APPENDIX B-3A

RFP # 2025-12

BEACON ISLAND PHASE 3
Packaged Wastewater Treatment Plant
and
Fire Pump House and Marine Inlet

REFERENCE DOCUMENTS (1 of 5)

Reference Documents

Beacon Island Phase 3 Program

- NYSDEC Article 11 and 15 Permits. Dated 11/10/22 - Page 1
- State Pollutant Discharge Elimination System (SPDES) Permit. Dated 10/1/23 - Page 21
- Stormwater Pollution Protection Plan (SWPPP). Dated 6/20/22 - Page 59
- Soil Management Plan. Dated 10/23/22 - Page 564
- Landfill Closure Certification Report. Dated 10/21/24 - Page 634
- Geotechnical Engineering Report. Dated 2/2/2023 - Page 659
- Army Corps of Engineers Permit. Date 4/10/23 - Page 779
- Community Air Monitoring Plan (CAMP). Dated 10/23/22 – Page 806

NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION

Division of Environmental Permits
625 Broadway, 4th Floor, Albany, New York 12233-1750
P: (518) 402-9167 | F: (518) 402-9168 | deppermitting@dec.ny.gov
www.dec.ny.gov

November 10, 2022

VIA EMAIL

Richard Hendrick
Albany Port District Commission
106 Smith Boulevard
Albany, NY 12202
rhendrick@portofalbany.us

RE: **NYSDEC Article 11 and Article 15 Permits**
Port of Albany Expansion Project
Marmen-Welcon Tower Manufacturing Plant
Beacon Island Parcel, Bethlehem NY, Albany County
DEC # 4-0122-00322

Dear Mr. Hendrick:

Enclosed is the Excavation and Fill in Navigable Waters Permit (ECL Article 15; 6 NYCRR Part 608) and Incidental Take Permit (ECL Article 11 and 6 NYCRR Part 182) for the Port of Albany Expansion Project. These permits are effective November 10, 2022 and will expire November 9, 2032. These permits are valid only for the Authorized Activities expressly authorized therein. Work beyond the scope of this permit and the approved documents is a violation of the law and may be subject to appropriate enforcement action.

A Compliance Checklist is enclosed for tracking the necessary notifications and plan/document submissions required by the permit conditions. Additionally enclosed is the required Implementation Agreement which was prepared pursuant to 6 NYCRR Part 182 for the take of habitat that is essential to the state and federally endangered Shortnose and Atlantic Sturgeon.

Be advised, the Uniform Procedures Regulations (6 NYCRR Part 621), provide that an applicant may request a hearing if a permit contains conditions which are unacceptable to them. Any such request must be made in writing within 30 calendar days of the date of this transmittal and must be addressed to the Chief Permit Administrator at the letterhead address. A copy should also be sent to the Chief Administrative Law Judge at the New York State Department of Environmental Conservation, 625 Broadway, 1st Floor, Albany, NY 12233-1550.

Please feel free to contact me at karen.gaidasz@dec.ny.gov if you have any questions about your obligations under these permits.

Sincerely,

Karen M. Gaidasz

Karen M. Gaidasz, Chief
Offshore Wind & Hydroelectric Section
Energy Project Management Bureau

Enclosures: NYSDEC Article 11 and 15 Permits
Compliance Checklist
Part 182 Implementation Agreement

ecc: McFarland-Johnson, Inc. – Steve Boisvert
USACE – Andrew Dangler (NAN-2021-00948-UDA)
DOS – David Newman (F-2021-0757)
OGS – Tom Laliberte
DEC Review Team

PERMIT
Under the Environmental Conservation Law (ECL)

Permittee and Facility Information

Permit Issued To:
ALBANY PORT DISTRICT COMMISSION
PORT OF ALBANY
106 SMITH BLVD
ALBANY, NY 12202
(518) 463-8763

Facility:
BEACON ISLAND
Beacon Island
Bethlehem, NY

Facility Location: in BETHLEHEM in ALBANY COUNTY **Village:** Town of Bethlehem
Facility Principal Reference Point: NYTM-E: 601.36 NYTM-N: 4717.582
Latitude: 42°36'14.4" Longitude: 73°45'51.7"
Project Location: 81.6-acre Beacon Island site located at Hudson River Mile 142
Authorized Activity:
Excavation and fill below mean high water (MHW) including construction of a 500-foot-long by 93-foot-wide wharf and associated dredging; and a new 3-span bridge over the Normans Kill. Approximately 75,100 cubic yards (CY) of sediment will be dredged from the Hudson River to a depth of 32 feet below the plane of mean low water (MLW), with approximately two (2) feet of allowable over-dredge, to 34 feet MLW. The area of dredging in the Hudson River will be approximately 2.72 acres (114,127 SF).

The dredging and wharf construction will result in the permanent loss of 0.76 acre of essential sturgeon habitat through habitat conversion and 0.21 acre of submerged aquatic vegetation. Additionally, there will be a temporary disturbance of 1.96 acres of essential sturgeon habitat from the dredging of previously unimpacted river bottom. This habitat impact will be offset through completion of the Net Conservation Benefit Project (NCBP) which will satisfy the requirements of ECL § 11-0535 and 6 NYCRR Part 182. The NCBP will create 1.0 acres of benthic habitat at Schodack Island State Park by converting habitat that is currently upland into habitat that can be used by sturgeon.

Permit Authorizations

Excavation & Fill in Navigable Waters - Under Article 15, Title 5
Permit ID 4-0122-00322/00002
New Permit Effective Date: 11/10/2022 Expiration Date: 11/9/2032
Endangered/Threatened Species (Incidental Take) - Under Article 11
Permit ID 4-0122-00322/00005
New Permit Effective Date: 11/10/2022 Expiration Date: 11/9/2032

NYSDEC Approval

By acceptance of this permit, the permittee agrees that the permit is contingent upon strict compliance with the ECL, all applicable regulations, and all conditions included as part of this permit.

Permit Administrator: KAREN M GAIDASZ, Deputy Chief Permit Administrator
Address: NYSDEC Headquarters
625 Broadway
Albany, NY 12233

Authorized Signature: Karen M. Gaidasz Date 11/ 10 / 2022

Permit Components

NATURAL RESOURCE PERMIT CONDITIONS

GENERAL CONDITIONS, APPLY TO ALL AUTHORIZED PERMITS

NOTIFICATION OF OTHER PERMITTEE OBLIGATIONS

NATURAL RESOURCE PERMIT CONDITIONS - Apply to the Following Permits: EXCAVATION & FILL IN NAVIGABLE WATERS; ENDANGERED/THREATENED SPECIES (INCIDENTAL TAKE)

GENERAL REQUIREMENTS

1. **Conformance With Plans** All activities authorized by this permit must be in strict conformance with the approved plans submitted by the applicant or applicant's agent as part of the permit application. Such approved plans were prepared by McFarland-Johnson, Inc., as further described in Condition #2, Approved Plans.

2. **Approved Plans** “Port of Albany Expansion Project - Joint Permit Application” package, submitted by the Albany Port District Commission, prepared by McFarland-Johnson, Inc., originally received on August 06, 2021, with subsequent revisions as listed below:

- Joint Permit Application Package (last revised May 12, 2022)
- Joint Permit Application Forms
- Appendix 1: Permit Sketches (Project Drawings) (last revised May 2022)
- Appendix 2: Interagency Pre-Application Meetings Documentation
- Appendix 3: Agency Correspondence
- Appendix 4: Rare Plant Species Investigation (June 11, 2019)
- Appendix 5: Supplemental Rare Plan Species Investigation (May 19, 2021)
- Appendix 6: Wetland Delineation Report (May 2019)

- Appendix 7: Supplemental Wetland Delineation Report (May 2021)
- Appendix 8: Submerged Aquatic Vegetation Survey (July 2020)
- Appendix 9: Freshwater Mussels Survey (July 2, 2020)
- Appendix 10: Soil Management Plan (last revised October 20, 2022)
- Appendix 11: Environmental Subsurface Investigation and Soil Sampling Report (October 22, 2020)
- Appendix 12 - Sediment Sampling and Analysis Report (September 24, 2020)
- Appendix 13 - Vegetation Management Plan for Vegetated Buffer at the Marmen-Welcon Offshore Wind Tower Manufacturing Plant (May 12, 2022)
- Appendix 14 - Port of Albany Marmen-Welcon Manufacturing Facility: 309 River Road Fire Flow (February 23, 2022)

"6 CRR-NY Part 182 Incidental Take Permit Application" package, submitted by the Albany Port District Commission, prepared by McFarland-Johnson, Inc., originally received on November 11, 2021, with subsequent revisions as listed below:

- 6 CRR-NY Part 182 Incidental Take Permit Application Package, last revised December 12, 2021
- Appendix 1: Permit Sketches (Project Drawings) (last revised December 12, 2021)
- Appendix 2: Submerged Aquatic Vegetation Survey (July 2020)
- Appendix 3: Freshwater Mussels Survey (July 2, 2020)
- Appendix 4: Sediment Sampling and Analysis Report (September 24, 2020)

3. **Approved Plans Discrepancies** If there is a discrepancy in the Approved Plans, the most recent document or plan takes precedence. If there is a discrepancy between the Approved Plans and any permit condition, the permit conditions take precedence.

4. **Revised, Modified, or New Plans** The Permittee must notify NYSDEC of material alterations to any Authorized Activity at least two weeks prior to starting that Activity. NYSDEC reserves the right to modify permit conditions upon review of revised, modified, or new plans.

5. **Excavation and Fill Activities** Excavation and fill below MHW includes construction of a 500-foot-long by 93-foot-wide wharf and associated dredging; and a new 3-span bridge over the Normans Kill. Final dredge volumes and areas were included in correspondence from McFarland-Johnson on October 31, 2022. Approximately 75,100 cubic yards (CY) of sediment will be dredged from the Hudson River to a depth of 32 feet below the plane of mean low water (MLW), with approximately two (2) feet of allowable over-dredge, to 34 feet MLW, within an approximately 2.72 acres (114,127 SF) area in the Hudson River.

6. **Normans Kill Bridge Work Plan** The installation of the new 3-span bridge across the Normans Kill will involve work below the MHW elevation, including the installation of temporary construction access and removal of remnant structures and material from the old railroad bridge. The removal of remnant structures and the installation of new structures shall be with vibratory or rotary methods only, no pile driving is authorized. Nets, tarps, and/or pans during construction of the bridge deck shall be implemented to prevent debris falling into the water. Remnant material from the previous railroad bridge shall be removed to at least 2 feet below the bed of the Normans Kill. The Permittee shall submit a detailed work plan for the Normans Kill bridge work to NYSDEC by February 1, 2023, for review and approval. The Normans Kill Bridge Work Plan must include finalized design plans and methods; procedures for removal of remnant materials and piers; construction access means and methods; and a dewatering plan.

7. **Stormwater SPDES** The Permittee must fully comply with the State Pollutant Discharge Elimination System (SPDES) General Permit for Stormwater Discharges from Construction Activity (GP-0-20-001).

8. **Groundwater Monitoring Wells** The Permittee shall prepare a Groundwater Monitoring Well Plan which specifies the installation of multiple groundwater wells along the banks of the Hudson River to monitor for potential contaminant migration resulting from construction activities over the coal ash landfill. The Plan shall outline the number of wells, location of wells and contaminants of concern. The wells shall be installed prior to soil surcharging activities so that baseline conditions can be established. Once soil surcharging commences, groundwater monitoring shall be conducted monthly until construction activities are completed. Monitoring shall continue post-construction every six (6) months until NYSDEC determines it can be discontinued based on review of sampling results. The Groundwater Monitoring Well Plan shall be submitted to NYSDEC by November 30, 2022, for review and approval.

9. **State Not Liable for Damage** The State of New York shall in no case be liable for any damage or injury to the structure or work herein authorized which may be caused by or result from future operations undertaken by the State for the conservation or improvement of navigation, or for other purposes, and no claim or right to compensation shall accrue from any such damage.

10. **State May Require Site Restoration** If upon the expiration or revocation of this permit, the project hereby authorized has not been completed, the applicant shall, without expense to the State, and to such extent and in such time and manner as the Department of Environmental Conservation may lawfully require, remove all or any portion of the uncompleted structure or fill and restore the site to its former condition. No claim shall be made against the State of New York on account of any such removal or alteration.

11. **State May Order Removal or Alteration of Work** If future operations by the State of New York require an alteration in the position of the structure or work herein authorized, or if, in the opinion of the Department of Environmental Conservation it shall cause unreasonable obstruction to the free navigation of said waters or flood flows or endanger the health, safety or welfare of the people of the State, or cause loss or destruction of the natural resources of the State, the owner may be ordered by the Department to remove or alter the structural work, obstructions, or hazards caused thereby without expense to the State, and if, upon the expiration or revocation of this permit, the structure, fill, excavation, or other modification of the watercourse hereby authorized shall not be completed, the owners, shall, without expense to the State, and to such extent and in such time and manner as the Department of Environmental Conservation may require, remove all or any portion of the uncompleted structure or fill and restore to its former condition the navigable and flood capacity of the watercourse. No claim shall be made against the State of New York on account of any such removal or alteration.

NOTIFICATIONS AND POSTINGS

12. **Notifications and Submissions** All notifications and submissions required by this permit must be provided by email at: DEPEnergy@dec.ny.gov.

13. **Notification of Commencement of Authorized Activities** At least forty-eight (48) hours prior to commencement of any Authorized Activities, the Permittee must notify the NYSDEC.

14. **Notification of Exceedance of Water Quality Standard** In the event of exceedance of a water quality standard, NYSDEC must be notified immediately, with the Permittee taking all necessary steps to return to compliance, including but not limited to, the cessation of any non-compliant discharges.

15. **Notification of Completion of Authorized Activities** Within ten (10) days of the completion of the Authorized Activities, the Permittee must notify the NYSDEC.

16. **Provide Project Status Reports Every Two Weeks** During construction phases involving work authorized by this permit, the Permittee shall provide project status reports via email to DEPEnergy@dec.ny.gov. The Project Status Reports shall include a description of the work undertaken during the preceding two-week period; representative photographs; and summary of any non-compliance.

WATER QUALITY

17. **Precautions Against Contamination of Waters** All necessary precautions shall be taken to preclude contamination of any wetland or waterway by suspended solids, sediments, fuels, solvents, lubricants, epoxy coatings, paints, concrete, leachate or any other environmentally deleterious materials associated with the project.

18. **Water Quality Standards** Authorized Activities shall comply with Section 301, 302, 303, 306, and 307 of the Federal Water Pollution Control Act, as amended and as implemented by the limitations, standards, and criteria of state statutory and regulatory requirements set forth in 6 NYCRR Section 608.9(a). Authorized Activities, as conditioned, shall also comply with applicable New York State water quality standards, including but not limited to effluent limitations, best usages, and thermal discharge criteria, as applicable, as set forth in 6 NYCRR Parts 701,702, 703, and 704.

19. **No Leachate or Drilling Fluid Discharge** There shall be no leachate or discharge of drilling fluid into the water column.

20. **Isolate In-Water Work Areas** All dredging must take place within the confines of a weighted full-depth Type II or III turbidity curtain. The turbidity curtain(s) must be in place prior to commencement of dredging activities and remain in place for at least 24 hours after dredging is completed. The Permittee must prevent both the discharge of sediment and a discharge that could cause a substantial visible contrast in ambient river water conditions outside the turbidity curtain(s). If a plume is observed outside the confines of the turbidity curtain, the turbidity curtain containment area must be examined for breaches. Any identified breaches in the curtain must be immediately repaired. Consultation with NYSDEC staff must commence immediately to determine whether additional best management practices for dredging should be implemented.

21. **Visual Monitoring Outside Turbidity Curtain Containment Area(s)** Visual monitoring for turbidity outside turbidity curtain containment area(s) must be conducted hourly during all in-water work. If a plume is observed to have migrated outside the turbidity curtain containment area, water column samples shall be collected to represent the entire depth of the water column within the plume and must be analyzed for TSS and PCBs. TSS must not exceed 100 ppm over ambient and PCBs must not exceed 0.2 ug/l per Aroclor in any plume identified emanating from the confines of the turbidity curtain.

22. **Water Quality Monitoring Outside Turbidity Curtain Containment Area(s)** Daily water column monitoring for PCBs shall be conducted just outside the confines of the turbidity curtain during dredging of areas with Class C concentrations of PCBs. Samples shall be collected downstream of the dredge area (based on the direction of flow) and represent the entire depth of the water column. If the results of the daily monitoring conclude that the concentration of PCBs exceeds the water quality standard (0.2 ug/l per Aroclor), then BMPs shall be implemented, such as slowing the rate of dredging. If the concentration (of PCBs) is consistently below the water quality standard, then water monitoring for PCBs outside the turbidity curtain containment area may be reduced to once per week. Any reduction in monitoring frequency must be approved in advance by NYSDEC staff.

CONSTRUCTION REQUIREMENTS

23. **No Interference With Navigation** There shall be no unreasonable interference with navigation by the work herein authorized.

24. **Seasonal Time of Year Restriction for Sturgeon** To minimize the risk for impacts to Atlantic and shortnose sturgeon, in-water work is only authorized from September 1 through December 31, or until river ice-in, whichever end date is earlier for any year in which this permit is in effect. In-water work is only authorized between the hours of 6 a.m. to 6 p.m.

25. **Entangled, Injured, or Dead Protected Species** If an entangled, injured, or dead protected species is encountered during the Authorized Activities, all work must immediately cease (allowing for retrieval of equipment, if necessary), and the Permittee must notify NYSDEC immediately. Protected species include species protected under the Marine Mammal Protection Act, and federal and state listed threatened and endangered species. Authorized Activities must not recommence until NYSDEC provides authorization to do so. A report including the following information must be prepared and provided to NYSDEC within 24 hours:

a. species identification (if known) or description of the animal;
b. time, date, and location (latitude/longitude) of the discovery;
c. condition of the animal (discoveries of injured or dead animals);
d. observed behaviors of the animal, if alive;
e. name(s) and contact information of the person(s) involved with the discovery;
f. weather conditions at the site for the previous forty-eight (48) hours;
g. if available, photographs or video footage of the animal; and
h. general circumstances under which the animal was discovered (discoveries of injured or dead animals).

26. **Certified Laboratory** All laboratory analyses required by this permit must be conducted by a laboratory certified by the New York State Department of Health (NYSDOH) Environmental Lab Approval Program (ELAP).

27. **Send Analytical Results to NYSDEC** All analytical results required by this permit must be sent to NYSDEC by email within forty-eight (48) hours of receipt of data results. Exceedances must be highlighted.

28. Provide Sediment Data in Electronic Digital Format All existing and future sediment data must be provided in an electronic digital format (EDD) file. Existing sediment data shall be provided in an EDD file to NYSDEC by December 15, 2022. Sediment data collected during the term of this permit shall be provided in an EDD file to NYSDEC within sixty (60) days of collecting the data.

29. Report Spills All spills must be reported through the NYSDEC Spills Hotline (1 800 457-7362) in accordance with the regulations.

30. Flocculant Use Requires NYSDEC Approval A flocculant may be added to enhance settling. If a flocculant is proposed to be used, the form "Water Treatment Chemical Usage Notification Requirements for SPDES Permittees" (WTC Form) must be submitted and approved by NYSDEC prior to its use. Additional information on submission and review of WTC Forms can be found at: http://www.dec.ny.gov/permits/93245.

31. Beneficial Use Determination Any material that is proposed for upland beneficial use within New York State must receive a NYSDEC Beneficial Use Determination (BUD).

DREDGING

32. Dredging Operations The following conditions apply to all dredging operations:

a. A closed environmental (clamshell) bucket with sealing gaskets or an overlapping sealed design at the jaws with seals or flaps positioned at locations of vent openings must be used to minimize sediment re-suspension during dredging.
b. Seals or flaps designed or installed at the jaws and locations of vent openings must tightly cover these openings while the bucket is lifted through the water column and into the barge.
c. The closed environmental (clamshell) bucket dredge must be equipped with sensors to ensure complete closure of the bucket before lifting through the water.
d. Dredged material must be placed in the barge in a controlled manner.
e. Excessive loss of material from the bucket must be investigated and repaired.
f. Bucket retrieval rates must be controlled to minimize turbidity.
g. Dredged material must be transferred directly from the barge to the disposal/processing site.
h. No barge overflow is authorized during dredging operations or transportation of dredge material to the disposal/processing site.
i. Bucket decanting and loss of dredged material into the Hudson River during barge loading must be controlled to minimize turbidity.
j. Wet dumping or sidecasting of the dredged material into the Hudson River or any associated waterbody is prohibited.
k. Washing of the gunwales must be avoided except to the extent necessary to ensure the safety of workers.
l. The use of a dragline for dredging is strictly prohibited.
m. All side slopes of the dredge channel shall have a maximum of 1:3 slope.
n. All dredging shall be conducted to leave a uniform bottom elevation free of mounds or holes.

33. Transporting Barges to be Inspected All sediment transporting barges must be inspected daily and certified by the Permittee as properly sealed. Only barges in good operating condition and appropriately designed to contain sediments are authorized. Loss of material during transport is prohibited.

34. **Submit a Plan to NYSDEC for Barge Decanting** Any proposed decanting of barges must be approved by NYSDEC prior to implementation. If decanting of barges is necessary, a detailed plan must be submitted to NYSDEC for review and approval at least thirty (30) days prior to commencing barge decanting.

35. **Final Dewatering Plan Must be Approved by NYSDEC** The final dewatering plan must be submitted to NYSDEC by December 15, 2022, for review and approval.

36. **Monitor Overlying Water Prior to Discharge** The Permittee shall monitor the overlying barge water for PCBs, TSS, and turbidity prior to discharge. If the overlying water meets the PCB, TSS, and turbidity limits in Table 1 in Condition 37, it can then be discharged back to the water column. If PCB, TSS, and turbidity limits are not met, then the permittee shall employ additional measures to achieve the limits prior to discharge.

37. **Dewatering and Return Water Monitoring Requirements** Transfer operations between barges must not cause discharges of sediment or turbid water causing a substantial visible contrast to receiving waters. Return water monitoring must commence upon discharge. Discharge limits and monitoring for return flow are in Table 1 below. The return water discharge must be configured so that water with the lowest possible turbidity is returned to the Hudson River.

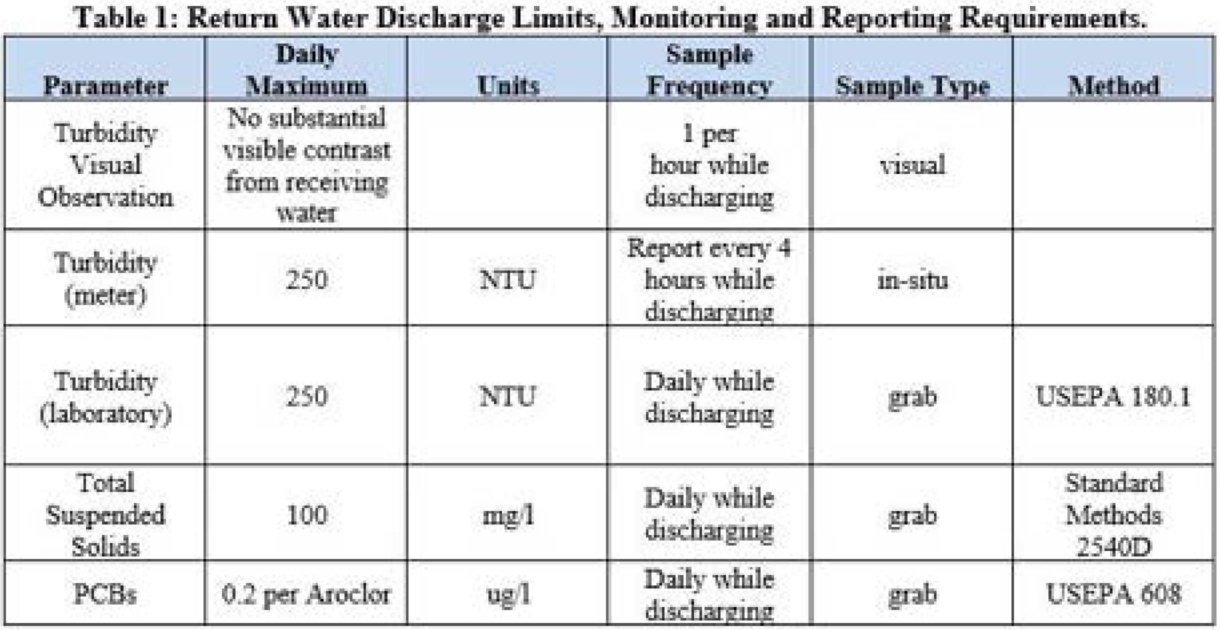

Table 1: Return Water Discharge Limits, Monitoring and Reporting Requirements.

Parameter	Daily Maximum	Units	Sample Frequency	Sample Type	Method
Turbidity Visual Observation	No substantial visible contrast from receiving water		1 per hour while discharging	visual	
Turbidity (meter)	250	NTU	Report every 4 hours while discharging	in-situ	
Turbidity (laboratory)	250	NTU	Daily while discharging	grab	USEPA 180.1
Total Suspended Solids	100	mg/l	Daily while discharging	grab	Standard Methods 2540D
PCBs	0.2 per Aroclor	ug/l	Daily while discharging	grab	USEPA 608

38. **Conduct Laboratory Analysis Within 24 to 48 Hours** Laboratory analysis of the return flow from dewatering must be conducted within 24 to 48 hours of the initial discharge.

39. **Operational Controls for Return Water Discharge** Operational controls must be implemented if TSS, turbidity, PCB limits are not met in the return water. Additional engineering controls can include, but are not limited to, the following: controlling the flow rate, increasing the detention time, and/or use of chemical additives to accelerate solids settling.

40. **Corrective Actions for Return Water Discharge Limit Exceedances** If two consecutive monitoring results of the return water exceed the 100 mg/l suspended solids limit, the 250 NTU turbidity limit, the 0.2 ug/l per Aroclor PCB limit, or a visual plume is observed, the Permittee must take immediate corrective actions to restore the discharge to acceptable levels and report the exceedance and the corrective actions to the NYSDEC. If corrective actions do not bring the effluent into compliance within 48 hours, then the Permittee must cease discharge and present a plan to the NYSDEC for approval prior to resuming the discharge. During implementation of corrective actions, NYSDEC may specify additional monitoring until compliance with permit levels is demonstrated. Samples must be collected until there is no longer a discharge of effluent from the site or until NYSDEC approves resumption of routine monitoring. If implementation of the corrective action does not return discharge water quality to an acceptable level, discharge must cease until a solution acceptable to NYSDEC.

41. **Post-Dredging Final Report** Within thirty (30) days of completion of the dredging operation, the Permittee must submit a report to NYSDEC summarizing the results of dredging, water quality monitoring, and dredged material management operations. The report must include:

a. Location and extent of dredging;
b. Total amount of material dredged;
c. Ultimate placement location of dredged material;
d. Description of problems or difficulties encountered during dredging;
e. Water quality monitoring results and corrective actions (when needed) taken; and
f. Return flow discharge monitoring results and corrective actions (when needed) taken.

42. **Maintenance Dredging** The Permittee must submit a Maintenance Dredging Plan to NYSDEC for review and approval at least ninety (90) days prior to maintenance dredging being conducted.

MITIGATION

43. **Implementation Agreement Funding Obligations** The Permittee is responsible for complying with the Funding Obligations contained in Section 4.0 of the "Port of Albany Expansion Project Implementation Agreement Pursuant to 6 NYCRR 182" signed by NYSDEC, New York State Office of Parks, Recreation and Historic Preservation and APDC on June 6, 2022. The Implementation Agreement was developed to mitigate for the temporary and permanent loss of sturgeon habitat. The habitat impact will be offset through completion of the Net Conservation Benefit Project (NCBP) which will satisfy the requirements of ECL § 11-0535 and 6 NYCRR Part 182. The NCBP will create 1.0 acres of benthic habitat at Schodack Island State Park by converting habitat that is currently upland into habitat that can be used by sturgeon.

44. **Hudson River Vegetated Buffer** A vegetated buffer within three (3) deed restricted areas along the Hudson River, totaling 2.87 acres, as shown on the Proposed Deed Restricted Area Boundary Map prepared by Colliers Engineering & Design, dated May 2, 2022, shall be protected in perpetuity by Restrictive Covenants. The Restrictive Covenants shall be filed with the Albany County Clerk's Office within 60 days of receipt of permits from the U.S. Army Corps of Engineers (USACE), or by June 1, 2023, whichever is sooner.

GENERAL CONDITIONS - Apply to ALL Authorized Permits:

1. **Facility Inspection by The Department** The permitted site or facility, including relevant records, is subject to inspection at reasonable hours and intervals by an authorized representative of the Department of Environmental Conservation (the Department) to determine whether the permittee is complying with this permit and the ECL. Such representative may order the work suspended pursuant to ECL 71- 0301 and SAPA 401(3).

The permittee shall provide a person to accompany the Department's representative during an inspection to the permit area when requested by the Department.

A copy of this permit, including all referenced maps, drawings and special conditions, must be available for inspection by the Department at all times at the project site or facility. Failure to produce a copy of the permit upon request by a Department representative is a violation of this permit.

2. **Relationship of this Permit to Other Department Orders and Determinations** Unless expressly provided for by the Department, issuance of this permit does not modify, supersede or rescind any order or determination previously issued by the Department or any of the terms, conditions or requirements contained in such order or determination.

3. **Applications For Permit Renewals, Modifications or Transfers** The permittee must submit a separate written application to the Department for permit renewal, modification or transfer of this permit. Such application must include any forms or supplemental information the Department requires. Any renewal, modification or transfer granted by the Department must be in writing. Submission of applications for permit renewal, modification or transfer are to be submitted to:

Deputy Chief Permit Administrator
NYSDEC Headquarters
625 Broadway
Albany, NY12233

4. **Submission of Renewal Application** The permittee must submit a renewal application at least 30 days before permit expiration for the following permit authorizations: Excavation & Fill in Navigable Waters, Endangered/Threatened Species (Incidental Take).

5. **Permit Modifications, Suspensions and Revocations by the Department** The Department reserves the right to exercise all available authority to modify, suspend or revoke this permit. The grounds for modification, suspension or revocation include:

a. materially false or inaccurate statements in the permit application or supporting papers;

b. failure by the permittee to comply with any terms or conditions of the permit;

c. exceeding the scope of the project as described in the permit application;

d. newly discovered material information or a material change in environmental conditions, relevant technology or applicable law or regulations since the issuance of the existing permit;

e. noncompliance with previously issued permit conditions, orders of the commissioner, any provisions of the Environmental Conservation Law or regulations of the Department related to the permitted activity.

6. Permit Transfer Permits are transferrable unless specifically prohibited by statute, regulation or another permit condition. Applications for permit transfer should be submitted prior to actual transfer of ownership.

NOTIFICATION OF OTHER PERMITTEE OBLIGATIONS

Item A: Permittee Accepts Legal Responsibility and Agrees to Indemnification
The permittee, excepting state or federal agencies, expressly agrees to indemnify and hold harmless the Department of Environmental Conservation of the State of New York, its representatives, employees, and agents ("DEC") for all claims, suits, actions, and damages, to the extent attributable to the permittee's acts or omissions in connection with the permittee’s undertaking of activities in connection with, or operation and maintenance of, the facility or facilities authorized by the permit whether in compliance or not in compliance with the terms and conditions of the permit. This indemnification does not extend to any claims, suits, actions, or damages to the extent attributable to DEC's own negligent or intentional acts or omissions, or to any claims, suits, or actions naming the DEC and arising under Article 78 of the New York Civil Practice Laws and Rules or any citizen suit or civil rights provision under federal or state laws.

Item B: Permittee's Contractors to Comply with Permit
The permittee is responsible for informing its independent contractors, employees, agents and assigns of their responsibility to comply with this permit, including all special conditions while acting as the permittee's agent with respect to the permitted activities, and such persons shall be subject to the same sanctions for violations of the Environmental Conservation Law as those prescribed for the permittee.

Item C: Permittee Responsible for Obtaining Other Required Permits
The permittee is responsible for obtaining any other permits, approvals, lands, easements and rights-of-way that may be required to carry out the activities that are authorized by this permit.

Item D: No Right to Trespass or Interfere with Riparian Rights
This permit does not convey to the permittee any right to trespass upon the lands or interfere with the riparian rights of others in order to perform the permitted work nor does it authorize the impairment of any rights, title, or interest in real or personal property held or vested in a person not a party to the permit.

COMPLIANCE CHECKLIST

Port of Albany Expansion Project - Marmen-Welcon Tower Manufacturing Plant

Town of Bethlehem, Albany County

DEC# 4-0122-00322

Notification Requirements

Required Submission (Natural Resource Condition #)	NYSDEC Notification Timeframe	Date Completed
Submit Revised, Modified or New Plans **(#4)**	At least 2 weeks prior to commencing activity.	If needed.
Notification of Commencement of Authorized Activities (#13)	At least 48 hours prior.	
Notification of Exceedance of Water Quality Standard (#14)	Immediately.	If needed.
Notification of Completion of Authorized Activities (#15)	Within 10 days.	
Project Status Reports (#16)	Every 2 weeks during construction activities.	Recurring Reports.
Observation of plume outside of turbidity curtain (#20)	Immediate repair and consultation.	If needed.
Entangled, Injured, or Dead Protected Species (#25)	Immediate notification and send report within 24 hours.	If needed.
Send Analytical Results (#26)	Within 48 hours of receipt of data results.	As necessary.
WTC Form (#30)	Prior to use of Water Treatment Chemical	If needed.
Corrective Actions for Return Water Discharge Limit Exceedances (#40)	Immediate notification and consultation with NYSDEC.	If needed.

Plan/Document Submission Requirements

Required Submission (Natural Resource Condition #)	NYSDEC Submission Timeframe	Date Completed
Normans Kill Bridge Work Plan (#6)	Submit Plan for review and approval by February 1, 2023.	
Groundwater Monitoring Well Plan (#8)	Submit Plan for review and approval by November 30, 2022.	
Sediment Data in EDD Format (#27)	Existing data by December 15, 2022. Future data within 60 days of collection.	
Barge Decanting Plan (#34)	At least 30 days prior to commencing barge decanting.	
Final Dewatering Plan (#35)	Submit Plan for review and approval by December 15, 2022.	
Post-Dredging Final Report (#41)	Submit Plan within 30 days of completion of dredging operations.	
Maintenance Dredging Plan (#42)	Submit Plan for review and approval at least 90 days prior to maintenance dredging.	
Implementation Agreement Funding Obligations (#43)	Submit payments to OPRHP per Section 4.0 of the signed Implementation Agreement.	
File Deed Restrictions with Albany County Clerk's Office **(#44)**	Within 60 days of receipt of USACE Permits or by June 1, 2023, whichever is sooner.	

Port of Albany Expansion Project
Implementation Agreement Pursuant to 6 NYCRR 182

By: Albany Port District Commission

Re: Port of Albany Expansion Project, Town of Bethlehem, Albany County, New York
Part 182/Incidental Take Permit Application (January 2022)
DEC #4-0122-00322

1.0 Introduction

This Implementation Agreement ("Agreement") has been prepared in accordance with the New York State Environmental Conservation Law ("ECL") § 11-0535 and the accompanying regulations found at Title 6 of the Official Compilation of Codes, Rules and Regulations of the State of New York ("NYCRR") Part 182. It sets forth the roles and responsibilities of each party that will be involved in the creation of one acre of new benthic[1] area at Schodack Island State Park ("Net Conservation Benefit Project") which is associated with an Incidental Take Permit to be issued by the New York State Department of Environmental Conservation ("NYSDEC") to the Albany Port District Commission ("Commission") for the Port of Albany Expansion Project ("Expansion Project") in the Town of Bethlehem, Albany County, New York.

The Expansion Project will modify an undeveloped industrially zoned property into an active marine terminal with specialized infrastructure capable of supporting a new manufacturing operation that would produce tower components for offshore wind developments. The Expansion Project will facilitate the marine-based import and export of materials and manufactured components to be used in the development of offshore wind projects. Site improvements include the construction of associated site utilities, roads and infrastructure; installation of a new bridge over Normans Kill; and a new wharf which requires dredging and associated shoreline improvements.

The proposed in-water work activities (*e.g.*, wharf and dredging) would occur along the western bank of the Hudson River in an area of approximately 740 feet in length by 167 feet in width. The general layout of the proposed wharf places the riverside face of structure coincident with the face of the existing timber revetment. The proposed wharf consists of a deep foundation-supported concrete-framed open-type wharf structure that provides overall dimensions of approximately 500 feet in length by 93 feet in width. The total area of the wharf is approximately 45,500 SF. The area of the wharf provided over water (outboard of the sheet pile cutoff wall), where shade will occur, is approximately 27,500 SF. Shade would vary and subject to time of the day and season of the year. The existing width of the Hudson River at this location is over 700 feet.

The Expansion Project will result in a take of habitat that is essential to the state and federally endangered Shortnose and Atlantic Sturgeon. This habitat impact will be offset through completion of the Net Conservation Benefit Project. The Net Conservation Benefit Project is one element of the larger Schodack Island Restoration Project ("Restoration Project") proposed by the New York State Office of Parks, Recreation and Historic Recreation ("OPRHP"). NYSDEC staff have determined that the Net Conservation Benefit Project will satisfy the requirements of ECL § 11-0535 and 6 NYCRR Part 182.

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[1] Benthic area will be inundated at mean high water.

2.0 Responsibilities of the Parties

The Commission shall be responsible for contributing funds to the Net Conservation Benefit Project as part of the larger Restoration Project. OPRHP shall use the funding received from the Commission for the Net Conservation Benefit Project. OPRHP shall be responsible for implementing the Net Conservation Benefit Project.

The Net Conservation Benefit Project will be implemented on lands owned and/or controlled by OPRHP at the Schodack Island State Park. The Net Conservation Benefit Project will be designed and permitted after a feasibility study is conducted. The feasibility study is being conducted by NYSDEC and will be completed by December 31, 2022.

2.1 Albany Port District Commission

- The Commission shall provide funding to OPRHP for the Net Conservation Benefit Project in accordance with the payment schedule and payment method set forth in Section 4 below.

2.2 OPRHP

- OPRHP shall administer the Net Conservation Benefit Project design that will create at least 1.0 area of benthic area at Schodack Island State Park, as identified in the Incidental Take Permit;

- OPRHP shall prepare and apply for the permits and approvals necessary to implement the Net Conservation Benefit Project;

- OPRHP shall administer the construction of the Net Conservation Benefit Project when sufficient funding is acquired and received;

- OPRHP shall consult NYSDEC staff to ensure the Net Conservation Benefit Project results in a net conservation benefit as required by the Commission's Incidental Take Permit; and

- OPRHP shall provide NYSDEC staff annual reports on progress of the design and construction of the Net Conservation Benefit Project, including three years post-construction monitoring, and other information as requested by NYSDEC staff to determine if the requirements of this Agreement and the Incidental Take Permit are met.

2.3 NYSDEC

- NYSDEC shall participate with OPRHP on the design review of the Net Conservation Benefit Project;

- NYSDEC shall assist OPRHP in obtaining all necessary NYSDEC and other permits/approvals for the Net Conservation Benefit Project;

- NYSDEC shall assist OPRPH with the oversight of the Net Conservation Benefit Project; and

- NYSDEC shall assist OPRHP with the three (3)-year post construction monitoring of the Net Conservation Benefit Project in accordance with 6 NYCRR Part 182.

3.0 Net Conservation Benefit Project Scope

The Expansion Project includes a wharf that is within the general vicinity of known Shortnose Sturgeon spawning habitat[2] and within essential foraging, rearing and migration habitat for both Shortnose and Atlantic Sturgeon. Both Sturgeon species are bottom feeders and have been documented to forage on benthic organisms found in the Hudson River. Habitat alteration is a key threat to Sturgeon populations.[3] The impacts on site will detrimentally impact these essential habitats for the species. Therefore, a restoration project must be completed that provides more benefit than the harm caused by the incidental take of Sturgeon to create a net conservation benefit to the species.

The Expansion Project will result in the permanent loss of 0.76 acres of essential Sturgeon habitat through permanent habitat conversion associated with the construction of the wharf. This area consists of the areas currently less than 10-feet in depth that will be converted to riprap or permanently shaded by wharf construction.

In addition to the permanent habitat loss, the Expansion Project will also result in an initial temporary disturbance of 1.96 acres of essential Sturgeon habitat through the dredging of previously unimpacted river bottom. Maintenance dredging at the site will likely be required once every 10 years to maintain the area, resulting in an annualized Sturgeon impact of approximately 0.20 acres. Temporary impacts will be confined to September 1 through October 31, to minimize both habitat impacts and the potential for direct impacts on feeding sturgeon. Habitat studies in other portions of the Hudson River indicate that the benthic fauna that Sturgeon feed on will recolonize the site within one year of dredging.[4]

The adverse habitat impacts to Sturgeon described above will be offset and improved by the Net Conservation Benefit Project. The area where Schodack Island State Park is located was historically several small islands separated by side channels. The side channels were filled in with spoils from dredging the Hudson River for commercial navigation. The Net Conservation Benefit Project will create 1.0 acres of benthic habitat at Schodack Island State Park by converting habitat that is currently upland into habitat that can be used by Sturgeon. The goal of this project is to return the area to habitat that is more similar to what existed before the dredge and fill operations, and ultimately create a self-sustaining habitat that is beneficial to Sturgeon. The benthic habitat creation will provide new foraging habitat for Shortnose Sturgeon and new refuge area for several fish species, including Atlantic Sturgeon. The Net Conservation Benefit Project shall be incorporated into the larger Schodack Island Restoration Project that will be developed by OPRHP and NYSDEC. The Net Conservation Benefit Project shall include:

- New benthic area created by the excavation of previously deposited dredge spoils to a depth that is inundated at Mean High Water;
- Benthic area will be inundated with Hudson River water to deliver river sediment and benthic organisms appropriate to enhance the food supply of Shortnose Sturgeon;
- Increased refuge area for fish species that increases diversity of habitat and survival of predation and hanging water flows;

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[2] Pendleton 2018 Acoustic Telemetry and Benthic Habitat Mapping Inform the Spatial Ecology of Shortnose Sturgeon in the Hudson River, New York.

[3] Title: Biological Assessment of shortnose sturgeon Acipenser periosteum, Published Date: 2010.

[4] Newell 1998 The Impact of Dredging works in coastal Waters: A Review of the sensitivity to disturbance and subsequent recovery of biological resources on the seabed.

- Stabilized restoration area to assure long-term benefit of benthic areas; and
- Monitoring for at least three years to document stability, water flows, and establishment of benthic organisms. Monitoring methodology will be consistent with other dredged areas to increase the understanding of Hudson River restoration.

4.0 The Commission's Funding Obligations and Method of Payment

The Commission shall provide OPRHP with a total of $700,000 for the Net Conservation Benefit Project in accordance with the payment schedule set forth in section 4.2. ORPHP shall dedicate these funds to be used for the Net Conservation Benefit Project. The total amount of $700,000 is estimated to be sufficient to fund the creation of 1.0 acres of benthic area identified as requiring restoration.

ORPHP shall submit invoices to the Commission requesting payment. All invoices shall be prepared in a standard invoice format and accompanied by a New York State Standard Voucher and Documentation Requirements. All invoices will be processed in accordance with established procedures of OPRHP and the New York State Office of the State Comptroller (OSC). The Commission shall transfer the funds to OPRHP through electronic payment. Such electronic payment shall be made in accordance with ordinary OPRHP and OSC procedures and practices.

4.1 Funding Source: The Albany Port District Commission shall provide the funding to OPRHP. Upon OPRHP's receipt of the final payment, in accordance with the funding schedule in 4.2, the obligation of the Albany Port District Commission for the Net Conservation Benefit Project will be complete.

4.2 Funding Schedule: To meet the obligations under the Incidental Take Permit, the Commission shall pay OPRHP a total of $700,000 in accordance with the following schedule:

Payment Amount	**Received by OPRHP on or before**	**Allocation of Funds**
$150,000	August 31, 2022	Design and Permitting of The Net Conservation Benefit Project
$137,500	January 1, 2023	Construction and Monitoring
$137,500	January 1, 2024	Construction and Monitoring
$137,500	January 1, 2025	Construction and Monitoring
$137,500	January 1, 2026	Construction and Monitoring

6/6/22

Thomas Berkman
Deputy Commissioner and General Counsel
New York State Department of Environmental Conservation
625 Broadway
Albany, New York 12233

Date

Mindy Scott Digitally signed by Mindy Scott
Date: 2022.05.20
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05/20/2022

Melinda Scott
Deputy Commissioner for Finance and Administration
New York State Office of Parks, Recreation, and Historic Preservation
625 Broadway
Albany, New York 12233

Date

Richard J. Hendrick Digitally signed by Richard J. Hendrick
Date: 2022.05.17
14:35:56 -04'00'

05/17/2022

Richard J. Hendrick
Chief Executive Officer
Albany Port District Commission
106 Smith Blvd
Albany, New York 12202

Date

State Pollutant Discharge Elimination System (SPDES) DISCHARGE PERMIT

SIC Code:	**3999**	NAICS Code:	**331221, 332114, 332312**	SPDES Number:	**NY0312924**
Discharge Class (CL):	**01**			DEC Number:	**4-0199-00059**
Toxic Class (TX):	**N**			Effective Date (EDP):	**10/1/2023**
Major-Sub Drainage Basin:	**1301**			Expiration Date (ExDP):	**9/30/2028**
Water Index Number:	**H**	Item No.:	**5**	Modification Dates (EDPM):	**-**
Compact Area:	**-**				

This SPDES permit is issued in compliance with Title 8 of Article 17 of the Environmental Conservation Law of New York State and in compliance with the Clean Water Act, as amended, (33 U.S.C. '1251 et. seq.)

PERMITTEE NAME AND ADDRESS					
Name:	**Albany Port District Commission**	Attention:	**Richard Hendrick**		
Street:	**106 Smith Blvd**				
City:	**Albany**	State:	**NY**	Zip Code:	**12202**
Email:	**Rhendrick@portofalbany.us**	Phone:	**518-463-8763**		

is authorized to discharge from the facility described below:

FACILITY NAME, ADDRESS, AND PRIMARY OUTFALL									
Name:	**Marmen – Welcon Manufacturing Plant**								
Address / Location:	**309 River Road**							County:	**Albany**
City:	**Albany**			State:	**NY**			Zip Code:	**12077**
Facility Location:		Latitude:	**42** ° **36** ’ **22** ” N			& Longitude:		**73** ° **45** ’ **57** ” W	
Primary Outfall No.:	**001**	Latitude:	**42** ° **36** ’ **24** ” N			& Longitude:		**73** ° **45** ’ **48** ” W	
Wastewater Description:	**Sanitary**	Receiving Water:	**Hudson River**	NAICS:	**-**	Class:	**C**	Standard:	**C**

and the additional outfalls listed in this permit, in accordance with: effluent limitations; monitoring and reporting requirements; other provisions and conditions set forth in this permit; and 6 NYCRR Part 750-1 and 750-2.

This permit and the authorization to discharge shall expire on midnight of the expiration date shown above and the permittee shall not discharge after the expiration date unless this permit has been renewed or extended pursuant to law. To be authorized to discharge beyond the expiration date, the permittee shall apply for permit renewal not less than 180 days prior to the expiration date shown above.

DISTRIBUTION:
- CO BWP - Permit Coordinator
- BWP – Permit Writer
- CO BWC - SCIS
- RWE
- RPA
- EPA Region II

Permit Administrator:	Karen M. Gaidasz		
Address:	625 Broadway Albany, NY 12233-1750		
Signature:	Karen M. Gaidasz	Date:	08 / 31 / 2023

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SUMMARY OF ADDITIONAL OUTFALLS

Outfall	Wastewater Description	NAICS Code	Outfall Latitude						Outfall Longitude					
01A	**Plain (no detergent) Wash Water and Compressor Condensate**		**42**	°	**36**	’	**18**	” N	**73**	°	**46**	’	**3**	” W
Receiving Water:	**Hudson River via Outfall 001**								Class:	**C**				

DEFINITIONS

TERM	DEFINITION
7-Day Geo Mean	The highest allowable geometric mean of daily discharges over a calendar week.
7-Day Average	The average of all daily discharges for each 7-days in the monitoring period. The sample measurement is the highest of the 7-day averages calculated for the monitoring period.
12-Month Rolling Average (12 MRA)	The current monthly value of a parameter, plus the sum of the monthly values over the previous 11 months for that parameter, divided by the number of months for which samples were collected in the 12-month period.
30-Day Geometric Mean	The highest allowable geometric mean of daily discharges over a calendar month, calculated as the antilog of: the sum of the log of each of the daily discharges measured during a calendar month divided by the number of daily discharges measured during that month.
Action Level	Action level means a monitoring requirement characterized by a numerical value that, when exceeded, triggers additional permittee actions and department review to determine if numerical effluent limitations should be imposed.
Compliance Level / Minimum Level	A compliance level is an effluent limitation. A compliance level is given when the water quality evaluation specifies a Water Quality Based Effluent Limit (WQBEL) below the Minimum Level. The compliance level shall be set at the Minimum Level (ML) for the most sensitive analytical method as given in 40 CFR Part 136, or otherwise accepted by the Department.
Daily Discharge	The discharge of a pollutant measured during a calendar day or any 24-hour period that reasonably represents the calendar day for the purposes of sampling. For pollutants expressed in units of mass, the ‘daily discharge’ is calculated as the total mass of the pollutant discharged over the day. For pollutants with limitations expressed in other units of measurement, the ‘daily discharge’ is calculated as the average measurement of the pollutant over the day.
Daily Maximum	The highest allowable Daily Discharge.
Daily Minimum	The lowest allowable Daily Discharge.
Effective Date of Permit (EDP or EDPM)	The date this permit is in effect.
Effluent Limitations	Effluent limitation means any restriction on quantities, quality, rates and concentrations of chemical, physical, biological, and other constituents of effluents that are discharged into waters of the state.
Expiration Date of Permit (ExDP)	The date this permit is no longer in effect.
Instantaneous Maximum	The maximum level that may not be exceeded at any instant in time.
Instantaneous Minimum	The minimum level that must be maintained at all instants in time.
Monthly Average	The highest allowable average of daily discharges over a calendar month, calculated as the sum of each of the daily discharges measured during a calendar month divided by the number of daily discharges measured during that month.
Outfall	The terminus of a sewer system, or the point of emergence of any waterborne sewage, industrial waste or other wastes or the effluent therefrom, into the waters of the State.
Range	The minimum and maximum instantaneous measurements for the reporting period must remain between the two values shown.
Receiving Water	The classified waters of the state to which the listed outfall discharges.
Sample Frequency / Sample Type / Units	See NYSDEC’s “DMR Manual for Completing the Discharge Monitoring Report for the SPDES” for information on sample frequency, type and units.

PERMIT LIMITS, LEVELS AND MONITORING

OUTFALL	DESCRIPTION	RECEIVING WATER	EFFECTIVE	EXPIRING
001	Sanitary	Hudson River	10/1/2023	9/30/2028

PARAMETER	EFFLUENT LIMITATION					MONITORING REQUIREMENTS				FN
								Location		
	Type	Limit	Units	Limit	Units	Sample Frequency	Sample Type	Inf.	Eff.	
Flow	Daily Maximum	13,300	GPD	-	-	Continuous	Recorder		X	
pH	Daily Minimum	6.0	SU	-	-	1/day	Grab		X	
	Daily Maximum	9.0	SU	-	-					
Temperature	Daily Maximum	Monitor	ºF	-	-	1/day	Grab		X	
BOD_5	Monthly Average	30	mg/L	-	-	1/month	6-hr. Comp.	X	X	
	Minimum Monthly Average	85	%	-	-	1/month	6-hr. Comp.		X	1
Total Suspended Solids (TSS)	Monthly Average	30	mg/L	-	-	1/month	Select	X	X	
	Minimum Monthly Average	85	%	-	-	1/month	6-hr. Comp.		X	1
Settleable Solids	Daily Maximum	0.3	mL/L	-	-	1/day	Grab		X	
Dissolved Oxygen	Daily Minimum	Monitor	mg/L	-	-	1/day	Grab		X	
Ammonia (as N) June 1st – October 31st	Monthly Average	4.2	mg/L	-	-	1/month	6-hr. Comp.		X	
Ammonia (as N) November 1st – May 31st	Monthly Average	6.4	mg/L	-	-	1/month	6-hr. Comp.		X	
Oil & Grease	Daily Maximum	15	mg/L	-	-	1/month	Grab		X	

EFFLUENT DISINFECTION Required Seasonal from May 1st - October 31st		Limit	Units	Limit	Units	Sample Frequency	Sample Type	Inf.	Eff.	FN
Coliform, Fecal	30-Day Geometric Mean	200	No./ 100 mL	-	-	1/month	Grab		X	
Coliform, Fecal	7-Day Geometric Mean	400	No./ 100 mL	-	-	1/month	Grab		X	
Chlorine, Total Residual	Daily Maximum	0.03	mg/L	-	-	1/month	Grab		X	2

For Footnotes, See Page 6

OUTFALL	DESCRIPTION	RECEIVING WATER	EFFECTIVE	EXPIRING
01A	Wash Water and Compressor Condensate	Hudson River via Outfall 001	10/1/2023	9/30/2028

PARAMETER	EFFLUENT LIMITATION					MONITORING REQUIREMENTS		FN
	Type	Limit	Units	Limit	Units	Sample Frequency	Sample Type	
Flow	Daily Maximum	2,100	GPD			1/month	Estimate	
pH	Daily Minimum	6.0	SU	-	-	1/month	Grab	
	Daily Maximum	9.0	SU	-	-			
Total Suspended Solids (TSS)	Daily Maximum	50	mg/L	-	-	1/month	Grab	
Oil & Grease	Daily Maximum	15	mg/L	-	-	1/month	Grab	
Foam (visible)	Daily Maximum	None	visible	-	-	1/month	Grab	3

FOOTNOTES:

1. Effluent shall not exceed 15% and 15% of influent concentration values for BOD_5 and TSS respectively.
2. Sampling and reporting for total residual chlorine is only necessary if chlorine is used for disinfection, elsewhere in the treatment process, or the facility otherwise has reasonable potential to discharge chlorine. Otherwise, the permittee shall report NODI-9 on the DMR.
3. **Visible Foam Analytical Method Procedure**
    a. Fill one (1) 500 mL narrow mouth bottle (glass or plastic) with effluent water to be tested.
    b. Upon return to the lab, fill a 1000 mL Wheaton narrow mouth glass sample bottle to the 200 mL mark with effluent from the 500mL bottle.
    c. Place the bottle with 200 mL of sample in a constant-temperature bath for a minimum of 1 hour and a maximum of 2 hours at 25 ± 1°C (77 ± 1.8°F).
    d. Measure the temperature of the sample and adjust to 25 ± 1°C (77 ± 1.8°F) if necessary.
    e. Remove the sample from the constant-temperature bath.
    f. Vigorously shake the sample bottle using a minimum of an 8-inch stroke and 40 shakes in less than 10 seconds.
    g. After completing 40 shakes, start a timer and allow the bottle to stand undisturbed.
    h. If any foam remains after 60 seconds, the sample will be noted as containing visible foam. If no foam remains after 60 seconds the sample will be noted as not containing visible foam.

STORMWATER POLLUTION PREVENTION REQUIREMENTS

Stormwater discharges at this facility are required to obtain coverage under the current Multi-Sector General Permit (MSGP) Sector AA (GP-0-23-001).

BEST MANAGEMENT PRACTICES (BMPs) FOR INDUSTRIAL FACILITIES

Note that for some facilities, especially those with few employees or limited industrial activities, some of the below BMPs may not be applicable. It is acceptable in these cases to indicate “Not Applicable” for the portion(s) of the BMP Plan that do not apply to your facility, along with an explanation.

1. **General** - The permittee shall develop, maintain, and implement a Best Management Practices (BMP) plan to prevent releases of significant amounts of pollutants to the waters of the State through plant site runoff; spillage and leaks; sludge or waste disposal; and stormwater discharges including, but not limited to, drainage from raw material storage. The BMP plan shall be documented in narrative form and shall include the 13 minimum BMPs and any necessary plot plans, drawings, or maps. Other documents already prepared for the facility such as a Safety Manual or a Spill Prevention, Control and Countermeasure (SPCC) plan may be used as part of the plan and may be incorporated by reference. A copy of the current BMP plan shall be submitted to the New York State Department of Environmental Conservation (Department) as required in item (2.) below and a copy must be maintained at the facility and shall be available to authorized Department representatives upon request.

2. **Compliance Deadlines** – The initial BMP plan shall be submitted in accordance with the Schedule of Submittals to the Regional Water Engineer. The BMP plan shall be implemented within 6 months of submission, unless a different time frame is approved by the Department. The BMP plan **shall be reviewed annually** and shall be modified whenever (a) changes at the facility materially increase the potential for releases of pollutants; (b) actual releases indicate the plan is inadequate, or (c) a letter from the Department identifies inadequacies in the plan. The permittee shall certify in writing, as an attachment to the December Discharge Monitoring Report (DMR), that the annual review has been completed. Subsequent modifications to or renewal of this permit does not reset or revise these deadlines unless a new deadline is set explicitly by such permit modification or renewal.

3. **Facility Review** - The permittee shall review all facility components or systems (including but not limited to material storage areas; in-plant transfer, process, and material handling areas; loading and unloading operations; storm water, erosion, and sediment control measures; process emergency control systems; and sludge and waste disposal areas) where materials or pollutants are used, manufactured, stored or handled to evaluate the potential for the release of pollutants to the waters of the State. In performing such an evaluation, the permittee shall consider such factors as the probability of equipment failure or improper operation, cross-contamination of storm water by process materials, settlement of facility air emissions, the effects of natural phenomena such as freezing temperatures and precipitation, fires, and the facility's history of spills and leaks. The relative toxicity of the pollutant shall be considered in determining the significance of potential releases. The review shall address all substances present at the facility that are identified in the SPDES application Form NY-2C (available at https://www.dec.ny.gov/docs/permits_ej_operations_pdf/form2c.pdf) or that are required to be monitored for by the SPDES permit.**13 Minimum BMPs:** Whenever the potential for a release of pollutants to State waters is determined to be present, the permittee shall identify BMPs that have been established to prevent or minimize such potential releases. Where BMPs are inadequate or absent, appropriate BMPs shall be established. In selecting appropriate BMPs, the permittee shall consider good industry practices and, where appropriate, structural measures such as secondary containment and erosion/sediment control devices and practices. USEPA guidance for development of stormwater elements of the BMP is available in *Developing Your Stormwater Pollution Prevention Plan A Guide for Industrial Operators*, February 2009, EPA 833-B-09-002.As a minimum, the plan shall include the following BMPs:

    1. BMP Pollution Prevention Team
    2. Reporting of BMP Incidents
    3. Risk Identification & Assessment
    4. Employee Training
    5. Inspections and Records
    6. Security
    7. Preventive Maintenance
    8. Good Housekeeping
    9. Materials/Waste Handling, Storage, & Compatibility
    10. Spill Prevention & Response
    11. Erosion & Sediment Control
    12. Management of Runoff
    13. Street Sweeping

BMPs FOR INDUSTRIAL FACILITIES (continued)

4. **Stormwater Pollution Prevention Plans (SWPPPs) Required for Discharges of Stormwater from Construction Activity to Surface Waters -** A SWPPP shall be developed prior to commencing any construction activity that will result in soil disturbance of one or more acres of uncontaminated area[1]. (Note: the disturbance threshold is 5000 SF in the New York City East of Hudson Watershed). The SWPPP shall conform to the current version of the SPDES General Permit for Stormwater Discharges from Construction Activity (CGP), including the *New York Standards and Specifications for Erosion and Sediment Control* and *New York State Stormwater Management Design Manual.* The permittee shall submit a copy of the SWPPP and any amendments thereto to the local governing body and any other authorized agency having jurisdiction or regulatory control over the construction activity **at least 30 days prior to soil disturbance**. The SWPPP shall be maintained on-site and submitted to the Department only upon request. When a SWPPP is required, a properly completed *Notice of Intent* (NOI) form shall be submitted (available at www.dec.ny.gov/chemical/43133.html) prior to soil disturbance. Note that submission of the NOI is required for informational purposes; the permittee is not eligible for and will not obtain coverage under any SPDES general permit for stormwater discharges. SWPPPs must be developed for subsequent site disturbances in accordance with the above requirements. The permittee is responsible for ensuring that the provisions of each SWPPP are properly implemented.

5. **Required Sampling For "Hot Spot" Identification** - Development of the BMP plan shall include sampling of waste stream segments for the purpose of pollutant "hot spot" identification. The economic achievability of effluent limits will not be considered until plant site "hot spot" sources have been identified, contained, removed or minimized through the imposition of site specific BMPs or application of internal facility treatment technology. For the purposes of this permit condition a "hot spot" is a segment of an industrial facility (including but not limited to soil, equipment, material storage areas, sewer lines etc.) which contributes elevated levels of problem pollutants to the wastewater and/or stormwater collection system of that facility. For the purposes of this definition, problem pollutants are substances for which treatment to meet a water quality or technology requirement may, considering the results of waste stream segment sampling, be deemed unreasonable. For the purposes of this definition, an elevated level is a concentration or mass loading of the pollutant in question which is sufficiently higher than the concentration of that same pollutant at the compliance monitoring location so as to allow for an economically justifiable removal and/or isolation of the segment and/or BAT treatment of wastewaters emanating from the segment.

6. **Facilities with Petroleum and/or Chemical Bulk Storage (PBS and CBS) Areas** - Compliance must be maintained with all applicable regulations including those involving releases, registration, handling and storage (6 NYCRR 595-599 and 612-614). Stormwater discharges from handling and storage areas should be eliminated where practical.

   A. Spill Cleanup - All spilled or leaked substances must be removed from secondary containment systems as soon as practical and for CBS storage areas within 24 hours, unless written authorization is received from the Department. The containment system must be thoroughly cleaned to remove any residual contamination which could cause contamination of stormwater and the resulting discharge of pollutants to waters of the State. Following spill cleanup, the affected area must be completely flushed with clean water three times and the water removed after each flushing for proper disposal in an on-site or off-site wastewater treatment plant designed to treat such water and permitted to discharge such wastewater. Alternately, the permittee may test the first batch of stormwater following the spill cleanup to determine discharge acceptability. If the water contains no pollutants at concentrations above the applicable effluent limits or Action Levels it may be discharged. Otherwise, it must be disposed of as noted above. See *Discharge Monitoring* below for the list of parameters to be sampled for.

   B. Discharge Operation - Stormwater must be removed before it compromises the required containment system capacity. Each discharge may only proceed with the prior approval of the permittee staff person responsible for ensuring SPDES permit compliance. Bulk storage secondary containment drainage systems must be locked in a closed position except when the operator is in the process of draining accumulated stormwater. Transfer area secondary containment drainage systems must be locked in a closed position during all transfers to or from these systems and must not be reopened unless the transfer area is clean of contaminants. Stormwater discharges from secondary containment systems should be avoided during periods of precipitation. A logbook shall be maintained on site noting the date, time and personnel supervising each discharge.

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[1] Uncontaminated area means soils which are free of contamination by any toxic or non-conventional pollutants identified in the tables of SPDES Application Form NY-2C. Disturbance of any size contaminated area(s) and the resulting discharge of contaminated stormwater is not authorized by this permit unless the discharge is under State or Federal oversight as part of a remedial program or after review by the Regional Water Engineer; nor is such discharge authorized by any SPDES general permit for stormwater discharges.

BMPs FOR INDUSTRIAL FACILITIES (continued)

C. Discharge Screening - Prior to each discharge from a secondary containment system the stormwater must be screened for contamination.* All stormwater must be inspected for visible evidence of contamination. Additional screening methods shall be developed by the permittee as part of the overall BMP Plan, e.g. the use of volatile gas meters to detect the presence of gross levels of gasoline or volatile organic compounds. If the screening indicates contamination, the permittee must collect and analyze a representative sample** of the stormwater. If the water contains no pollutants at concentrations above the applicable effluent limits or Action Levels it may be discharged. Otherwise, it must either be disposed of in an onsite or off-site wastewater treatment plant designed to treat and permitted to discharge such wastewater or the Regional Water Engineer can be contacted to determine if it may be discharged without treatment.

D. Discharge Monitoring - Unless the discharge from any bulk storage containment system outlet is identified in the SPDES permit as an outfall with explicit effluent and monitoring requirements, the permittee shall monitor the outlet as follows:

(i) *Bulk Storage Secondary Containment Systems:*

(a) The volume of each discharge from each outlet must be monitored. Discharge volume may be calculated by measuring the depth of water within the containment area times the wetted area converted to gallons or by other suitable methods. A representative sample shall be collected of the first discharge* following any cleaned-up spill or leak. The sample must be analyzed for pH, the substance(s) stored within the containment area and any other pollutants the permittee knows or has reason to believe are present.**

(b) Every fourth discharge* from each outlet must be sampled for pH, the substance(s) stored within the containment area and any other pollutants the permittee knows or has reason to believe are present.**

(ii) *Transfer Area Secondary Containment Systems:*

The first discharge* following any spill or leak must be sampled for flow, pH, the substance(s) transferred in that area and any other pollutants the permittee knows or has reason to believe are present.**

E. Discharge Reporting - Any results of monitoring required above, excluding screening data, must be submitted to the Department by appending them to the corresponding DMR. Failure to perform the required discharge monitoring and reporting shall constitute a violation of the terms of the SPDES permit.

F. Prohibited Discharges - **In all cases, any discharge which contains a visible sheen, foam, or odor, or may cause or contribute to a violation of water quality is prohibited.** The following discharges are prohibited unless specifically authorized elsewhere in this SPDES permit: spills or leaks, tank bottoms, maintenance wastewaters, wash waters where detergents or other chemicals have been used, tank hydrotest and ballast waters, contained firefighting runoff, fire training water contaminated by contact with pollutants or containing foam or fire-retardant additives, and unnecessary discharges of water or wastewater into secondary containment systems.

* Discharge includes stormwater discharges and snow and ice removal. If applicable, a representative sample of snow and/or ice should be collected and allowed to melt prior to assessment.

** If the stored substance is gasoline or aviation fuel then sample for oil & grease, benzene, ethylbenzene, naphthalene, toluene and total xylenes. If the stored substance is kerosene, diesel fuel, fuel oil, or lubricating oil then sample for oil & grease and polynuclear aromatic hydrocarbons (PAHs). The analytical methods selected for monitoring the stored substances are to be the most sensitive in detecting and quantifying the target analytes as approved under 40 CFR Part 136 and in compliance with NYSDOH ELAP certified methods or as directed by the Department. If the substance(s) are listed in the tables of SPDES Application Form NY-2C then sampling is required. Contact the facility inspector for further guidance. In all cases flow and pH monitoring is required.

MERCURY MINIMIZATION PROGRAM (MMP) - Type IV

On June 17, 2023, the permittee submitted a Conditional Exclusion Certification, certifying that the facility does not have any of the mercury sources listed in Part III.A.3. of DOW 1.3.10.

1. General - The permittee must develop, implement, and maintain a mercury minimization program (MMP), containing the elements set forth below.
2. MMP Elements - The MMP must be a written document and must include any necessary drawings or maps of the facility and/or collection system. Other related documents already prepared for the facility may be used as part of the MMP and may be incorporated by reference. At a minimum, the MMP must include the following elements[2] as described in detail below:

    a. Conditional Exclusion Certification - A certification (Appendix D of *DOW 1.3.10*), signed in accordance with 750-1.8 Signature of SPDES forms, must be submitted once every five (5) years to the Regional Water Engineer and to the Bureau of Water Permits certifying that Outfalls for the facility are neither a mercury source nor receives flows from a mercury source. Criteria to determine if a facility has a mercury source are as follows:

        - The facility is or receives discharge from 1) individually permitted combined sewer overflow (CSOs)[3] communities and/or 2) Type II sanitary sewer overflow (SSO)[4] facilities;
        - One or more effluent samples which exceed 12 ng/L, including samples taken as a result of the SPDES application process;
        - Internal or tributary waste stream samples exceed the GLCA effluent limitation **AND** the final effluent samples are less than the GLCA due primarily to dilution by uncontaminated or less contaminated waste streams. Both components of this criterion may include samples taken as a result of the SPDES application process;
        - A permit application or other information indicates that mercury is handled on site and could be discharged through outfalls;
        - Outfalls which contain legacy mercury contamination;
        - The facility's collection system receives discharges from a dental and/or categorical industrial user (CIU)[5] that may discharge mercury;
        - The facility accepts hauled wastes; or,
        - The facility is defined as a categorical industry that may discharge mercury. This may also include dentists, universities, hospitals, or laboratories which have their own SPDES permit.

    b. Control Strategy - The control strategy must contain the following minimum elements:
        i. Equipment and Materials – Equipment and materials (e.g., thermometers, thermostats) used by the permittee, which may contain mercury, must be evaluated by the permittee. As equipment and materials containing mercury are updated/replaced, the permittee must use mercury-free alternatives, if possible.
        ii. Bulk Chemical Evaluation – For chemicals, used at a rate which exceeds 1,000 gallons/year or 10,000 pounds/year, the permittee must obtain a manufacturer's certificate of analysis, a chemical analysis performed by a certified laboratory, and/or a notarized affidavit which describes the substances' mercury concentration and the detection limit achieved. If possible, the permittee must only use bulk chemicals utilized in the wastewater treatment process which contain <10 ppb mercury.

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[2] Neither monitoring nor outreach is required for facilities meeting the criteria for MMP Type IV, but monitoring and/or outreach can be included in the permittee's control strategy.

[3] CSO permits are included under the 05 and 07 permit classifications.

[4] These are overflow retention facilities (ORFs) and are included under the 05 and 07 permit classifications.

[5] CIUs include those listed under Federal Regulation in 40 CFR Part 400.

MERCURY MINIMIZATION PROGRAM (MMP) – Type IV (Continued)

c. **<u>Status Report</u>** - An **annual** status report must be developed and maintained on site, in accordance with the <u>Schedule of Additional Submittals</u>, summarizing:

   i. Review of criteria to determine if the facility has a potential mercury source;
      a. If the permittee no longer meets the criteria for MMP Type IV, the permittee must notify the Department for a permittee-initiated permit modification;
   ii. All actions undertaken, pursuant to the control strategy, during the previous year; and
   iii. Actions planned, pursuant to the control strategy, for the upcoming year.

   The permittee must maintain a file with all MMP documentation. The file must be available for review by Department representatives and copies must be provided upon request in accordance with 6 NYCRR 750-2.1(i) and 750-2.5(c)(4).

3. <u>MMP Modification</u> - The MMP must be modified whenever:
   a. Changes at the facility, or within the collection system, increase the potential for mercury discharges;
   b. A letter from the Department identifies inadequacies in the MMP.

   The Department may use information in the annual status reports, in accordance with 2.c of this MMP, to determine if the permit limitations and MMP Type is appropriate for the facility.

DEFINITIONS:

Potential mercury source – a source identified by the permittee that may reasonably be expected to have total mercury contained in the discharge. Some potential mercury sources include switches, fluorescent lightbulbs, cleaners, degreasers, thermometers, batteries, hauled wastes, universities, hospitals, laboratories, landfills, Brownfield sites, or raw material storage.

DISCHARGE NOTIFICATION REQUIREMENTS

(a) The permittee shall install and maintain identification signs at all outfalls to surface waters listed in this permit, unless the permittee has obtained a waiver in accordance with the Discharge Notification Act (DNA). Such signs shall be installed before initiation of any new discharge location.

(b) Subsequent modifications to or renewal of this permit does not reset or revise the deadline set forth in (a) above, unless a new deadline is set explicitly by such permit modification or renewal.

(c) The Discharge Notification Requirements described herein do not apply to outfalls from which the discharge is composed exclusively of storm water, or discharges to ground water.

(d) The sign(s) shall be conspicuous, legible and in as close proximity to the point of discharge as is reasonably possible while ensuring the maximum visibility from the surface water and shore. The signs shall be installed in such a manner to pose minimal hazard to navigation, bathing or other water related activities. If the public has access to the water from the land in the vicinity of the outfall, an identical sign shall be posted to be visible from the direction approaching the surface water.

The signs shall have **minimum** dimensions of eighteen inches by twenty-four inches (18" x 24") and shall have white letters on a green background and contain the following information:

N.Y.S. PERMITTED DISCHARGE POINT

SPDES PERMIT No.: NY__________

OUTFALL No. :____

For information about this permitted discharge contact:

Permittee Name: ____

Permittee Contact: ____

Permittee Phone: ( ) - ### - ####

OR:

NYSDEC Division of Water Regional Office Address:

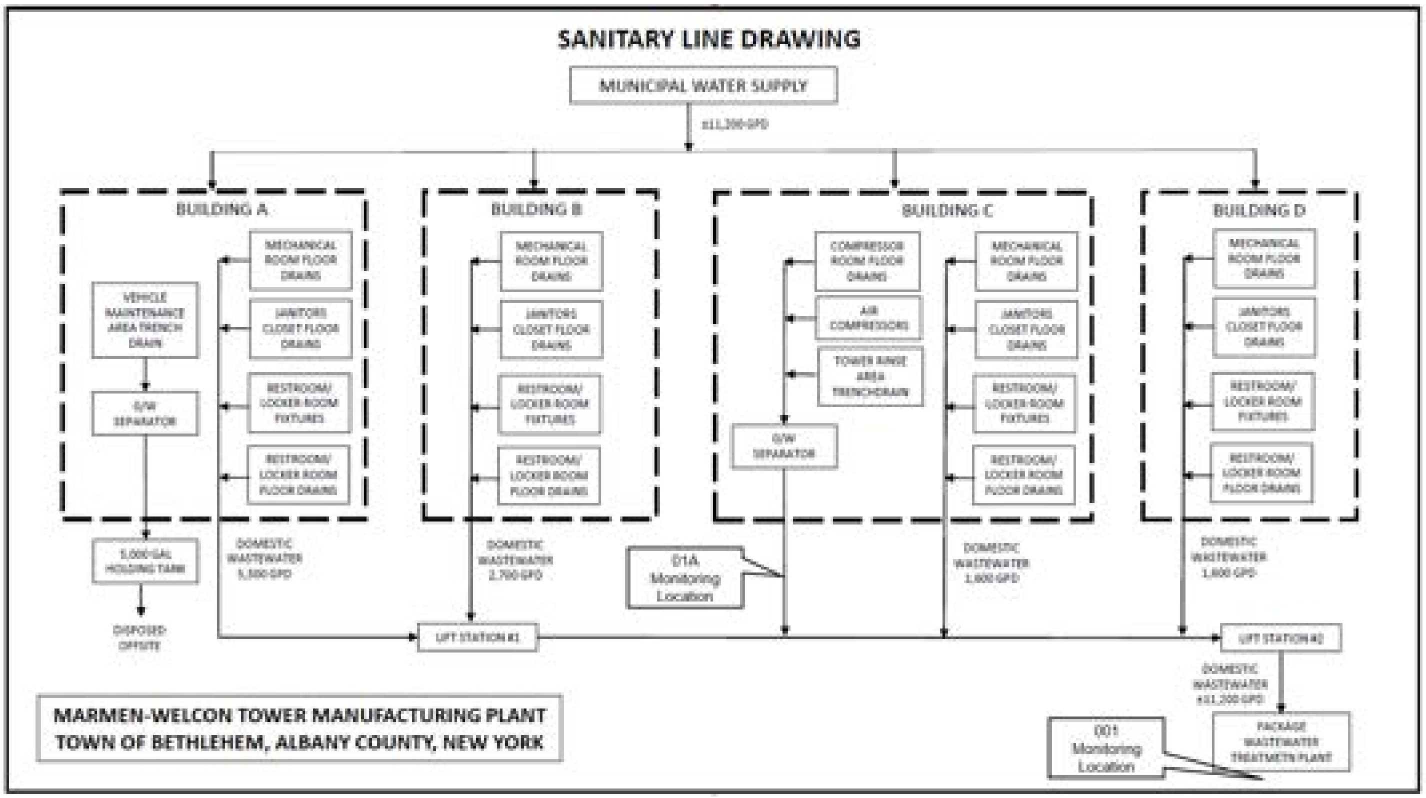
NYSDEC Division of Water Regional Phone: ( ) - ### -####

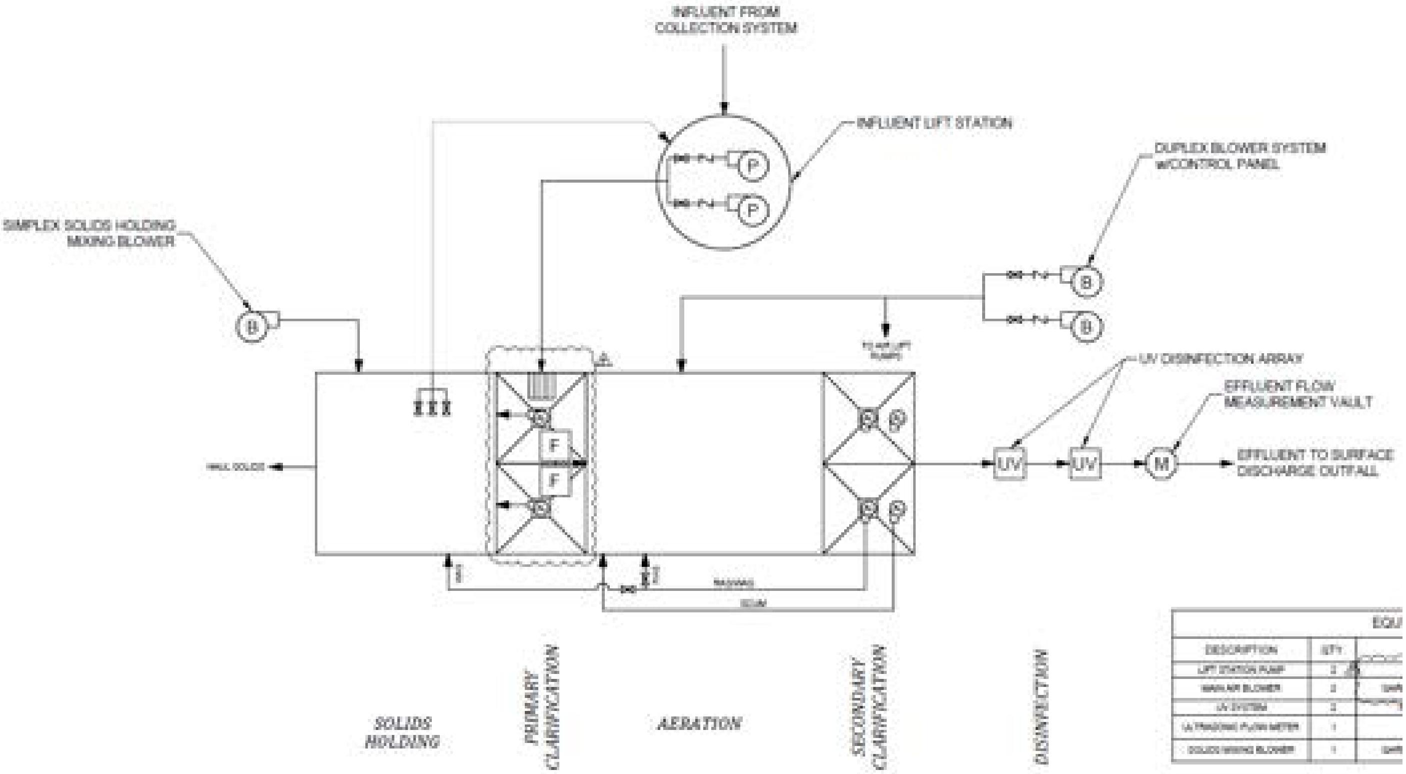
(e) Upon request, the permittee shall make available electronic or hard copies of the sampling data to the public. In accordance with the RECORDING, REPORTING AND ADDITIONAL MONITORING REQUIREMENTS page of your permit, each DMR shall be maintained (either electronically or as a hard copy) on record for a period of five years.

(f) The permittee shall periodically inspect the outfall identification sign(s) in order to ensure they are maintained, are still visible, and contain information that is current and factually correct. Signs that are damaged or incorrect shall be replaced within 3 months of inspection.

MONITORING LOCATIONS

The permittee shall take samples and measurements, to comply with the monitoring requirements specified in this permit, at the locations(s) specified below:





DESCRIPTION	QTY.
LIFT STATION PUMP	2
MAIN AIR BLOWER	2
UV SYSTEM	2
ULTRASONIC FLOW METER	1
SOLIDS MIXING BLOWER	1

GENERAL REQUIREMENTS

A. The regulations in 6 NYCRR Part 750 are hereby incorporated by reference and the conditions are enforceable requirements under this permit. The permittee shall comply with all requirements set forth in this permit and with all the applicable requirements of 6 NYCRR Part 750 incorporated into this permit by reference, including but not limited to the regulations in paragraphs B through H as follows:

B. <u>General Conditions</u>

1.	Duty to comply	6 NYCRR 750-2.1(e) & 2.4
2.	Duty to reapply	6 NYCRR 750-1.16(a)
3.	Need to halt or reduce activity not a defense	6 NYCRR 750-2.1(g)
4.	Duty to mitigate	6 NYCRR 750-2.7(f)
5.	Permit actions	6 NYCRR 750-1.1(c), 1.18, 1.20 & 2.1(h)
6.	Property rights	6 NYCRR 750-2.2(b)
7.	Duty to provide information	6 NYCRR 750-2.1(i)
8.	Inspection and entry	6 NYCRR 750-2.1(a) & 2.3

C. Operation and Maintenance

1.	Proper Operation & Maintenance	6 NYCRR 750-2.8
2.	Bypass	6 NYCRR 750-1.2(a)(17), 2.8(b) & 2.7
3.	Upset	6 NYCRR 750-1.2(a)(94) & 2.8(c)

D. Monitoring and Records

1.	Monitoring and records	6 NYCRR 750-2.5(a)(2), 2.5(a)(6), 2.5(c)(1), 2.5(c)(2), & 2.5(d)
2.	Signatory requirements	6 NYCRR 750-1.8 & 2.5(b)

E. Reporting Requirements

1.	Reporting requirements for non-POTWs	6 NYCRR 750-2.5, 2.6, 2.7, &1.17
2.	Anticipated noncompliance	6 NYCRR 750-2.7(a)
3.	Transfers	6 NYCRR 750-1.17
4.	Monitoring reports	6 NYCRR 750-2.5(e)
5.	Compliance schedules	6 NYCRR 750-1.14(d)
6.	24-hour reporting	6 NYCRR 750-2.7(c) & (d)
7.	Other noncompliance	6 NYCRR 750-2.7(e)
8.	Other information	6 NYCRR 750-2.1(f)

F. Sludge Management
The permittee shall comply with all applicable requirements of 6 NYCRR Part 360.

G. SPDES Permit Program Fee
The permittee shall pay to the Department an annual SPDES permit program fee within 30 days of the date of the first invoice, unless otherwise directed by the Department, and shall comply with all applicable requirements of ECL 72-0602 and 6 NYCRR Parts 480, 481 and 485. Note that if there is inconsistency between the fees specified in ECL 72-0602 and 6 NYCRR Part 485, the ECL 72-0602 fees govern.

H. Water Treatment Chemicals (WTCs)
New or increased use and discharge of a WTC requires prior Department review and authorization. At a minimum, the permittee must notify the Department in writing of its intent to change WTC use by submitting a completed *WTC Notification Form* for each proposed WTC. The Department will review that submittal and determine if a SPDES permit modification is necessary or whether WTC review and authorization may proceed outside of the formal permit administrative process. The majority of WTC authorizations do not require SPDES permit modification. In any event, use and discharge of a WTC shall not proceed without prior authorization from the Department. Examples of WTCs include biocides, coagulants, conditioners, corrosion inhibitors, defoamers, deposit control agents, flocculants, scale inhibitors, sequestrants, and settling aids.

1. WTC use shall not exceed the rate explicitly authorized by this permit or otherwise authorized by the Department.
2. The permittee shall maintain a logbook of all WTC use, noting for each WTC the date, time, exact location, and amount of each dosage, and, the name of the individual applying or measuring the chemical. The logbook must also document that adequate process controls are in place to ensure excessive levels of WTCs are not used.
3. The permittee shall submit a completed WTC Annual Report Form each year that they use and discharge WTCs. This form shall be submitted in electronic format and attached to either the December DMR or the annual monitoring report required below. The *WTC Notification Form and WTC Annual Report Form* are available from the Department's website at: http://www.dec.ny.gov/permits/93245.html

RECORDING, REPORTING AND ADDITIONAL MONITORING REQUIREMENTS

A. The monitoring information required by this permit shall be retained for a period of at least five years from the date of the sampling for subsequent inspection by the Department or its designated agent.

B. Discharge Monitoring Reports (DMRs): Completed DMR forms shall be submitted for each 1 month reporting period in accordance with the DMR Manual available on Department's website.

DMRs must be submitted electronically using the electronic reporting tool (NetDMR) specified by NYSDEC. Instructions on the use of NetDMR can be found at https://www.dec.ny.gov/chemical/103774.html. **Hardcopy paper DMRs will only be received at the address listed below, directed to the Bureau of Water Compliance, if a waiver from the electronic submittal requirements has been granted by DEC to the facility.**

The first monitoring period begins on the effective date of this permit, and, unless otherwise required, the reports are due no later than the 28th day of the month following the end of each monitoring period.

C. Additional information required to be submitted by this permit shall be summarized and reported to the Regional Water Engineer and Bureau of Water Permits at the following addresses:

Department of Environmental Conservation
Division of Water, Bureau of Water Permits
625 Broadway, Albany, New York 12233-3505 Phone: (518) 402-8111

Department of Environmental Conservation
Regional Water Engineer, Region 4
1130 North Westcott Road, Schenectady, New York, 12306-2014 Phone: (518) 357-2045

D. Schedule of Additional Submittals:

The permittee shall submit the following information to the Regional Water Engineer and to the Bureau of Water Permits, unless otherwise instructed:

Outfall(s)	SCHEDULE OF ADDITIONAL SUBMITTALS - Required Action	Due Date
001, 01A	COMPLETION OF CONSTRUCTION The permittee shall provide a Certificate of Completion[6] (COC) to DEC that the treatment system has been completed in accordance with the approved Design Documents.	With 30 days of Completion of Construction
001, 01A	COMMENCEMENT OF OPERATION Upon submission of the COC, the permittee shall comply with the final effluent limitations described in this permit.	Upon Submission of COC

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[6] 6 NYCRR 750-2.10 (c)

Outfall(s)	SCHEDULE OF ADDITIONAL SUBMITTALS - Required Action	Due Date
001, 01A	SHORT TERM MONITORING PROGRAM For Outfalls 001 and 01A, the permittee shall complete a short term monitoring program. The permittee must collect quarterly samples for a period of 1 year for pollutants as identified in the NY-2C SPDES application, Tables A – D. Samples must be representative of normal discharge conditions and treatment operations at the facility. The permittee must use approved EPA analytical methods with the lowest possible detection limit as promulgated under 40 CFR Part 136 for the determination of the concentrations of parameters listed. Method 1631 shall be used for Mercury analysis. Method 1633 shall be used for analysis of PFAS. Quarterly sampling must begin during the first quarter of 2025, unless otherwise approved by DEC. DEC must be notified at least 30 days before sampling is to commence.	Submit sample results no later than January 28, 2026
-	BMP PLAN The permittee shall submit and annually review the completed BMP plan. The BMP plan shall be modified whenever: (a) changes at the facility materially increase the potential for releases of pollutants, (b) actual releases indicate the plan is inadequate, or (c) a letter from the Department identifies inadequacies in the plan. The permittee shall certify in writing, as an attachment to the December Discharge Monitoring Report (DMR), that the annual review has been completed. All BMP plan revisions must be submitted to the Regional Water Engineer within 30 days.	EDP + 6 Months, Annually thereafter on January 28th
001, 01A	WATER TREATMENT CHEMICAL (WTC) ANNUAL REPORT FORM The permittee shall submit a completed WTC Annual Report Form each year that Water Treatment Chemicals are used. The form shall be attached to the December DMR.	Annually on January 28th if WTCs are used
001	MERCURY MINIMIZATION PLAN - SAMPLING Complete Mercury Sampling as required in Short Term Monitoring Program above.	Submit sample results no later than January 28, 2026
001	MERCURY MINIMIZATION PLAN – ANNUAL STATUS REPORT The permittee must complete and maintain onsite an annual mercury minimization status report in accordance with the requirements of this permit.	Annually on January 28th
001	MERCURY - CONDITIONAL EXCLUSION CERTIFICATION Permittee must submit a mercury conditional exclusion certification every five years to maintain MMP Type IV status.	EDP + 5 years, every 5 years thereafter

Unless noted otherwise, the above actions are one-time requirements.

E. Monitoring and analysis shall be conducted using sufficiently sensitive test procedures approved under 40 CFR Part 136, unless other test procedures have been specified in this permit.

F. More frequent monitoring of the discharge(s), monitoring point(s), or waters of the State than required by the permit, where analysis is performed by a certified laboratory or where such analysis is not required to be performed by a certified laboratory, shall be included in the calculations and recording of the data on the corresponding DMRs.

G. Calculations which require averaging of measurements shall utilize an arithmetic mean unless otherwise specified in this permit.

H. Unless otherwise specified, all information recorded on the DMRs shall be based upon measurements and sampling carried out during the most recently completed reporting period.

I. Any laboratory test or sample analysis required by this permit for which the State Commissioner of Health issues certificates of approval pursuant to section 502 of the Public Health Law shall be conducted by a laboratory which has been issued a certificate of approval. Inquiries regarding laboratory certification should be directed to the New York State Department of Health, Environmental Laboratory Accreditation Program.

SPDES Permit Fact Sheet Albany Port District Commission Marmen-Welcon Manufacturing Plant NY0312924

NEW YORK STATE | Department of Environmental Conservation

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Summary of Permit Changes

A new State Pollutant Discharge Elimination System (SPDES) permit has been drafted for the Marmen-Welcon Manufacturing Plant.

This factsheet summarizes the information used to determine the effluent limitations (limits) and other conditions contained in the permit. General background information including the regulatory basis for the effluent limitations and other conditions are in the Appendix linked throughout this factsheet.

Administrative History

6/1/2022	The Albany Port District Commission submitted a NY-2C permit application for a new facility. Revised NY-2C permit applications were submitted 8/3/22 and 5/17/23.

The Notice of Complete Application, published in the Environmental Notice Bulletin and newspapers, contains information on the public notice process.

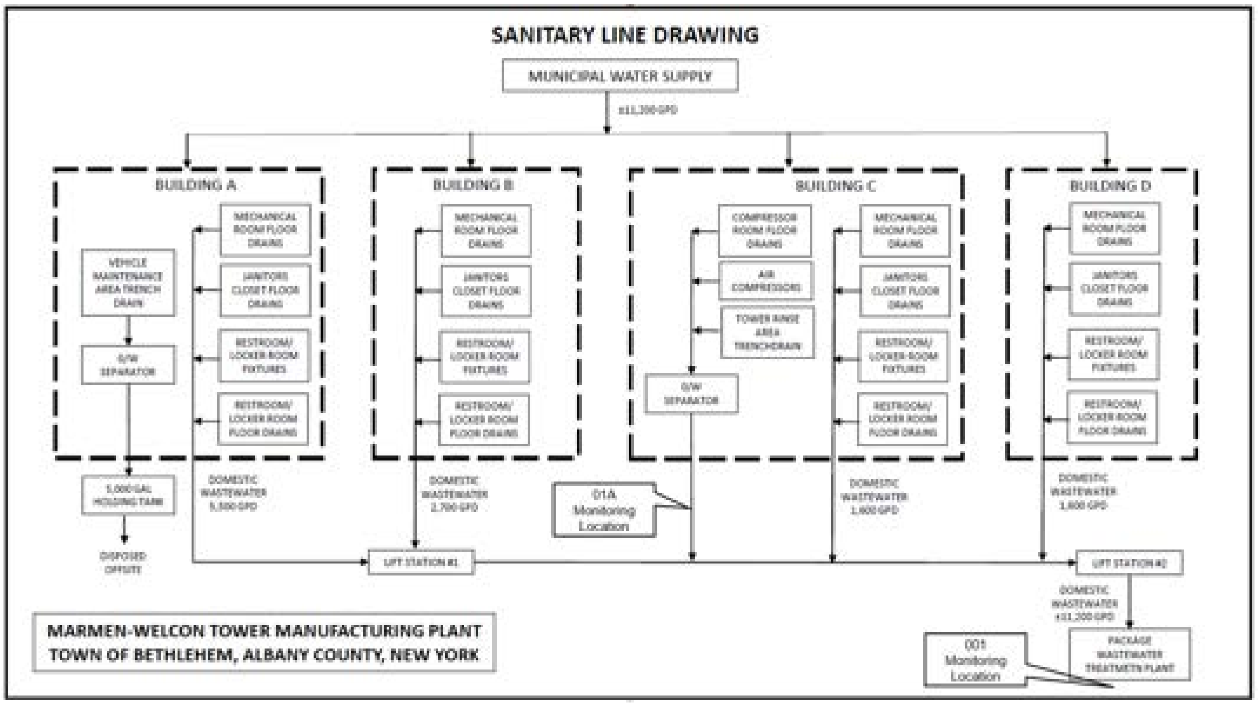
Facility Information

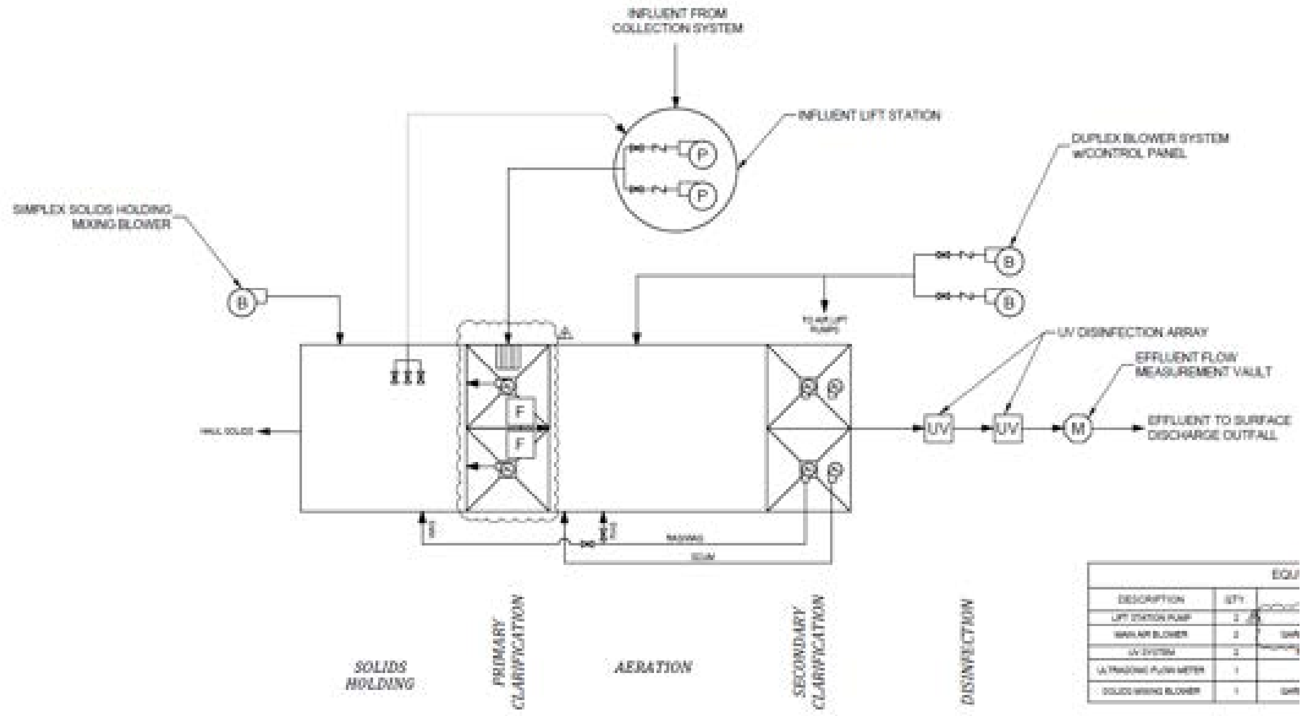
This is an industrial facility (SIC codes 3499, 3449, 3999) that fabricates and manufactures offshore wind tower sections and transition piece sections. The tower sections will be coated in Ecogel (primary ingredient propylene glycol) and rinsed for final phases of manufacturing. The treatment system will be constructed in 2023. Tower washdown and compressor condensate will be treated through an oil and water separator. The vehicle maintenance area will go to an oil and water separator followed by a holding tank for off-site disposal. Restrooms and locker room floor drains will go to the package wastewater treatment plant for treatment. The package plant will provide aerobic digestion and includes the following treatment units:

- Preliminary Treatment: Screening
- Primary Treatment: Primary Clarification
- Secondary Treatment: Activated Sludge
- Disinfection: UV

Sludge will be wet hold and haul.

Site Overview





Existing Effluent Quality

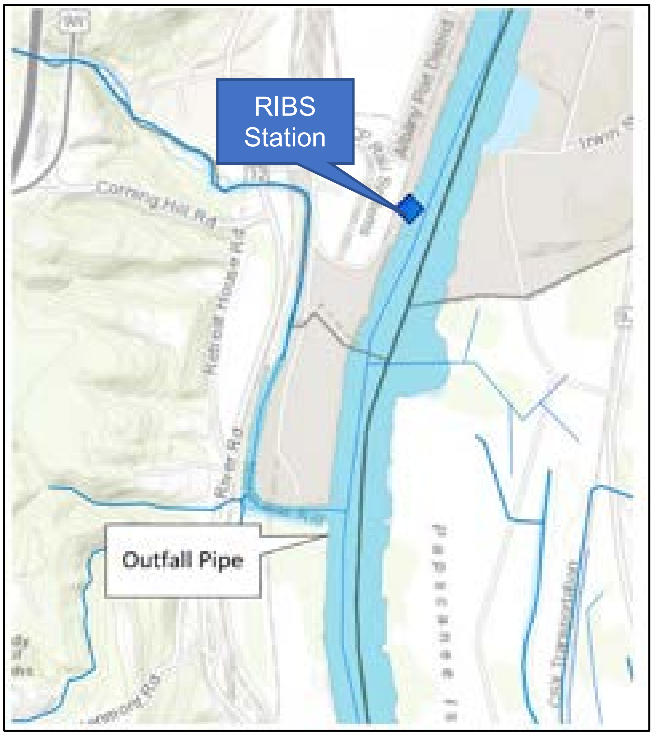
The Pollutant Summary Table presents the existing effluent quality and effluent limitations. The existing effluent quality was determined from the application submitted by the permittee. Appendix Link

Receiving Water Information

The facility proposes to discharge via the following outfalls:

Outfall No.	SIC Code	Wastewater Type	Receiving Water
001		Treated Sanitary Sewage	Hudson River, Class C
01A (internal)		Plain (no detergent) wash water, air compressor condensate	Hudson River via 001, Class C
DR-1		Stormwater	Authorized under MSGP permit
DR-2		Stormwater	Authorized under MSGP permit
DR-3		Stormwater	Authorized under MSGP permit
DR-4		Stormwater	Authorized under MSGP permit
DR-5		Stormwater	Authorized under MSGP permit
DR-6		Stormwater	Authorized under MSGP permit
DR-7		Stormwater	Authorized under MSGP permit

Reach Description: The outfall for the facility discharges to the Hudson River (PWL ID 1301-0002) at the confluence of the Normans Kill (H-221-4). The outfall pipe will be above Mean High Water (MHW).



See the Outfall and Receiving Water Summary Table and Appendix for additional information.

Impaired Waterbody Information

The Hudson River segment (PWL No. 1301-0002) was first listed on the 1998 New York State Section 303(d) List of Impaired/TMDL Waters as impaired due to PCBs from contaminated sediment. The segment continues to be listed as of the 2018 NYS Section 303(d) List. A TMDL has not been developed to address the impairment, and therefore, there are no applicable wasteload allocations (WLAs) for this facility.

Critical Receiving Water Data & Mixing Zone

Outfall 001 discharges to Hudson, which is a classified tidal waterbody. New York State Department of Environmental Conservation (NYSDEC or Department) Guidance (TOGS 1.3.1) states that a dilution ratio of 10:1 is appropriate for a fully submerged outfall discharging directly into a tidal waterbody. The discharge terminus point for this outfall is at the shoreline (bank discharge) and therefore the effluent mixing with the tidal waterbody will take place along the shoreline rather than the open waters of the Hudson River. The mixing intensity will be reduced due to low momentum of the discharge resulting in less dilution than the specified guidance value; therefore, a dilution ration of 5:1 for acute, chronic, and Human, Aesthetic, Wildlife (HEW) protections is appropriate and has been included in this permit.

Outfall No.	Acute Dilution Ratio A(A)	Chronic Dilution Ratio A(C)	Human, Aesthetic, Wildlife Dilution Ratio (HEW)	Basis
001	5:1	5:1	5:1	TOGS 1.3.1 (for ponded or tidal waterbodies)

Critical receiving water data are listed in the Pollutant Summary Table at the end of this fact sheet. Appendix Link

Permit Requirements

The technology based effluent limitations (TBELs), water quality-based effluent limitations (WQBELs), Existing Effluent Quality and a discussion of the selected effluent limitation for each pollutant present in the discharge are provided in the Pollutant Summary Table.

Whole Effluent Toxicity (WET) Testing

None of the seven criteria that are indicative of potential toxicity are applicable to this facility; therefore, WET testing is not included in the permit. Appendix Link

Anti-backsliding

This is a new permit; therefore, anti-backsliding does not apply.

Appendix Link

Antidegradation

The permit contains effluent limitations which ensure that the best usages of the receiving waters will be maintained. The Notice of Complete Application published in the Environmental Notice Bulletin contains information on the State Environmental Quality Review (SEQR)[1] determination. Appendix Link

Discharge Notification Act Requirements

In accordance with the Discharge Notification Act (ECL 17-0815-a), the permittee is required to post a sign at each point of wastewater discharge to surface waters, unless a waiver is obtained. This requirement is new.

Additionally, the permit contains a requirement to make the Discharge Monitoring Report (DMR) sampling data available to the public upon request. This requirement is new.

Best Management Practices (BMPs) for Industrial Facilities

In accordance with 6 NYCRR 750-1.14(f) and 40 CFR 122.44(k), the permittee is required to develop and implement a BMP plan that prevents, or minimizes the potential for, the release of toxic or hazardous pollutants to state waters. The BMP plan requires annual review by the permittee.

Stormwater Pollution Prevention Requirements

The facility discharges stormwater associated with industrial activity and requires SPDES permit coverage under 40 CFR 122.26(a)(6).

Stormwater discharges at this facility are required to obtain coverage under the current Multi-Sector General Permit (MSGP) Sector [AA] (GP-0-23-001).

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[1] As prescribed by 6 NYCRR Part 617

Schedule(s) of Additional Submittals

A schedule of additional submittals has been included for the following (Appendix Link):

- Complete Construction – Provide a Certificate of Completion
- Commence Operation
- Short Term Monitoring for New Discharges
    - Since this is a new facility, parameters listed under Tables A-D of the NY-2C SPDES application are required to be sampled to identify any other parameters associated with the industrial activities at the facility. In accordance with TOGS 1.3.13, emerging contaminants will be sampled as well.
- Water Treatment Chemical (WTC) Annual Report Form
- Best Management Practices Plan
- Mercury Minimization Plan – Sampling
- Mercury Minimization Plan – Annual Status Report
- Mercury – Conditional Exclusion Certification

OUTFALL AND RECEIVING WATER SUMMARY TABLE

Outfall	Latitude	Longitude	Receiving Water Name	Water Class	Water Index No. / Priority Waterbody Listing (PWL) No.	Major / Sub Basin	Hardness (mg/l)	1Q10 (MGD)	7Q10 (MGD)	30Q10 (MGD)	Critical Effluent Flow (GPD)	Dilution Ratio		
												A(A)	A(C)	HEW
001	42° 36' 24" N	73° 45' 48" W	Hudson River	C	H-5 PWL: 1301-0002	13 / 01	80[5]	Tidal Waterbody			13,300	5:1	5:1	5:1
01A	42° 36' 18" N	73° 46' 3" W	Hudson River via Outfall 001	C	H-5 PWL: 1301-0002	13 / 01	80[2]	Tidal Waterbody			2,100	5:1	5:1	5:1

POLLUTANT SUMMARY TABLE

Outfall 001

<table>
<tr><td colspan="2">**Outfall #**</td><td>001</td><td colspan="13">**Description of Wastewater:** Treated Sanitary Sewer and Plain (no detergent) Wash Water, Air Compressor Condensate from Outfall 01A.</td></tr>
<tr><td colspan="3"></td><td colspan="13">**Type of Treatment:** Screening, Primary Clarification, Activated Sludge, UV Disinfection</td></tr>
<tr><td rowspan="2">Effluent Parameter</td><td rowspan="2">Units</td><td rowspan="2">Averaging Period</td><td colspan="3">Existing Discharge Data</td><td colspan="2">TBELs</td><td colspan="6">Water Quality Data & WQBELs</td><td rowspan="2">ML</td><td rowspan="2">Basis for Permit Requirement</td></tr>
<tr><td>Permit Limit</td><td>Existing Effluent Quality[3]</td><td># of Data Points Detects / Non-Detects</td><td>Limit</td><td>Basis</td><td>Ambient Bkgd. Conc.</td><td>Projected Instream Conc.</td><td>WQ Std. or GV</td><td>WQ Type</td><td>Calc. WQBEL</td><td>Basis for WQBEL</td></tr>
<tr><td colspan="16">**General Notes:** This is a new facility; therefore, existing discharge data is not available. Proposed data was obtained from the application provided by the permittee. All applicable water quality standards were reviewed for development of the WQBELs. The standard and WQBEL shown below represent the most stringent.</td></tr>
<tr><td rowspan="2">Flow Rate</td><td>GPD</td><td>Monthly Avg</td><td>-</td><td>-</td><td>-</td><td>**13,300**</td><td>Design Flow</td><td colspan="5">Narrative: No alterations that will impair the waters for their best usages.</td><td>703.2</td><td>-</td><td>TBEL</td></tr>
<tr><td colspan="15">The flow limit is set at the design flow of the wastewater treatment facility.</td></tr>
<tr><td rowspan="3">pH</td><td rowspan="2">SU</td><td>Minimum</td><td>-</td><td>-</td><td>-</td><td>**6.0**</td><td rowspan="2">TOGS 1.2.1</td><td rowspan="2">7.90[4]</td><td rowspan="2">-</td><td rowspan="2">6.5 – 8.5</td><td rowspan="2">Range</td><td rowspan="2">-</td><td rowspan="2">703.3</td><td rowspan="2">-</td><td rowspan="2">TBEL</td></tr>
<tr><td>Maximum</td><td>-</td><td>-</td><td>-</td><td>**9.0**</td></tr>
<tr><td colspan="15">Consistent with TOGS 1.2.1, TBELs reflect the available treatment technology listed in Attachment C. Given the available dilution an effluent limitation equal to the TBEL is reasonably protective of the WQS.</td></tr>
</table>

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[2] Ambient hardness data obtained from RIBS Station 13-LHUD-125.8.

[3] Existing Effluent Quality: Permittee's estimate based on NY-2C application submitted for a new facility.

[4] Ambient pH obtained from HRECOS Port of Albany Water Quality 2016 data, 80th percentile.

Outfall #	001	**Description of Wastewater:** Treated Sanitary Sewer and Plain (no detergent) Wash Water, Air Compressor Condensate from Outfall 01A.
		Type of Treatment: Screening, Primary Clarification, Activated Sludge, UV Disinfection

Effluent Parameter	Units	Averaging Period	Existing Discharge Data			TBELs		Water Quality Data & WQBELs						ML	Basis for Permit Requirement
			Permit Limit	Existing Effluent Quality^3	# of Data Points Detects / Non-Detects	Limit	Basis	Ambient Bkgd. Conc.	Projected Instream Conc.	WQ Std. or GV	WQ Type	Calc. WQBEL	Basis for WQBEL		
Temperature	°F	Daily Max	-	-	-	**Monitor**	750-1.13 Monitor	-	Narrative (Non-Trout): The water temperature at the surface of a stream shall not be raised to more than 90F at any point and... shall not be raised or lowered to more than 5F over the temperature that existed before the addition				704.2	-	TBEL
	Consistent with 6 NYCRR 750-1.13(a), monitoring is required and may be used to inform future permitting decisions.														
Dissolved Oxygen (DO)	mg/L	Daily Min	-	-	-	2.0	TOGS 1.3.3	-	-	(Non-Trout) 4.0 mg/L	Narrative	-	703.3	-	Monitor
	As the facility is discharging to the tidal portion on the Hudson River (downstream of Troy Dam) DO sag curve models cannot be performed. As the flow of the facility is orders of magnitude smaller then flow of the Hudson River the need for WQBELS for DO and BOD_5 are unnecessary and the TBELs are protective of water quality.														
5-day Biochemical Oxygen Demand (BOD_5)	mg/L	Monthly Avg	-	-	-	**30**	TOGS 1.3.3	-	See Dissolved Oxygen			-	703.3	-	TBEL
		7 Day Avg	-	-	-	-	-					-			
	lbs/d	Monthly Avg	-	-	-	-	-					-			
		7 Day Avg	-	-	-	-	-					-			
	% Rem	Minimum	-	-	-	**85**	TOGS 1.3.3					-			
	Consistent with TOGS 1.3.3 for POTWs, TBELs reflect secondary treatment standards. See justification for Dissolved Oxygen. Sampling will be conducted monthly, therefore, 7-day average limit is not necessary.														
Total Suspended Solids (TSS)	mg/L	Monthly Avg	-	-	-	**30**	TOGS 1.3.3	-	Narrative: None from sewage, industrial wastes or other wastes that will cause deposition or impair the waters for their best usages.				703.2	-	TBEL
		7 Day Avg	-	-	-	-	-								
	lbs/d	Monthly Avg	-	-	-	-	-								
		7 Day Avg	-	-	-	-	-								
	% Rem	Minimum	-	-	-	**85**	TOGS 1.3.3								
	Consistent with TOGS 1.3.3 for POTWs, TBELs reflect secondary treatment standards. Given that adequate dilution is available, an effluent limitation equal to the TBEL, and consistent with TOGS 1.3.3, is reasonably protective of water quality standards. Sampling will be conducted monthly, therefore, 7-day average limit is not necessary.														

Outfall #	001	**Description of Wastewater:** Treated Sanitary Sewer and Plain (no detergent) Wash Water, Air Compressor Condensate from Outfall 01A.
		Type of Treatment: Screening, Primary Clarification, Activated Sludge, UV Disinfection

Effluent Parameter	Units	Averaging Period	Existing Discharge Data			TBELs		Water Quality Data & WQBELs						ML	Basis for Permit Requirement
			Permit Limit	Existing Effluent Quality^3	# of Data Points Detects / Non-Detects	Limit	Basis	Ambient Bkgd. Conc.	Projected Instream Conc.	WQ Std. or GV	WQ Type	Calc. WQBEL	Basis for WQBEL		
Settleable Solids	mL/L	Daily Max	-	-	-	**0.3**	TOGS 1.3.3	-	Narrative: None from sewage, industrial wastes or other wastes that will cause deposition or impair the waters for their best usages				703.2	-	TBEL
	Consistent with TOGS 1.3.3, the effluent limitation is equal to the TBEL of 0.3 mL/L for POTWs providing secondary treatment without filtration. Given that adequate dilution is available the TBEL is reasonably protective of WQS.														
Nitrogen, Ammonia (as N) June 1st – Oct. 31st	mg/L	Monthly Avg	-	-	-	-	-	0.081	-	.91	A(C)	**4.2**	703.5	-	WQBEL
	The WQS for Ammonia was determined from TOGS 1.1.1 from a summer pH of 7.9 and a temperature of 25. The temperature of the receiving waterbody was an assumed value and consistent with TOGS 1.3.1E. The pH of the receiving waterbody was calculated from HRECOS Port of Albany Water Quality 2016 data, 80th percentile. In accordance with TOGS 1.3.1E, the HEW dilution ratio was applied to calculate the applicable WQBEL for the facility.														
Nitrogen, Ammonia (as N) Nov. 1st – May 31st	mg/L	Monthly Avg	-	-	-	-	-	0.081	-	1.34	A(C)	**6.4**	703.5	-	WQBEL
	The WQS for Ammonia was determined from TOGS 1.1.1 from a summer pH of 7.9 and a temperature of 10. The temperature of the receiving waterbody was an assumed value and consistent with TOGS 1.3.1E. The pH of the receiving waterbody was calculated from HRECOS Port of Albany Water Quality 2016 data, 80th percentile. In accordance with TOGS 1.3.1E, the HEW dilution ratio was applied to calculate the applicable WQBEL for the facility.														
Total Mercury	ng/L	Daily Max	-	-	-	-	ILCA	-	-	0.7	H(FC)	-	-	-	DOW 1.3.10
	ng/L	12 MRA	-	-	-	-	EEQ	-	-	0.7	H(FC)	-	-	-	
	There are no known sources of mercury at the facility. Confirmatory sampling will be conducted after operation commences. See Mercury section of this factsheet for more details.														

Outfall #	001	**Description of Wastewater:** Treated Sanitary Sewer and Plain (no detergent) Wash Water, Air Compressor Condensate from Outfall 01A.
		Type of Treatment: Screening, Primary Clarification, Activated Sludge, UV Disinfection

Effluent Parameter	Units	Averaging Period	Existing Discharge Data			TBELs		Water Quality Data & WQBELs						ML	Basis for Permit Requirement
			Permit Limit	Existing Effluent Quality^3	# of Data Points Detects / Non-Detects	Limit	Basis	Ambient Bkgd. Conc.	Projected Instream Conc.	WQ Std. or GV	WQ Type	Calc. WQBEL	Basis for WQBEL		
Coliform, Fecal	#/100 ml	30d Geo Mean	-	-	-	**200**	TOGS 1.3.3	-	Narrative: The monthly geometric mean, from a minimum of five examinations, shall not exceed 200.				703.4	-	TBEL
		7d Geo Mean	-	-	-	**400**	TOGS 1.3.3	-							
	Consistent with TOGS 1.3.3, effluent disinfection is required seasonally from May 1st - October 31st, due to the class of the receiving waterbody. Fecal coliform limits equal to the TBEL are specified.														
Total Residual Chlorine (TRC)	mg/L	Daily Max	-	-	-	2.0	TOGS 1.3.3	-	-	0.005	A(C)	0.025	703.5	**0.03**	ML
	Seasonal effluent disinfection is being added to the permit. Only if you are using chlorine for disinfection. Due to the low dilution, the calculated WQBEL is less than the TBEL and less than the minimum level of detection. Therefore, an effluent limitation equal to the minimum level of detection of 0.030 mg/L is appropriate.														
Additional Pollutants Detected															
Oil & Grease	mg/L	Daily Max	-	-	-	**15**	TOGS 1.2.1	-	No residue attributable to sewage, industrial wastes or other wastes, nor visible oil film nor globules of grease.				703.2		TBEL
	Consistent with TOGS 1.2.1, TBELs reflect the available treatment technology listed in Attachment C.														
1,2 Propylene Glycol (CAS 57-55-6)	mg/L	Daily Max	-	-	-	-	-	-	-	-	-	-	-	-	**No Limitation**
	There is no applicable water quality standard or guidance value for propylene glycol. Propylene glycol has an oxygen demanding component. The current limits for BOD$_5$ and dissolved oxygen will account and be protective of the dissolved oxygen in the receiving waterbody.														
[2-(2-Methoxymethylethoxy) methylethoxy] propanol (CAS 25498-49-1)	mg/L	Daily Max	-	-	-	-	-	-	-	-	-	-	-	-	**No Limitation**
	There is no applicable water quality standard or guidance value for [2-(2-Methoxymethylethoxy) methylethoxy] propanol. No monitoring is required at this time.														
Hydroxyethyl Cellulose (CAS 9004-62-0)	mg/L	Daily Max	-	-	-	-	-	-	-	-	-	-	-	-	**No Limitation**
	There is no applicable water quality standard or guidance value for hydroxyethyl cellulose. No monitoring is required at this time.														

Outfall #	001	**Description of Wastewater:** Treated Sanitary Sewer and Plain (no detergent) Wash Water, Air Compressor Condensate from Outfall 01A.													
		Type of Treatment: Screening, Primary Clarification, Activated Sludge, UV Disinfection													
Effluent Parameter	Units	Averaging Period	Existing Discharge Data			TBELs		Water Quality Data & WQBELs						ML	Basis for Permit Requirement
			Permit Limit	Existing Effluent Quality[3]	# of Data Points Detects / Non-Detects	Limit	Basis	Ambient Bkgd. Conc.	Projected Instream Conc.	WQ Std. or GV	WQ Type	Calc. WQBEL	Basis for WQBEL		
Diethanolamine (CAS 111-42-2)	mg/L	Daily Max	-	-	-	-	-	-	-	-	-	-	-	-	**No Limitation**
	There is no applicable water quality standard or guidance value for diethanolamine.														

Outfall 01A

Outfall #	01A	**Description of Wastewater:** Plain (no detergent) Wash Water, Air Compressor Condensate													
		Type of Treatment: Oil & Water Separator													
Effluent Parameter	Units	Averaging Period	Existing Discharge Data			TBELs		Water Quality Data & WQBELs						ML	Basis for Permit Requirement
			Permit Limit	Existing Effluent Quality[5]	# of Data Points Detects / Non-Detects	Limit	Basis	Ambient Bkgd. Conc.	Projected Instream Conc.	WQ Std. or GV	WQ Type	Calc. WQBEL	Basis for WQBEL		
General Notes: This is a new facility. Therefore, existing discharge data is not available. Proposed obtained from the application provided by the permittee. All applicable water quality standards were reviewed for development of the WQBELs. The standard and WQBEL shown below represent the most stringent.															
Flow Rate	GPD	Monthly Avg	-	-	-	**2,100**	Design Flow	Narrative: No alterations that will impair the waters for their best usages.					703.2	-	TBEL
	The flow limit is set at the design flow of the wastewater treatment facility.														
pH	SU	Minimum	-	-	-	**6.0**	TOGS 1.2.1	-	-	6.5 – 8.5	Range	-	703.3	-	TBEL
		Maximum	-	-	-	**9.0**									
	Consistent with TOGS 1.2.1, TBELs reflect the available treatment technology listed in Attachment C. Given the available dilution an effluent limitation equal to the TBEL is reasonably protective of the WQS.														
Temperature	°F	Daily Max	-	-	-	-	-	-	-				-	-	**No Limitation**
	Outfall 01A is an internal outfall. Temperature will be monitored at Outfall 001.														
Total Suspended Solids	mg/L	Monthly Avg	-	-	-	**50**	BPJ	-	Narrative: None from sewage, industrial wastes or other wastes that will cause deposition or impair the waters for their best usages.				703.2	-	TBEL
		7 Day Avg	-	-	-	-	-								

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[5] Existing Effluent Quality: Permittee’s estimate based on NY-2C application submitted for a new facility.

Outfall #	01A	**Description of Wastewater:** Plain (no detergent) Wash Water, Air Compressor Condensate
		Type of Treatment: Oil & Water Separator

Effluent Parameter	Units	Averaging Period	Existing Discharge Data			TBELs		Water Quality Data & WQBELs						ML	Basis for Permit Requirement
			Permit Limit	Existing Effluent Quality[5]	# of Data Points Detects / Non-Detects	Limit	Basis	Ambient Bkgd. Conc.	Projected Instream Conc.	WQ Std. or GV	WQ Type	Calc. WQBEL	Basis for WQBEL		
Additional Pollutants Detected															
Oil & Grease	mg/L	Daily Max	-	-	-	**15**	TOGS 1.2.1	-	No residue attributable to sewage, industrial wastes or other wastes, nor visible oil film nor globules of grease.				703.2		TBEL
	Consistent with TOGS 1.2.1, TBELs reflect the available treatment technology listed in Attachment C.														
1,2 Propylene Glycol (CAS 57-55-6)															
	Not listed in TOGS 1.1.1 or 703.5. Propylene Glycol has an oxygen demanding component. On Outfall 001 the BOD_5 will help ensure the DO standard in protected in the Hudson.														
[2-(2-Methoxymethylethoxy) methylethoxy] propanol (CAS 25498-49-1)															
	Not list in TOGS 1.1.1 or 703.5. No monitoring required at this time.														
Hydroxyethyl Cellulose (CAS 9004-62-0)															
	Not list in TOGS 1.1.1 or 703.5. No monitoring required at this time.														
Diethanolamine (CAS 111-42-2)															
	Not list in TOGS 1.1.1 or 703.5. No monitoring required at this time.														

Outfall #	01A	**Description of Wastewater:** Plain (no detergent) Wash Water, Air Compressor Condensate													
		Type of Treatment: Oil & Water Separator													
Effluent Parameter	Units	Averaging Period	Existing Discharge Data			TBELs		Water Quality Data & WQBELs						ML	Basis for Permit Requirement
			Permit Limit	Existing Effluent Quality[5]	# of Data Points Detects / Non-Detects	Limit	Basis	Ambient Bkgd. Conc.	Projected Instream Conc.	WQ Std. or GV	WQ Type	Calc. WQBEL	Basis for WQBEL		
Foam (visible)	mg/L	Daily Max	-	-	-	**No visible foam**	BPJ	-	-				-		TBEL
	Facility shall use plain, non-detergent water for rinsing. No visible foam shall be present.														

Appendix: Regulatory and Technical Basis of Permit Authorizations

The Appendix is meant to supplement the factsheet for multiple types of SPDES permits. Portions of this Appendix may not be applicable to this specific permit.

Regulatory References

The provisions of the permit are based largely upon 40 CFR 122 subpart C and 6 NYCRR Part 750 and include monitoring, recording, reporting, and compliance requirements, as well as general conditions applicable to all SPDES permits. Below are the most common citations for the requirements included in SPDES permits:

- Clean Water Act (CWA) 33 section USC 1251 to 1387
- Environmental Conservation Law (ECL) Articles 17 and 70
- Federal Regulations
    - 40 CFR, Chapter I, subchapters D, N, and O
- State environmental regulations
    - 6 NYCRR Part 621
    - 6 NYCRR Part 750
    - 6 NYCRR Parts 700 - 704 – Best use and other requirements applicable to water classes
    - 6 NYCRR Parts 800 – 941 - Classification of individual surface waters
- NYSDEC water program policy, referred to as Technical and Operational Guidance Series (TOGS)
- USEPA Office of Water Technical Support Document for Water Quality-based Toxics Control, March 1991, Appendix E

The following is a quick guide to the references used within the factsheet:

SPDES Permit Requirements	**Regulatory Reference**
Anti-backsliding	6 NYCRR 750-1.10(c)
Best Management Practices (BMPS) for CSOs	6 NYCRR 750-2.8(a)(2)
Environmental Benefits Permit Strategy (EBPS)	6 NYCRR 750-1.18, NYS ECL 17-0817(4), TOGS 1.2.2 (revised January 25,2012)
Exceptions for Type I SSO Outfalls (bypass)	6 NYCRR 750-2.8(b)(2), 40 CFR 122.41
Mercury Multiple Discharge Variance	Division of Water Program Policy 1.3.10 (DOW 1.3.10)
Mixing Zone and Critical Water Information	TOGS 1.3.1 & Amendments
PCB Minimization Program	40 CFR Part 132 Appendix F Procedure 8, 6 NYCRR 750-1.13(a) and 750-1.14(f), and TOGS 1.2.1
Pollutant Minimization Program (PMP)	6 NYCRR 750-1.13(a), 750-1.14(f), TOGS 1.2.1
Schedules of Compliance	6 NYCRR 750-1.14
Sewage Pollution Right to Know (SPRTK)	NYS ECL 17-0826-a, 6 NYCRR 750-2.7
State Administrative Procedure Act (SAPA)	State Administrative Procedure Act Section 401(2), 6 NYCRR 621.11(l)
State Environmental Quality Review (SEQR)	6 NYCRR Part 617
USEPA Effluent Limitation Guidelines (ELGs)	40 CFR Parts 405-471
USEPA National CSO Policy	33 USC Section 1342(q)
Whole Effluent Toxicity (WET) Testing	TOGS 1.3.2
General Provisions of a SPDES Permit Department Request for Additional Information	NYCRR 750-2.1(i)

Outfall and Receiving Water Information

Impaired Waters

The NYS 303(d) List of Impaired/TMDL Waters identifies waters where specific best usages are not fully supported. The Department must consider the development of a Total Maximum Daily Load (TMDL) or other strategy to reduce the input of the specific pollutant(s) that restrict waterbody uses, in order to restore and protect such uses. SPDES permits must include effluent limitations necessary to implement a WLA of an EPA-approved TMDL (6 NYCRR 750-1.11(a)(5)(ii)), if applicable. In accordance with 6 NYCRR 750-1.13(a), permittees discharging to waters which are on the list but do not yet have a TMDL developed may be required to perform additional monitoring for the parameters causing the impairment. Accurate monitoring data is needed to determine the existing capabilities of the wastewater treatment plants and to assure that wasteload allocations (WLAs) are allocated equitably.

Existing Effluent Quality

The existing effluent quality is determined from a statistical evaluation of effluent data in accordance with TOGS 1.2.1 and the USEPA Office of Water, Technical Support Document for Water Quality-based Toxics Control, March 1991, Appendix E (TSD). The existing effluent quality is equal to the 95th (monthly average) and 99th (daily maximum) percentiles of the lognormal distribution of existing effluent data. When there are greater than three non-detects, a delta-lognormal distribution is assumed, and delta-lognormal calculations are used to determine the monthly average and daily maximum pollutant concentrations. Statistical calculations are not performed for parameters where there are less than ten data points. If additional data is needed, a monitoring requirement may be specified either through routine monitoring or a short-term high intensity monitoring program. The Pollutant Summary Table identifies the number of sample data points available.

Permit Requirements

Basis for Effluent Limitations

Sections 101, 301, 304, 308, 401, 402, and 405 of the CWA and Titles 5, 7, and 8 of ECL Article 17, as well as their implementing federal and state regulations, and related guidance, provide the basis for the effluent limitations and other conditions in the permit.

When conducting a full technical review of an existing permit, the previous effluent limitations form the basis for the next permit. Existing effluent quality is evaluated against the existing effluent limitations to determine if these should be continued, revised, or deleted. Generally, existing limitations are continued unless there are changed conditions at the facility, the facility demonstrates an ability to meet more stringent limitations, and/or in response to updated regulatory requirements. Pollutant monitoring data is also reviewed to determine the presence of additional contaminants that should be included in the permit based on a reasonable potential analysis to cause or contribute to a water quality standards violation.

Anti-backsliding

Anti-backsliding requirements are specified in the CWA sections 402(o) and 303(d)(4), ECL 17-0809, and regulations at 40 CFR 122.44(*l*) and 6 NYCRR 750-1.10(c) and (d). Generally, the relaxation of effluent limitations in permits is prohibited unless one of the specified exceptions applies, which will be cited on a case-by-case basis in this factsheet. Consistent with current case law[6] and USEPA interpretation[7] anti-backsliding requirements do not apply should a revision to the final effluent limitation take effect before the scheduled date of compliance for that final effluent limitation.

Antidegradation Policy

The Department implements the antidegradation portion of the CWA based upon two documents: (1) Organization and Delegation Memorandum #85-40, “Water Quality Antidegradation Policy” (September 9, 1985); and, (2) TOGS 1.3.9, “Implementation of the NYSDEC Antidegradation Policy – Great Lakes Basin (Supplement to Antidegradation Policy dated September 9, 1985).” The permit for the facility contains effluent limitations which ensure that the existing best usage of the receiving waters will be maintained. To further support the antidegradation policy, SPDES applications have been reviewed in accordance with the State Environmental Quality Review Act (SEQR) as prescribed by 6 NYCRR Part 617.

Effluent Limitations

In developing a permit, the Department determines the technology-based effluent limitations (TBELs) and then evaluates the water quality expected to result from technology controls to determine if any exceedances of water quality criteria in the receiving water might result. If there is a reasonable potential for exceedances of water quality criteria to occur, water quality-based effluent limitations (WQBELs) are developed. A WQBEL is designed

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[6] American Iron and Steel Institute v. Environmental Protection Agency, 115 F.3d 979, 993 n.6 (D.C. Cir. 1997)

[7] U.S. EPA, Water Quality Standards; Establishment of Numeric Criteria for Priority Toxic Pollutants for the State of California; 65 Fed. Reg. 31682, 31704 (May 18, 2000); Proposed Water Quality Guidance for the Great Lakes System, 58 Fed. Reg. 20802, 20837 & 20981 (April 16, 1993)

to ensure that the water quality standards of receiving waters are met. In general, the CWA requires that the effluent limitations for a particular pollutant are the more stringent of either the TBEL or WQBEL.

Technology-based Effluent Limitations (TBELs) for Industrial Facilities

A TBEL requires a minimum level of treatment for industrial point sources based on currently available treatment technologies and/or Best Management Practices (BMPs). CWA sections 301(b) and 402, ECL sections 17-0509, 17-0809 and 17-0811, and 6 NYCRR 750-1.11 require technology-based controls on effluents. TBELs are set based upon an evaluation of New Source Performance Standards (NSPS), Best Available Technology Economically Achievable (BAT), Best Conventional Pollutant Control Technology (BCT), Best Practicable Technology Currently Available (BPT), and/or Best Professional Judgment (BPJ).

USEPA Effluent Limitation Guidelines (ELGs) Applicable to Facility

In many cases, BPT, BCT, BAT and NSPS limitations are based on effluent guidelines developed by USEPA for specific industries, as promulgated under 40 CFR Parts 405-471. Applicable guidelines, pollutants regulated by these guidelines, and the effluent limitation derivation for facilities subject to these guidelines is in the USEPA Effluent Limitation Guideline Calculations Table.

Best Professional Judgement (BPJ)

For substances that are not explicitly limited by regulations, the permit writer is authorized to use BPJ in developing TBELs. Consistent with CWA 402(a)(1) and ECL Section 17-0811, the Department is authorized to issue a permit containing “any further limitations necessary to ensure compliance with water quality standards adopted pursuant to state law”. BPJ limitations may be set on a case-by-case basis using any reasonable method that takes into consideration the criteria set forth in 40 CFR 125.3. Applicable state regulations include 6 NYCRR 750-1.11. The BPJ limitation considers the existing technology present at the facility, the statistically calculated existing effluent quality for that parameter, and any unique or site-specific factors relating to the facility. Technology limitations generally achievable for various treatment technologies are included in TOGS 1.2.1, Attachment C. These limitations may be used for the listed parameters when the technology employed at the facility is listed.

Water Quality-Based Effluent Limitations (WQBELs)

In addition to the TBELs, permits must include additional or more stringent effluent limitations and conditions, including those necessary to protect water quality. CWA Section 101 and 301(b)(1)(C), 40 CFR 122.44(d)(1), and 6 NYCRR 750-1.11 require that permits include limitations for all pollutants or parameters which are or may be discharged at a level which may cause or contribute to an exceedance of any State water quality standard adopted pursuant to ECL 17-0301. Water quality standards can be found under 6 NYCRR Parts 700-704. The limitations must be stringent enough to ensure that water quality standards are met and must be consistent with any applicable WLA which may be in effect through a TMDL for the receiving water. These and other requirements are summarized in TOGS 1.1.1, 1.3.1, 1.3.2, 1.3.5 and 1.3.6. The Department considers a mixing zone analysis, critical flows, and reasonable potential analysis when developing a WQBEL.

Mixing Zone Analyses

In accordance with TOGS 1.3.1., the Department may perform additional analysis of the mixing condition between the effluent and the receiving waterbody. Mixing zone analyses using plume dispersion modeling are conducted in accordance with the following:
“EPA Technical Support Document for Water Quality-Based Toxics Control” (March 1991); EPA Region VIII’s “Mixing Zones and Dilution Policy” (December 1994); NYSDEC TOGS 1.3.1, “Total Maximum Daily Loads and Water Quality-Based Effluent Limitations” (July 1996); “CORMIX v11.0” (2019).

Critical Flows

In accordance with TOGS 1.2.1 and 1.3.1, WQBELs are developed using dilution ratios that relate the critical low flow condition of the receiving waterbody to the critical effluent flow. The critical low flow condition used in the dilution ratio will be different depending on whether the limitations are for aquatic or human health protection. For chronic aquatic protection, the critical low flow condition of the waterbody is typically represented by the 7Q10 flow and is calculated as the lowest average flow over a 7-day consecutive period within 10 years. For acute aquatic protection, the critical low flow condition is typically represented by the 1Q10 and is calculated as the lowest 1-day flow within 10 years. Nevertheless, NYSDEC considers using 50% of the 7Q10 to be equivalent to the 1Q10 flow. For the protection of human health, the critical low flow condition is typically represented by the 30Q10 flow and is calculated as the lowest average flow over a 30-day consecutive period within 10 years. Nevertheless, NYSDEC considers using 1.2 x 7Q10 to be equivalent to the 30Q10. The 7Q10 or 30Q10 flow is used with the critical effluent flow to calculate the dilution ratio. The critical effluent flow can be the maximum daily flow reported on the permit application, the maximum of the monthly average flows from discharge monitoring reports for the past three years, or the facility design flow. When more than one applicable standard exists for aquatic or human health protection for a specific pollutant, a reasonable potential analysis is conducted for each applicable standard and corresponding critical flow to ensure effluent limitations are sufficiently stringent to ensure all applicable water quality standards are met as required by 40 CFR 122.44(d)(1)(i). For brevity, the pollutant summary table reports the results of the most conservative scenario.

Reasonable Potential Analysis (RPA)

The Reasonable Potential Analysis (RPA) is a statistical estimation process, outlined in the 1991 USEPA Technical Support Document for Water Quality-based Toxics Control (TSD), Appendix E. This process uses existing effluent quality data and statistical variation methodology to project the maximum amounts of pollutants that could be discharged by the facility. This projected instream concentration (PIC) is calculated using the appropriate ratio and compared to the water quality standard (WQS). When the RPA process determines the WQS may be exceeded, a WQBEL is required. The procedure for developing WQBELs includes the following steps:

1) Identify the pollutants present in the discharge(s) based upon existing data, sampling data collected by the permittee as part of the permit application or a short-term high intensity monitoring program, or data gathered by the Department;

2) Identify water quality criteria applicable to these pollutants;

3) Determine if WQBELs are necessary (i.e. reasonable potential analysis (RPA)). The RPA will utilize the procedure outlined in Chapter 3.3.2 of EPA's Technical Support Document (TSD). As outlined in the TSD, for parameters with limited effluent data the RPA may include multipliers to account for effluent variability; and

4) Calculate WQBELs (if necessary). Factors considered in calculating WQBELs include available dilution of effluent in the receiving water, receiving water chemistry, and other pollutant sources.

The Department uses modeling tools to estimate the expected concentrations of the pollutant in the receiving water and develop WQBELs. These tools were developed in part using the methodology referenced above. If the estimated concentration of the pollutant in the receiving water is expected to exceed the ambient water quality standard or guidance value (i.e., numeric interpretation of a narrative water quality standard), then there is a reasonable potential that the discharge may cause or contribute to an exceedance of any State water quality standard adopted pursuant to ECL 17-0301. If a TMDL is in place, the facility's WLA for that pollutant is applied as the WQBEL.

For carbonaceous and nitrogenous oxygen demanding pollutants, the Department uses a model which incorporates the Streeter-Phelps equation. The equation relates the decomposition of inorganic and organic materials along with oxygen reaeration rates to compute the downstream dissolved oxygen concentration for comparison to water quality standards.

A Watershed Maximum Daily Load (WMDL) may be developed by the Department to account for the cumulative effect of multiple discharges of conservative toxic pollutants to ensure water quality standards are met in downstream segments. The WMDL uses a simple dilution model, assuming full mix in the receiving stream, to calculate the maximum allowable pollutant load that can be discharged and still meet water quality standards during critical low flow in downstream segments such as those with sensitive receptors (e.g., public water supply) or higher water classification. WQBELs are established to ensure that the cumulative mass load from point source discharges does not exceed the maximum allowable load to ensure permit limits are protective of water quality.

Minimum Level of Detection

Pursuant to 40 CFR 122.44(i)(1)(iv) and 6 NYCRR 750-2.5(d), SPDES permits must contain monitoring requirements using sufficiently sensitive test procedures approved under 40 CFR Part 136. A method is "sufficiently sensitive" when the method's minimum level (ML) is at or below the level of the effluent limitation established in the permit for the measured pollutant parameter; or the lowest ML of the analytical methods approved under 40 CFR Part 136. The ML represents the lowest level that can be measured within specified limitations of precision and accuracy during routine laboratory operations on most effluent matrices. When establishing effluent limitations for a specific parameter (based on technology or water quality requirements), it is possible that the calculated limitation will fall below the ML established by the approved analytical method(s). In these instances, the calculated limitation is included in the permit with a compliance level set equal to the ML of the most sensitive method.

Monitoring Requirements

CWA Section 308, 40 CFR 122.44(i), 6 NYCRR 750-1.13, and 750-2.5 require that monitoring be included in permits to determine compliance with effluent limitations. Additional effluent monitoring may also be required to gather data to determine if effluent limitations may be required. The permittee is responsible for conducting the monitoring and reporting results on DMRs. The permit contains the monitoring requirements for the facility. Monitoring frequency is based on the minimum sampling necessary to adequately monitor the facility's performance and characterize the nature of the discharge of the monitored flow or pollutant. Variable effluent flows and pollutant levels may be required to be monitored at more frequent intervals than relatively constant effluent flow and pollutant levels (6 NYCRR 750-1.13). For industrial facilities, sampling frequency is based on guidance provided in TOGS 1.2.1. For municipal facilities, sampling frequency is based on guidance provided in TOGS 1.3.3.

Other Conditions

Schedules of Compliance

Schedules of compliance are included in accordance with 40 CFR Part 132 Attachment F, Procedure 9, 40 CFR 122.47 and 6 NYCRR 750-1.14. Schedules of compliance are intended to, in the shortest reasonable time, achieve compliance with applicable effluent standards and limitations, water quality standards, and other applicable requirements. Where the time for compliance is more than nine months, the schedule of compliance must include interim requirements and dates for their achievement. If the time necessary to complete the interim milestones is more than nine months, and not readily divisible into stages for completion, progress reports must be required.

Schedule(s) of Additional Submittals

Schedules of Additional Submittals are used to summarize the deliverables required by the permit not identified in a separate Schedule of Compliance.

Best Management Practices (BMP) for Industrial Facilities

BMP plans are authorized for inclusion in SPDES permits pursuant to CWA 304(e) and 402 (a)(1) and 6 NYCRR 750-1.14(f). The regulations pertaining to BMPs are promulgated under 40 CFR Part 125, Subpart K. These regulations specifically address surface water discharges.

STORMWATER POLLUTION PREVENTION PLAN (SWPPP)

For

MARMEN-WELCON TOWER MANUFACTURING PLANT

PREPARED FOR:

PORT OF ALBANY

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FINAL SITE PLAN SUBMISSION

JUNE 20, 2022
(Field Change Amendment – August 4, 2025)

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1. INTRODUCTION

A stormwater management assessment has been conducted for the proposed project in order to protect the waters of the State of New York from the adverse impacts of stormwater runoff. This report presents an analysis of the project in accordance with the *New York State Department of Environmental Conservation SPDES General Permit for Stormwater Discharges from Construction Activity Permit No. GP-0-20-001.* A separate Drainage Design Report (included as Appendix C of this report) has been developed to address the *New York State Stormwater Management Design Manual* ("The Manual"). As required, the Stormwater Pollution Prevention Plan is designed, where appropriate, to incorporate green infrastructure techniques that preserve natural resources and utilize the existing hydrology of the site, provide runoff reduction practices, water quality treatment practices, apply volume and peak control practices for channel protection, overbank flood control, and extreme flood control as appropriate.

In accordance with Appendix B, Table 2 of the SPDES General Permit for Construction Activity, GP-0-20-001 (included as Appendix E of this report), industrial facilities that involve a soil disturbance of one or more acres require the preparation of a full SWPPP that includes post-construction stormwater management practices. In total, approximately 72.7 acres of soil disturbance is expected during the construction of this project ***and 0.7 acres of disturbance for NG utility access drives.*** Therefore, this project includes the development of erosion and sediment controls, green infrastructure site planning techniques, runoff reduction volume practices and post-construction stormwater management practices.

The general contractor and subcontractors performing any activity that involves soil disturbance will be required to comply with the terms and conditions of the SWPPP for the project identified as a condition of authorization to discharge stormwater. The Contractor shall provide signed certifications (Form CONR 5) for itself and all applicable subcontractors at the preconstruction meeting. These signed certifications shall be included as part of the SWPPP. The SPDES General Permit and SWPPP must be kept on file at the Project Field Office. As required by the conditions described in the SPDES general permit, the SWPPP shall be kept current, and updates will be made to reflect changes in the design, construction and operation, or maintenance of the project.

The complete set of construction drawings and specifications are provided as separate documents; however, they should be considered an integral component of the SWPPP and are referenced throughout this document. Prior to the start of construction activities, a Notice of Intent (NOI) must be filed and accepted by the NYSDEC. A Draft NOI has been included in this document as Appendix J. The applicant must retain all documentation for 5 years after NYSDEC accepts the Notice of Termination (NOT).

1.1 Scope of the Project

The proposed development is an offshore wind (OSW) manufacturing operation that will produce wind turbine tower components. The site development includes 603,500 +/- square feet of OSW manufacturing spread over four (4) buildings with ancillary impervious areas including parking for automobiles and trucks, roadway, bridge, and a maritime wharf. The remainder of the site will be used for tower storage and be made up of dense graded aggregate (compacted gravel). There will also be small pervious areas of grass and unaltered brush and trees.

1.2 Location of Project

The Project is situated on 81.62 acres of land on Beacon Island ("Expansion Site"), located at the confluence of the Normans Kill and Hudson River. The project also includes development within 4.4 acres of the adjoining parcel owned by National Grid, the extension and improvement of Normanskill Street (Normanskill Street Improvements) and widening of Rt. 144 (Offsite Improvements). The project owner, Albany Port District Commission (APDC), is proposing to develop the vacant parcels of land (tax parcels 98.00-2-10.23 and 98.01-2-1.0) to expand the existing Port of Albany in the Town of Bethlehem, Albany County, New York. Refer to the Location Map in Appendix A.

The project is not located within a TMDL and does not discharge into a 303(d) listed waterbody.

Table 1 - Location Table

Approximate Coordinate Position @ Center of Project	
Latitude	42° 36’ 10.8” N
Longitude	73° 45’ 57.0” W

1.3 Project Type and Size

The project is a new development construction project that has a disturbance area of approximately 72.7 +/- acres. The new impervious area is approximately 65.9 acres.

1.4 Project Description

The proposed project will include development of an OSW tower manufacturing (Marmen-Welcon) facility consisting of five (5) separate buildings totaling up to 625,539+/- square feet of floor space. The following is a breakdown of the function and size of each building:

- Building A Plate Preparation & Welding (299,250 SF)
- Building B Welding Finishing (111,023 SF)
- Building C Blast Metallization Plant (131,968 SF)
- Building D Internal Assembly Finishing (61,550 SF)
- Building E Material Receiving (21,748 SF)

Tower production will occur within four (4) buildings (Buildings A-D) at the main facility on the Port Expansion property located in the Town of Bethlehem. The 5th building (Building E) will be located at 700 Smith Boulevard within the existing Port District in the City of Albany. A proposed gated bridge over the Normans Kill will provide a truck transportation route in and out of the main facility, by connecting Beacon Island and the 14.7-acre offsite parcel at 700 Smith Boulevard. In conjunction with the proposed bridge, Normanskill Street is to be extended from its existing end point to the bridge. The existing pavement will be improved to accommodate the proposed trucking route. River Road (Rt. 144) will be widened to accommodate the employee entrance. Employee parking will be situated on the adjoining land owned by National Grid with access from River Road. A proposed 500 LF wharf and associated dredging along the Hudson River will be used to load and ship completed tower sections. A separate stormwater analysis and SWPPP has been prepared for the 14.7-acre Building E site at 700 Smith Boulevard and the portion of Normanskill Street. located in the City of Albany, as the sites are separated by approximately 1-mile and are under separate MS4 jurisdictions.

The purpose of this report is to assess the stormwater quality, quantity, and erosion and sediment control for the development of the site. This report has been developed in accordance with the New York State Department of Environmental Conservation (NYSDEC) State Pollution Discharge Elimination System (SPDES) General Permit for Stormwater Discharges from Construction Activity, GP-0-20-001 (Permit) and the NYSDEC Stormwater Management Design Manual (The Manual). The project site is located within the

Town of Bethlehem, Albany County, New York, which is an MS4 community, requiring this report and project to receive approval from the Town.
The soil disturbance area addressed in this is report is contained within the Town of Bethlehem. This report does not include coverage for work within the Mean High Water (MHW) elevation of the Hudson River or the Normans Kill. Work within the MHW line will be covered under a separate permit.

The total disturbance area is ***73.4 +/-*** acres and includes the Expansion Site, Normanskill Street Improvements, **NG access drives** and Offsite Improvement. The existing impervious area is approximately 5.16 +/- acres, 7.09% of the total disturbance area. The proposed site development will consist of ***66.3 +/-*** acres of impervious cover, 90.6% of the total disturbance area.

Due to the amount of soil disturbance proposed for this project, a 5-acre disturbance waiver is being requested. The 5-acre waiver request along with all required documentation is included as Appendix L of this report.

1.5 Cultural Resources

A Supplemental Environmental Impact Statement (SEIS) has been developed as part of the SEQR process for the Port of Albany Expansion Project. A SHPO review was conducted, and the current status reflects “No Adverse Effect”. The SHPO review letter for the GEIS review process, dated September 13, 2019. A follow up SEIS SHPO review letter, dated December 9, 2021, resulted in a follow up noise study and coordination with the Stockbridge-Munsee Tribal Historic Preservation (SMTHP). Multiple coordination meetings were conducted resulting in some modifications to the site design and additional information provided. As a result, the SMTHP provided a follow up letter dated March 3, 2022, confirming the project has “No Adverse Effect”. In response to the SMTHP letter, SPHO has issued a letter of “No Adverse Effect” dated March 25, 2022, contingent upon a deed restriction to be filed by the Owner to protect the Tree and Vegetation buffer area during the construction phase as well as permanently after construction is completed. All referenced letters have been included in Appendix F of this report.

1.6 On-site Wetlands

As part of the Draft Generic Environmental Impact Statement (DGEIS) and SEIS, impact to aquatic resources, including wetlands, were evaluated. The New York State Freshwater Wetland and Tidal Wetlands mapping of the project site indicates there are no NYSDEC jurisdictional wetlands within or adjacent to the project area. Review of USFWS National Wetlands Inventory (NWI) mapping of the project site indicates that the majority of the project area is mapped as palustrine emergent wetlands (PEM) and palustrine forested wetlands (PFO). It should be noted that NWI mapping does not have any regulatory consequence, but rather indicates areas that may meet federal wetland criteria as identified by the USFWS using aerial photography.

A wetland delineation was conducted in April 2019 by McFarland Johnson (MJ) for the FGEIS. The results of the delineation indicated that there are 8 freshwater wetlands located within the project limits. These wetlands are hereafter referred to as Wetlands 1, 3, 4, 5, 6, 7, 8, and 9. Wetlands within the original study area totaled approximately 2.33 acres. A Supplemental Wetland Delineation was performed by MJ in April 2021 within the 18.22 acres on the National Grid Parcel. One contiguous wetland, comprising of approximately 7.13 acres, was delineated within the 18.22-acre area. The delineated wetland represents an extension of the 2019 wetland delineation and previously identified as Wetland 1. Wetland 1 drains in a northerly direction into 40-inch corrugated metal pipe (CMP) which discharges directly to the Normans Kill.

The Project will result in direct impacts to 0.81 acres of Wetland 1 located in Beacon Island (original Project Area) and 0.01 acres of direct impact to Wetland 1 on National Grid property for the construction of a retaining wall. In addition, there is a 0.04-acre impact to Wetland 9 for the bridge over the Normans Kill and a 0.02-acre impact to Wetland 7 for roadway improvements. There will also be approximately 0.33 acres of temporary impacts to wetlands during construction. Total permanent wetland impacts are estimated to be approximately 0.86 acre.

Compensatory wetland mitigation will be satisfied through a federally approved In-Lieu Fee Mitigation Program or off-site mitigation bank (The Wetland Trust). Mitigation in accordance with USACE rules and regulations will ensure no net loss of wetlands and will be included as part of the Joint application Permit submitted to the USACE and NYSDEC.

2. PROJECT MAPS AND PLANS

2.1 Location Map

See Appendix A

2.2 Soil Maps

See Appendix E of the Drainage Design Report (included as Appendix C of this report)

2.3 Erosion and Sediment Control Plans

See Appendix B of this report

2.4 Existing and Proposed Subcatchment Maps

See Appendix A and B of the Drainage Design Report (included as Appendix C of this report)

3. PROJECT SOILS

3.1 NRCS Soil Map

See Appendix E of the Drainage Design Report (included as Appendix C of this report)

3.2 Soil Types

The following soil type(s) and hydrologic group(s) are present within the project area of disturbance:

Table 2 – Soil Types

Symbol	Soil Name	Hydrologic Soil Group
HuE	Hudson silt loam, 25 to 45 percent slopes	C/D
NrD	Nassau very channery silt loam, hilly, very rocky	D
Ug	Udorthents, loamy	A
Ur	Urban land	
Wo	Wayland soils complex, non-calcareous substratum, 0 to 3 percent slopes, frequently flooded	B/D

3.3 Discussion of Soil Characteristics and Soil Erosion Hazard Potential

According to the Natural Resources Conservation Service (NRCS) web soil survey, there are five (5) mapped soil units identified within the project boundary. See Appendix E of the Drainage Design Report (included as Appendix C of this report). The majority of the soil at the expansion site falls within the hydrologic soil group B/D. The first letter corresponds to drained soil's properties under drained conditions and the second to saturated conditions. Group B soils have moderate infiltration and runoff rates while group D have a low infiltration rate and a high runoff rate. The soils with dual group identifiers have been modeled with the more conservative of the two, in this case a D soils group. Most of the soil adjacent to Normanskill Street is within soil group A. Group A soils have a high infiltration rate.

Geotechnical studies have been undertaken to evaluate the subsurface conditions of the site. These investigations have been summarized in the following reports:

- *Preliminary Geotechnical Evaluation and Interpretive Report*, CME Associates, Inc., April 5, 2017
- *Supplemental Geotechnical Report*, Dente Group, July 20, 2017

Copies of these reports were included in the TOWN OF BETHLEHEM PLANNING BOARD, DRAFT GENERIC ENVIRONMENTAL IMPACT STATEMENT For ALBANY PORT DISTRICT COMMISSION PORT OF ALBANY EXPANSION PROJECT, Appendix E.

- *Draft Geotechnical Engineering Report,* Terracon, October 15, 2021

A copy of this reports is included in the TOWN OF BETHLEHEM PLANNING BOARD, SUPPLEMENTAL ENVIRONMENTAL IMPACT STATEMENT For ALBANY PORT DISTRICT COMMISSION PORT OF ALBANY EXPANSION PROJECT.

Based on these previous investigations, the subsurface conditions of the Expansion site are generally characterized by historic fills of various depths overlying, in sequence with depth; river sediments, alluvial sands, glaciolacustrine silt/ clay, glacial till, and shale bedrock. The fill was noted at specific boring locations ranging from 6 to 23 feet below existing grade. The fill material is characterized as a random landfill deposit containing natural and solid waste deposits such as, but not limited to, foundry sand waste, sand, silt, coal ash, gravel, and organic matter. A predominant component of the fill was reported as coal ash.

Shale bedrock was found beneath the glacial till soils at select boring locations. The depth to rock ranged from approximately 61 feet below grade near the northwest portion of the site, to greater than 148 feet at the southeast portion of the site. The rock depths appear shallowest on the north and west sides of the site and increase to the east towards the Hudson River and in a south direction across the site. Based on the New York State Museum and Science Service's Geologic Map of New York: State Hudson-Mohawk Sheet, and the geotechnical rock core samples, the bedrock appears to be consistent with the Normans kill Shale Formation.

According to the geotechnical reports, shallow groundwater was observed at depths ranging from approximately 1.5 to 13.7 feet below existing grade. However, due to the subsurface conditions, the shallower observations could be representative of perched groundwater zones due to discontinuous impermeable layers. Shallow groundwater fluctuations should be expected to occur at this site depending on several factors such as rainfall, seasonal changes, prevailing climate, ambient weather conditions, and the tidal influences of the Hudson River.

Historically, the project site was composed of small islands and river channels subject to natural shifts due to flows associated with the Hudson River and the former Island Creek, a side channel of the Hudson River. Island Creek historically flowed along the western side of the site through the current power line corridor and discharged to the Hudson River at the southern end of the site. Based on available mapping, sometime between 1936 and 1961, Island Creek channel was diverted at the north end of the site directly to the Hudson River, whereupon it was referred to solely as Normans Kill, the main tributary to this former channel. The site was subject to historic filling operations to create usable lands and a portion of the site was operated as a coal ash (fly ash) disposal site by Niagara Mohawk from approximately 1952 to 1970. As such, there are large areas of fly ash deposits on the site that must be considered during the design and construction of the site infrastructure and stormwater management facilities. Excavated fly ash material will need to be appropriately handled and properly disposed of.

Due to the presence of fly ash on the Expansion Site, in addition to a NYSDEC SPDES, a Site Management Plan (SMP) has been prepared in accordance with 6 NYCRR Part 375 and DER Technical Guidance for Site Investigation and Remediation and submitted to the NYSDEC, Division of Environmental Remediation and the NYSDOH. The SMP includes: a Health and Safety Plan (HASP), to inform and protect the contractor and their work force; a Community Air Monitoring Plan (CAMP), to monitor and protect the surrounding communities; and an Excavation Work Plan (EWP), to direct the activities of the contractor during construction. The EWP includes a detailed description of the work to be performed, the anticipated environmental conditions, and engineering controls to mitigate the movement of fly ash. The SMP pertains only to the Expansion Site portion of the project (see Appendix H of this report).

3.4 Soil Infiltration Test

Supplemental to the above noted geotechnical reports, additional subsurface investigation and infiltration testing was completed by Terracon on May 12, 13, and 18, 2022. Test pit locations were selected based on the potential for stormwater infiltration practices to be utilized. Test locations were along the shore of the Hudson River and Normans Kill as well as within the boundary of Infiltration Basin #1 and #2. The testing locations are shown on a map included in Appendix F of the Drainage Report (included as Appendix C to this report).

The subsurface investigation and infiltration testing concluded that locations IT-1, IT-1A, IT-7, IT-7A, IT-8, IT-10, IT-12, IT-12A, and IT-15 can provide an infiltration rate of 0.5 in/hr or greater. The remaining locations had test results below 0.5 in/hr which is not sufficient for infiltration chambers or infiltration basins. In addition to infiltration rate, a soil classification was also provided at the test depth. The soil classifications found were fill (coal ash with crushed stone and slag), sandy silt (ML), silt (ML), silt and clay (CL-ML), silt (ML), silt with sand (ML), silty sand (ML), silty clay (CL-ML), fill (sandy silt), and fill (coal ash). The entire subsurface investigation report including soil profiles is located in Appendix F of the Drainage Report (included as Appendix C to this report).

4. CONSTRUCTION PHASING

4.1 Sequence of Construction Activities

The Contractor’s work schedule and methods shall be consistent with the SWPPP or amended SWPPP. Once approved, the progress schedule shall become a part of the SWPPP. It should be noted that there is a NYSDEC approved SMP for this site, which is included as Appendix H of this report. Any disturbance of the site must comply with the SMP.

The following list is a suggested sequence of major construction activities for the project to meet the NYSDEC Phase II erosion control requirements:

1. Conduct a pre-construction meeting with the MS4, Engineer, and Contractor to review the SWPPP.
2. Notify the NYSDEC within 14 days, and no less than 3 days, prior to commencing work activities that may affects areas of the subject site that are impacted with ash, as required by Section 3.2 of the SMP.
3. Clearly identify project work limits, identifying all areas where construction disturbance shall be permitted.
4. Install erosion control measures prior to commencing earthwork operations. Construct temporary earthen berms, diversion swales, sediment control dams and associated erosion control measures necessary to divert runoff from entering planned areas of disturbance and to protect the adjacent waterway.
5. All installed erosion and sediment control measures are to be inspected and certified as correctly installed by the owner’s qualified inspector and Town of Bethlehem staff.
6. Establish temporary/permanent storm water management ponds/erosion control basins.
    a. Consult the SMP for the appropriate measures to handle or dispose of any encountered contaminated soils.
7. Remove vegetation and dispose of off-site.
8. Strip and stockpile topsoil from proposed pavement, structural fill and cut areas (stockpile materials in locations as directed by owner’s representative).
    a. Consult the SMP for appropriate measures to handle or dispose of any encountered contaminated soils.
9. Establish mass earthwork subgrade elevations.
    a. Consult the SMP the appropriate measures to handle or dispose of any encountered contaminated soils.
10. All temporary erosion and sediment control measures as well as stockpiles are to be mulched and seeded for temporary vegetative cover immediately following grading.
11. Import the aggregate fill material to serve as a surcharge for the proposed building and concrete pad areas.
12. After surcharging compaction is completed, places fabric and geogrid on the subgrade and spread the aggregate material in layers with additional geogrid as specified.
13. Construct utility lines (water/electric/gas/communications/sanitary sewers/storm sewers), construct building and install infrastructure improvements.
14. Box out roadway and pavement areas and install concrete curbing.
15. Construct asphalt pavement section, up to binder course.
16. Fine grade and spread topsoil, install landscaping plantings and hardscapes, site amenities and permanent seeding.
17. Town of Bethlehem (MS4) shall conduct a site inspection to determine (1) that the site has achieved 80% stabilization and (2) the installed stormwater facilities are operational.
18. Remove temporary erosion and sediment control features upon establishment of permanent ground cover and inspection/approval from a Town official or representative.
19. Notify owner’s representative of completion of final site stabilization.
20. File Notice of Termination.

5. EROSION AND SEDIMENT CONSTROL MEASURES

5.1 Erosion Control Plan

An erosion control plan has been developed in accordance with the “New York Standards and Specifications for Erosion and Sediment Control”. The erosion control plan employs permanent and temporary erosion and sediment control methods including silt fence, erosion control matting, construction entrances, and other appropriate measures. It should be noted that there is a NYSDEC approved SMP for the Expansion Site, which is included as Appendix H of this report. Any disturbance of this area must comply with the SMP. As stated in Section 3.2 of the SMP, NYSDEC must be notified within 14 days, and no less than 3 days, prior to commencing work activities that may affect areas of the subject site that are impacted with ash.

5.1.1 Temporary Surface Stabilization

All work and prior NYSDEC notification shall be in accordance with the SMP. Areas within the project limits that may be disturbed more than once during the construction activities will be stabilized using temporary seed and mulch item or as directed by the Engineer. Areas remaining unpaved and undisturbed for more than seven (7) days during construction operations shall be stabilized temporarily. Other areas that might need to be stabilized temporarily will be at the discretion of the Engineer.

5.1.2 Drainage Pipe Inlet / Outlet Stabilization

As part of the permanent erosion control measure, the inlet and outlet of the culvert pipes will be provided with either stone riprap apron or an apron consisting of erosion control product with vegetation to provide the required erosion control which blends in with the surrounding natural features and topography. The location and type of stabilization to be provided is shown on project plans.

5.1.3 De-watering

Any groundwater that is suspected of being contaminated shall be handled in accordance with Section 4.2 of the SMP. If required, de-watering of miscellaneous areas within the site will be performed utilizing a pump and filter bag system. The filter bags should be made of non-woven geotextile material capable of trapping particles larger than 150 microns. Filter bags should be replaced when they are half full or a no longer functioning per the manufacturer’s requirements. Filter bags should be located in a well vegetated/grassy area and discharge into stable erosions resistant areas. Where this is not possible a geotextile flow path should be established. Bags shall not be placed on slopes greater than 5%. The pump discharge hose shall be inserted into the bags in the manner specified by the manufacturer and securely clamped. Pumping rate shall not be greater than 750 GPM or ½ the maximum specified by the manufacturer, whichever is less. Pump intakes shall be floated and screened.

5.1.4 Construction Entrance

As required, at least one (1) stabilized construction entrance will be constructed to access the Contractors Staging/Storage Area. This entrance/area shall conform to the details. Refer to the Erosion and Sediment Control (E&SC) Plans (included as Appendix B of this report) for location of construction entrance(s).

5.1.5 Concrete Truck Washout / Concrete Batch Plant Protection

As required, a temporary excavated or above ground lined pit where concrete truck mixers and equipment can be washed after their loads have been discharged, to prevent highly alkaline runoff from entering storm drainage systems or leaching into soil shall be constructed. See E&SC Plans (included Appendix B of this report) for location of concrete washout. If a concrete batch plant is installed at the site, temporary containment to prevent discharge of runoff from entering storm drainage systems or leaching into soil shall be constructed.

5.1.6 Permanent Stabilization

Stabilization of the graded surfaces will be accomplished by using various seed mixtures for establishing permanent vegetation.

5.1.7 Dust Control

Dust shall be controlled and monitored in accordance with Section 4.3 of the SMP. The contractor will be required to minimize dust generation during the construction activities. Provisions such as applying water on haul roads, wetting equipment, and excavation faces, spraying water on buckets during excavation and dumping, hauling materials in properly tarped or watertight containers, restricting vehicle speeds to 10 mph, covering excavated areas and material after excavation activity ceases, and reducing the excavation size and/or number of excavations have proven effective in dust control.

5.1.8 Silt Fence

Silt fence will be placed per the E&SC Plans (Appendix B of this report), down slope of all disturbed areas, soil stockpiles, and spoil areas. Along the bank of the Normans Kill, two layers of silt fence are to be installed due to the proximity to the Mean Higher High Water (MHHW) level. The purpose of the silt fence is to remove sediment from sheet flow in these areas. Silt fence shall remain in place and functional until the contributing area has been permanently stabilized. Sediment socks or mulch dikes may be used in lieu of silt fence, where approved by the Engineer of Record. Erosion Control shall be in accordance with the SMP.

5.1.9 Temporary Sediment Basins

Temporary sediment basins have been designed to store sediment runoff from the Expansion Site. Basin #1 will be converted into a permanent stormwater quality pond. All basins have been designed in accordance with Section 5 of the NYS Standards and Specifications for Erosion and Sediment Control (Blue Book). Calculations for the basins are included as Appendix I to this report. Locations and a detail of the basins are included in the E&SC Plans (Appendix B of this report).

5.1.10 Weekly Inspections

A qualified inspector shall conduct site inspections at least once every seven (7) calendar days. After a 5-acre waiver is granted; site inspections shall occur at least twice every seven (7) calendar days while there are more than 5 acres of soil disturbance. The qualified inspector shall inspect all erosion and sediment control practices and pollution prevention measures to ensure integrity and effectiveness; all post-construction stormwater management practices under construction to ensure that they are constructed in conformance with the SWPPP; all areas of disturbance that have not achieved final stabilization, all points of discharge to natural surface waterbodies located within, or immediately adjacent to, the property boundaries of the construction site; and all points of discharge from the construction site. The qualified construction inspector shall also prepare an inspection report after every inspection. Complete inspection and maintenance requirements can be found in Part IV of the SPDES General Permit GP-0-20-001 (included as Appendix E of this report). Inspection reports shall be submitted to the Town's Stormwater Program Manager.

5.1.11 Final Inspection

Prior to the project being finally accepted, it shall be inspected for any evidence of erosion or slope failure.

If any such condition becomes apparent upon final inspection, temporary soil erosion and sediment controls shall be installed immediately as directed by the Engineer. The situation shall be corrected per a schedule agreed to by the MS4 (Town of Bethlehem).

5.1.12 Wharf E&SC

Erosion and sediment control measures associated with the construction of the wharf and dredging of the Hudson River are to be designed and approved prior to disturbance below the Mean High High Water (MHHW) elevation. These E&SC measures are being designed in conjunction with the NYSDEC, USACE and other State agencies through a separate permitting process. All additional permits required for the wharf and dredging will also be in place prior to disturbance below the MHHW. Draft conceptual E&SC Plans associated with the wharf construction and dredging are included in Appendix G of this report.

5.2 Permanent Erosion and Sediment Control Measures

Table 3 – List of Permanent Erosion & Sediment Control Measures

Permanent Feature	Converted Temporary Practice?	Location: ESC Plan	Receiving Waterbody Protected (where applicable)
Riprap outlet protection	Yes	See Plans	Hudson River, Normans Kill
Soil Stabilization	Yes	See Plans	Hudson River, Normans Kill
Check Dam	Yes	See Plans	N/A
Diversion Dike	Yes	See Plans	N/A

5.3 Installation Sequence

See the intended sequence of construction activities noted in Section 4 above.

5.4 Maintenance Schedule

The Contractor is required to inspect all E&SC devices in their active work area daily and after each rain event; and repair any deficiencies in accordance with the SPDES permit.

5.5 SWPPP Implementation Responsibilities

Implementation of all E&SC devices will be by the Contractor as indicated in the contract documents.

6. POLLUTION PREVENTION MEASURES

6.1 Material Management Practices

All waste materials, including construction debris and trash that occur onsite shall be handled and disposed of in a lawful manner that is in accordance with state and local regulations. No waste material shall be buried on site.

- An effort will be made to store only enough products required for the project.
- All materials stored within the site will be stored in a neat orderly manner in their appropriate containers and if possible, an enclosed area.
- Products shall be kept in their original containers with the original manufacturer’s labels. Manufacturer’s recommendations for proper use and disposal shall be followed.
- Hazardous materials shall be disposed of in a lawful manner and in accordance with State and

Local regulations.

- Sanitary waste will be collected from portable units as required and shall be disposed of in a lawful manner.

The following materials are expected to be on-site during construction:

- Concrete
- Asphalt
- Paints (Enamel and Latex)
- Petroleum based products
- Fertilizers
- Metal building components
- Detergents
- Cleaning Solvents
- Roofing Materials
- Tar

These materials and other materials used during construction with the potential to impact stormwater will be stored, managed, used, and disposed of in a lawful manner that minimizes the potential for releases to the environment and especially into stormwater.

Emergency contacts for the project will be posted at the project office and are included at the end of this section.

6.2 Spill Control Practices

The contractor will be responsible for preparing a project area specific spill control plan in accordance with Local and NYSDEC regulations. At a minimum, this plan shall:

1. Stop the source of the spill.
2. Contain the spill.
3. Reduce stormwater contact if there is a spill.
4. Dispose of contaminated material in lawful manner and in accordance with manufacturer's procedures and NYSDEC regulations.
5. Identify responsible trained personnel.
6. Ensure spill area is well ventilated.

For any work within National Grid property, the National Grid Environmental Guidance EG 501 regarding reporting of a release of oil, chemical or hazardous material must be followed. National Grid EG 501 has been included as Appendix M of this document.

6.3 General Material Handling Practices

The following general practices will be used throughout the project to reduce the potential for spills:

1. Potential pollutants will be stored and used in a manner consistent with the manufacturer's instructions in a secure location. To the extent practicable, material storage areas should not be located near storm drain inlets and should be equipped with covers, roofs, or secondary containment as needed to prevent stormwater from contacting stored materials. Potential pollutants should not be stored within 100 feet of a water course or wetland. Chemicals that are not compatible shall be stored in segregated areas so that spilled materials cannot combine and react.

2. Materials disposal will be in accordance with manufacturer's instructions and applicable local state and federal regulations.
3. Materials no longer required for construction will be removed from the site as soon as practicable.
4. Adequate garbage, construction waste, and sanitary waste handling and disposal facilities will be provided/utilized to the extent necessary to keep the site clear of obstruction and Best Management Practices (BMPs) clear and functional.

6.4 Product Specific Practices

The following product specific practices will be followed within the project area.

6.4.1 Petroleum Products

All project related vehicles shall be monitored for leaks and receive regular preventative maintenance to reduce the potential of leakage. Petroleum products shall be stored in tightly sealed containers, which are clearly labeled. Any asphalt substances used during construction shall be applied according to manufacturer's recommendations.

6.4.2 Fertilizers

Fertilizers used shall be applied only in the minimum amounts recommended by the manufacturer. Once applied, fertilizer shall be worked into the soil to limit exposure to stormwater. Fertilizers shall be stored in covered or other contained areas.

6.4.3 Paints

All containers shall be tightly sealed and stored when not required for use. Excess paint shall not be discharged into the storm sewer system but shall be disposed of according to manufacturer's instructions or State regulations.

6.4.4 Concrete Trucks

Concrete Trucks shall be allowed to wash out within project areas that the contractor provides an area which collects and contains any concrete / slurry material washed from trucks for recovery and disposal at a later time. No concrete or slurry shall be discharged from the property at any time of construction. The concrete washout area shall conform to the detail found on sheet ESC-06 (Appendix B of this report).

6.5 Spill Response

The primary objective in responding to a spill is to quickly contain the material(s) and prevent or minimize their migration into stormwater runoff or conveyance systems. If the release has impacted on-site stormwater, it is critical to contain the released material on-site and prevent their release into receiving waters.

If a spill of pollutants threatens stormwater on-site, the spill response procedures outline below must be implemented in a timely manner to prevent release of the pollutant:

1. The site superintendent will be notified immediately when a spill or the threat of a spill is observed. The superintendent will assess the situation and determine the appropriate response.
2. If spills represent an imminent threat of escaping E&SC facilities and entering the receiving waters, facility personnel will respond immediately to contain the release and notify the superintendent after the situation has been stabilized.
3. Spill kits containing materials and equipment for spill response and clean-up will be maintained onsite. Each spill kit may contain:
    - Oil absorbent pads (one bale)

- Oil absorbent booms (40 feet)
- 55-gallon drums (2)
- 9-mil plastic bags (10)
- Personal protective equipment including gloves and goggles

4. If an oil sheen is observed on surface water, absorbent pads and/or booms will be applied to contain and remove the oil. The source of the oil sheen will also be identified and removed or repaired as necessary to prevent further releases.
5. The site superintendent, or their designee, will be responsible for completing a spill reporting form to the appropriate state or local agency.
6. Spill response equipment will be inspected and maintained as necessary to replace any materials used in spill response activities.

6.6 Notification

In the event of a spill, make the appropriate notification(s) consistent with the following procedures:

1. Any spill of oil which a) violates water quality standards, b) produces a sheen on a surface water, c) causes a sludge or emulsion must be reported immediately by telephone to the National Response Center Hotline at (800) 424-8802.
2. Any oil, hazardous substance, or hazardous waste release which exceeds the reportable quantity must be reported immediately by telephone to the National Response Center Hotline at (800) 424-8802.
3. Any spill of oil or hazardous substance to waters of the state must be reported immediately by telephone to the NYSDEC.
4. Any release of hazardous substance that may be a threat to human health or the environment must be reported to the NYSDEC immediately upon discovery.

7. EXISTING SITE CONDITIONS

The existing site is Beacon Island, located in the Town of Bethlehem, Albany County, New York. The site is currently vacant and consists primarily of brush and trees with a small gravel area as well as abandoned railroad tracks.

7.1 Existing Watershed Information

The project area is in close proximity and includes shorelines to both the Hudson River and Normans Kill, which are the receiving waterbodies for runoff from the current site.

The existing drainage condition is split up into seven (7) drainage areas. Drainage area DR-A drains to analysis point #1A, drainage areas DR-B and DR-F drain to analysis point #1, drainage areas DR-D and DR-E drain to analysis point #2. Drainage area DR-G drains to analysis point #3. Drainage area DR-C drains to a self-contained depression for storage. See Appendix A of the Drainage Design Report (included as Appendix C of this report) for the Pre-Development Site Drainage Areas Map.

Runoff from DR-A travels via sheet and shallow concentrated flow directly to a wetland located in the northwest corner of the site (Wetland 1). During large storm events the wetland overflows into an existing 40” pipe with direct outlet to the Normans Kill. Analysis of the existing capacity of the outlet pipe is provided in section IV of the Drainage Design Report (included as Appendix C of this report). Runoff from areas DR-B, DR-D, DR-E, and DR-F travel via sheet and concentrated flow to low areas with eventual outfall directly to the Normans Kill and Hudson River. An approximately 30-acre internal portion of the site (DR-C) was determined to be self-contained within the site capable of storing and infiltrating the 100-year storm event. Runoff from area DR-G sheet flows to the west side of River Road and travels to a low spot adjacent to the roadway where it is stored and eventually infiltrated.

The existing site falls within the Normans Kill watershed of the Middle Hudson Sub-Basin for the Lower Hudson River Basin (HUC10: 0202000602, Water Index No H-221-4) which is listed as a Class C water. Neither the Normans Kill nor the Hudson River are listed in the Manual's Appendix C as a watershed where enhanced phosphorus removal standards are required. Additionally, neither are listed in the Manual's Appendix E as a watershed impaired by pollutants related to construction activity.

7.2 Table of Receiving Waterbodies

Table 4: Receiving Waterbodies

Stormwater Structure	*Receiving Waterbody*	*NYSDEC Regulated*
None	Hudson River	Yes – Class C
40" Outlet Pipe	Normans Kill	Yes – Class C

8. STORMWATER MANAGEMENT ASSESSMENT

8.1 Methodology

To analyze the hydrologic impacts of the proposed development, a storm water management model was developed in accordance with the Manual. HydroCAD™, by HydroCAD Software Solutions LLC was used to model both the existing and proposed conditions: soil data from the NRCS Web Soil Survey was entered into the software; land coverage areas were estimated using aerial photography and site visits; watershed areas were developed using the surveyed topography; time of concentrations were estimated using USDA, Urban Hydrology for Small Watersheds, TR-55 (TR-55) methodology; and finally runoff and routing calculations were performed using the SCS Unit Hydrograph method.

Green Infrastructure practices were designed in accordance with the Manual using the NYSDEC Runoff Reduction Worksheets available through the NYSDEC's Construction Stormwater Toolbox, available on their website.

The following general steps are followed when conducting a stormwater design:

1. **Site Planning:** The existing natural resource areas and drainage patterns including wetlands, waterways, floodplains, and soils are identified. Conservation of natural resources are maximized given the proposed site.
2. **Pre and Post-Development Conditions Analysis:** The pre and post-development stormwater runoff conditions for the 1, 10, and 100-year storm events are determined using HydroCAD (detailed HydroCAD reports for this project can be found in Appendix A and B or the Drainage Design Report, provided in Appendix C of this report).
3. **Water Quality:** The Water Quality Volume and Runoff Reduction Volume are calculated using Chapter 4 of the Manual and Green Infrastructure Worksheets (see Appendix C of the Drainage Design Report, provided in Appendix C of this report).
4. **Water Quantity:** Peak runoff and stormwater retention/detention are evaluated using the Manual.

8.1.1 Water Quality Volume (WQv) / Runoff Reduction Volume (RRv)

Section 4.2 of the Manual states that Water Quality Volume (WQv) is intended to improve the water quality by capturing and treating runoff from small, frequent storm events that contain higher pollutant levels created through the increase of impervious surfaces. Impervious surfaces accumulate pollutants that quickly wash off and rapidly enter downstream waters as well as prevent natural groundwater recharge.

The WQv required for the proposed site is based upon the 90% rainfall event number, percent of impervious cover, and the total site area. Calculations were done using the Green Infrastructure worksheets and can be found in Appendix C of the Drainage Report (included as Appendix C of this report). The total WQv required is 273,874 cubic feet.

Runoff Reduction Volume (RRv) is the reduction of the total WQv by application of green infrastructure techniques and stormwater management practices to replicate pre-development hydrology more closely. The intent of RRv is to recognize the water quality benefits of certain site design practices to address flow as a pollutant of concern. Calculations were done using the Green Infrastructure worksheets and can be found in Appendix C of the Drainage Report (included as Appendix C of this report). The minimum RRv was determined to be 57,313 cubic feet.

Due to the level of contamination present in the existing soils across the Expansion Site, stormwater infiltration practices were located only in areas along the Hudson River and Normans Kill where contamination is not expected to be present. However, the Normanskill Street project area does not contain contaminated soils, so all treatment practices selected in this area utilize infiltration and therefore include RRv.

The total Water Quality Volume and Runoff Reduction Volume requirements are met for this project.

8.1.2 Channel Protection Volume (CPv)

Stream Channel Protection Volume Requirements (CPv) are designed to protect stream channels from erosion. The Manual was used to determine the water quantity requirements of CPv; specifically, providing 24-hour extended detention for the 1-year storm event or discharging directly to tidal waters.

According to Section 4.4 of the Manual, the Stream Channel Protection Volume (CPV) requirement does not apply when the site discharges to a tidal waterbody.

The CPv requirement does not apply in certain conditions, including the following:

- Reduction of the entire CPv volume is achieved at a site through green infrastructure or infiltration systems.
- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams.

The Hudson River and Normans Kill are classified as tidal waters at the project site. Therefore, 24-hour extended detention of the 1-year storm event is not required for drainage areas that outlet directly to the Hudson River or Normans Kill.

Drainage areas DR-8 and DR-9 convey large storm events to Wetland #1, and therefore require water quantity controls. A pre-and post-development analysis of the inflow to Wetland #1 was performed and analyzed as Analysis Point #1A. Hydrologic analysis for the 1-year storm event from existing to proposed is provided in Appendix A and B of the Drainage Report (included as Appendix C of this report) and summarized in Table 8.

8.1.3 Overbank Flood Control (Qp)

The primary purpose of the overbank flood control sizing criterion is to prevent an increase in the frequency and magnitude of out-of-bank flooding generated by urban development. The Manual was used to determine the water quantity requirements of Qp; specifically, providing sufficient retention volume to discharge all runoff from the proposed 10-year storm event at a rate equal to or less than the existing peak 10-year runoff rate or discharging directly to tidal waters.
According to Section 4.5 of the Manual, the Overbank Flood Control Criteria (Qp) requirement does not apply when the site discharges to a tidal waterbody.

The overbank flood control requirement (Qp) does not apply in certain conditions, including:

- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams.

The Hudson River and Normans Kill are classified as tidal waters at the project site. Therefore, 24-hour extended detention of the 10-year storm event is not required for drainage areas that outlet directly to the Hudson River or Normans Kill.

Drainage areas DR-8 and DR-9 convey large storm events to Wetland #1, and therefore require water quantity controls. A pre-and post-development analysis of the inflow to Wetland #1 was performed and analyzed as Analysis Point #1A. Hydrologic analysis for the 10-year storm event from existing to proposed is provided in Appendix A and B of the Drainage Report (included as Appendix C of this report) and summarized in Table 8.

8.1.4 Extreme Flood Control (Qf)

The intent of the extreme flood criteria is to prevent the increased risk of flood damage from large storm events, maintain the boundaries of the predevelopment 100-year floodplain, and protect the physical integrity of stormwater management practices. The Manual was used to determine the water quantity requirements of Qf; specifically, providing sufficient retention volume to discharge all runoff from the proposed 100-year storm event at a rate equal to or less than the existing peak 100-year runoff rate or discharging directly to tidal waters.

According to Section 4.6 of the Manual, the Extreme Flood Control Criteria (Qf) requirement does not apply when the site discharges to a tidal waterbody.

The 100-year storm control requirement can be waived if:

- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams.

The Hudson River and Normans Kill are classified as tidal waters at the project site. Therefore, 24-hour extended detention of the 100-year storm event is not required for drainage areas that outlet directly to the Hudson River or Normans Kill.

Drainage areas DR-8 and DR-9 convey large storm events to Wetland #1, and therefore require water quantity controls. A pre-and post-development analysis of the inflow to Wetland #1 was performed and analyzed as Analysis Point #1A. Hydrologic analysis for the 100-year storm event from existing to proposed is provided in Appendix A and B of the Drainage Report (included as Appendix C of this report) and summarized in Table 8.

8.2 Evaluation of Green Infrastructure

According to Section 4.3 of the Manual, meeting the RRv (through green infrastructure) may not be feasible due to limitations that prevent the use of an infiltration technique and/or infiltration of the total WQv. The Beacon Island portion of the project site does not allow for the infiltration of any stormwater runoff due to the presence of fly ash across the site. However, green infrastructure practices were evaluated for potential use.

8.2.1 Conservation of Natural Areas

The existing vegetation located along the bank of the Hudson River is conserved in a deed restricted area in the proposed plan. Credit has been applied.

8.2.2 Sheetflow to Riparian Buffers and Filter Strips

Riparian Buffers and Filter Strips were not utilized, due to the lack of space on the Port Authority owned land.

8.2.3 Vegetated Swales

The developed site contains vegetated swales where there is sufficient room. Due to the SMP not recommending infiltration within the Expansion Site, no credit has been applied.

8.2.4 Tree Planting / Tree Pits

New landscaping will complement the existing environment and provide aesthetics for the buildings and parking areas. No credit has been applied for proposed tree plantings.

8.2.5 Disconnection of Rooftop Runoff

Rooftop disconnection was not considered for this project, as the buildings will have large, sloped roofs in a single direction and would have to discharge to the compacted gravel areas that would potentially cause erosion and instability of the dense graded aggregate due to over saturation; as well as encourage infiltration into the fly ash sub-surface layer which is not recommended by the SMP.

8.2.6 Stream Daylighting

Stream daylighting is not available for the proposed project.

8.2.7 Bioretention

The developed site does not have sufficient area to accommodate Bioretention. The proposed stormwater treatment ponds designed for this site are located either partially or fully on the adjacent National Grid property. As required by National Grid, the treatment on their property must hold at least the 10-year storm. There is not adequate space to use bioretention systems instead of stormwater ponds in these locations given the quantity of water required to retain. This practice promotes infiltration into the fly ash sub-surface layer which is not recommended by the SMP.

8.2.8 Green Roofs

Green roofs were not considered to be feasible as pre-engineered metal buildings are proposed for this project.

8.2.9 Stormwater Planter

Stormwater Planters were not considered due to the poor soils, excessive rooftop runoff volume, and would promote infiltration into the fly ash sub-surface layer which is not recommended by the SMP.

8.2.10 Rain Barrels and Cisterns

The developed site does not have sufficient area for Rain Barrels or Cisterns to accommodate the storage of the roof runoff volume.

8.2.11 Porous Pavement

Porous pavement was not considered due to the large loads associated with the Tower sections being moved and stored on site that would cause the porous pavement surface to crumble. This practice also promotes infiltration, which is not recommended by the SMP.

8.2.12 Infiltration System

Two infiltration basins were designed to treat runoff from a portion of the Normanskill Street Improvements. Two dry swales were designed to treat runoff from the Offsite and a portion of the Normanskill Street Improvements. Three infiltration chamber systems were designed to provide runoff reduction along the Hudson River and Normans Kill.

9. POST CONSTRUCTION STORMWATER CONTROL PRACTICES

9.1 Table of Post Construction Practices

See Table 5 below.

9.2 Post Construction Practices Plan

Locations of Post Construction Practices are found in the Erosion & Sediment Control Plans and Details (included as Appendix B of this report).

To best mitigate the water quality requirements of the proposed site, two (2) stormwater quality ponds, six (6) manufactured stormwater filtering systems, three (3) infiltration chamber systems, two (2) infiltration basins, and two (2) dry swales were designed. All practices were designed in accordance with the Manual. Each practice was sized to provide WQv; however, they do not all provide storm event flow mitigation (see sections 8.1.2 through 8.1.4 above).

Drainage Areas DR-1 through DR-7 will provide WQv using manufactured water quality systems. Drainage Areas DR-8 and DR-9 will drain to stormwater ponds providing WQv. The total of all practices providing water quality volume is ***217,428*** cubic feet (cf). A full description of the designed stormwater treatment practices is provided in Section III.B of the Drainage Design Report (included as Appendix C of this report). The WQv is summarized in Table 5 below:

Table 5 – Water Quality Volume Practice Summary

Drainage Area	**Stormwater Practice**	**WQv (cf)**
DR-1	Filter Type 2	18,005
	Infiltration Chamber	2,802
DR-2	Filter Type 1	21,971
DR-3	Filter Type 1	43,939
DR-4	Filter Type 3	14,989
	Infiltration Chamber	2,052
DR-5	Infiltration Chamber	2,047
DR-6	Filter Type 1	48,060
DR-7	Filter Type 2	34,826
DR-8	Stormwater Pond #1	***14,209***
DR-9	Stormwater Pond #2	***13,622***
DR-14	Infiltration Basin #2	141
DR-15	Dry Swale #1	129
DR-17	Dry Swale #2	636
	Total WQv	***217,428***

Due to the presence of fly ash across the Expansion Site, infiltration practices were located in areas not expected to be contaminated. However, the Normanskill Street and Offsite Improvement portions of this project are in an area of uncontaminated soil with high infiltration rates. Therefore, all treatment practices selected infiltrate into the ground and provide RRv. The total RRv provided is 63,333 CF. The RRv is summarized in Table 6 below:

Table 6 – Runoff Reduction Practice Summary

Drainage Area	**Stormwater Practice**	**RRv (cf)**
DR-1	Infiltration Chamber	21,248
DR-4	Infiltration Chamber	18,464
DR-5	Infiltration Chamber	18,426
DR-11	Conservation of Natural Areas	868
DR-13	Infiltration Basin #1	1,995
DR-14	Infiltration Basin #2	2,245
DR-15	Dry Swale #1	87
	Total RRv	**63,333**

Drainage Areas DR-10, DR-11, DR-12, and DR-16 are to remain as naturally vegetated and therefore do not require water quality treatment.

However, Drainage Area DR-11 will be deed restricted to ensure the perpetual protection of the proposed area. Therefore, this area qualifies under the “Conservation of Natural Areas” volume reduction practice shown in the chart above.

9.3 Hydraulic Analysis of Pre- and Post-Development Conditions

The site was analyzed in both the pre- and post-construction stormwater conditions. Water quantity controls were required for drainage areas contributing to analysis point #1A as it is a Wetland. Using Chapter 4 of the Manual for new development, the project meets the total water quality volume required. Table 7 below summarizes the impervious cover of the pre- and post-development conditions.

Table 7 – Impervious Cover

	Pre-Development	Post-Development
Impervious Area	5.16 ac	65.9 ac
% Impervious Cover	7.1%	90.6%

The existing site has no water quality treatment measures. A portion of all stormwater not stored within the site is directly discharged into the Hudson River and Normans Kill. Per Chapter 4 of the Manual, new development projects are required to provide water quality treatment. As shown below, the project can meet the total water quality volume required. The peak discharge for all storm events draining to Analysis Point #1A is decreased in the post-development condition. The post-development peak discharge rates for the 1-year, 10-year and 100-year storm events exceed the pre-development peak discharge rates for the remaining analysis points; however, as described in Section 8.1 above, this requirement is waived when discharging directly to tidal waters. A summary of the peak discharge and stormwater management plan is shown in Table 8 below.

Table 8 – Peak Discharge and Stormwater Management Plan Summary

Storm Event	Pre-Development	Post-Development
Analysis Point #1A (Wetland #1)		
1-yr Discharge	27.32 cfs	***4.68 cfs***
10-yr Discharge	73.24 cfs	***12.50 cfs***
100-yr Discharge	163.60 cfs	***30.43 cfs***
Analysis Point #1		
1-yr Discharge	3.25 cfs	58.54 cfs
10-yr Discharge	14.96 cfs	124.96 cfs
100-yr Discharge	43.20 cfs	205.59 cfs
Analysis Point #2		
1-yr Discharge	7.17 cfs	82.79 cfs
10-yr Discharge	20.65 cfs	166.00 cfs
100-yr Discharge	48.06 cfs	290.19 cfs
Analysis Point #3		
1-yr Discharge	0.60 cfs	1.20 cfs
10-yr Discharge	1.84 cfs	2.62 cfs
100-yr Discharge	4.39 cfs	5.25 cfs
Total Area of Soil Disturbance	***73.4 acres***	
WQv Target	***275,516 cf***	
WQv Provided	***280,761 cf (max)***	
RRv Target	57,313 cf	
RRv Provided	**63,333 cf**	

Analysis Point #1A analyzes the peak discharge at Wetland #1. In larger storm events, stormwater quality ponds #1 and #2 will provide a “first flush” treatment for up to a 10-year storm event with stabilized emergency spillways to direct flow to the surrounding area for storms greater than the 10-year event. Due to the topography of the surrounding undisturbed area, water will flow toward Wetland #1. In the post-development condition, Analysis Point #1A has a peak discharge less than the pre-development condition during all storm events, therefore the required water quantity controls are met.

In the post-development condition, Analysis Point #1 has a total drainage area of 0.12 square miles (75.28 acres). This point drains to the Normans Kill with a drainage area of 162 square miles (103,680 acres). The project area makes up approximately 0.07% of the total drainage area of the Normans Kill. With an overall project time of concentration of around 10 minutes, the proposed project will have a negligible impact on the total Normans Kill hydrology as the site-produced runoff will be conveyed prior to the Normans Kill peak and will not impact the overall flood conditions of the Normans Kill.

In the post-development condition, Analysis Point #2 has a total drainage area of 0.04 square miles (23.6 acres). This point drains to the Hudson River with a drainage area of 8,090 square miles (5,177,600 acres). The project area makes up approximately 0.0005% of the total drainage area of the Hudson. With an overall project time of concentration of around 10 minutes, the proposed project will have a negligible impact on the total Hudson River hydrology, as the site-produced runoff will be conveyed prior to the Hudson River peak and will not impact the overall flood conditions of the Hudson River.
At Analysis Point #3, the post-development discharge rates are higher than the pre-development condition. However, analysis point #3 drains to the surrounding area which stores runoff to be gradually infiltrated. Runoff from this analysis point does not flow to a stream or wetland.

9.4 Deviation from NYS Stormwater Management Design Manual

The proposed stormwater management design deviates from The Manual by utilizing manufactured stormwater filtering systems for new development.

The need for alternative stormwater management practices is rooted in the extremely limited space available as well as the current site conditions. The proposed Offshore Wind Manufacturing Facility requires 85 acres of usable manufacturing and storage space along the Hudson River. It also requires close proximity to an existing port. Such requirements narrow the available project locations to a select few plots of unoccupied land in the entire state and this site was selected through a solicitation process by the state for off-shore wind development. This site was chosen given it is located adjacent to the existing Port of Albany and is directly on the Hudson River. However, the usable portion of the site adjacent to the Hudson River, is only 66-acres. Therefore, the entirety of the site is needed for the OSW manufacturing process, with an ancillary receiving site located at 700 Smith Boulevard.

The Expansion Site also extends onto the adjacent National Grid property from which APDC is leasing approximately 4.4 acres. However, National Grid has prohibited the installation of permanent stormwater infiltration practices within their property.

In addition to space limitations, the existing soils conditions prevent infiltration from being utilized as a stormwater management practice over most of the site. The existing soils are classified as Hydrologic Group D and B/D which provide little to no infiltration and are underlaid by fly ash. Infiltration practices were utilized in the areas where greater than 0.5 in/hr infiltration rates were achieved.

To adequately satisfy the WQv requirements of the Manual, manufactured systems are needed. The Contech Jellyfish units designed meet both the performance and sizing requirements of Chapter 4 of the Manual. The units are also certified by Washington State Department of Ecology (TAPE) and the Maryland Department of the Environment, adequate sources accepted by the NYSDEC. Specifications and details for the proposed units are provided in Appendix D of the Design Report (included as Appendix C of this report).

9.5 Maintenance Schedule of Post-Construction Stormwater Control Practices

See Appendix D of this report for the maintenance inspection checklists and requirement for the facilities to be maintained as summarized below in Table 9.

Table 9 – Maintenance Schedule of Post-Construction Stormwater Management Facilities

Maintained By	Name of Entity
Name, Address, Phone of Responsible Party	Albany Port District Commission 106 Smith Boulevard Albany, NY 12202 (518) 463-8763
Facilities to be Maintained	Jellyfish Filter (9 units at 6 locations) Infiltration Chamber (3 systems) Stormwater Quality Ponds (2) Infiltration Basins (2) Dry Swales (2) Stormwater Collection & Conveyance Systems
Description of Maintenance Activity for each Facility and Frequency	See Appendix D for maintenance guidelines, as recommended by the manufacturer and NYSDEC.
Description of Applicable Easements	N/A
Access and Safety Issues	Maintenance forces have access to all drainage facilities within the site.
Local and Non-Local Permits	Joint Permit Application
Legal Agreements	N/A

9.6 Drainage Structure Catchment Areas

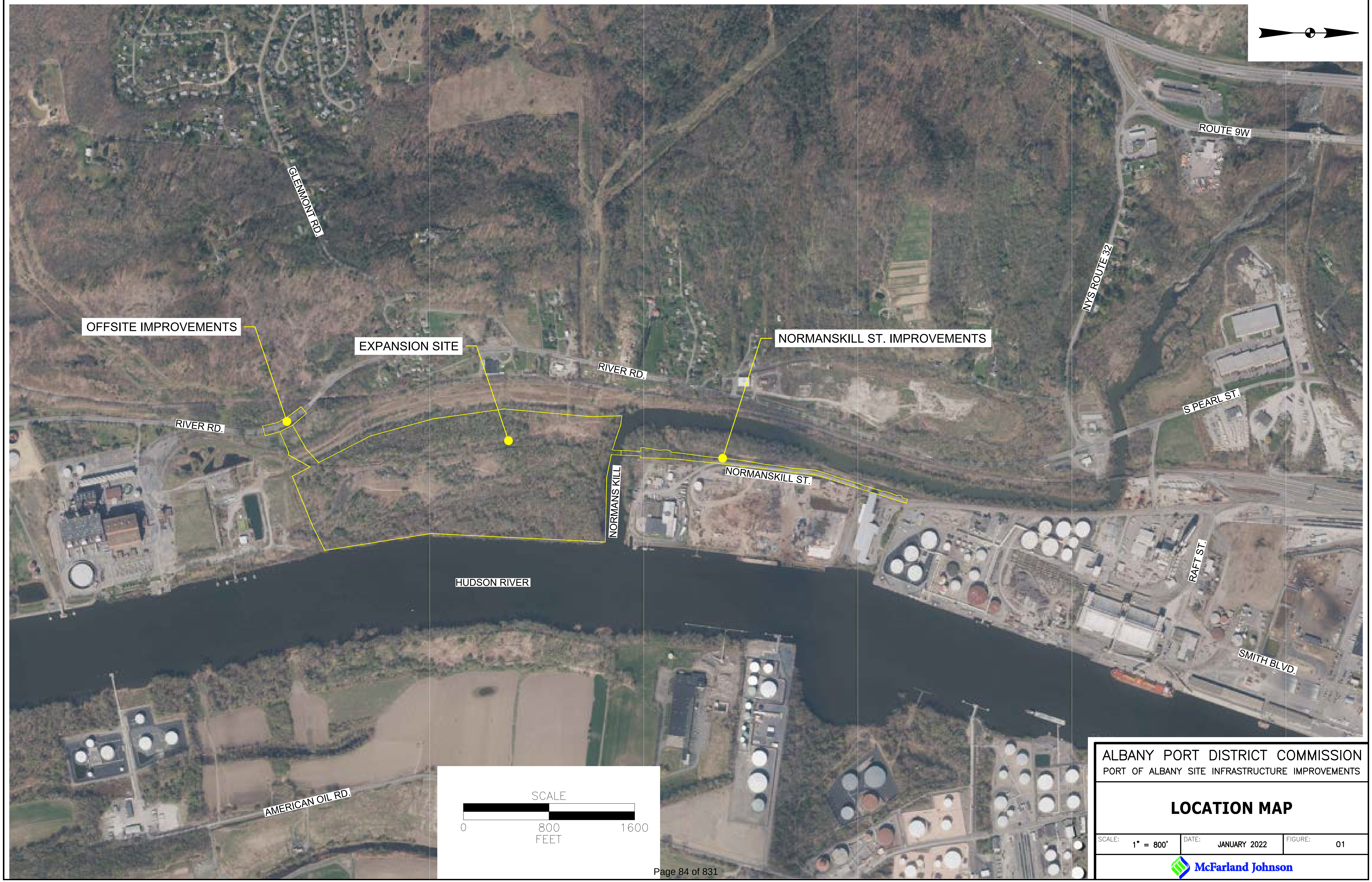
See Appendix C of the Drainage Design Report (included as Appendix C of this report).

9.7 Hydraulic Analysis of Stormwater Sewer System

All elements of the closed drainage system have been designed to be non-erosive during a 2-year storm event and capable of conveying a 10-year storm event. The profiles were created in AutoCAD Civil 3D which incorporates the rational method and Manning's Equation to iteratively calculate the hydraulic capacity, grade lines, and inlet spreads. Printouts of the closed drainage system analysis are in Appendix C of the Drainage Design Report (included as Appendix C of this report).

APPENDIX A

LOCATION MAP



ALBANY PORT DISTRICT COMMISSION

PORT OF ALBANY SITE INFRASTRUCTURE IMPROVEMENTS

LOCATION MAP

SCALE: 1" = 800'	DATE: JANUARY 2022	FIGURE: 01

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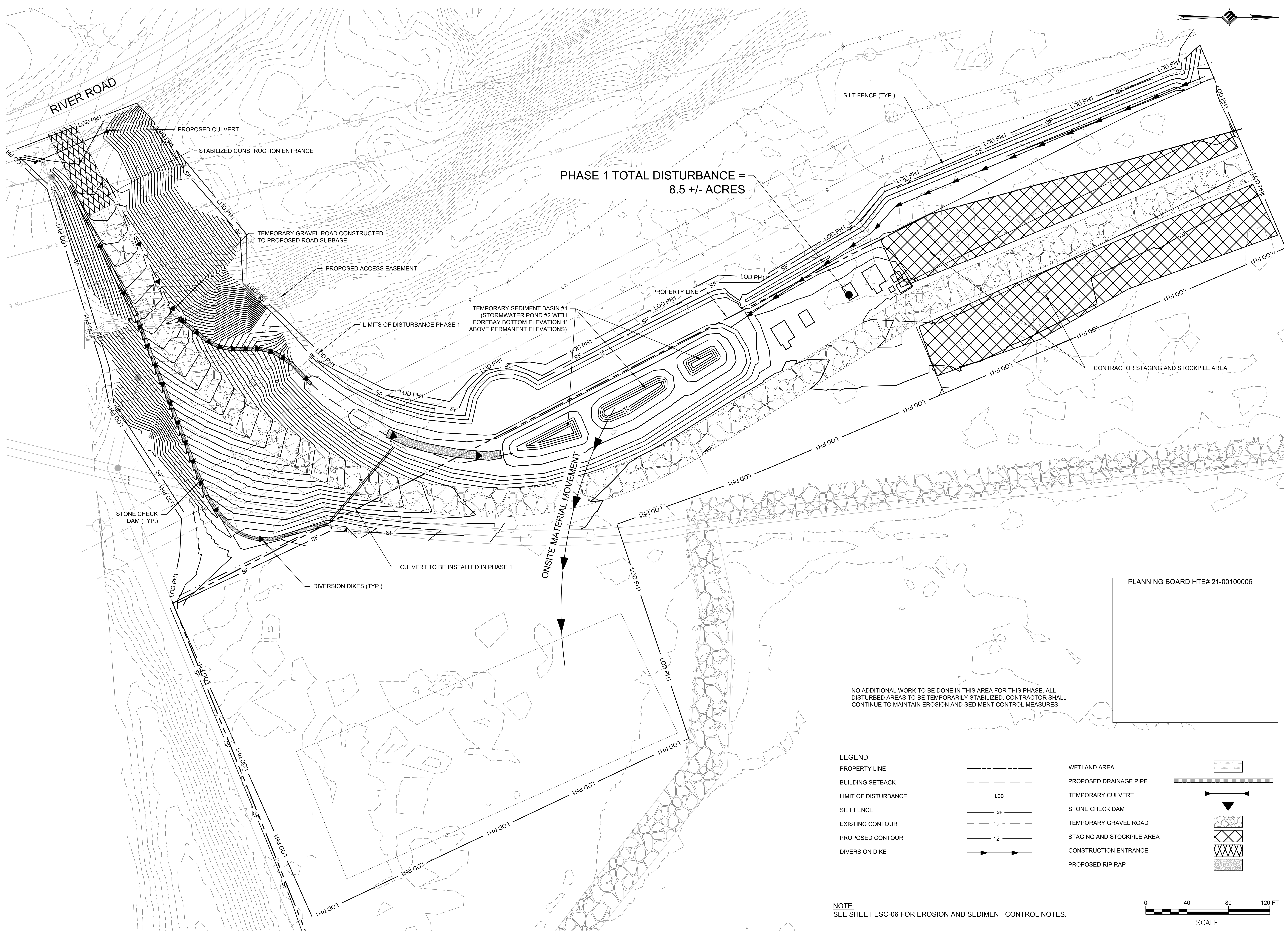
APPENDIX B

EROSION & SEDIMENT CONTROL PLANS, DETAILS & NOTES



PLANNING BOARD HTE# 21-00100006

LEGEND

PROPERTY LINE	— - — - —
BUILDING SETBACK	— — — —
LIMIT OF DISTURBANCE	—— LOD ——
SILT FENCE	—— SF ——
EXISTING CONTOUR	— — 12 - — —
PROPOSED CONTOUR	—— 12 ——
DIVERSION DIKE	—▶——▶—
WETLAND AREA	
PROPOSED DRAINAGE PIPE	
TEMPORARY CULVERT	▶——◀
STONE CHECK DAM	▼
TEMPORARY GRAVEL ROAD	
STAGING AND STOCKPILE AREA	▨
CONSTRUCTION ENTRANCE	▨
PROPOSED RIP RAP	

NOTE:
SEE SHEET ESC-06 FOR EROSION AND SEDIMENT CONTROL NOTES.

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PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION
1	05/20/22	TOWN COMMENTS

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	AS SHOWN
DATE	05/10/2022
PROJECT	18641.00

ADAM J FROSINO
088870

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
EROSION & SEDIMENT CONTROL PLAN-PH.1

DRAWING NUMBER
ESC-01

57 OF 68

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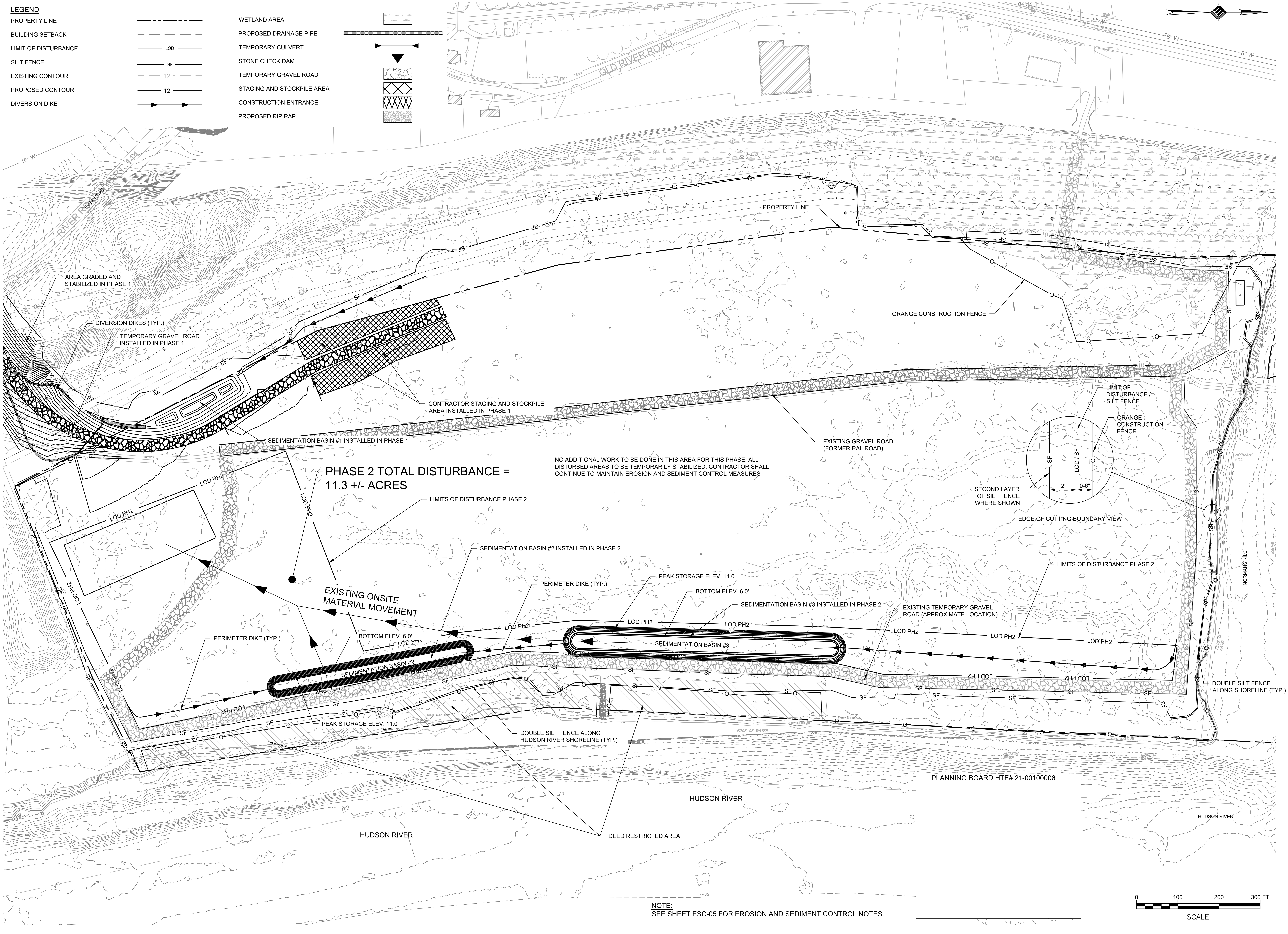
LEGEND

PROPERTY LINE	— · — · —
BUILDING SETBACK	— — —
LIMIT OF DISTURBANCE	—— LOD ——
SILT FENCE	—— SF ——
EXISTING CONTOUR	— — 12 — —
PROPOSED CONTOUR	—— 12 ——
DIVERSION DIKE	——▶——▶
WETLAND AREA	▭
PROPOSED DRAINAGE PIPE	▬
TEMPORARY CULVERT	▶——◀
STONE CHECK DAM	▼
TEMPORARY GRAVEL ROAD	▨
STAGING AND STOCKPILE AREA	▨
CONSTRUCTION ENTRANCE	▨
PROPOSED RIP RAP	▨



NO ADDITIONAL WORK TO BE DONE IN THIS AREA FOR THIS PHASE. ALL DISTURBED AREAS TO BE TEMPORARILY STABILIZED. CONTRACTOR SHALL CONTINUE TO MAINTAIN EROSION AND SEDIMENT CONTROL MEASURES

PLANNING BOARD HTE# 21-00100006

NOTE:
SEE SHEET ESC-05 FOR EROSION AND SEDIMENT CONTROL NOTES.

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DRAWING TITLE
EROSION & SEDIMENT CONTROL PLAN-PH.2

DRAWING NUMBER
ESC-02

58 OF 68

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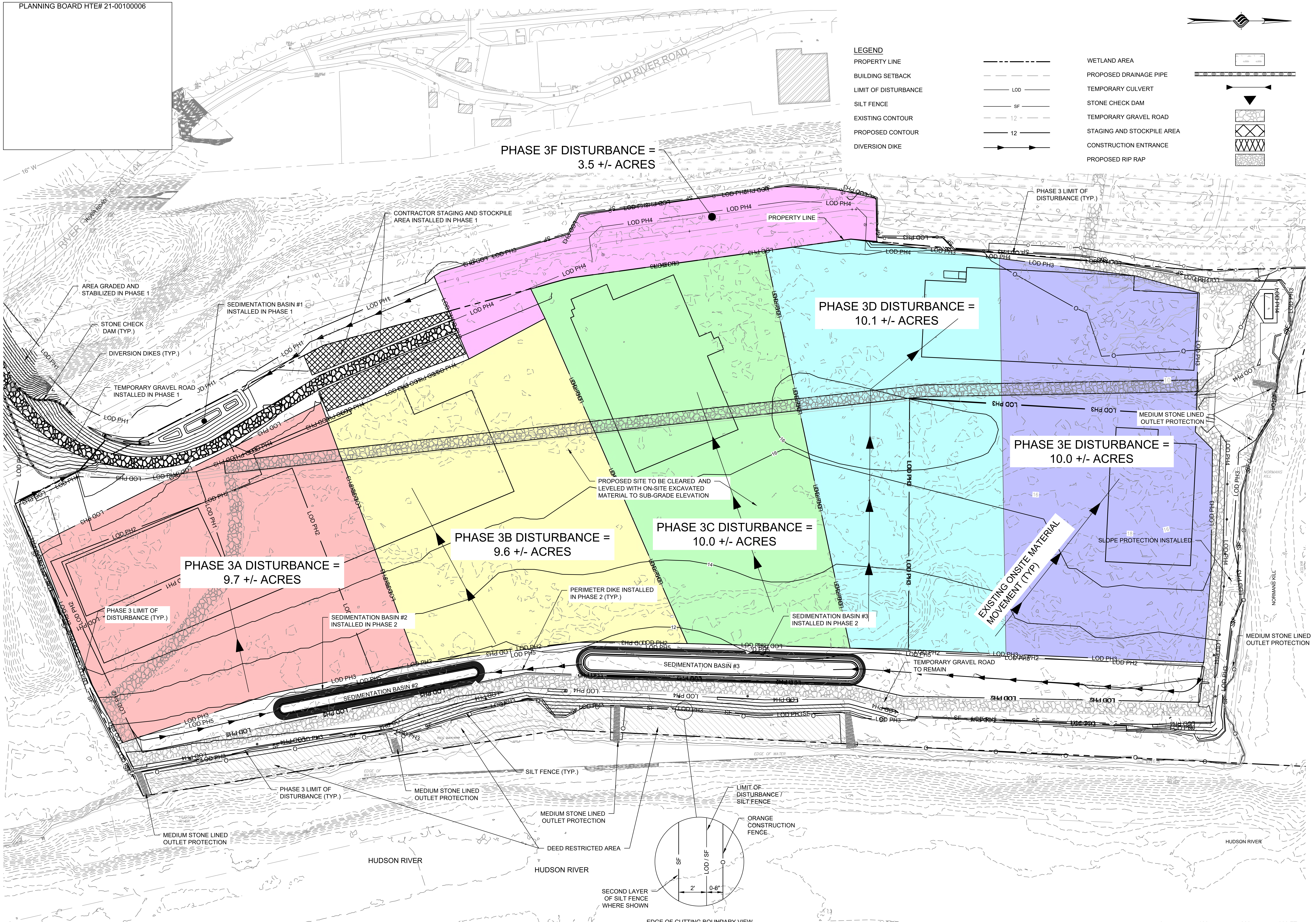
PLANNING BOARD HTE# 21-00100006



NOTE:
SEE SHEET ESC-05 FOR EROSION AND SEDIMENT CONTROL NOTES.

LEGEND

PROPERTY LINE	— - - — - - —
BUILDING SETBACK	— — — — —
LIMIT OF DISTURBANCE	—— LOD ——
SILT FENCE	—— SF ——
EXISTING CONTOUR	— — 12 — —
PROPOSED CONTOUR	—— 12 ——
DIVERSION DIKE	—▶——▶—
WETLAND AREA	☐
PROPOSED DRAINAGE PIPE	▬
TEMPORARY CULVERT	▶——◀
STONE CHECK DAM	▼
TEMPORARY GRAVEL ROAD	▨
STAGING AND STOCKPILE AREA	☒
CONSTRUCTION ENTRANCE	▨
PROPOSED RIP RAP	▨

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DRAWING TITLE
EROSION & SEDIMENT CONTROL PLAN-PH.3

DRAWING NUMBER
ESC-03

59 OF 68

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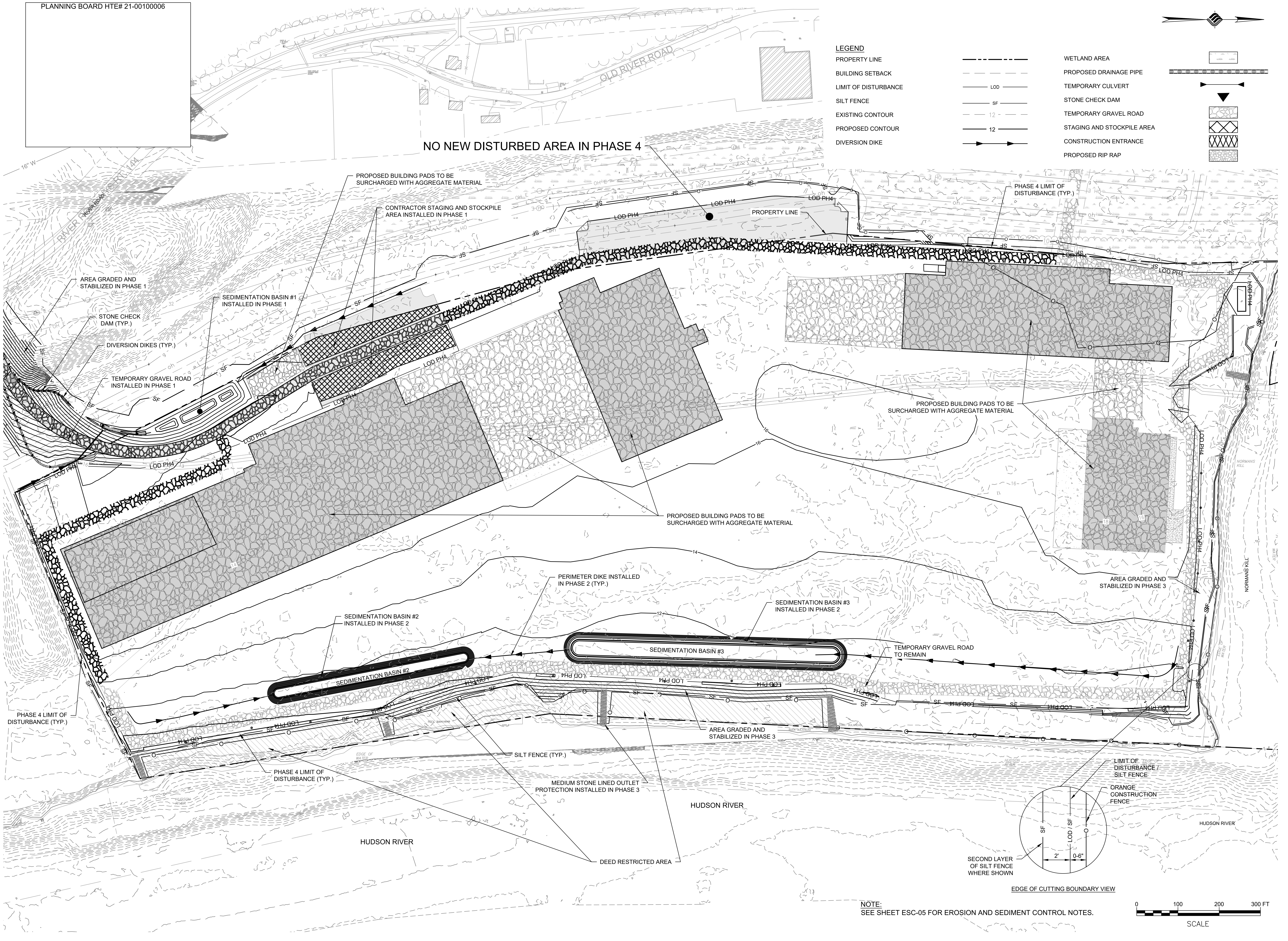
PLANNING BOARD HTE# 21-00100006



LEGEND

PROPERTY LINE	WETLAND AREA
BUILDING SETBACK	PROPOSED DRAINAGE PIPE
LIMIT OF DISTURBANCE — LOD —	TEMPORARY CULVERT
SILT FENCE — SF —	STONE CHECK DAM ▼
EXISTING CONTOUR — 12 —	TEMPORARY GRAVEL ROAD
PROPOSED CONTOUR — 12 —	STAGING AND STOCKPILE AREA
DIVERSION DIKE	CONSTRUCTION ENTRANCE
	PROPOSED RIP RAP

NOTE:
SEE SHEET ESC-05 FOR EROSION AND SEDIMENT CONTROL NOTES.

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ALBANY, NEW YORK

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DRAWING TITLE
EROSION & SEDIMENT CONTROL PLAN-PH.4

DRAWING NUMBER
ESC-04

60 OF 68

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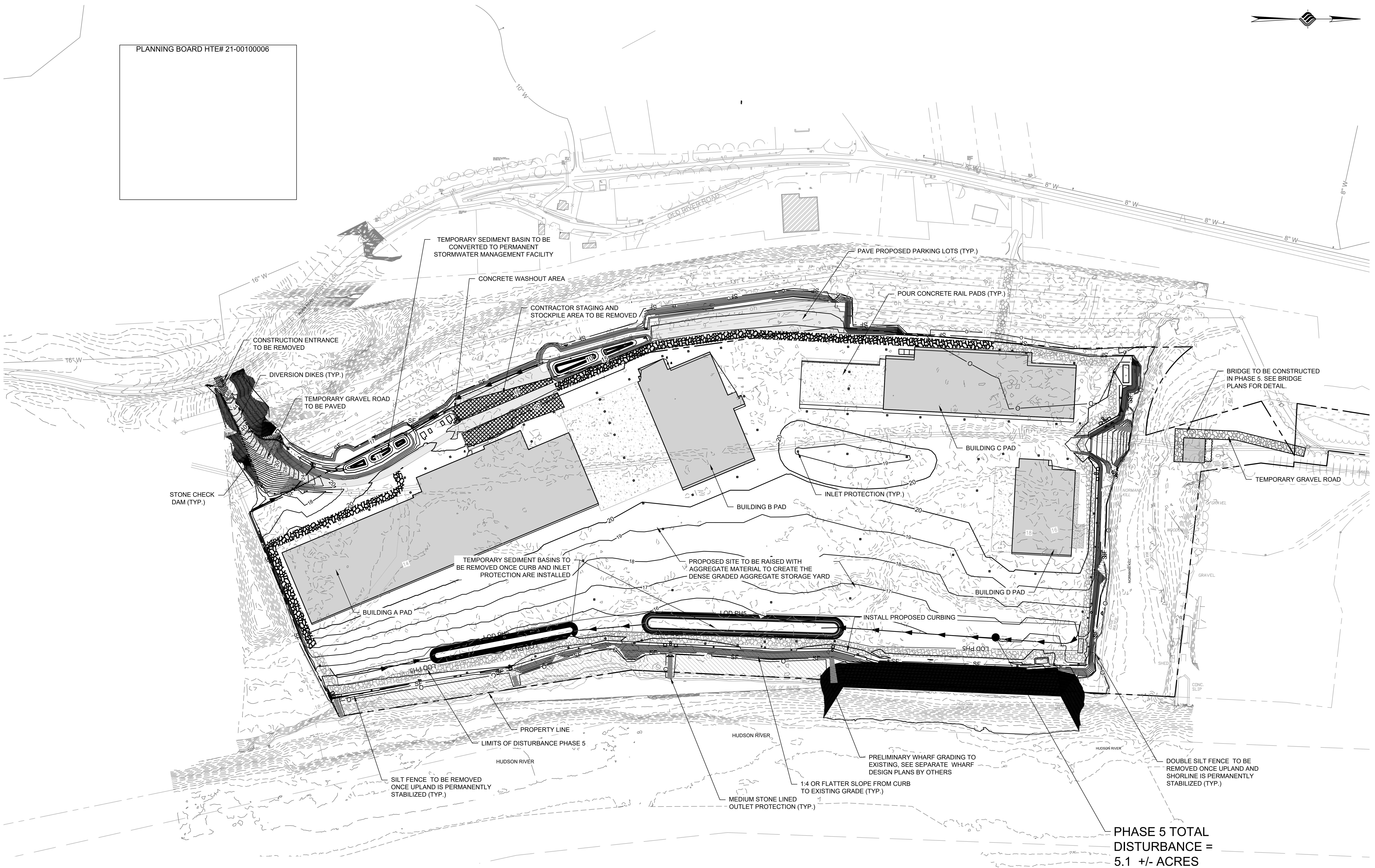
PLANNING BOARD HTE# 21-00100006



PHASE 5 TOTAL DISTURBANCE = 5.1 +/- ACRES

LEGEND

PROPERTY LINE	— - - — - - —	WETLAND AREA	▭
BUILDING SETBACK	— — — —	PROPOSED DRAINAGE PIPE	▬
LIMIT OF DISTURBANCE	—— LOD ——	TEMPORARY CULVERT	▶—◀
SILT FENCE	—— SF ——	STONE CHECK DAM	▼
EXISTING CONTOUR	— — 12 — —	TEMPORARY GRAVEL ROAD	▨
PROPOSED CONTOUR	—— 12 ——	STAGING AND STOCKPILE AREA	⊠
DIVERSION DIKE	—▶——▶—	CONSTRUCTION ENTRANCE	▩
		PROPOSED RIP RAP	▨

NOTE:
SEE SHEET ESC-05 FOR EROSION AND SEDIMENT CONTROL NOTES.

0 150 300 450 FT
SCALE

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DRAWING TITLE
EROSION & SEDIMENT CONTROL PLAN-PH.5

DRAWING NUMBER
ESC-05

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EROSION AND SEDIMENT CONTROL PLAN NOTES:

1. THE EROSION AND SEDIMENT CONTROL PLAN IS INTENDED TO REPRESENT A CONCEPTUAL APPROACH TO EROSION AND SEDIMENT CONTROL. IT IS FURTHER INTENDED THAT THE OWNER AND CONTRACTOR SHALL IMPLEMENT PRACTICES, AS REQUIRED, TO CONTROL EROSION AND SEDIMENT IN ACCORDANCE WITH THE NEW YORK STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL AND SWPPP.

2. INSTALL SILT FENCE, AND ALL OTHER EROSION CONTROL MEASURES AS INDICATED ON THE PLAN PRIOR TO THE START OF ANY EXCAVATION WORK. EROSION CONTROL MEASURES WILL BE IMPLEMENTED IN ACCORDANCE WITH THE NEW YORK STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL, NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION AND THE GOVERNING MUNICIPALITY REQUIREMENTS.

3. REMOVE AND STOCKPILE TOPSOIL IN ACCORDANCE WITH THE EROSION AND SEDIMENT CONTROL PLAN. REPLACE TOPSOIL TO A MINIMUM 4" DEPTH. ALL DISTURBED AREAS ARE TO BE HYDROSEEDED IN ACCORDANCE WITH THE EROSION AND SEDIMENT CONTROL PLANS.

4. CONTRACTOR SHALL BE RESPONSIBLE FOR THE MAINTENANCE AND REMOVAL OF TEMPORARY SEDIMENTATION CONTROLS, INCLUDING INLET PROTECTION AND SILT FENCE. EROSION CONTROL MEASURES SHALL NOT BE REMOVED BEFORE AREAS HAVE BEEN PROPERLY STABILIZED.

5. CONTRACTOR SHALL MAINTAIN A STOCK PILE OF EROSION AND SEDIMENT CONTROL MEASURES ON SITE AS INDICATED ON THE PLAN.

6. NO PETROLEUM PRODUCTS ARE TO BE STORED ON SITE WITHOUT PRIOR APPROVAL OF THE LOCAL STORMWATER INSPECTOR. ANY PETROLEUM ON SITE WILL COMPLY WITH ALL LOCAL, STATE, AND FEDERAL GOVERNMENT REGULATIONS.

7. WRAP YARD INLET GRATES IN FILTER FABRIC PROGRESSIVELY AS STORM SEWER AND YARD INLETS ARE INSTALLED.

8. ALL EROSION CONTROL MEASURES ARE TO BE REPLACED WHENEVER THEY BECOME CLOGGED OR INOPERABLE AND SHALL BE REPLACED AT A MINIMUM OF EVERY 3 MONTHS.

9. JUTE MESH WILL BE USED ON SLOPES STEEPER THAN 3:1 AND WHEREVER NECESSARY TO CONTROL EROSION AND SILTATION OF EXISTING DRAINAGE SYSTEMS AS ORDERED BY THE ENGINEER.

10. ALL DISTURBED AREAS SHALL BE FINISH GRADED TO PROMOTE VEGETATION ON ALL EXPOSED AREAS AS SOON AS PRACTICABLE. STABILIZATION PRACTICES (TEMPORARY/PERMANENT SEEDING, MULCHING, GEOTEXTILES, ETC.) MUST BE IMPLEMENTED WITHIN SEVEN (7) DAYS WHERE CONSTRUCTION ACTIVITIES HAVE TEMPORARILY OR PERMANENTLY CEASED, AND NOT EXPECTED TO RESUME WITHIN FOURTEEN (14) DAYS.

11. ALL RIP-RAP OUTLET PROTECTION TO BE CONSTRUCTED PER NYSDEC STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL.

12. CONTRACTOR SHALL TAKE THE NECESSARY MEASURES, INCLUDING WATER SPRINKLING, TO PROVIDE DUST CONTROL DURING CONSTRUCTION.

13. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF ALL TEMPORARY AND PERMANENT EROSION CONTROL FEATURES THROUGHOUT THE DURATION OF CONSTRUCTION.
    A. ALL SEDIMENT TRAPPING DEVICES AND INLET PROTECTION DEVICES SHALL BE CLEANED OF ACCUMULATED SILT WHEN STORAGE CAPACITY HAS BEEN REDUCED BY 50% OF THEIR DESIGN CAPACITY.
    B. ALL SEDIMENT SHALL BE REMOVED FROM BEHIND SILT FENCE AND STRAW BALES WHEN IT ACCUMULATES TO A MAXIMUM HEIGHT OF 6".
    C. AFTER VEGETATION HAS BEEN SUBSTANTIALLY ESTABLISHED, EXCAVATE SWALES OF ACCUMULATED SILT. RE-ESTABLISHED VEGETATION ON DISTURBED AREAS.
    D. SEDIMENT COLLECTED BY EROSION CONTROL MEASURES SHALL BE DISPOSED OF BY SPREADING ON-SITE OR HAULED AWAY IF DETERMINED TO BE UNSUITABLE FOR FILL.

12. ALL DISTURBED AREAS SHALL BE STABILIZED, SEEDED AND MULCHED WITHIN 7 DAYS OF CEASED CONSTRUCTION ACTIVITY.

13. TOTAL PROJECT DISTURBANCE AREA PER THE NYSDEC SPDES STANDARDS IS 79 ACRES.

14. ALL AREAS TO REMAIN AS PERVIOUS VEGETATED AREAS SHALL BE RESTORED IN ACCORDANCE WITH THE NYS STORMWATER MANAGEMENT DESIGN MANUAL TABLE 5.3 SOIL RESTORATION REQUIREMENTS.

PERMANENT SEEDING NON-SLOPED AREAS:

1. IF SOILS ARE COMPACTED, SCARIFY UPPER TWO INCHES BY BACKBLADING WITH DOZER, RAKING, OR DISKING.

2. PLACE TOPSOIL TO A MINIMUM DEPTH OF 4 INCHES.

3. SEED PER SCHEDULE SPECIFIED ON LANDSCAPE PLANS.

4. FERTILIZE WITH 600 POUNDS PER ACRE OF 10-10-10. LIME TO ACHIEVE A PH OF NOT LESS THAN 5.5 OR GREATER THAN 7.6. IF HYDROSEEDER IS NOT USED, SEED AND FERTILIZER SHOULD BE LIGHTLY RAKED INTO SOIL.

5. MULCH WITH CLEAN (WEED FREE) STRAW IF SPECIFIED ON PLANS.

PERMANENT SEEDING SLOPED AREAS:

1. IF SOILS ARE COMPACTED, SCARIFY UPPER TWO INCHES BY BACKBLADING WITH DOZER, RAKING, OR DISKING.

2. PLACE TOPSOIL TO A MINIMUM DEPTH OF 4 INCHES.

3. FERTILIZE WITH 600 POUNDS PER ACRE OF 10-10-10. LIME TO ACHIEVE A PH OF NOT LESS THAN 5.5 OR GREATER THAN 7.6. IF HYDROSEEDER IS NOT USED, SEED AND FERTILIZER SHOULD BE LIGHTLY RAKED INTO SOIL.

4. IMMEDIATELY SEED PER SEED SCHEDULE SPECIFIED ON LANDSCAPE PLAN.

5. PROVIDE JUTE MESH IF SPECIFIED ON PLANS OR MULCH WITH CLEAN (WEED FREE) STRAW.

EROSION AND SEDIMENT CONTROL SEQUENCE:

THE TOWN OF BETHLEHEM SHOULD BE NOTIFIED PRIOR TO CONSTRUCTION ACTIVITIES STARTING AND CEASING DISTURBANCE OF OVER 5 ACRES AT ONE TIME.

PHASE I:

- INSTALL CONSTRUCTION ENTRANCE ROADS
- ESTABLISH THE PROJECT CONSTRUCTION STAGING/OFFICE AREA
- USE ANY ACCESS ROAD CUT MATERIAL AS FILL FOR THE CONSTRUCTION STAGING AREA
- TEMPORARILY STABILIZE ALL DISTURBED AREAS
- INSTALL SILT FENCE DOWNSTREAM OF ALL DISTURBED AREAS
- CONSTRUCT SEDIMENTATION BASIN #1 FOREBAYS TO 1' HIGHER THEN PROPOSED GRADING FOR THE PERMANENT STORM WATER MANAGEMENT POND #2 FOREBAYS.
- MATERIAL FROM THE POND EXCAVATION TO BE PLACED AND COMPACTED AS PART OF THE BUILDING A EMBANKMENT.
- BASED ON THE POTENTIAL FOR PROPOSED ROCK CUT WHEN EXCAVATING THE ROADWAY SECTION PHASE II MAY PROGRESS PRIOR TO THE COMPLETION OF PHASE I. THE PHASE I AREA SHALL BE STABILIZED TO THE MINIMIZE DISTURBANCE AREA PRIOR TO PROGRESSION TO PHASE II

PHASE II:

- INSTALL PERIMETER CONTROLS
- INSTALL ADDITIONAL CONSTRUCTION ACCESS ROAD
- CONSTRUCT SEDIMENTATION BASINS AND DIVERSION DIKES TO BASINS
- THE PROPOSED TEMPORARY STORM WATER TREATMENT FACILITIES SHALL BE INSTALLED BEFORE PROGRESSING INTO PHASE III

PHASE III:

- SITE TO BE GRUBBED AND GRADED TO THE TOP OF SUB-GRADE ELEVATION IN SUB-PHASES
- BALANCE CUT AND FILLS IN THE SITE.
- COMPACT/IMPROVE EXISTING GROUND CONDITIONS ACCORDING TO GEOTECHNICAL REPORT
- IMPORT MATERIAL TO RAISE THE SITE TO PROPOSED SUB-GRADE ELEVATIONS
- LIMITS OF DISTURBANCE DISTURBANCE TO BE MINIMIZED IN EACH SUB-PHASE BY STABILIZING AREAS WITHIN 2 DAYS AFTER FINAL GRADE IS ACHIEVED
- THE SUB-PHASE AREAS WILL BE DISTURBED AND STABILIZE IN A ROLLING OPERATION AS THE EARTHWORK PROGRESSES FROM THE SOUTH END OF THE SITE TO THE NORTH END. TO AVOID STOCKPILING AVAILABLE CUT MATERIAL FROM ONE SUB-PHASE AREA MAY BE DEPOSITED AND STABILIZED WITHIN ANOTHER SUB-PHASE AREA; HOWEVER THE OVERALL TOTAL DISTURBED AREA SHALL NOT EXCEED 11 ACRES.
- PHASE IV AGGREGATE PLACEMENT WORK WILL OCCUR SIMULTANEOUSLY AND PROVIDE STABILIZATION ONCE SUB-GRADE ELEVATIONS HAVE BEEN ACHIEVED.

PHASE IV:

- HAUL IN PROPOSED AGGREGATE MATERIAL TO SURCHARGE THE BUILDING FOOTPRINTS AND CONCRETE AREAS
- MAINTAIN EXISTING PHASE III EROSIONAL AND SEDIMENT CONTROL MEASURES
- MONITOR SETTLEMENT OF THE SUB-GRADE MATERIAL IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.

PHASE V:

- INSTALL STORM SEWER SYSTEM WITH INLET PROTECTION FOR DRAINAGE STRUCTURES AND STONE LINING OUTLET PROTECTION
- INSTALL SITE UTILITIES
- SPREAD AGGREGATE MATERIAL TO STORAGE AREAS
- INSTALL INFILTRATION CHAMBERS
- POUR ALL PROPOSED CONCRETE RAIL PADS AND SIDEWALKS
- INSTALL PROPOSED CONCRETE CURBING
- PAVE PARKING LOT AREAS
- REMOVE CONSTRUCTION STAGING AREA
- CONVERT TEMPORARY SEDIMENT BASIN TO PERMANENT STORM WATER MANAGEMENT FACILITIES BY EXCAVATING THE PERMANENT POOL AND FOREBAYS DOWN TO FINAL GRADE AND CONVERTING THE OUTLET STRUCTURE.
- REMOVE TEMPORARY SEDIMENT BASINS, WHICH ARE NOT TO BE CONVERTED TO PERMANENT PRACTICES
- FINAL STABILIZATION FOR EMBANKMENT SLOPES ALONG THE NORMANS KILL AND HUDSON RIVER

TEMPORARY SEEDING:

1. IF SOILS ARE COMPACTED, SCARIFY UPPER TWO INCHES BY BACKBLADING WITH DOZER, RAKING, OR DISKING. FERTILIZE WITH 300 POUNDS PER ACRE OF 10-10-10.

2. NOTE: NO FERTILIZER SHOULD BE USED AFTER OCTOBER 1ST IF THERE IS DANGER OF LEACHING INTO WATER RESOURCE.

3. IMMEDIATELY SEED PER SEED SCHEDULE SPECIFIED ON LANDSCAPE PLAN.

4. APPLY STRAW MULCH AS NECESSARY TO HOLD IN MOISTURE, PROTECT SOIL FROM EROSION, HOLD SEED IN PLACE, AND KEEP SOIL TEMPERATURES MORE CONSTANT; 2 TONS PER ACRE.

SOIL RESTORATION NOTES:

SOIL RESTORATION PROCEDURE:

DURING PERIODS OF RELATIVELY LOW TO MODERATE SUBSOIL MOISTURE, THE DISTURBED SUBSOILS ARE RETURNED TO ROUGH GRADE AND THE FOLLOWING SOIL RESTORATION STEPS APPLIED:

1. APPLY 3 INCHES OF COMPOST OVER SUBSOIL

2. TILL COMPOST INTO SUBSOIL TO A DEPTH OF AT LEAST 12 INCHES USING A CAT-MOUNTED RIPPER, TRACTOR-MOUNTED DISC, OR TILLER, MIXING, AND CIRCULATING AIR AND COMPOST INTO SUBSOILS

3. ROCK-PICK UNTIL UPLIFTED STONE/ROCK MATERIALS OF FOUR INCHES AND LARGER SIZE ARE CLEANED OFF THE SITE

4. APPLY TOPSOIL TO A DEPTH OF 6 INCHES

5. VEGETATE AS REQUIRED BY APPROVED PLAN.

AT THE END OF THE PROJECT AN INSPECTOR SHOULD BE ABLE TO PUSH A 3/8” METAL BAR 12 INCHES INTO THE SOIL JUST WITH BODY WEIGHT. TILLING (STEP 2 ABOVE) SHOULD NOT BE PERFORMED WITHIN THE DRIP LINE OF ANY EXISTING TREES OR OVER UTILITY INSTALLATIONS THAT ARE WITHIN 24 INCHES OF THE SURFACE.

COMPOST SPECIFICATIONS:

COMPOST SHALL BE AGED, FROM PLANT DERIVED MATERIALS, FREE OF VIABLE WEED SEEDS, HAVE NO VISIBLE FREE WATER OR DUST PRODUCED WHEN HANDLING, PASS THROUGH A HALF INCH SCREEN AND HAVE A PH SUITABLE TO GROW DESIRED PLANTS.

WINTER STABILIZATION:

1. PREPARE A SNOW MANAGEMENT PLAN WITH ADEQUATE STORAGE FOR SNOW AND CONTROL OF MELT WATER, REQUIRING CLEARED SNOW TO BE STORED IN A MANNER NOT AFFECTING ONGOING CONSTRUCTION ACTIVITIES.
2. TO ENSURE ADEQUATE STABILIZATION OF DISTURBED SOIL IN ADVANCE OF A MELT EVENT, AREAS OF DISTURBED SOIL SHOULD BE STABILIZED AT THE END OF EACH WORK DAY UNLESS:
    A. WORK WILL RESUME WITHIN 24 HOURS IN THE SAME AREA AND NO PRECIPITATION IS FORECAST OR;
    B. THE WORK IS IN DISTURBED AREAS THAT COLLECT AND RETAIN RUNOFF, SUCH AS OPEN UTILITY TRENCHES, FOUNDATION EXCAVATIONS, OR WATER MANAGEMENT AREAS.
3. IF THE SITE WILL NOT HAVE EARTH DISTURBING ACTIVITIES ONGOING DURING THE "WINTER SEASON", ALL BARE EXPOSED SOIL MUST BE STABILIZED BY ESTABLISHED VEGETATION, STRAW OR OTHER ACCEPTABLE MULCH, MATTING, ROCK OR OTHER APPROVED MATERIAL SUCH AS ROLLED EROSION CONTROL PRODUCTS. SEEDING OF AREAS WITH MULCH COVER IS PREFERRED BUT SEEDING ALONE IS NOT ACCEPTABLE FOR PROPER STABILIZATION.

SOIL DISTURBANCE PHASING

PHASE	DISTURBANCE AREA
1	8.5 ACRES
2	11.3 ACRES
3	11 ACRES MAX.
3A	9.7 ACRES
3B	9.6 ACRES
3C	10.0 ACRES
3D	10.1 ACRES
3E	10.0 ACRES
3F	3.5 ACRES
4	NO NEW AREAS (MAY OCCUR SIMULTANEOUSLY WITH PHASE 3)
5	5.1 ACRES

NOTE: A 5-ACRE WAIVER REQUEST MUST BE APPROVED BY THE TOWN OF BETHLEHEM (MS4) PRIOR TO DISTURBING MORE THAN 5 ACRES.

PLANNING BOARD HTE# 21-00100006

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PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION
1	05/20/22	TOWN COMMENTS

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

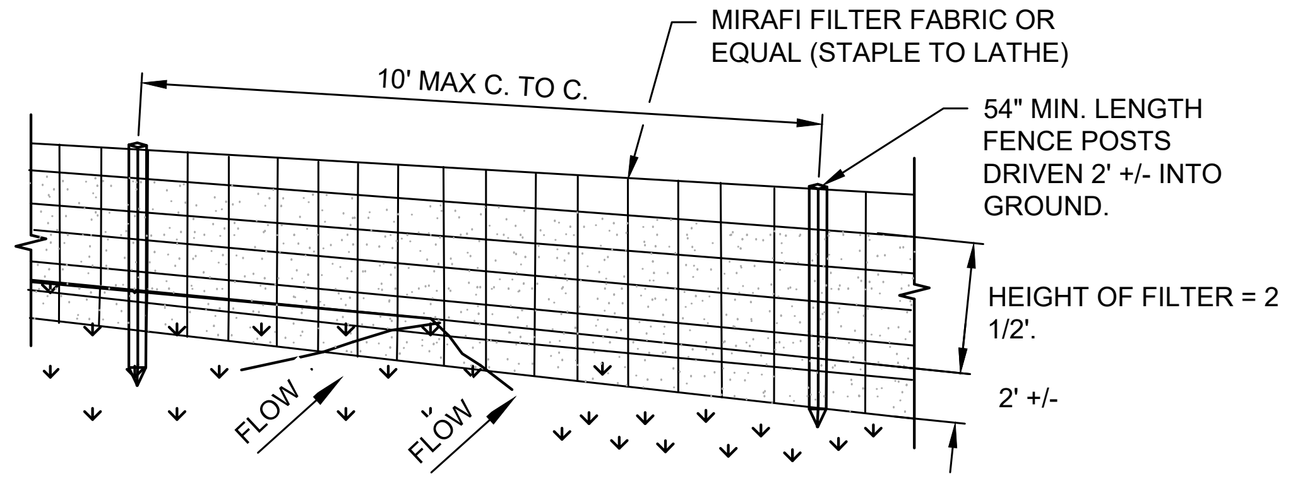
DRAWN JES
DESIGNED NSO
CHECKED AJF
SCALE AS SHOWN
DATE 05/10/2022
PROJECT 18641.00

088870

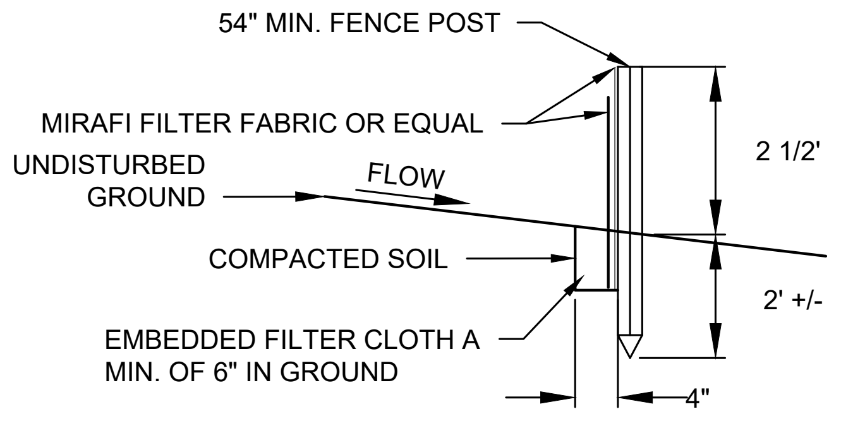
DRAWING TITLE
EROSION & SEDIMENT CONTROL NOTES

DRAWING NUMBER
ESC-06
62 OF 68

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES\18641.00-ESC-DETS.DWG



PERSPECTIVE VIEW

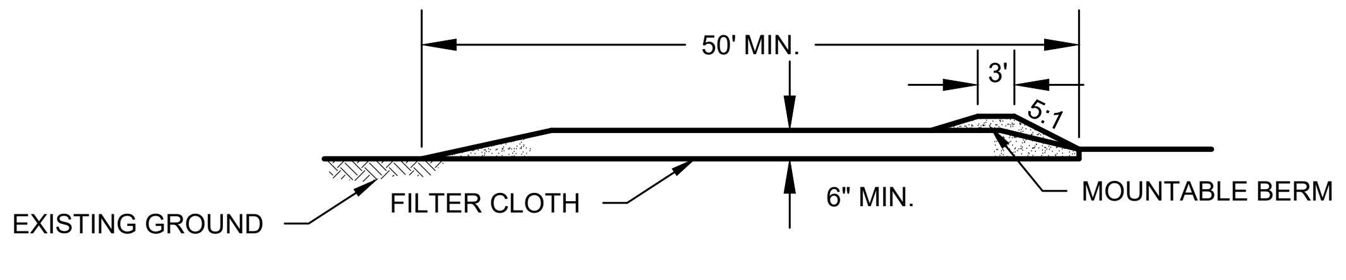


SECTION VIEW

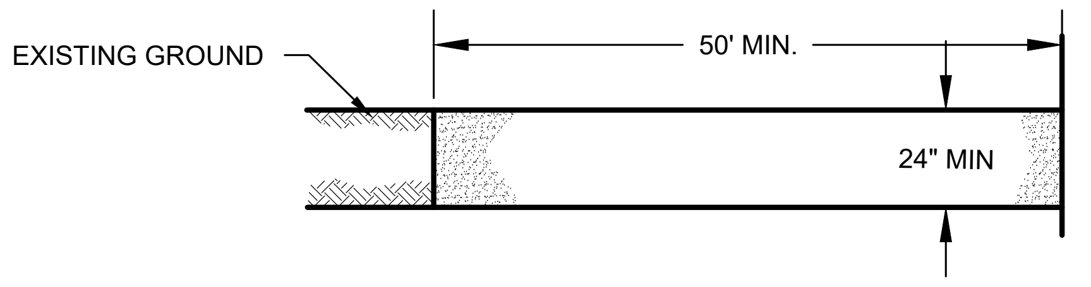
NOTES:
1. MIRAFI FILTER FABRIC TO BE SECURED TO FENCE POSTS WITH STAPLES. POSTS SHALL BE STEEL EITHER "T" OR "U" TYPE OR HARDWOOD.
2. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER THEY SHALL BE OVER-LAPPED BY SIX INCHES AND FOLDED.
3. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN "BULGES" DEVELOP IN THE SILT FENCE.

SILT FENCE

N.T.S.



PROFILE

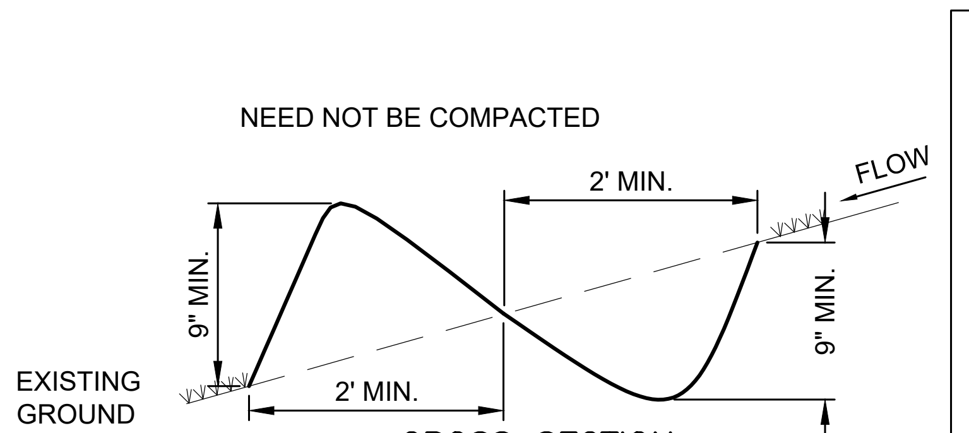


PLAN VIEW

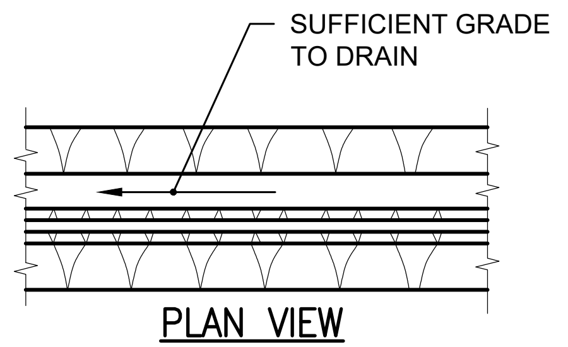
NOTES:
1. STONE SIZE - USE #3 CRUSHED STONE OR GRAVEL (PER NYSDOT SECTION 209).
2. LENGTH - NOT LESS THAN 50 FEET.
3. THICKNESS - NOT LESS THAN SIX (6) INCHES.
4. WIDTH - TWENTY-FOUR (24) FOOT IF SINGLE ENTRANCE TO SITE.
5. FILTER CLOTH - WILL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING OF STONE.
6. SURFACE WATER - ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED ACROSS THE ENTRANCE. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM WITH 5:1 SLOPES WILL BE PERMITTED.
7. MAINTENANCE - THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY, ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACTED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED IMMEDIATELY.
8. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE AND WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.
9. PERIODIC INSPECTION AND NEEDED MAINTENANCE SHALL BE PROVIDED AFTER EACH RAIN.

STABILIZED CONSTRUCTION ENTRANCE

N.T.S.



CROSS SECTION



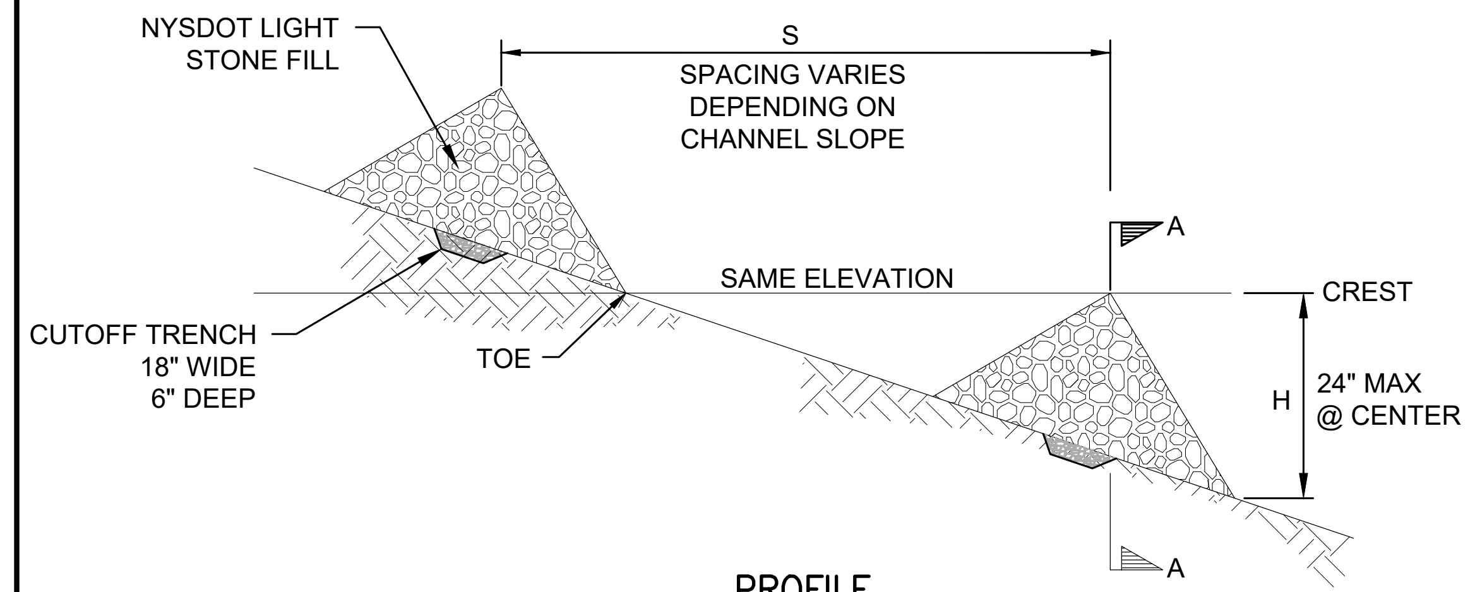
PLAN VIEW

NOTES:
1. ALL PERIMETER DIKE/SWALE SHALL HAVE UNINTERRUPTED POSITIVE GRADE TO AN OUTLET.
2. DIVERTED RUNOFF FROM A DISTURBED AREA SHALL BE CONVEYED TO A SEDIMENT TRAPPING DEVICE.
3. DIVERTED RUNOFF FROM AN UNDISTURBED AREA SHALL OUTLET INTO AN UNDISTURBED STABILIZED AREA AT NON-EROSION VELOCITY.
4. THE SWALE SHALL BE EXCAVATED OR SHAPED TO LINE GRADE, AND CROSS SECTION AS REQUIRED TO MEET THE CRITERIA SPECIFIED IN THE STANDARD.
5. STABILIZATION OF THE AREA DISTURBED BY THE DIKE AND SWALE SHALL BE DONE IN ACCORDANCE WITH THE STANDARD AND SPECIFICATIONS FOR TEMPORARY SEEDING AND MULCHING, AND SHALL BE DONE WITHIN 10 DAYS.
6. PERIODIC INSPECTION AND REQUIRED MAINTENANCE MUST BE PROVIDED AFTER EACH RAIN EVENT.

PERIMETER DIKE

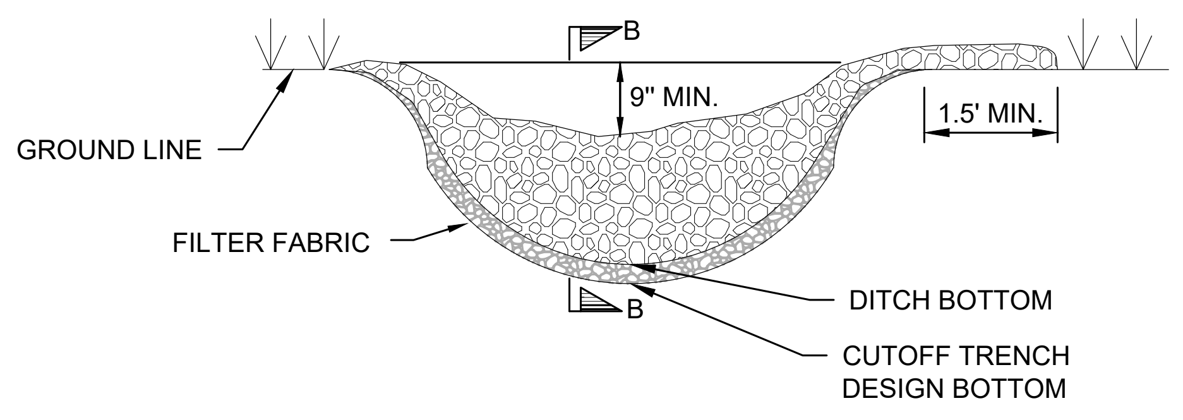
N.T.S.

PLANNING BOARD HTE# 21-00100006

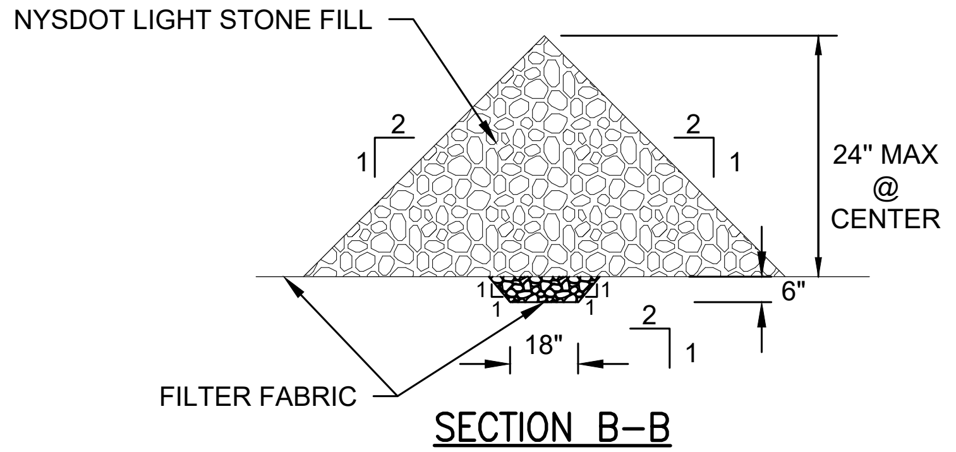


PROFILE

$$S = \frac{H\ (FT)}{SLOPE\ (FT/FT)}$$



SECTION A–A

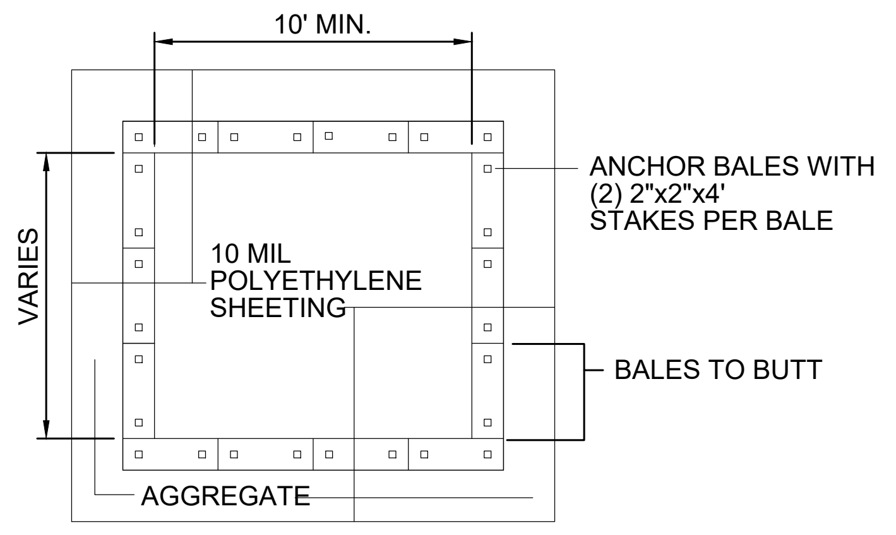


SECTION B–B

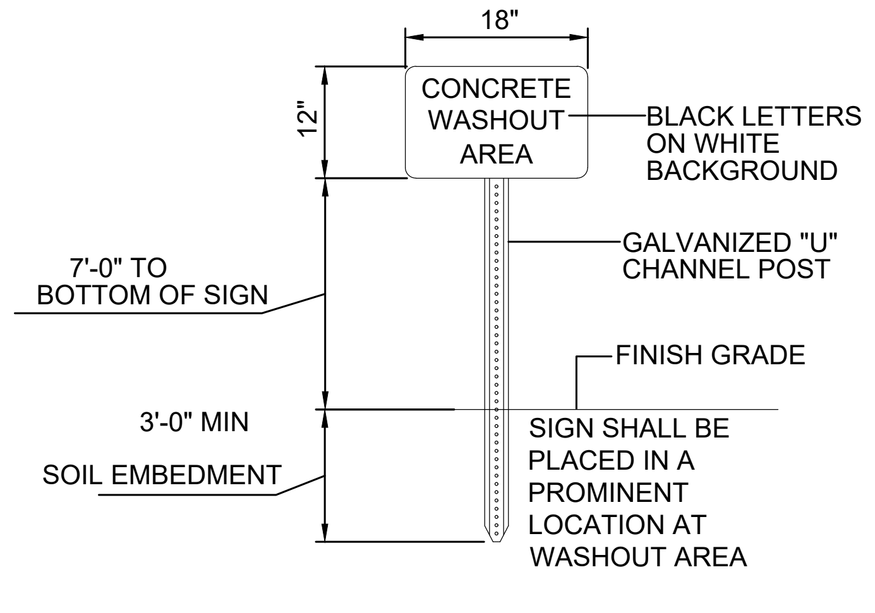
CONSTRUCTION SPECIFICATIONS
1. STONE WILL BE PLACED ON A FILTER FABRIC FOUNDATION TO THE LINES GRADES AND LOCATIONS SHOWN ON THE PLAN.
2. SET SPACING OF CHECK DAMS TO ASSUME THAT THE ELEVATIONS OF THE CREST OF THE DOWNSTREAM DAM IS AT THE SAME ELEVATION OF THE TOE OF THE UPSTREAM DAM.
3. EXTEND THE STONE A MINIMUM OF 1.5' BEYOND THE DITCH BANKS TO PREVENT CUTTING AROUND THE DAM.
4. PROTECT THE CHANNEL DOWNSTREAM OF THE LOWESTCHECK DAM FROM SCOUR AND EROSION WITH STONE OR LINER AS APPROPRIATE.
5. ENSURE THAT CHANNEL APPURTENANCES SUCH AS CULVERT ENTRANCES BELOW CHECK DAMS ARE NOT SUBJECT TO DAMAGE OR BLOCKAGE FROM DISPLACED STONES.

LIGHT STONE CHECK DAM

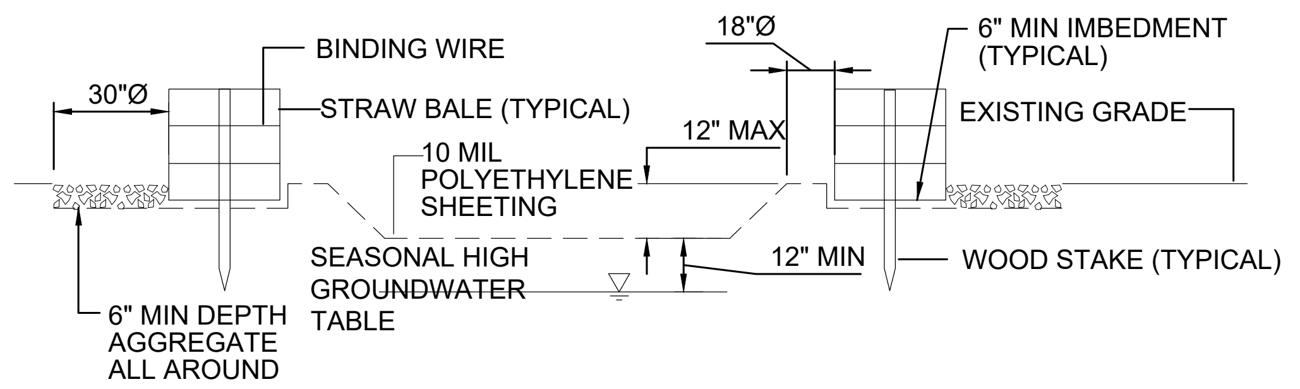
N.T.S.



PLAN



WASHOUT SIGN

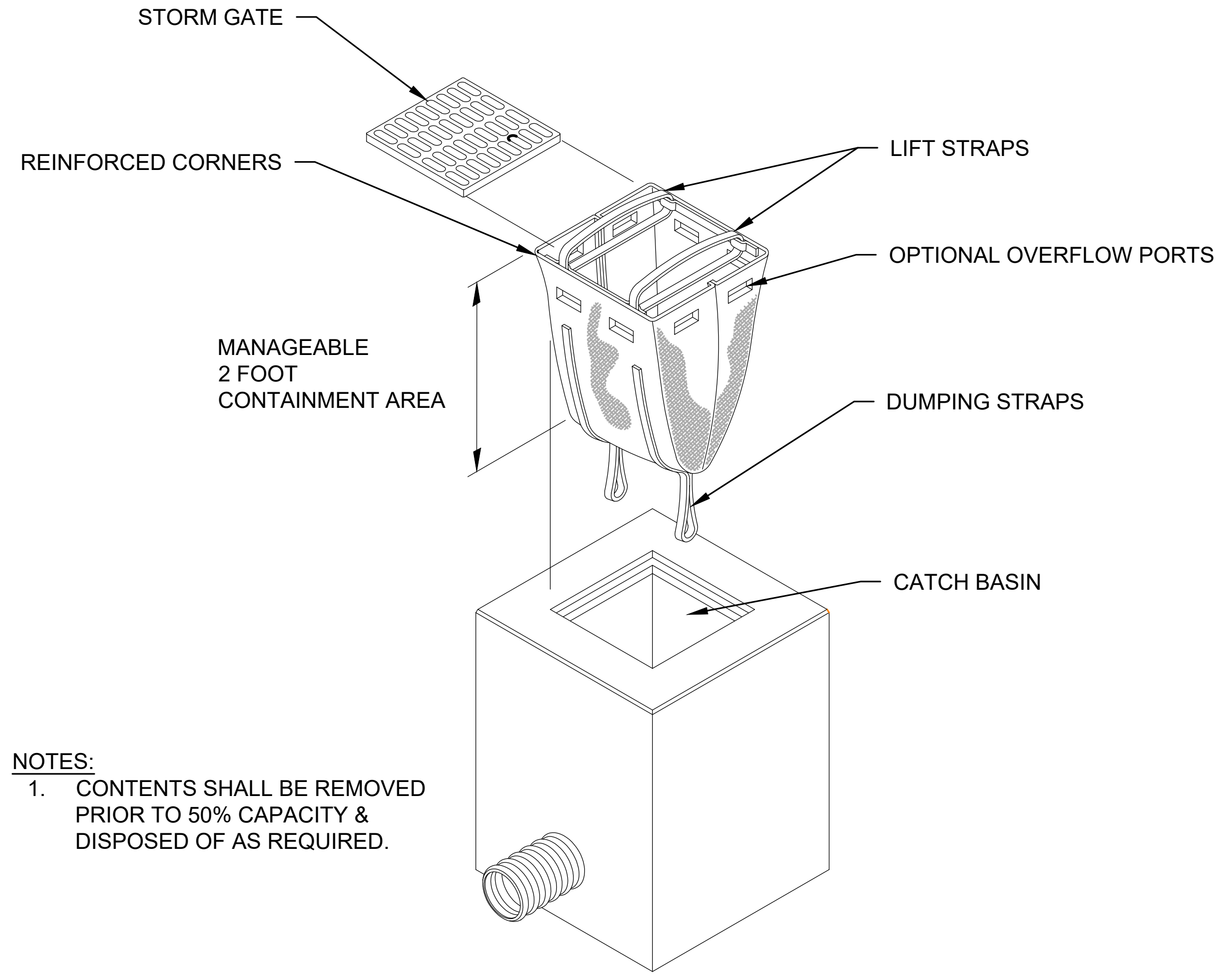


TYPICAL SECTION

NOTES:
1. CONTAINMENT MUST BE STRUCTURALLY SOUND AND LEAK FREE AND CONTAIN ALL LIQUID WASTES.
2. CONTAINMENT DEVICES MUST BE OF SUFFICIENT QUANTITY OR VOLUME TO COMPLETELY CONTAIN THE LIQUID WASTES GENERATED.
3. WASHOUT MUST BE CLEANED OR NEW FACILITIES CONSTRUCTED AND READY TO USE ONCE WASHOUT IS 75% FULL.
4. WASHOUT AREA(S) SHALL BE INSTALLED IN A LOCATION EASILY ACCESSIBLE BY CONCRETE TRUCKS.
5. ONE OR MORE AREAS MAY BE INSTALLED ON THE CONSTRUCTION SITE AND MAY BE RELOCATED AS CONSTRUCTION PROGRESSES.
6. AT LEAST WEEKLY REMOVE ACCUMULATION OF SAND AND AGGREGATE AND DISPOSE OF PROPERLY.

CONCRETE WASHOUT

N.T.S.



NOTES:
1. CONTENTS SHALL BE REMOVED PRIOR TO 50% CAPACITY & DISPOSED OF AS REQUIRED.

INLET PROTECTION

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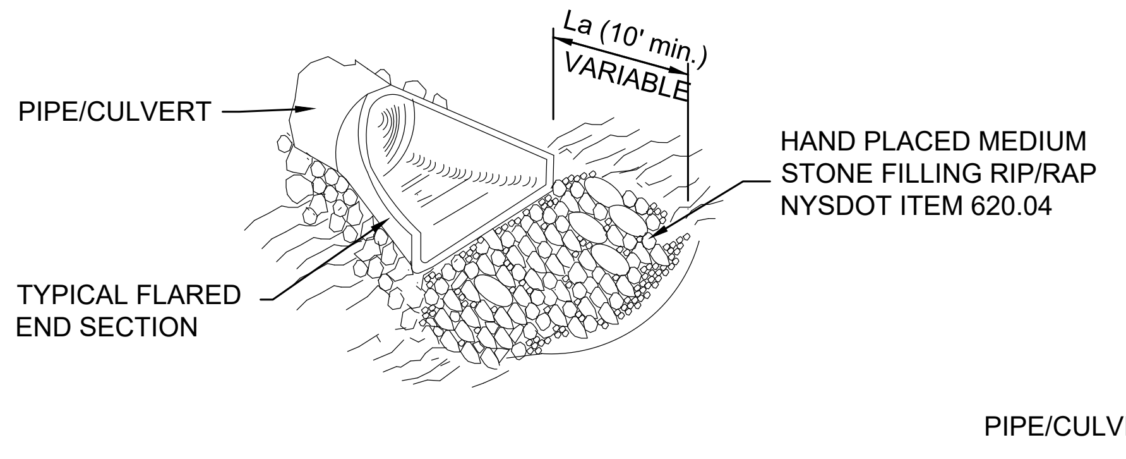
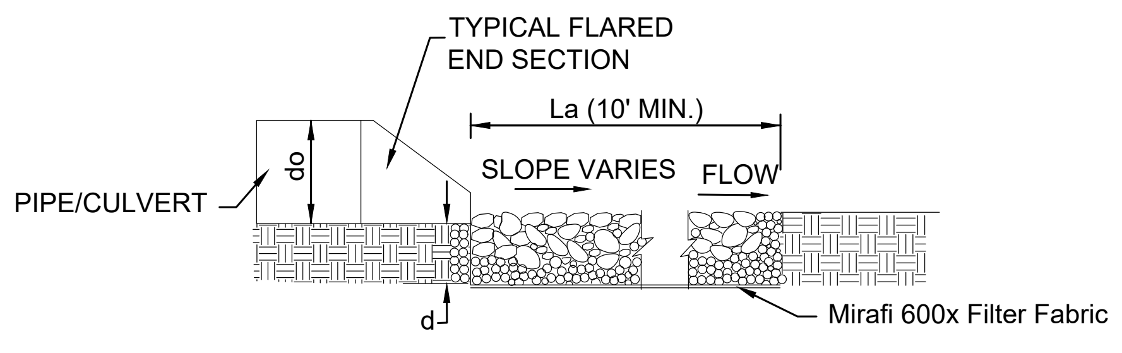
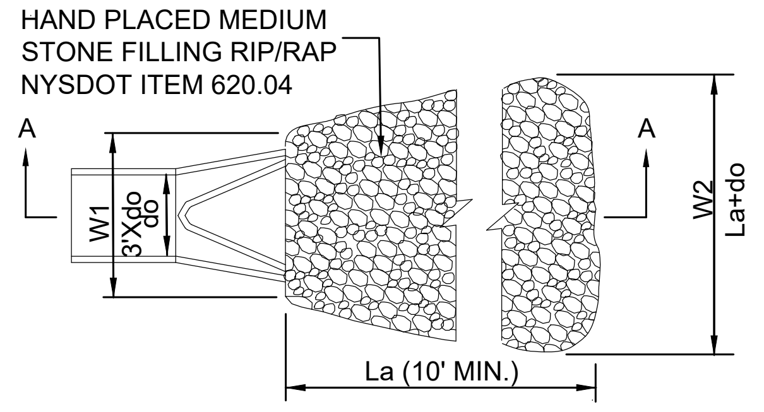


ISOMETRIC VIEW



SECTION A–A



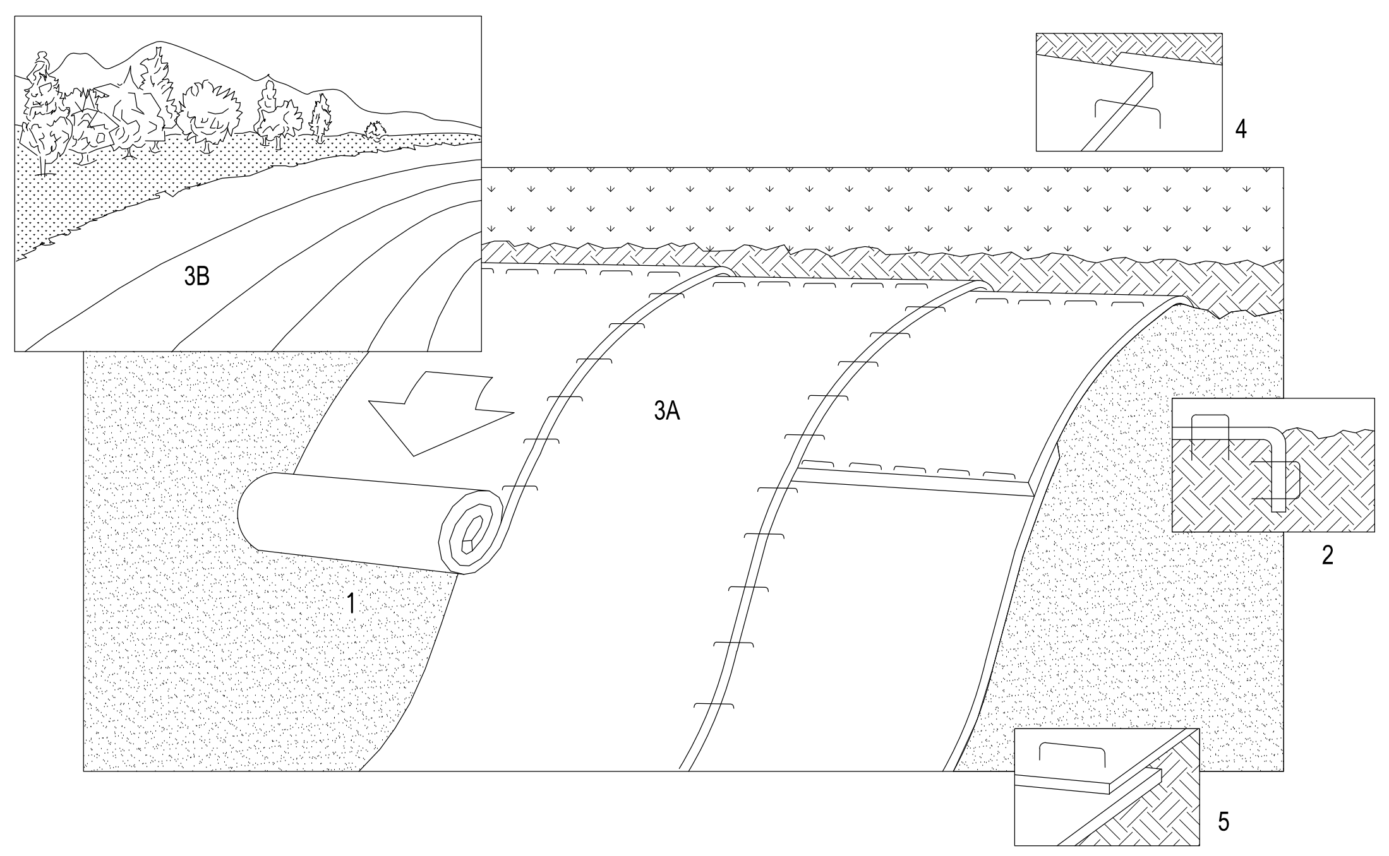
PLAN

do = Pipe Dia. (see plans)
La = Apron Length
W = Apron Width (centered on pipe)
d = Apron Depth

NOTES:
1. d = 1.5 TIMES THE MAXIMUM STONE DIAMETER BUT NO LESS THAN 6".
2. INSTALL FILTER MIRAFI 500X OR APPROVED EQUAL FILTER FABRIC BETWEEN RIP-RAP AND SUBGRADE.

OUTLET PROTECTION - RIP RAP

N.T.S.



INSTALLATION OF GEOSYNTHETICS EROSION CONTROL MATS:
1. EROSION CONTROL MATS SHALL BE UTILIZED ON ALL SLOPES OF 3:1 OR GREATER REQUIRING PERMANENT STABILIZATION.
2. SLOPES SHALL BE PREPARED AND EROSION CONTROL MATS SHALL BE INSTALLED, ANCHORED AND SOIL FILLED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS AND INSTALLATION PROCEDURES.
3. SEEDING SHALL BE COMPLETED IN ACCORDANCE WITH THE PERMANENT SEEDING SPECIFICATIONS ABOVE PRIOR TO PLACEMENT OF THE EROSION CONTROL MAT. A SECOND APPLICATION OF PERMANENT SEEDING SHALL BE APPLIED AFTER THE MAT IS IN PLACE, PRIOR TO SOIL FILLING THE MAT.
4. CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTENANCE, RESEEDING AND REPAIR OF MATS AND SLOPES UNTIL PERMANENT STABILIZATION IS ACHIEVED.

NOTES:
REFER TO GENERAL STAPLE PATTERN GUIDE FOR CORRECT STAPLE PATTERN RECOMMENDATIONS FOR SLOPE INSTALLATIONS.
1. PREPARE SOIL BEFORE INSTALLING BLANKETS, INCLUDING APPLICATION OF LIME, FERTILIZER, AND SEED. NOTE: WHEN USING CELL-O-SEED DO NOT SEED PREPARED AREA. CELL-O-SEED MUST BE INSTALLED WITH PAPER SIDE DOWN.
2. BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN 6" DEEP X 6" WIDE TRENCH. BACKFILL AND COMPACT THE TRENCH AFTER STAPLING.
3. ROLL THE BLANKETS (A) DOWN OR (B) HORIZONTALLY ACROSS THE SLOPE.
4. THE EDGES OF PARALLEL BLANKETS MUST BE STAPLED WITH APPROXIMATELY 2" OVERLAP.
5. WHEN BLANKETS MUST BE SPLICED DOWN THE SLOPE, PLACE BLANKETS END OVER END (SHINGLE STYLE) WITH APPROXIMATELY 4" OVERLAP. STAPLE THROUGH OVERLAPPED AREA, APPROXIMATELY 12" APART.
6. MANUFACTURER'S INSTALLATION INSTRUCTIONS SHALL SUPERCEDE THIS DETAIL.

EROSION CONTROL MAT INSTALLATION

N.T.S.

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION
1	05/20/22	TOWN COMMENTS

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	AS SHOWN
DATE	05/10/2022
PROJECT	18641.00

ADAM J FROSINO
088870

DRAWING TITLE
EROSION & SEDIMENT CONTROL DETAILS

DRAWING NUMBER
ESC-07

63 OF 68

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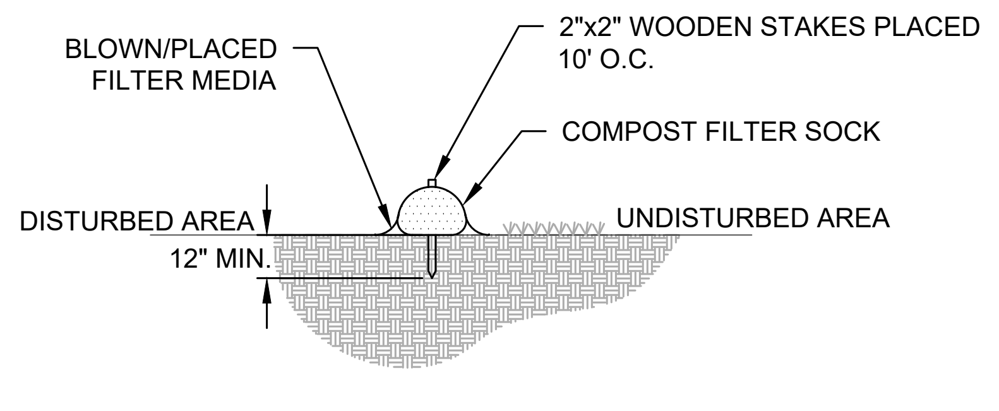
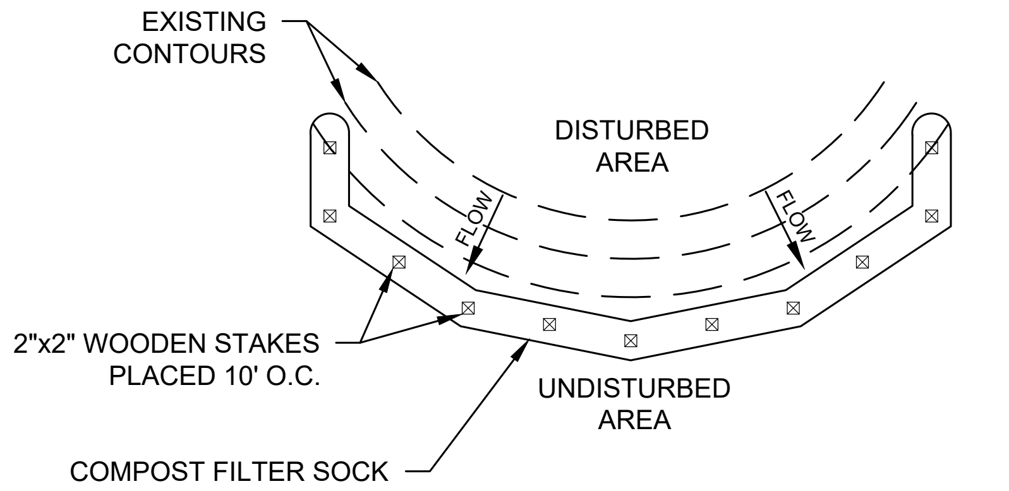
PLANNING BOARD HTE# 21-00100006



SECTION VIEW



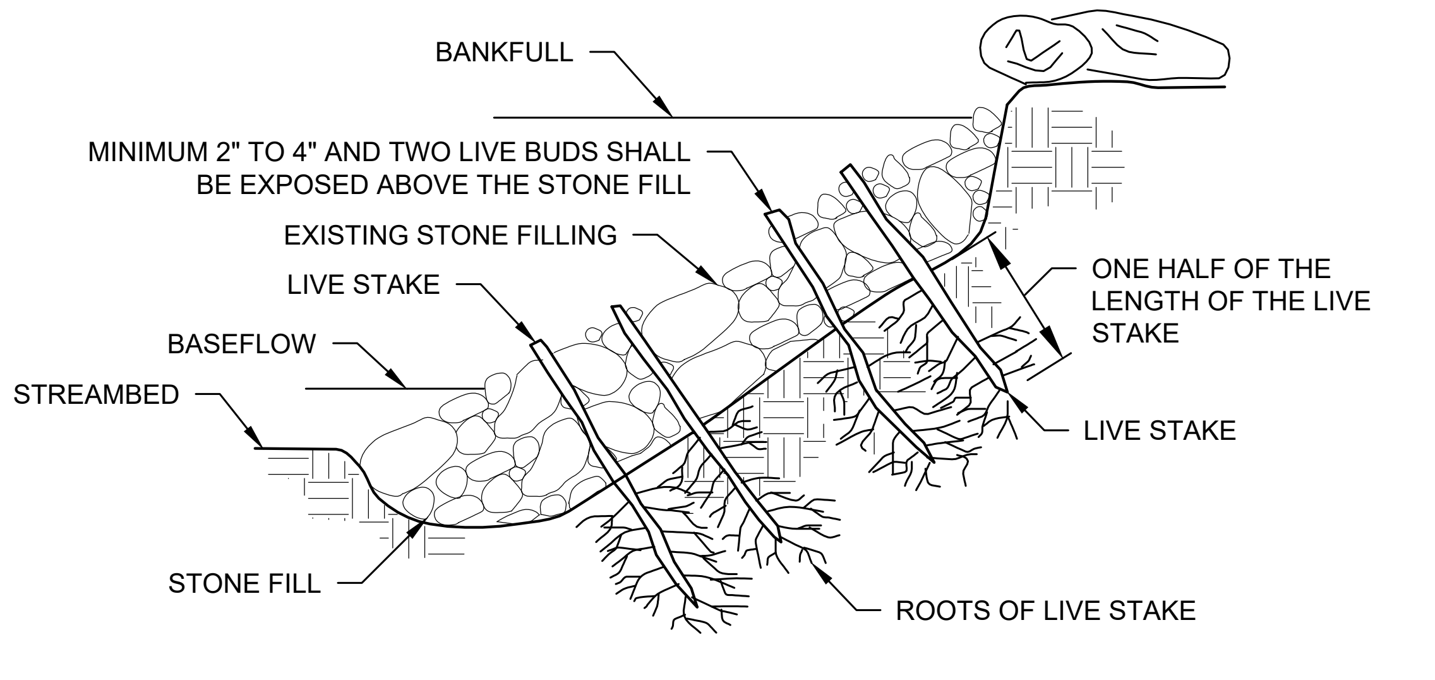
PLAN VIEW

NOTES:

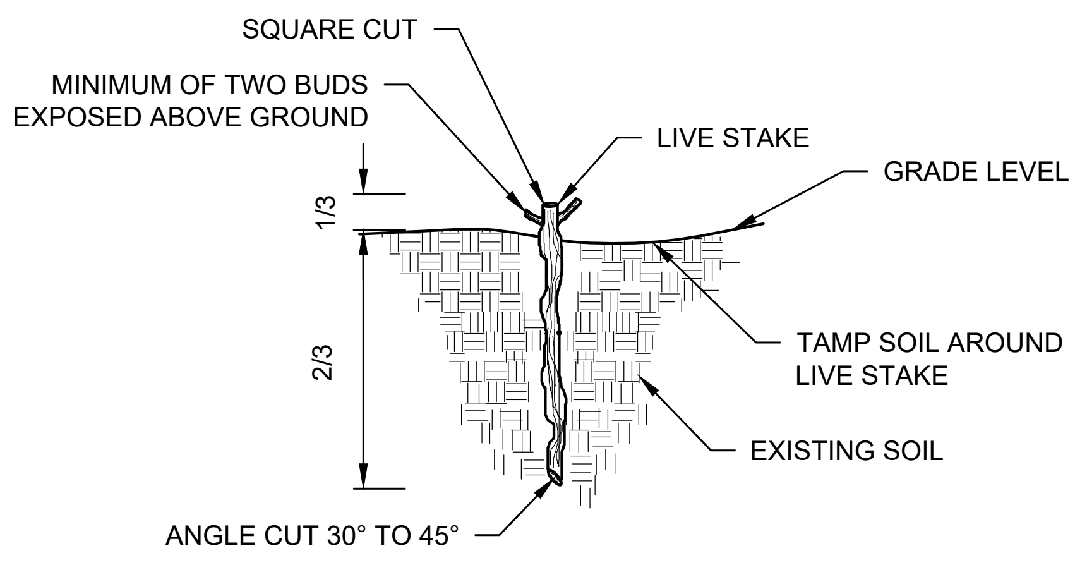
1. SOCK FABRIC SHALL MEET STANDARDS OF TABLE 5.1 OF NYS STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL. COMPOST SHALL MEET THE STANDARDS LISTED ON TABLE 5.2 OF NYS STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL.
2. COMPOST FILTER SOCK SHALL BE PLACED AT EXISTING LEVEL GRADE. BOTH ENDS OF THE SOCK SHALL BE EXTENDED AT LEAST 8 FEET UP SLOPE AT 45° TO THE MAIN SOCK ALIGNMENT. MAXIMUM SLOPE LENGTH ABOVE ANY SOCK SHALL NOT EXCEED THAT SHOWN ON FIGURE X.X OF NYS STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL. STAKES MAY BE INSTALLED IMMEDIATELY DOWNSLOPE OF THE SOCK IF SO SPECIFIED BY THE MANUFACTURER.
3. TRAFFIC SHALL NOT BE PERMITTED TO CROSS FILTER SOCKS.
4. ACCUMULATED SEDIMENT SHALL BE REMOVED WHEN IT REACHES HALF THE ABOVEGROUND HEIGHT OF THE SOCK AND DISPOSED IN THE MANNER DESCRIBED ELSEWHERE IN THE PLAN.
5. SOCKS SHALL BE INSPECTED WEEKLY AND AFTER EACH RUNOFF EVENT. DAMAGED SOCKS SHALL BE REPAIRED ACCORDING TO MANUFACTURER'S SPECIFICATIONS OR REPLACED WITHIN 24 HOURS OF INSPECTION.
6. BIODEGRADABLE FILTER SOCKS SHALL BE REPLACED AFTER 6 MONTHS; PHOTODEGRADABLE SOCKS AFTER 1 YEAR. POLYPROPYLENE SOCKS SHALL BE REPLACED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS.
7. UPON STABILIZATION OF THE AREA TRIBUTARY TO THE SOCKS. STAKES SHALL BE REMOVED. THE SOCK MAY BE LEFT IN PLACE AND VEGETATED OR REMOVED. IN THE LATTER CASE, THE MESH SHALL BE CUT OPEN AND THE MULCH SPREAD AS A SOIL SUPPLEMENT.

COMPOST FILTER SOCK

N.T.S.



LIVE STAKE JOINT PLANTING CROSS SECTION

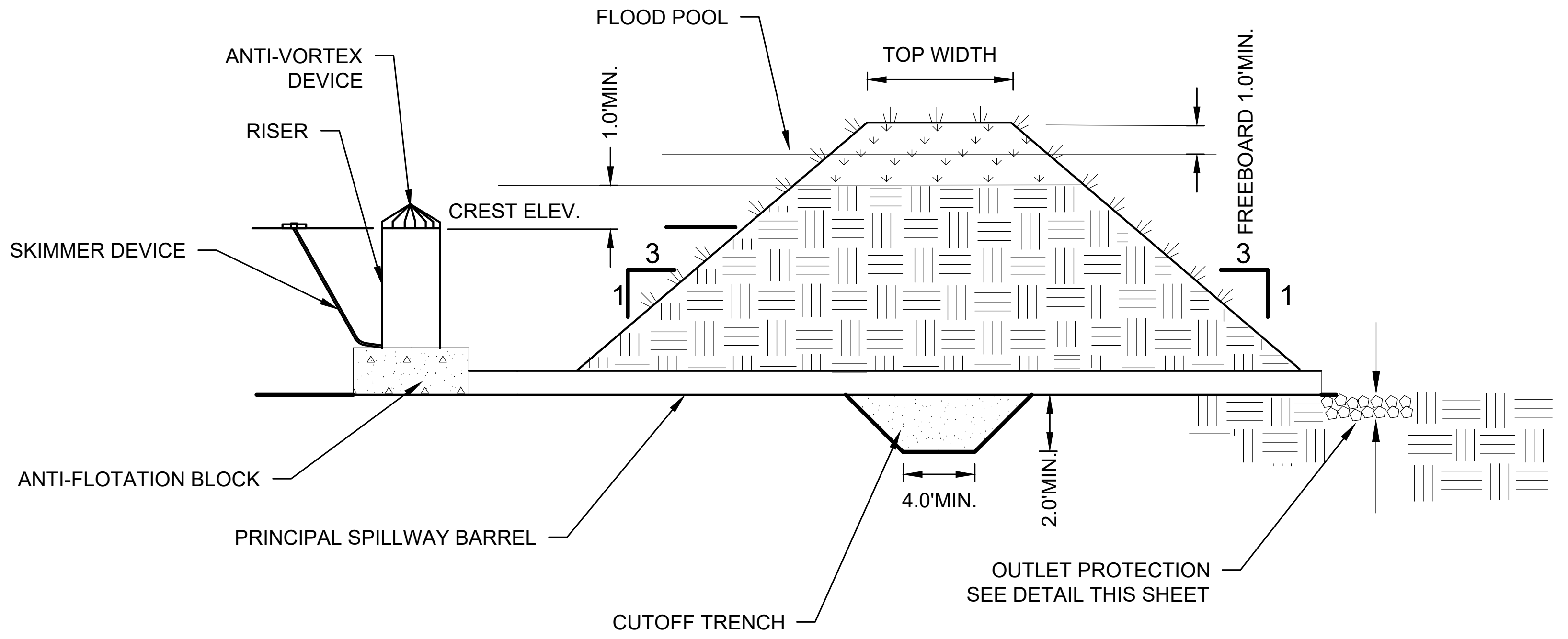


LIVE STAKE CROSS SECTION

NOTES:

1. CARE SHALL BE TAKEN NOT TO DAMAGE THE LIVE STAKES DURING INSTALLATION. THOSE DAMAGED SHALL BE LEFT IN PLACE AND SUPPLEMENTED WITH AN INTACT LIVE STAKE.
2. THE LENGTHS OF LIVE STAKES DEPENDS UPON THE APPLICATION. THE LENGTH SHALL EXTEND THROUGH THE SURFACE OF THE STONE FILL AT LEAST HALF THE LENGTH SHALL BE INSERTED IN TO THE SOIL, BELOW THE STONE FILL.
3. A PILOT HOLE IS REQUIRED TO ENSURE THAT THE LIVE STAKE IS NOT DAMAGED WHEN DRIVEN THROUGH THE STONE FILLING. ACCESS SHALL BE MADE THROUGH THE USE OF A DIBBLE BAR, OR SIMILAR TOOL TO WORK AN OPENING THROUGH THE ROCK LAYER.
4. MINIMUM 2" TO 4" AND TWO LIVE BUDS OF THE LIVE STAKE SHALL BE EXPOSED ABOVE THE STONE FILLING.
5. LIVE STAKES SHALL RANGE FROM 1" TO 4" IN DIAMETER AND BE FROM 5' TO 6' IN LENGTH.
6. LIVE STAKES SHALL BE CUT TO A POINT ON THE BASAL END FOR INSERTION IN THE GROUND.
7. USE A DEAD BLOW HAMMER TO DRIVE STAKES INTO THE GROUND. (HAMMER HEAD FILLED WITH SHOT OR SAND). A DIBBLE, IRON BAR, OR SIMILAR TOOL SHALL BE USED TO MAKE A PILOT HOLE TO PREVENT DAMAGING THE MATERIAL DURING INSTALLATION.
8. WHEN POSSIBLE, TAMP SOIL AROUND LIVE STAKES.

LIVE STAKE



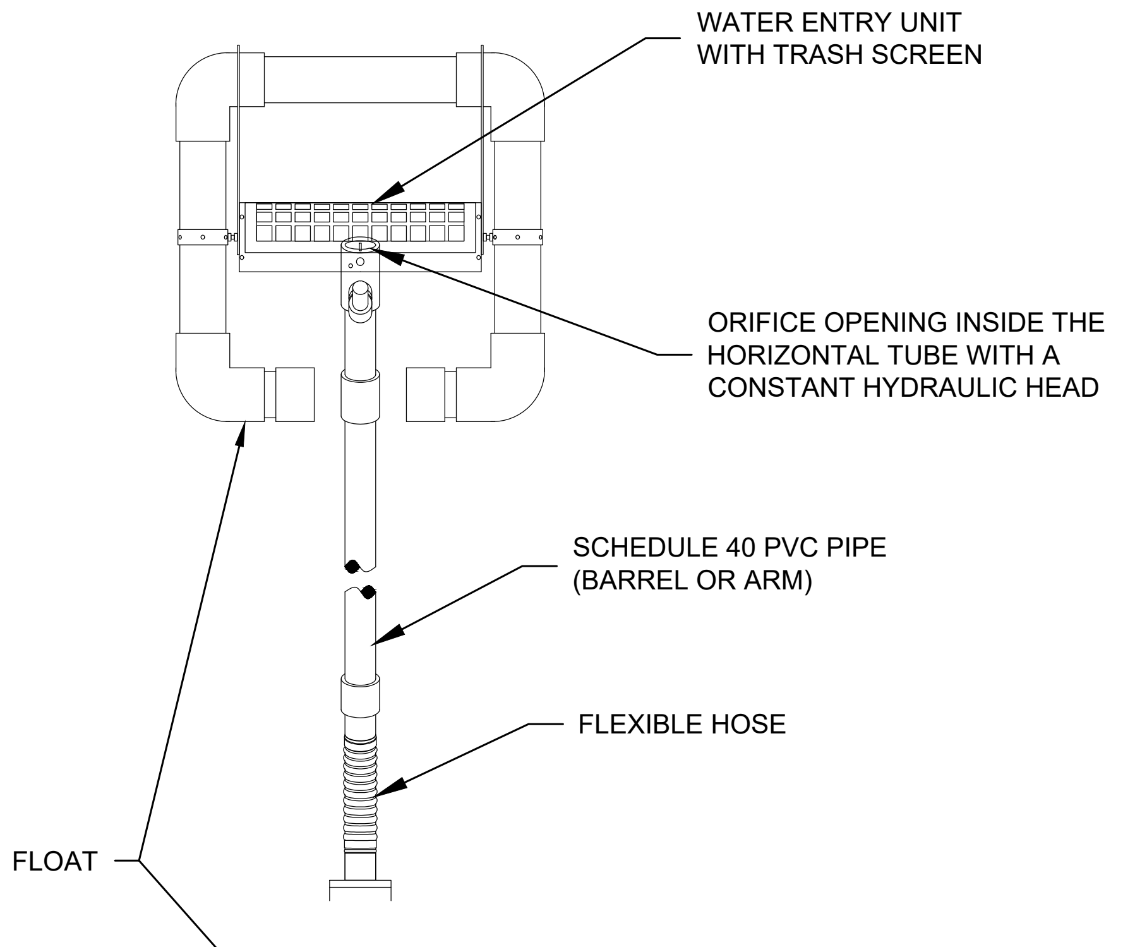
SEDIMENT BASIN CHART											
BASIN NUMBER	BOTTOM ELEV. (FT)	TOP OF DAM ELEV. (FT)	TOP OF RISER ELEV. (FT)	INV. OUT (FT)	SEDIMENT STORAGE ZONE VOLUME REQ'D (FTˆ)	SEDIMENT STORAGE ZONE VOLUME PROVIDED (FTˆ)	SEDIMENT STORAGE ZONE ELEV. (FT)	DEWATERING ZONE VOLUME REQ'D (FTˆ)	DEWATERING ZONE VOLUME PROVIDED (FTˆ)	DEWATERING ZONE ELEV. (FT)	CLEANOUT ELEVATION (FT.)
1	10	14	-	-	5300	5300	-	19080	19080	-	-
2	6	14	11	6.5	15200	16370	7.5	54720	55416	11	6.75
3	6	14	11	6.5	33000	36859	7.5	118800	119226	11	6.75

NOTES:

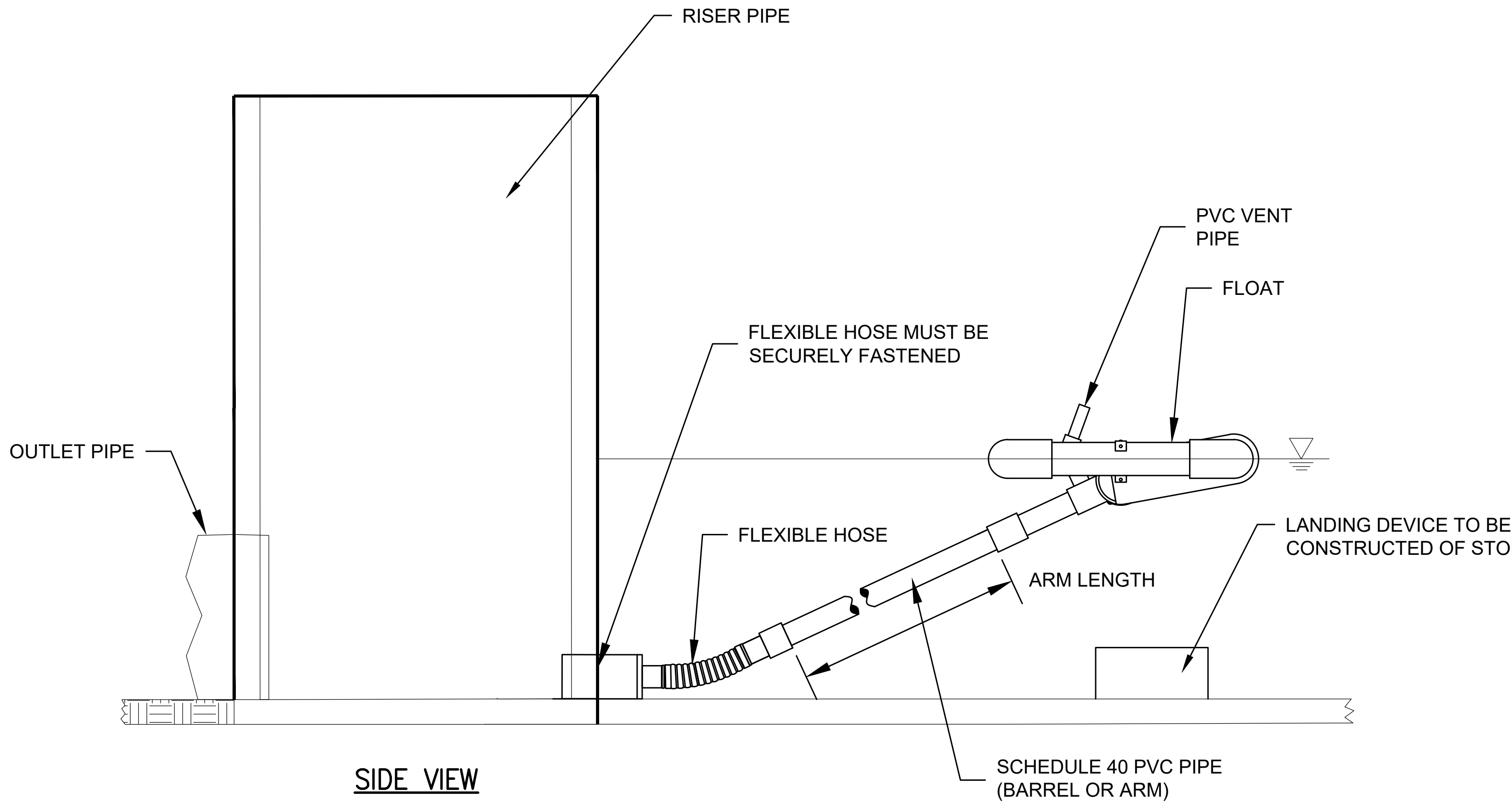
1. TEMPORARY SEDIMENT BASIN 1 CALCULATIONS HAVE BEEN SHOWN FOR CAPACITY VERIFICATION ONLY.
2. BASIN 1 WILL BE GRADED OUT PER WQV POND DETAIL ON SHEET GR-14. ALL OUTLET STRUCTURES ARE TO BE COVERED WITH FILTER FABRIC DURING CONSTRUCTION. EXCAVATION OF BASIN 1 TO FINAL GRADE ELEVATIONS SHALL OCCUR ONCE FINAL STABILIZATION HAS BEEN REACHED.
3. EMBANKMENT MUST BE COMPACTED TO DESIGN SPECIFICATIONS.
4. EROSION PROTECTION MUST BE INSTALLED ALONG THE EMBANKMENT AND AT THE DISCHARGE END OF THE PIPE.

TEMPORARY SEDIMENT BASIN

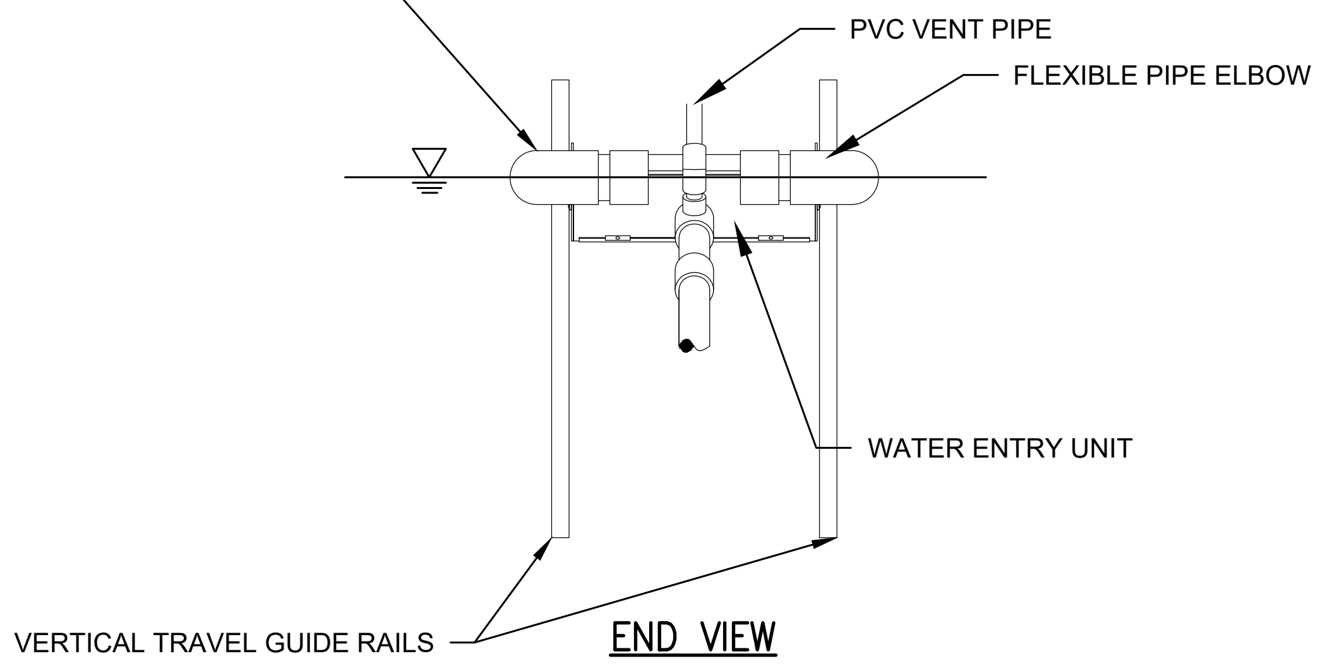
N.T.S.



TOP VIEW



SIDE VIEW



END VIEW

NOTES:

1. SKIMMER SHALL BE CONSTRUCTED WITH A 4 FOOT LONG FLEXIBLE PIPE ELBOW TO ALLOW FOR VERTICAL MOVEMENT OF THE SKIMMER FOR ITS DESIGNATED RANGE OF OPERATION.
2. THE SKIMMER WILL BE PROVIDED WITH VERTICAL TRAVEL GUIDES AND A LANDING DEVICE CONSTRUCTED OF STONE.
3. INSPECT SYSTEM REGULARLY TO ENSURE IT IS FUNCTIONING IN A CORRECT MANNER.

SKIMMER DEWATERING DEVICE							
BASIN NUMBER	WATER SURFACE ELEVATION (FT)	ARM LENGTH (FT)	ARM DIA. (in)	ORIFICE SIZE (in)	TOP OF LANDING DEVICE ELEVATION (FT)	FLEXIBLE HOSE LENGTH (in)	FLEXIBLE HOSE ATTACHMENT ELEVATION (FT)
2	11	16	5	5	7.5	12	6
3	11	16	7	7	7.5	12	6

SKIMMER DEWATERING DEVICE DETAILS

N.T.S.

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION
1	05/20/22	TOWN COMMENTS

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	AS SHOWN
DATE	05/10/2022
PROJECT	18641.00

ADAM J FROSINO
088870

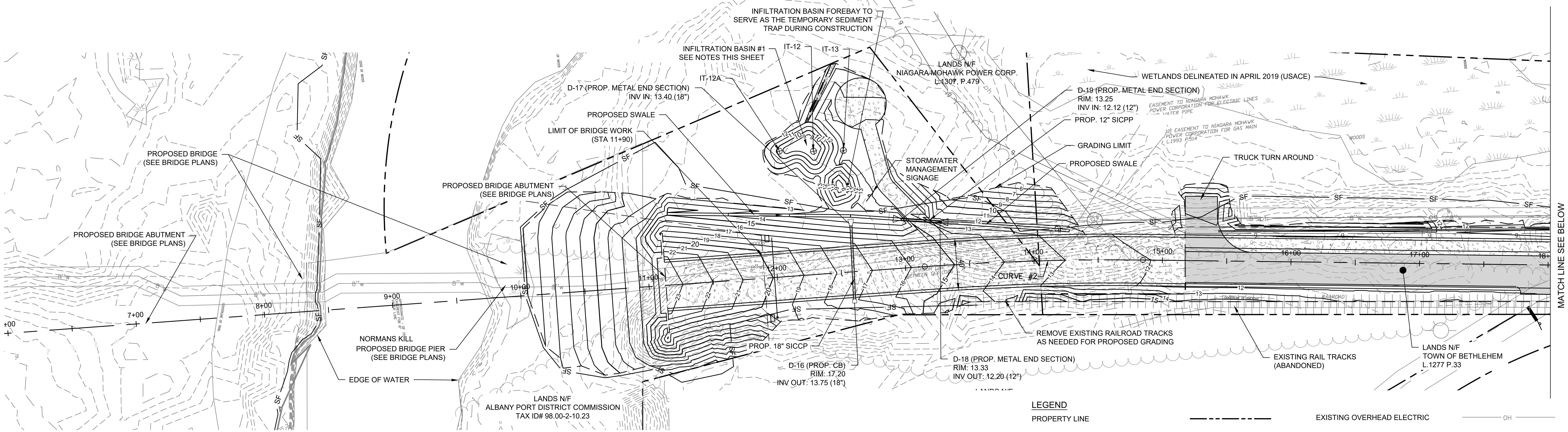
IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

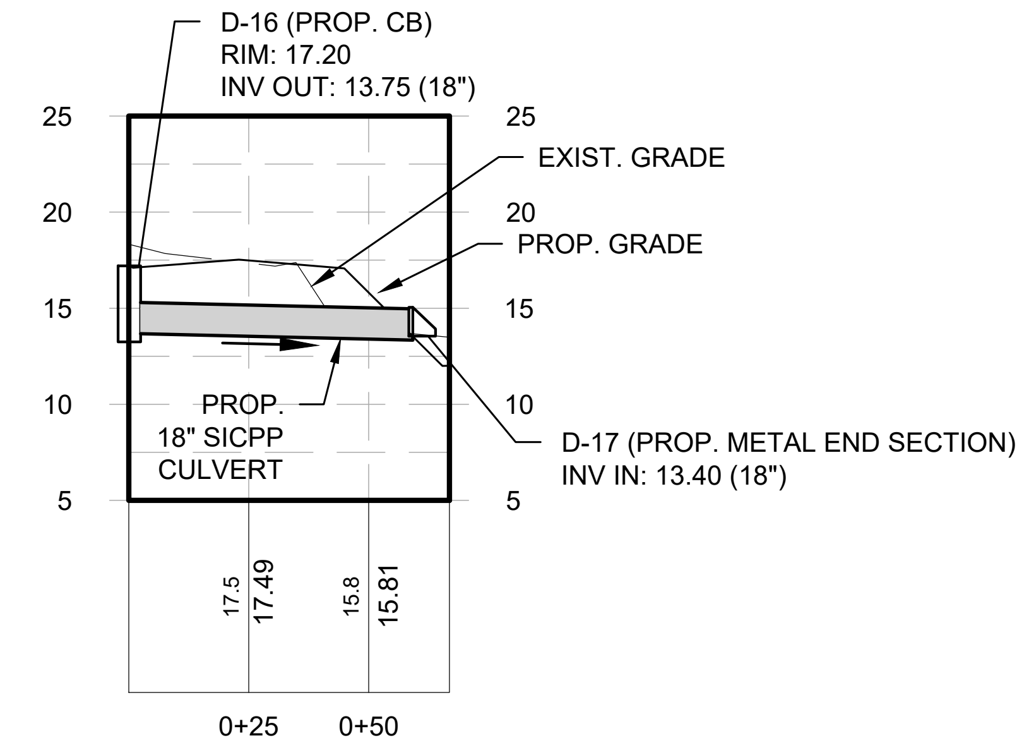
DRAWING TITLE
EROSION & SEDIMENT CONTROL DETAILS

DRAWING NUMBER
ESC-08

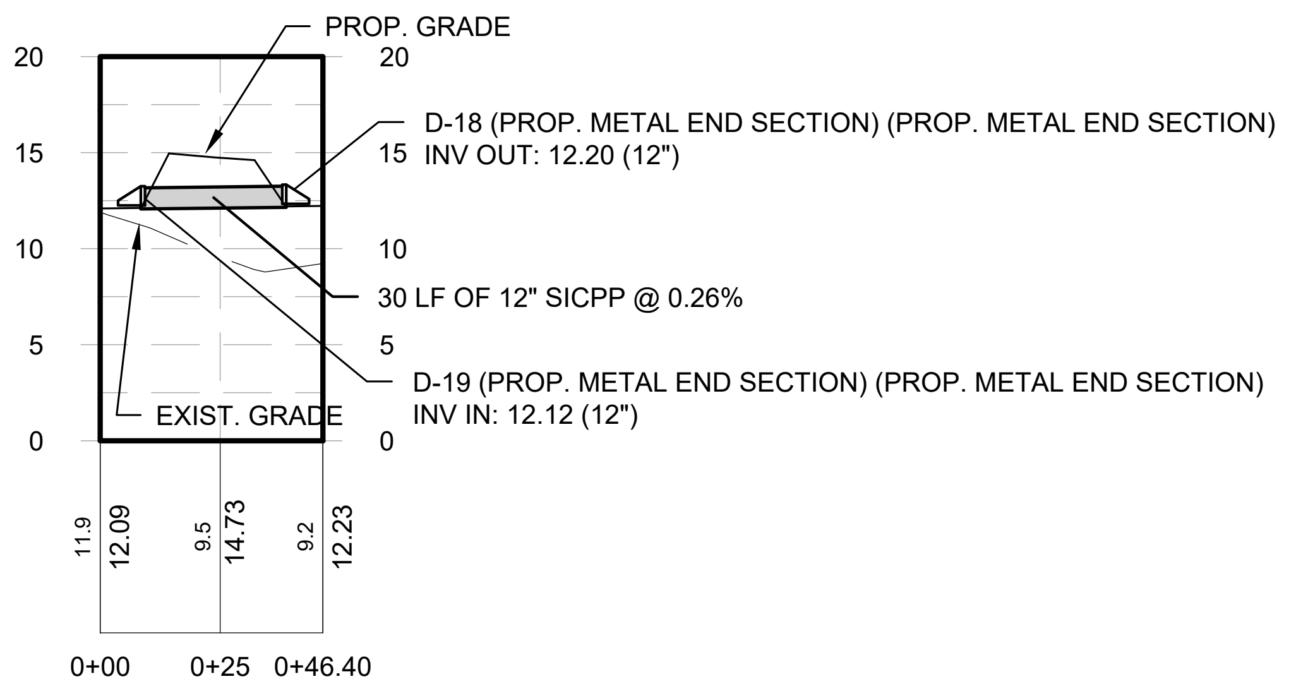
64 OF 68

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES\18641.00-ESC-DETS.DWG





D-16 TO D-17
Horizontal Scale: 1" = 40'
Vertical Scale: 1" = 10'



D-18 TO D-20
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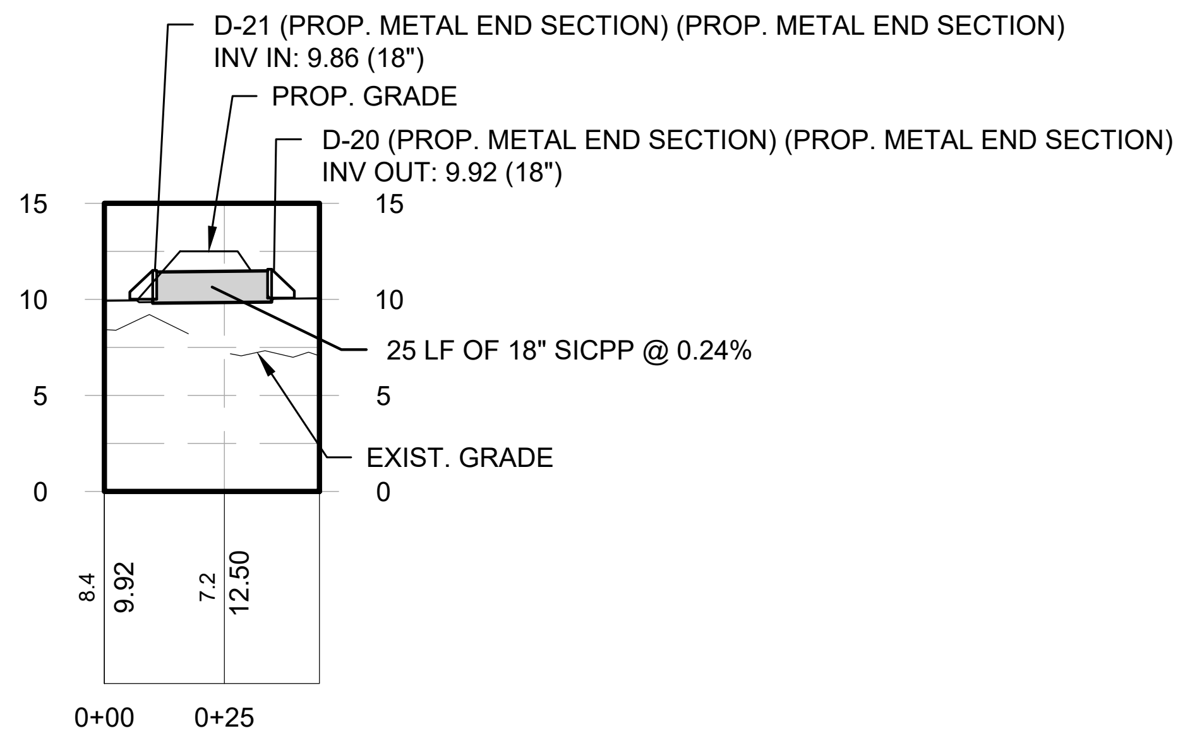
Vertical Scale: 1" = 10'



D-20 TO D-21
Horizontal Scale: 1" = 40'
Vertical Scale: 1" = 10'

LEGEND

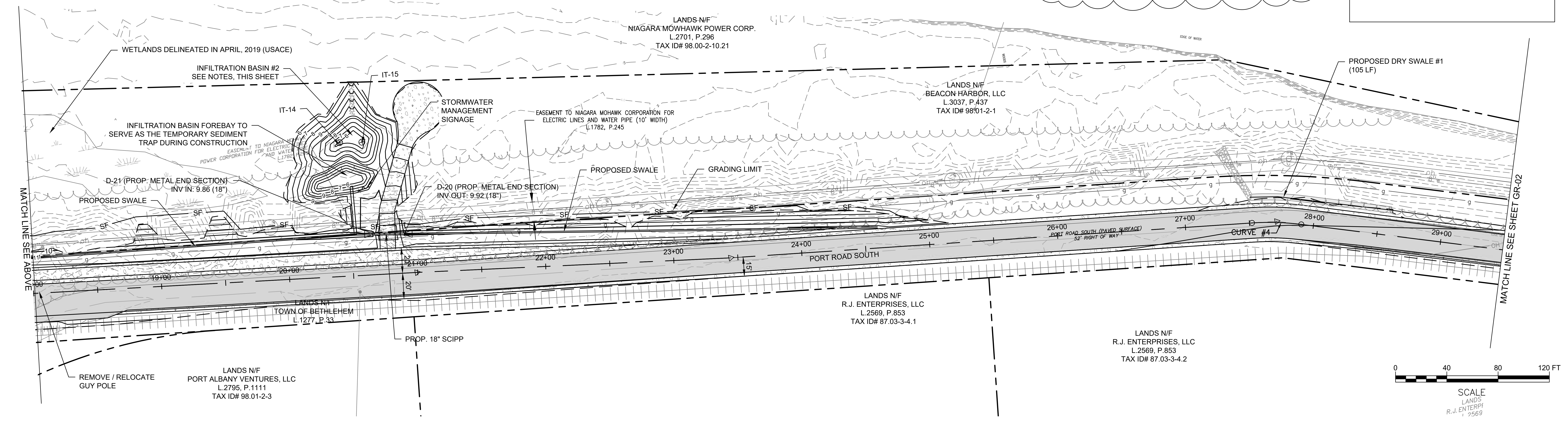
PROPERTY LINE	— - — - —	EXISTING OVERHEAD ELECTRIC	OH
PROPOSED BOX BEAM GUIDE RAIL		EXISTING GAS LINE	G
DITCH CENTERLINE		EXISTING WATERLINE	W
WETLAND AREA		PROPOSED PAVEMENT	■
SECURITY FENCE		PROPOSED CONCRETE	▨
SILT FENCE	SF	RIP-RAP WATER EMBANKMENT STABILIZATION	▨
CATCH BASIN	☐		

NOTES:

1. UPSTREAM CONSTRUCTION SHALL BE COMPLETED AND STABILIZED BEFORE CONNECTION TO A DOWNSTREAM INFILTRATION FACILITY. A DENSE AND VIGOROUS VEGETATIVE COVER SHALL BE ESTABLISHED OVER THE CONTRIBUTING PERVIOUS DRAINAGE AREAS BEFORE RUNOFF CAN BE ACCEPTED INTO THE FACILITY.
2. PROPOSED INFILTRATION BASINS SHALL BE OWNED , OPERATED AND MAINTAINED BY THE PORT OF ALBANY AND BE ACCESSIBLE BY THE TOWN FOR INSPECTION.
3. SEE TABLE ON SHEET GR-04 FOR INFILTRATION TEST INFORMATION.

2

PLANNING BOARD HTE# 21-00100006



McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION
1	05/20/22	TOWN COMMENTS
2	06/06/22	TOWN COMMENTS

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
NORMANSKILL ST. REHAB

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	1"=40'
DATE	05-10-2022
PROJECT	18641.00

ADAM J FROSINO
088870

DRAWING TITLE

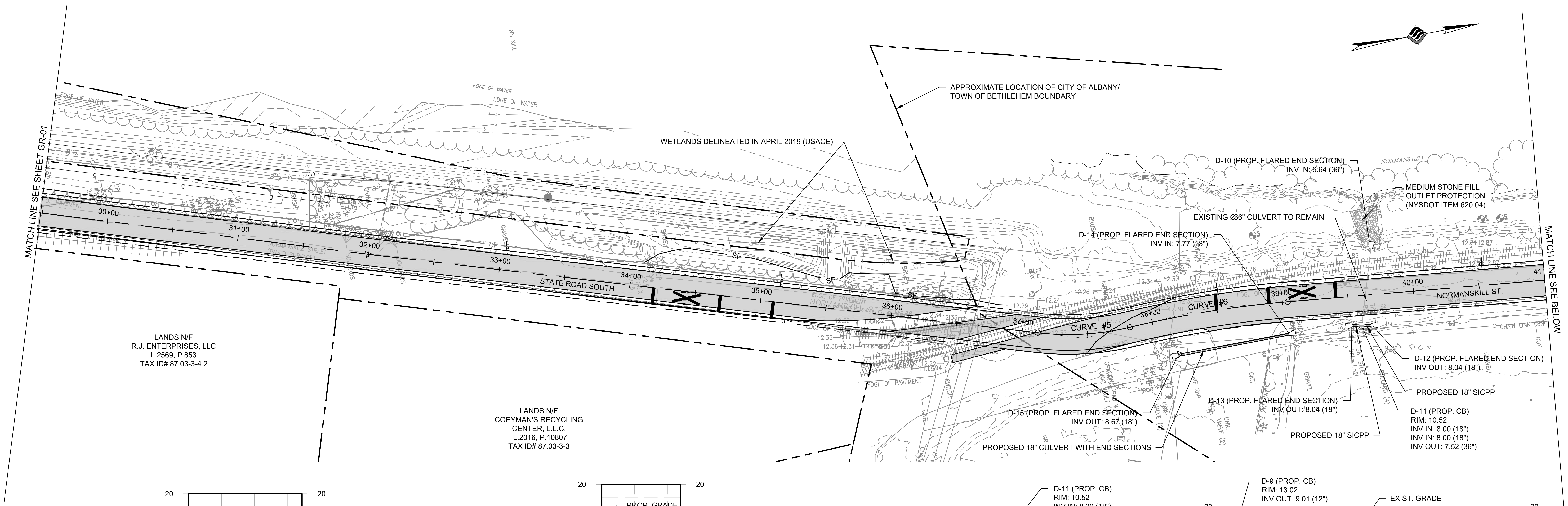
GRADING, DRAINAGE, EROSION AND SEDIMENT CONTROL

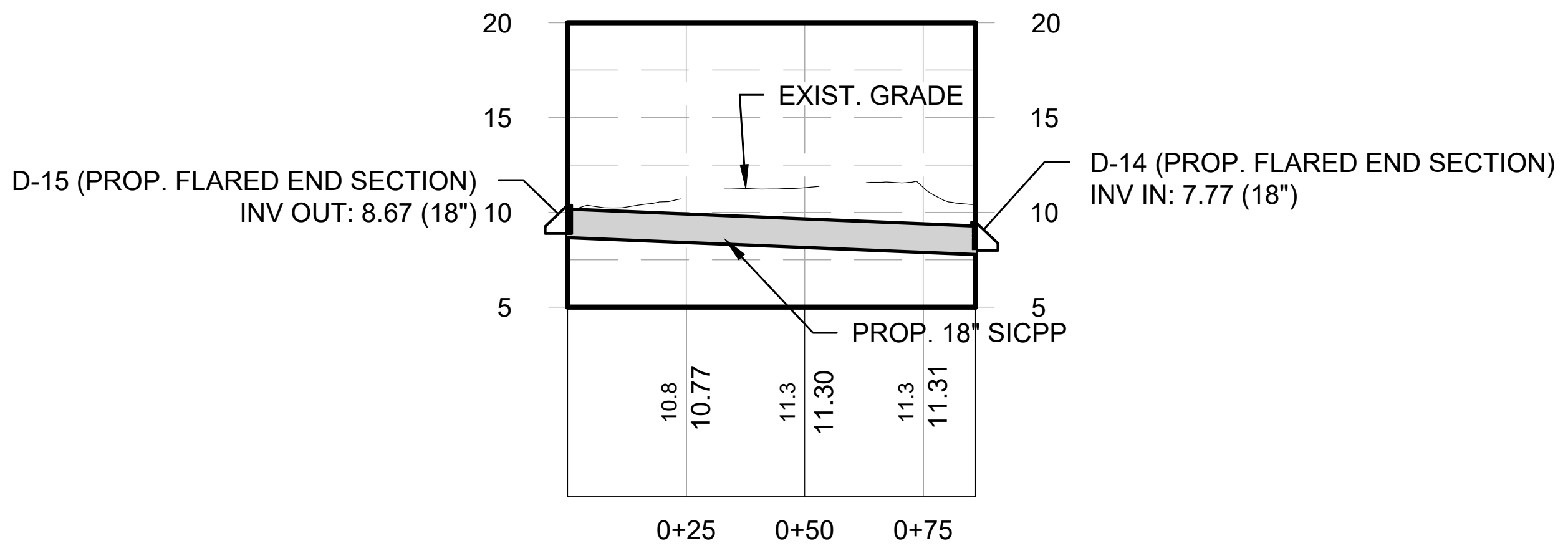
DRAWING NUMBER

GR-01

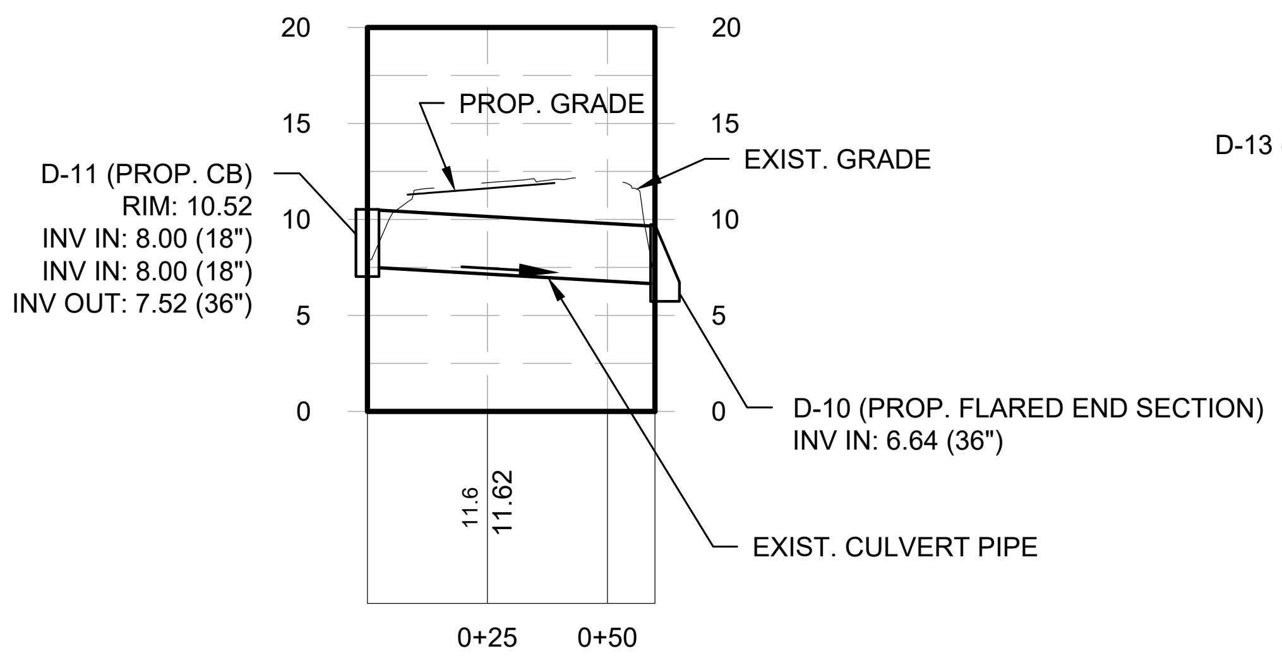
10 OF 16

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES - NORMANSKILL ST\18641.00-GRAD.DWG





D-15 TO D-14
Horizontal Scale: 1" = 40'
Vertical Scale: 1" = 10'



D-11 TO D-10
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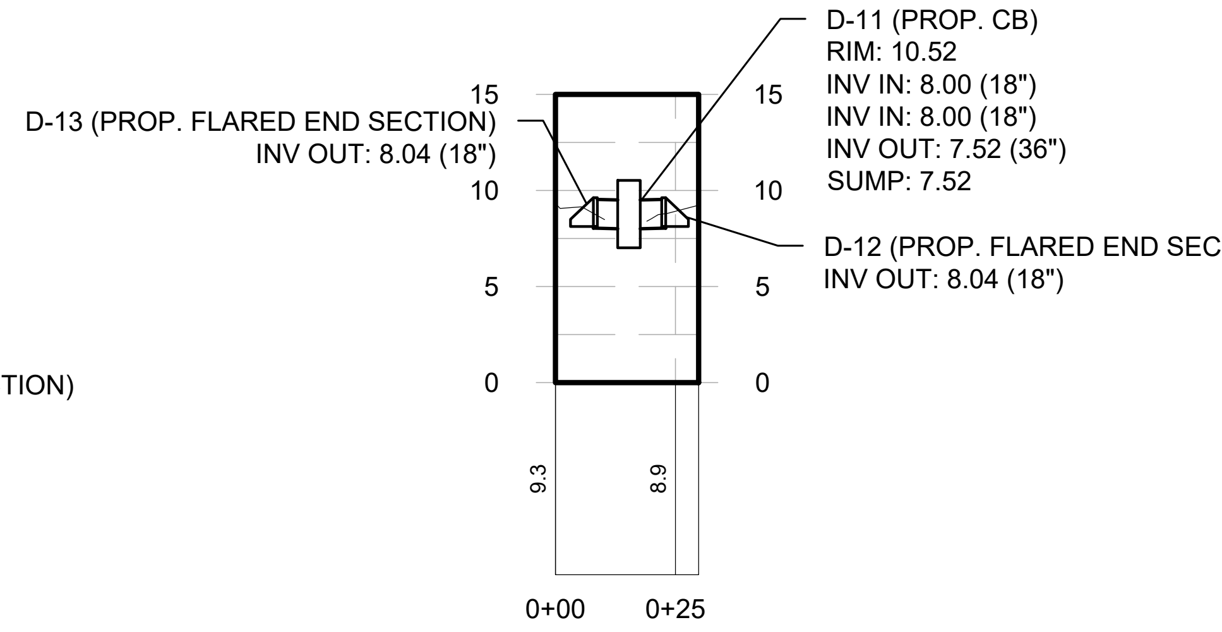
Vertical Scale: 1" = 10'



D-13 TO D-10
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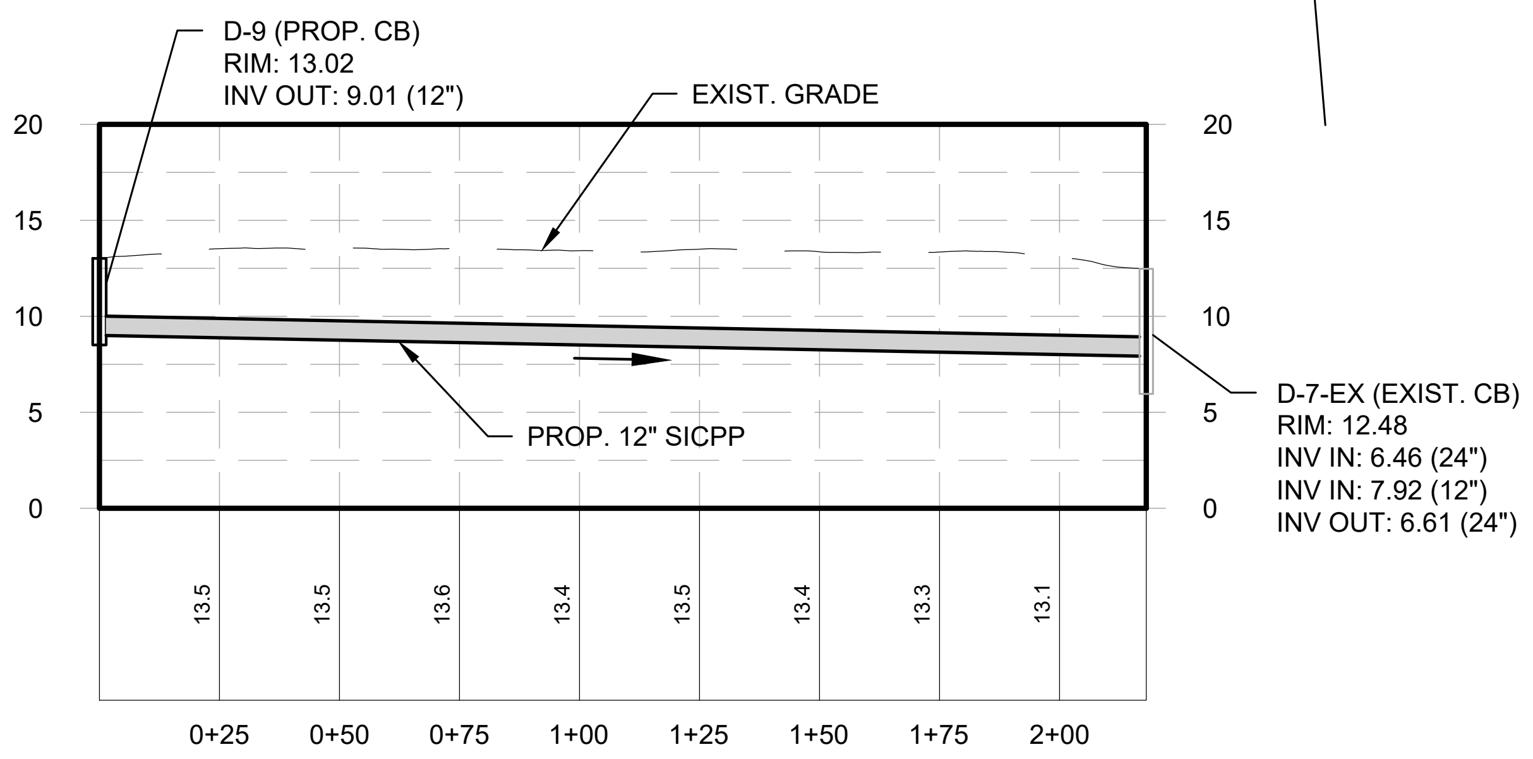
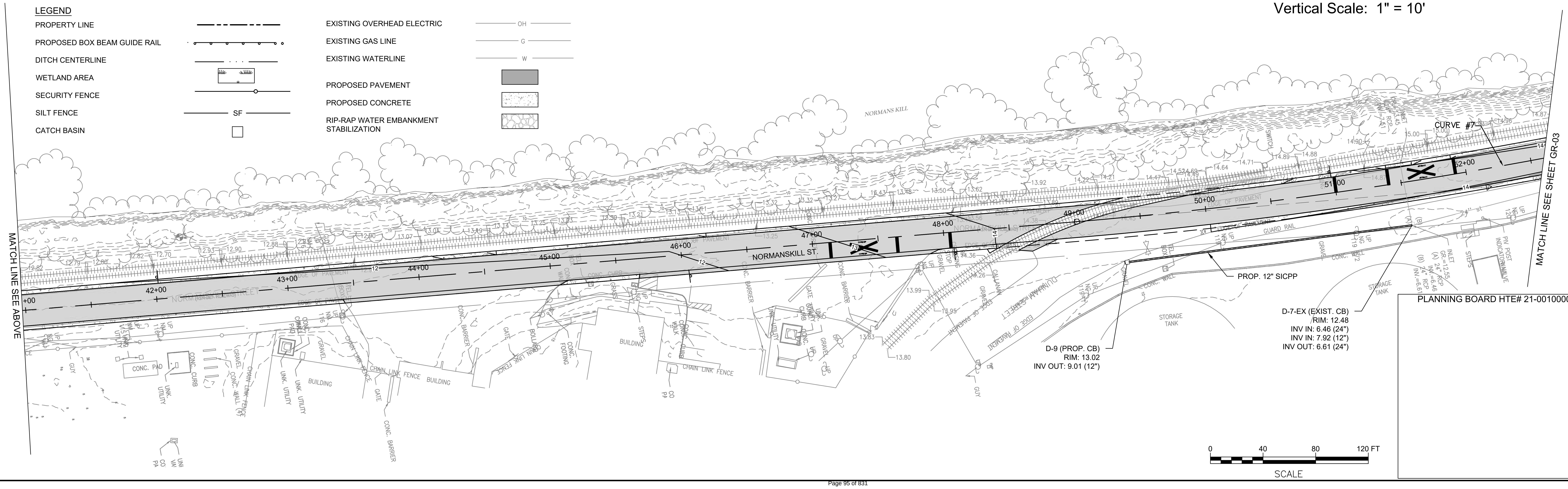
Vertical Scale: 1" = 10'



D-9 TO D-7-EX
Horizontal Scale: 1" = 40'
Vertical Scale: 1" = 10'

LEGEND

PROPERTY LINE	— · — · —
PROPOSED BOX BEAM GUIDE RAIL	∘—∘—∘
DITCH CENTERLINE	. . .
WETLAND AREA	
SECURITY FENCE	——∘——
SILT FENCE	—— SF ——
CATCH BASIN	☐
EXISTING OVERHEAD ELECTRIC	—— OH ——
EXISTING GAS LINE	—— G ——
EXISTING WATERLINE	—— W ——
PROPOSED PAVEMENT	■
PROPOSED CONCRETE	▨
RIP-RAP WATER EMBANKMENT STABILIZATION	▨



PLANNING BOARD HTE# 21-00100006

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES - NORMANSKILL ST\18641.00-GRAD.DWG

McFarland Johnson
60 RAILROAD PLACE
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P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION
1	05/20/22	TOWN COMMENTS

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
NORMANSKILL ST. REHAB

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	1"=40'
DATE	05-10-2022
PROJECT	18641.00

ADAM J FROSINO
088870

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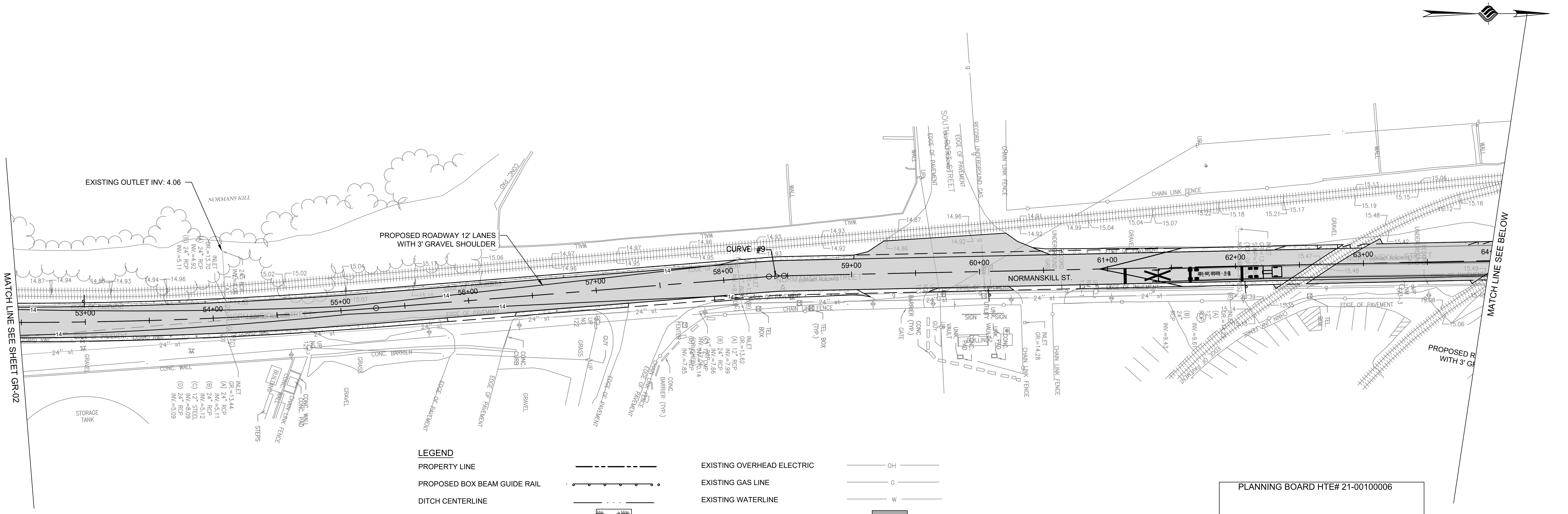
DRAWING TITLE
GRADING, DRAINAGE, EROSION AND SEDIMENT CONTROL

DRAWING NUMBER
GR-02

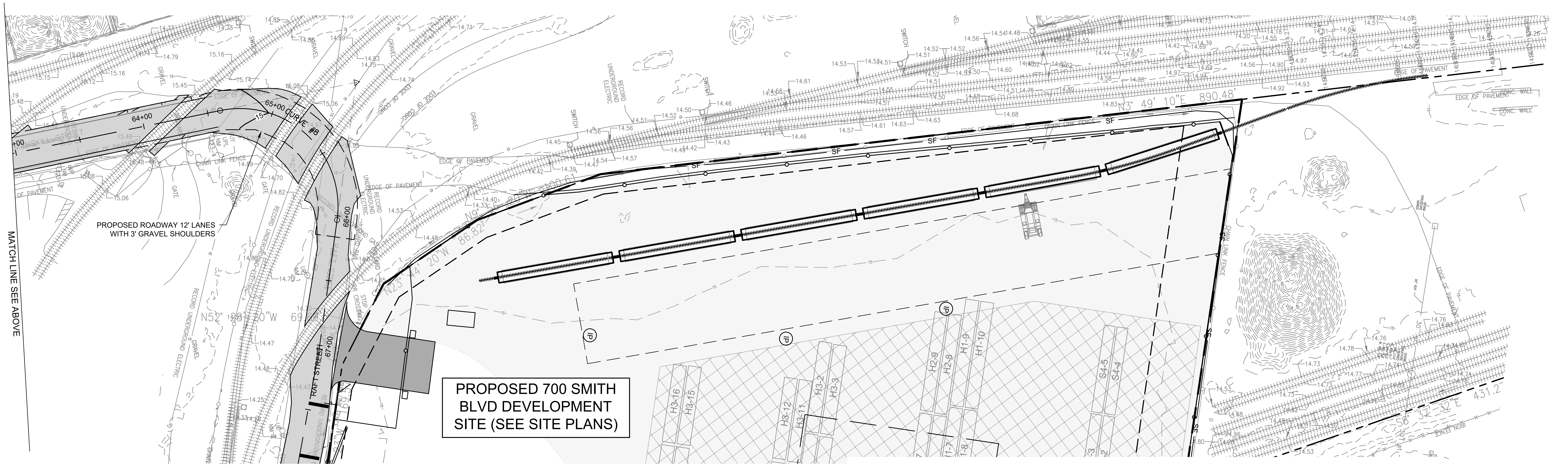
11 OF 16



LEGEND

PROPERTY LINE	— - - — - - —	EXISTING OVERHEAD ELECTRIC	—— OH ——
PROPOSED BOX BEAM GUIDE RAIL	·o—o—o—o—o—o o	EXISTING GAS LINE	—— G ——
DITCH CENTERLINE	—— · · · ——	EXISTING WATERLINE	—— W ——
WETLAND AREA	▭	PROPOSED PAVEMENT	▩
SECURITY FENCE	——o——	PROPOSED CONCRETE	▨
SILT FENCE	—— SF ——	RIP-RAP WATER EMBANKMENT STABILIZATION	▦
CATCH BASIN	☐		

PLANNING BOARD HTE# 21-00100006



0 40 80 120 FT
SCALE

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION
1	05/20/22	TOWN COMMENTS

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
NORMANSKILL ST. REHAB

DRAWN: JES
DESIGNED: NSO
CHECKED: AJF
SCALE: 1"=40'
DATE: 05-10-2022
PROJECT: 18641.00

ADAM J FROSINO
088870

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DRAWING TITLE
GRADING, DRAINAGE, EROSION AND SEDIMENT CONTROL

DRAWING NUMBER
GR-03

12 OF 16

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APPENDIX C

DRAINAGE DESIGN REPORT

- August 4, 20205 Amendment
- Original Drainage Design Report

McFARLAND JOHNSON
90 East Avenue • Saratoga Springs, NY 12866
Phone: (518) 580-9380
www.mjinc.com

July 25, 2025

Mr. Eric Johnson, PE
Town Engineer
Town of Bethlehem
Department of Economic Development & Planning
445 Delaware Avenue, 2nd Floor
Delmar, NY 12054

Re: SWPPP Amendment – National Grid Proposal Gravel Access Drive
Albany Port District Commission
Marmen-Welcon Tower Manufacturing Plant
Tax ID 98.01-2-1.0 / 98.00-2-10.23

Dear Eric:

I am writing on behalf of the Albany Port District Commission and National Grid to request an amendment to the current NYSDEC SPDES Permit # NYR11K128 to accommodate a National Grid access drive to the adjacent transmission lines.

Proposed Additional Work:

This proposed work was discussed during a coordination meeting on Thursday, June 26, 2025. To summarize that meeting, the gravel access road is needed for installation of National Grid transmission poles/switches on National Grid's property installed by National Grid crews/subcontractors. There is an existing gravel drive directly off Route 144; however, it does not meet the standards necessary for installation and maintenance of the new equipment. The existing access drive also raises safety concerns due to the limited sight distance on Route 144. Given that this National Grid infrastructure (i.e., transmission poles and switches) will be installed by National Grid and serve the Port's site/substation and that the Port has an existing SWPPP, it was suggested that the Port pursue a modification/amendment to this open permit with the Town as the MS4 to cover the National Grid construction activity.

This proposed access work is shown on the attach "National Grid Access Driveway" drawings GR-01 and SP-01 prepared by McFarland Johnson dated July 2025. A new 16' wide gravel driveway will provide access to a 100' x 75' gravel area surrounding the utility poles in the upper area. This work will result in around 0.54 acres of disturbance. Access to the lower new poles will be provided by a 15' wide access drive 330' long that extend from the previously approved gravel drive around the electrical substation. This work will result in around 0.19 acres of disturbance with 0.11 acres of new permanent impervious area.

Stormwater Impacts:

The proposed work will alter the stormwater flow path and add additional gravel area where there was existing woods/brush. As shown on the attached post development site drainage map, the upper access drive, the access road area is currently included within drainage area DR-10 and after construction the stormwater flow in the upper isolated area will be directed to the roadside swale and into Pond #2 as an expansion to drainage area DR-09. We have updated the stormwater HydroCAD model by removing 0.54 acres of woods/shrubs from DR-10 and added 0.27 acres of gravel and 0.27 acres of grass lawn area to the DR-09 catchment area. This is a conservative approach as there is an existing driveway and some gravel areas there today of which we are not taking any impervious area credit.

The lower access drive with in the lower area and will have the access drive runoff staying in drainage area DR-10. We have updated the stormwater HydroCAD model by removing 0.11 acres of woods/shrubs from DR-10 and added 0.11 acres of gravel to this catchment area.

The tables below summarize the updated results when applying this changes noted above to the model:

Storm Event	Pre-Development	Post-Development	NG Access Addition
Analysis Point #1A (Wetland #1)			
1-yr Discharge	27.32 cfs	4.77 cfs	4.68 cfs
10-yr Discharge	73.24 cfs	12.46 cfs	12.50 cfs
100-yr Discharge	163.60 cfs	30.97 cfs	30.43 cfs
Analysis Point #1			
1-yr Discharge	3.25 cfs	58.54 cfs	58.54 cfs
10-yr Discharge	14.96 cfs	127.40 cfs	124.96 cfs
100-yr Discharge	43.20 cfs	216.11 cfs	205.59 cfs

SW Pond #2 Elevations		
	Post-Development	NG Access Addition
1-yr Storm	15.45'	15.53'
10-yr Storm	16.47'	16.55'
100-yr Storm	17.07'	17.18'

Based on the model analysis the proposed Stormwater Retention Basin #2 will continue to operate as originally designed with the increase in catchment area only raising the basin’s water level by approximately 1-1.5” during the 1, 10 and 100-year storm events. The modification does not have an impact on the overall site outlets to the Hudson River and adjacent wetlands as there is a negligible change in the peak flow rates for each storm event at Analysis Points 1 and 1A.

From a water quality standpoint the proposed onsite stormwater treatment practices provide more WQv than what was previously required and the increase in impervious form these two minor areas will increase the required WQv; however it will not exceed the maximum WQv that can be provided by all the site’s stormwater treatment practices. See the updated WQv table below, note that the increase in WQv from the previous report is based on assuming that Pond #1 and Pond #2’s full capacity of WQv is utilized. It was previously cut off to only the required WQv from the contributing watershed area.

Total Area of Soil Disturbance	72.7 acres	73.4 acres
WQv Target	273,874 cf	275,516 cf
WQv Provided	**274,728 cf**	**280,761 cf (max)**
RRv Target	57,313 cf	57,643 cf
RRv Provided	**63,333 cf**	**63,333 cf**

Conclusion:
With the implementation of the proposed temporary erosion and sediment control measures shown in the attached National Grid Access Driveway drawing, the proposed additional improvements within National Grid property to provide access to their infrastructure will not have an impact on the temporary and permanent drainage system at the Port of Albany Expansion site. The development site will continue to detain and treat the stormwater in accordance with the original Stormwater Pollution Prevention Plan.

This letter and the attached documents will be added to the SWPPP binder as record of addition of the National Grid gravel driveway adjacent to the proposed Port of Albany expansion site. Please let us know if any additional documentation is required by the Town as the NYSDEC MS4.

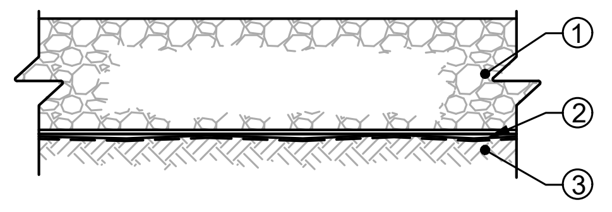
Sincerely yours,
McFARLAND-JOHNSON, INC.

Adam J. Frosino, PE, PTOE
Project Manager

CC: Joesph Cleveland, Matt Hoffman – Town of Bethlehem
Meredithe Mathias – National Grid
Roddy Yagan – APDC
Ed Larkin, Chris LaPointe – LaBella Associates

LEGEND

PROPERTY LINE	── - - ── - - ──
BUILDING SETBACK	— — — — —
EXISTING RAIL LINE	┼┼┼┼┼┼┼┼┼┼
EXISTING TREE LINE	︵︵︵︵︵︵
EXISTING CONTOUR	— — 12 - — —
PROPOSED CONTOUR	──── 12 ────
DITCH CENTERLINE	—— · · · ——— ——



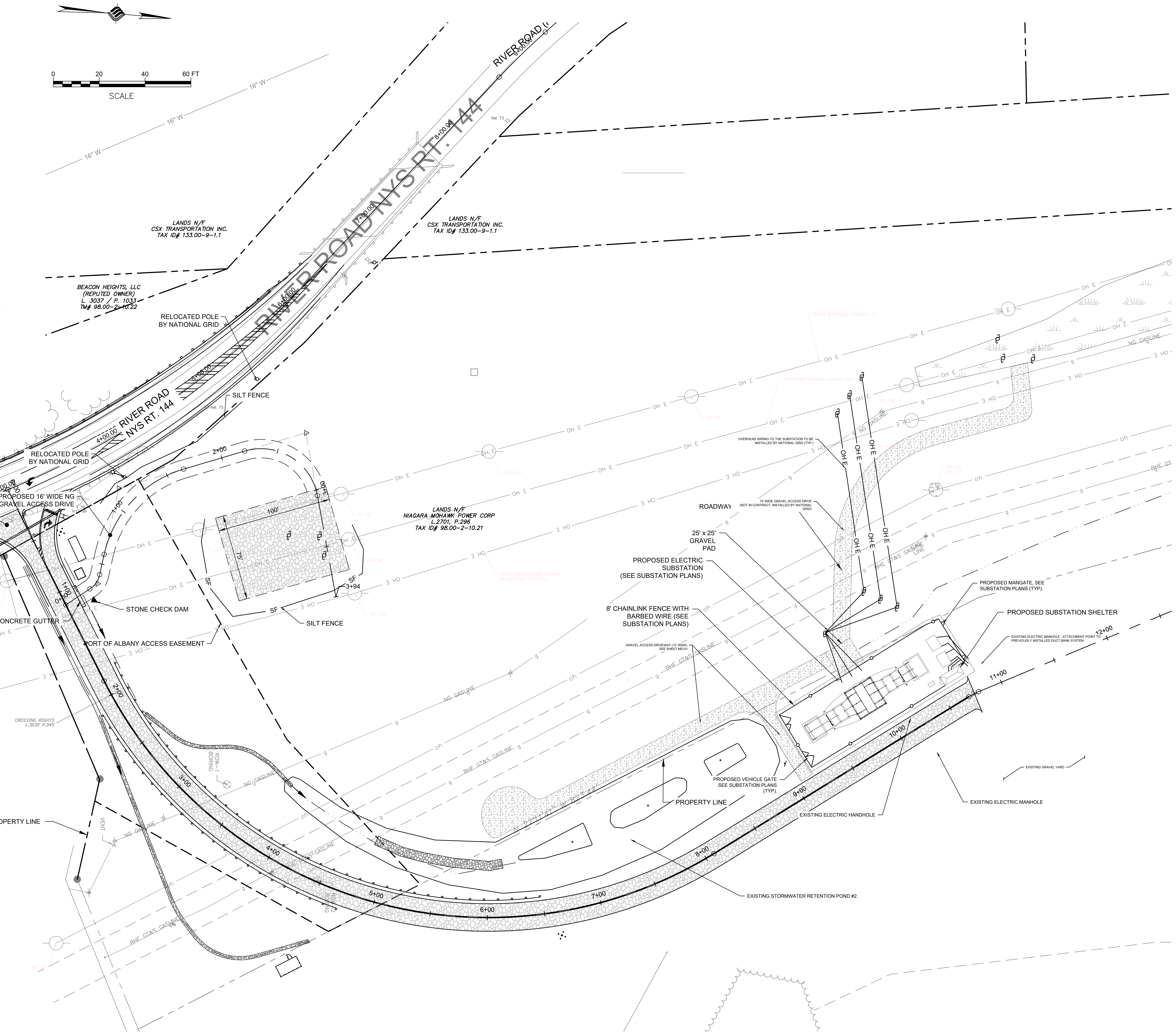
① 8" SUBBASE COURSE, NYSDOT ITEM 304.15 OPTIONAL TYPE
② MIRAFI 600X GEOTEXTILE FABRIC OR APPROVED EQUAL
③ COMPACTED SUBGRADE

NOTES:

1. COMPACT SUBGRADE TO A MODIFIED PROCTOR DENSITY OF 95%
2. SUBBASE COURSE SHALL HAVE NO MORE THAN (7%) SEVEN PERCENT BY WEIGHT FINER THAN NO. 200 SIEVE.
3. REFER TO GEOTECHNICAL ENGINEERING REPORT FOR SUBGRADE PREPARATION/COMPACTION REQUIREMENTS.

GRAVEL ACCESS ROAD SECTION

N.T.S.



McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
NG ACCESS

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
NATIONAL GRID ACCESS DRIVE

DRAWN	JES
DESIGNED	JES
CHECKED	AJF
SCALE	1"=20'
DATE	JULY 2025
PROJECT	18641.00

ADAM J FROSINO
088870

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DRAWING TITLE
NATIONAL GRID
ACCESS DRIVEWAY

DRAWING NUMBER
SP-01

1 OF 1

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES\NG ACCESS FROM RT 144\18641.00-NG ACCESS_V2.DWG

LEGEND

PROPERTY LINE

BUILDING SETBACK

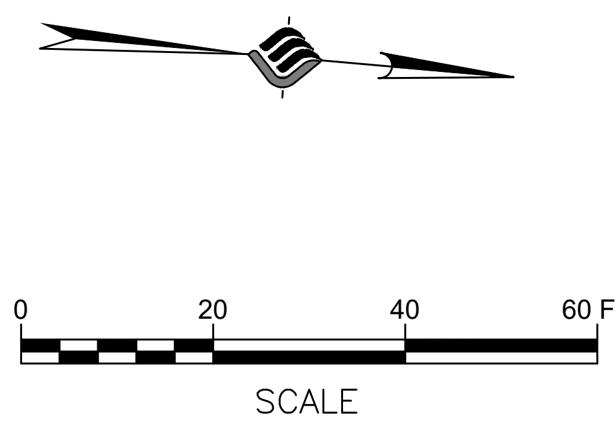
EXISTING RAIL LINE

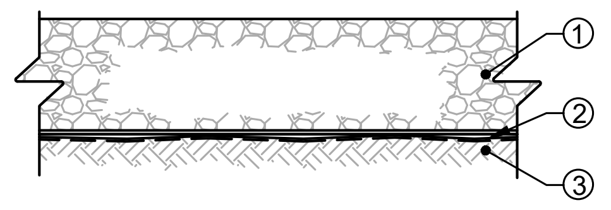
EXISTING TREE LINE

EXISTING CONTOUR

PROPOSED CONTOUR

DITCH CENTERLINE





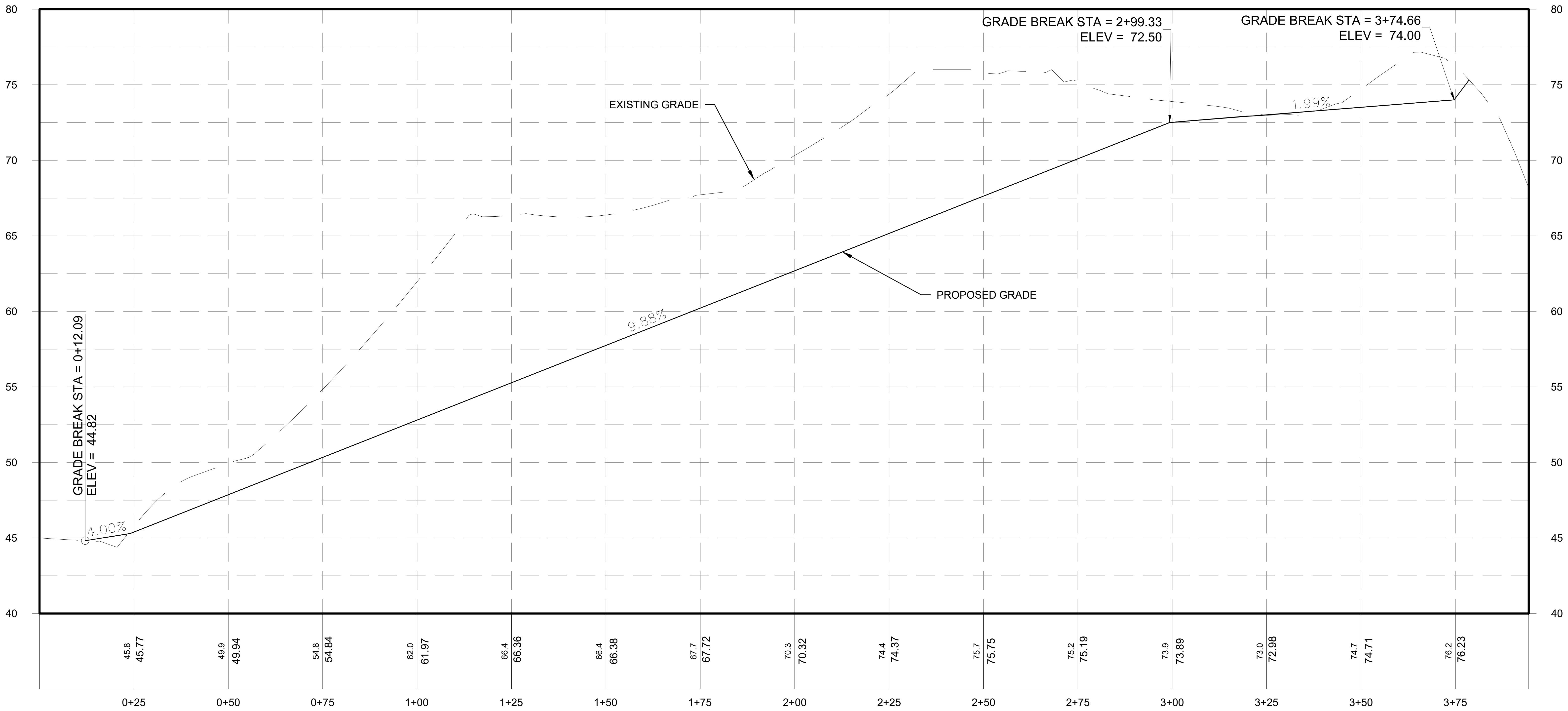
① 8" SUBBASE COURSE, NYSDOT ITEM 304.15 OPTIONAL TYPE
② MIRAFI 600X GEOTEXTILE FABRIC OR APPROVED EQUAL
③ COMPACTED SUBGRADE

NOTES:

1. COMPACT SUBGRADE TO A MODIFIED PROCTOR DENSITY OF 95%
2. SUBBASE COURSE SHALL HAVE NO MORE THAN (7%) SEVEN PERCENT BY WEIGHT FINER THAN NO. 200 SIEVE.
3. REFER TO GEOTECHNICAL ENGINEERING REPORT FOR SUBGRADE PREPARATION/COMPACTION REQUIREMENTS.

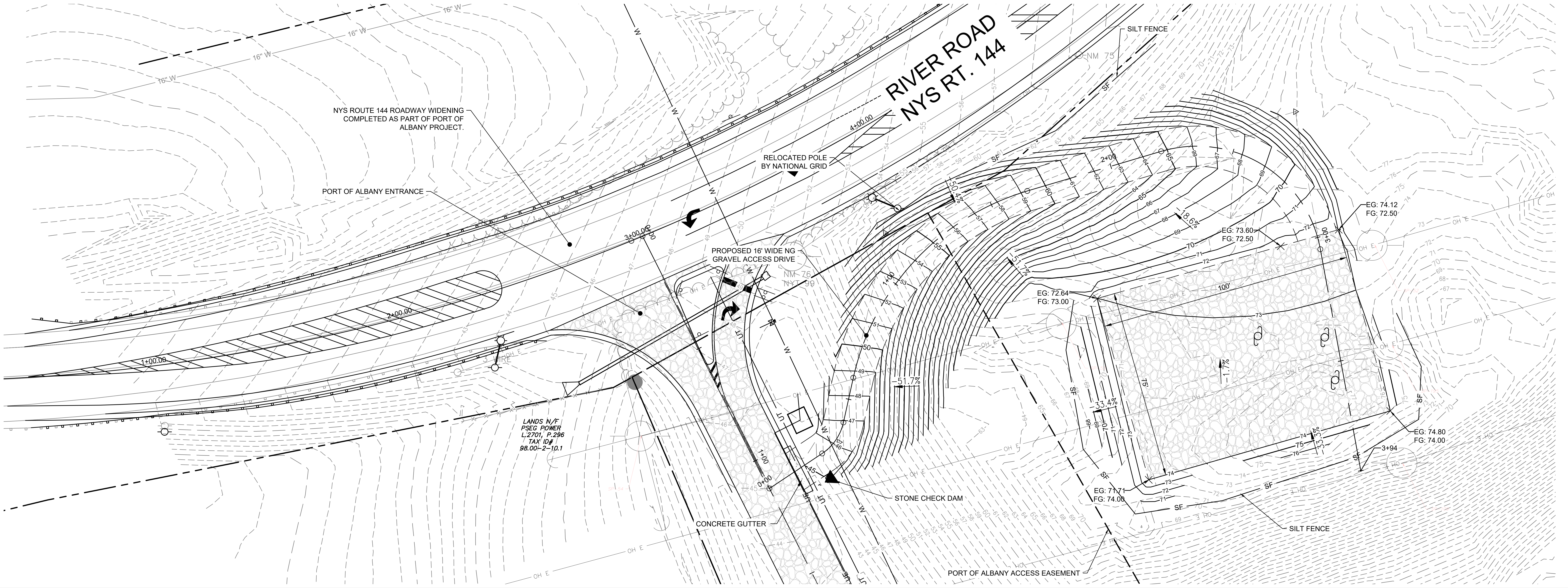
GRAVEL ACCESS ROAD SECTION

N.T.S.



ACCESS ROAD PROFILE

Horizontal Scale: 1" = 20'
Vertical Scale: 1" = 5'



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McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
NG ACCESS

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
NATIONAL GRID ACCESS DRIVE

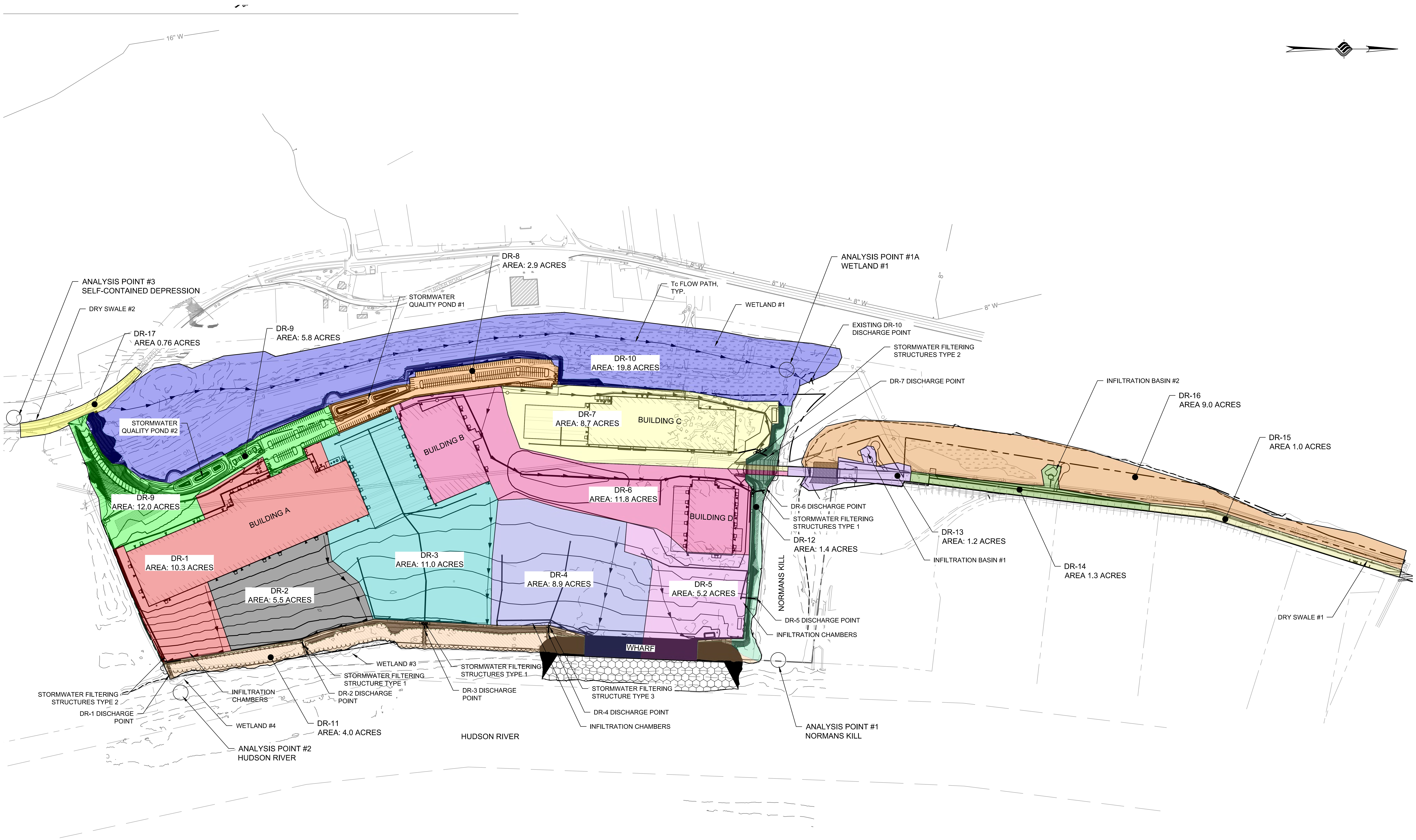
DRAWN	JES
DESIGNED	JES
CHECKED	AJF
SCALE	1"=20'
DATE	JULY 2025
PROJECT	18641.00

ADAM J FROSINA
088870

DRAWING TITLE
NATIONAL GRID
ACCESS DRIVEWAY

DRAWING NUMBER
GR-01

1 OF 1



LEGEND

- ■ DA-1
- ■ DA-2
- ■ DA-3
- ■ DA-4
- ■ DA-5
- ■ DA-6
- ■ DA-7
- ■ DA-8
- ■ DA-9
- ■ DA-10
- ■ DA-11
- ■ DA-12
- ■ DA-13
- ■ DA-14
- ■ DA-15
- ■ DA-16
- ■ DA-17

NOTE:
1. DRAINAGE AREAS DR-5, DR-6, DR-7, DR-8, DR-9, DR-10, DR-12, DR-13, DR-14, DR-15 AND DR-16 DRAIN TO ANALYSIS POINT #1.
2. DRAINAGE AREAS DR-1, DR-2, DR-3, DR-4 AND DR-11 DRAIN TO ANALYSIS POINT #2.
3. DRAINAGE AREA DR-17 DRAINS TO ANALYSIS POINT #3.

0 200 400 600 FT
SCALE

N:\18641.00 ALBANY PORT EXPANSION\REPORTS\DRAINAGE REPORT\18641.00-PROPOSED CONDITION.DWG

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE

NFC

NO.	DATE	DESCRIPTION
1	05/20/22	TOWN COMMENTS

CLIENT:
ALBANY PORT DISTRICT COMMISSION
TOWN OF BETHLEHEM, NEW YORK

PROJECT:
PORT OF ALBANY SITE INFRASTRUCTURE IMPROVEMENTS

DRAWN NSO
DESIGNED NSO
CHECKED AJF
SCALE 1" = 200'
DATE MAY 2022
PROJECT 18641.00

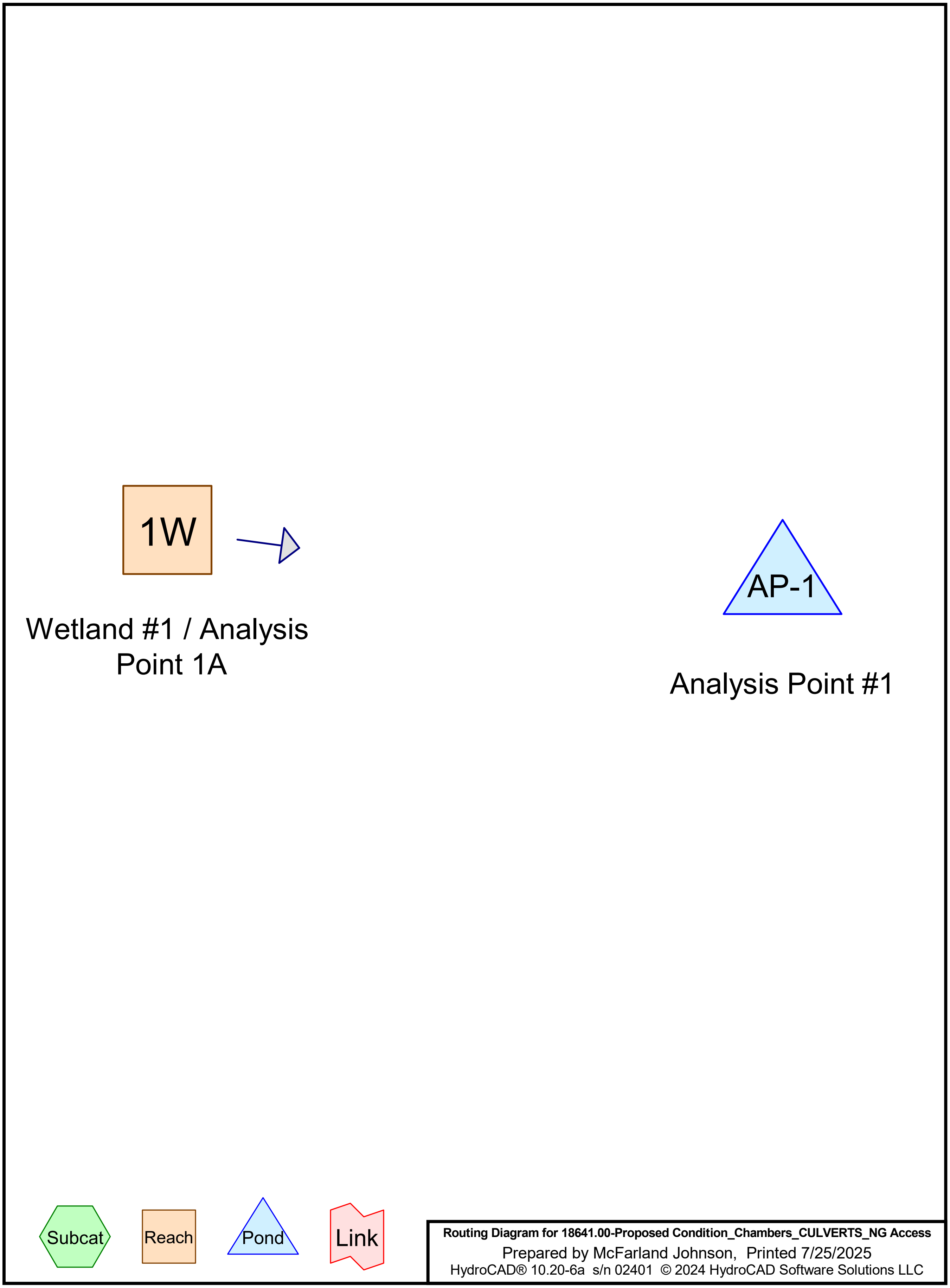
DRAWING TITLE

POST-DEVELOPMENT
SITE DRAINAGE
AREAS

DRAWING NUMBER

DR-PR

01 OF 01



Routing Diagram for 18641.00-Proposed Condition_Chambers_CULVERTS_NG Access
Prepared by McFarland Johnson, Printed 7/25/2025
HydroCAD® 10.20-6a s/n 02401 © 2024 HydroCAD Software Solutions LLC

Summary for Reach 1W: Wetland #1 / Analysis Point 1A

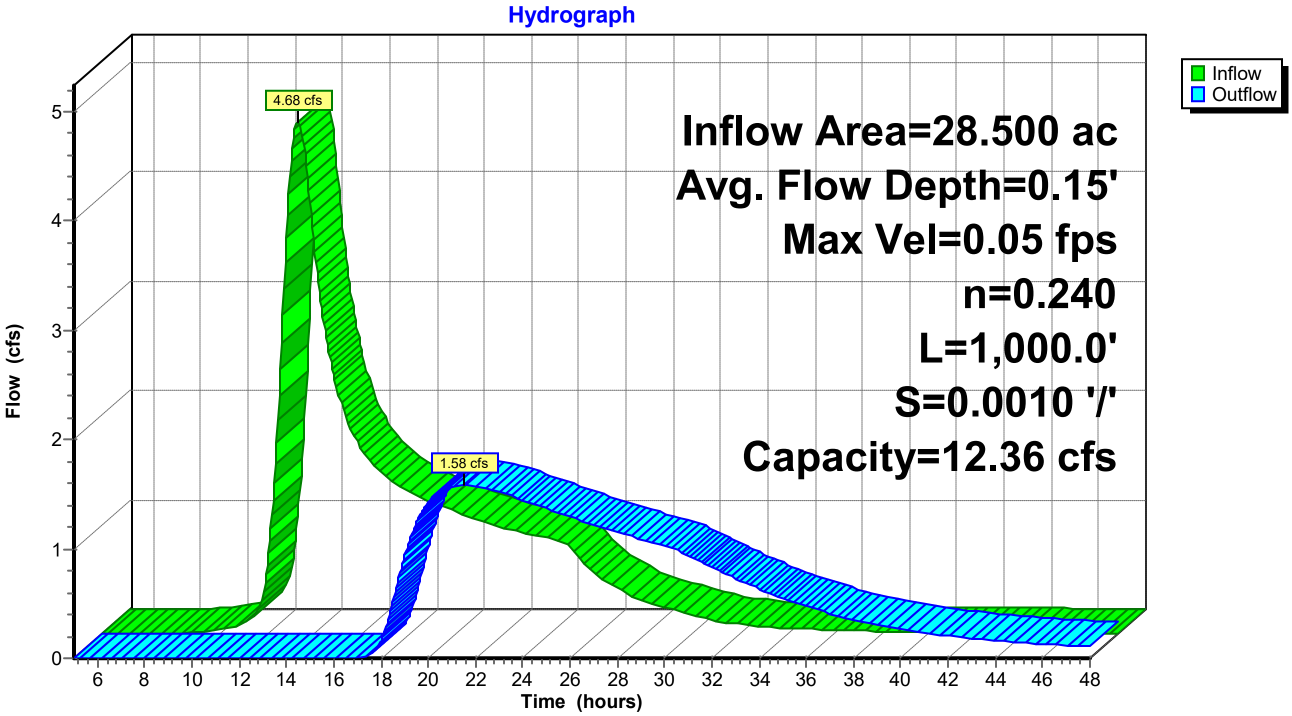
Inflow Area = 28.500 ac, 13.16% Impervious, Inflow Depth > 0.84" for 1-Year event
Inflow = 4.68 cfs @ 13.30 hrs, Volume= 1.998 af
Outflow = 1.58 cfs @ 21.48 hrs, Volume= 1.863 af, Atten= 66%, Lag= 491.2 min
Routed to Reach 3R : Outlet Pipe

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.05 fps, Min. Travel Time= 307.6 min
Avg. Velocity = 0.03 fps, Avg. Travel Time= 564.8 min

Peak Storage= 29,246 cf @ 16.36 hrs
Average Depth at Peak Storage= 0.15' , Surface Width= 200.88'
Bank-Full Depth= 0.50' Flow Area= 100.8 sf, Capacity= 12.36 cfs

200.00' x 0.50' deep channel, n= 0.240
Side Slope Z-value= 3.0 '/' Top Width= 203.00'
Length= 1,000.0' Slope= 0.0010 '/'
Inlet Invert= 6.00', Outlet Invert= 5.00'

Reach 1W: Wetland #1 / Analysis Point 1A



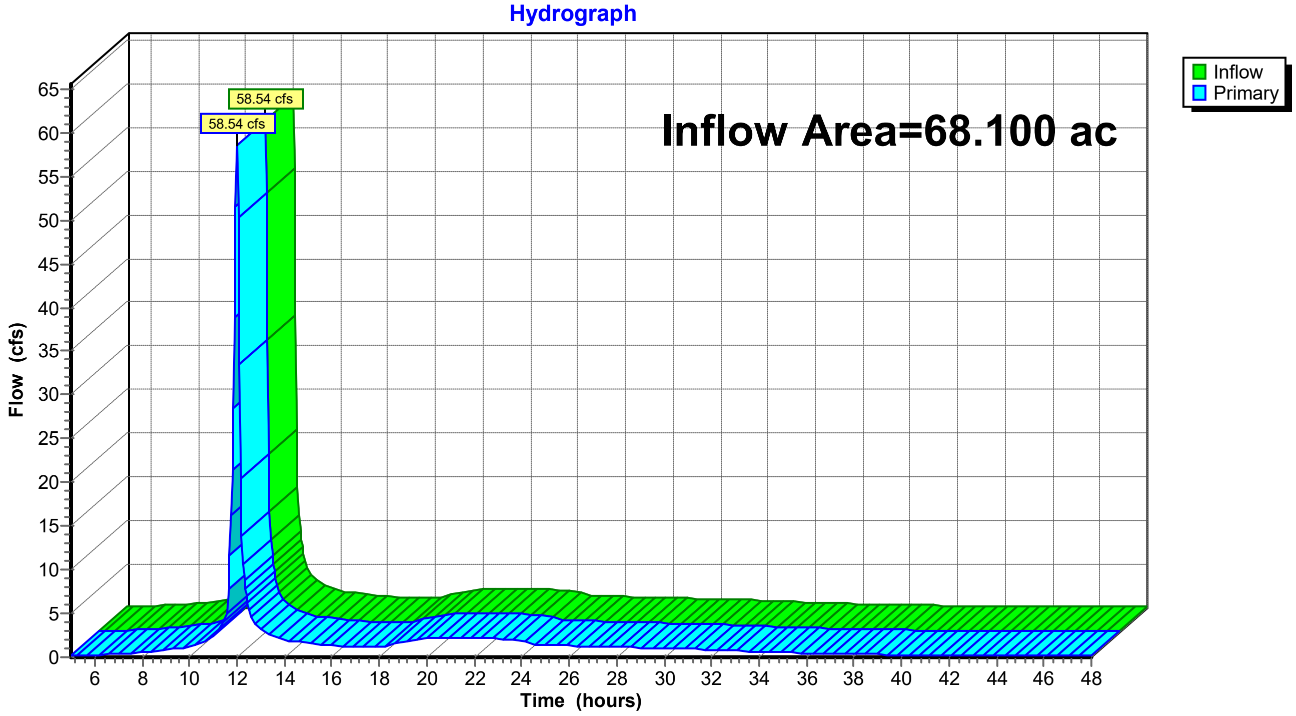
Summary for Pond AP-1: Analysis Point #1

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 68.100 ac, 20.36% Impervious, Inflow Depth > 0.92" for 1-Year event
Inflow = 58.54 cfs @ 12.00 hrs, Volume= 5.232 af
Primary = 58.54 cfs @ 12.00 hrs, Volume= 5.232 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

Pond AP-1: Analysis Point #1



Summary for Reach 1W: Wetland #1 / Analysis Point 1A

[55] Hint: Peak inflow is 101% of Manning's capacity

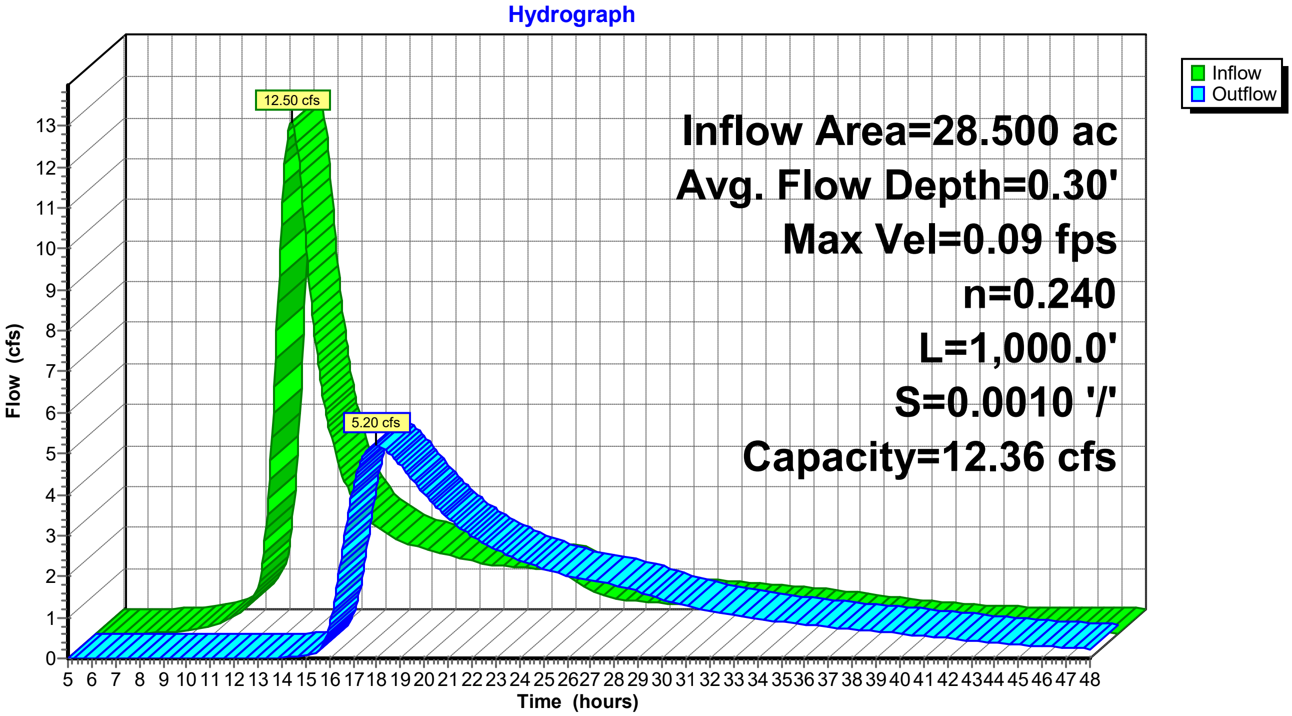
```
Inflow Area =        28.500 ac,  13.16% Impervious,  Inflow Depth >   1.94"    for  10-Year event
Inflow      =        12.50 cfs @   13.21 hrs,  Volume=           4.614 af
Outflow     =         5.20 cfs @   17.94 hrs,  Volume=           4.397 af,  Atten= 58%,  Lag= 283.8 min
     Routed to Reach 3R : Outlet Pipe
```

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.09 fps, Min. Travel Time= 191.6 min
Avg. Velocity = 0.04 fps, Avg. Travel Time= 403.6 min

Peak Storage= 59,798 cf @ 14.75 hrs
Average Depth at Peak Storage= 0.30' , Surface Width= 201.79'
Bank-Full Depth= 0.50' Flow Area= 100.8 sf, Capacity= 12.36 cfs

200.00' x 0.50' deep channel, n= 0.240
Side Slope Z-value= 3.0 '/' Top Width= 203.00'
Length= 1,000.0' Slope= 0.0010 '/'
Inlet Invert= 6.00', Outlet Invert= 5.00'

Reach 1W: Wetland #1 / Analysis Point 1A

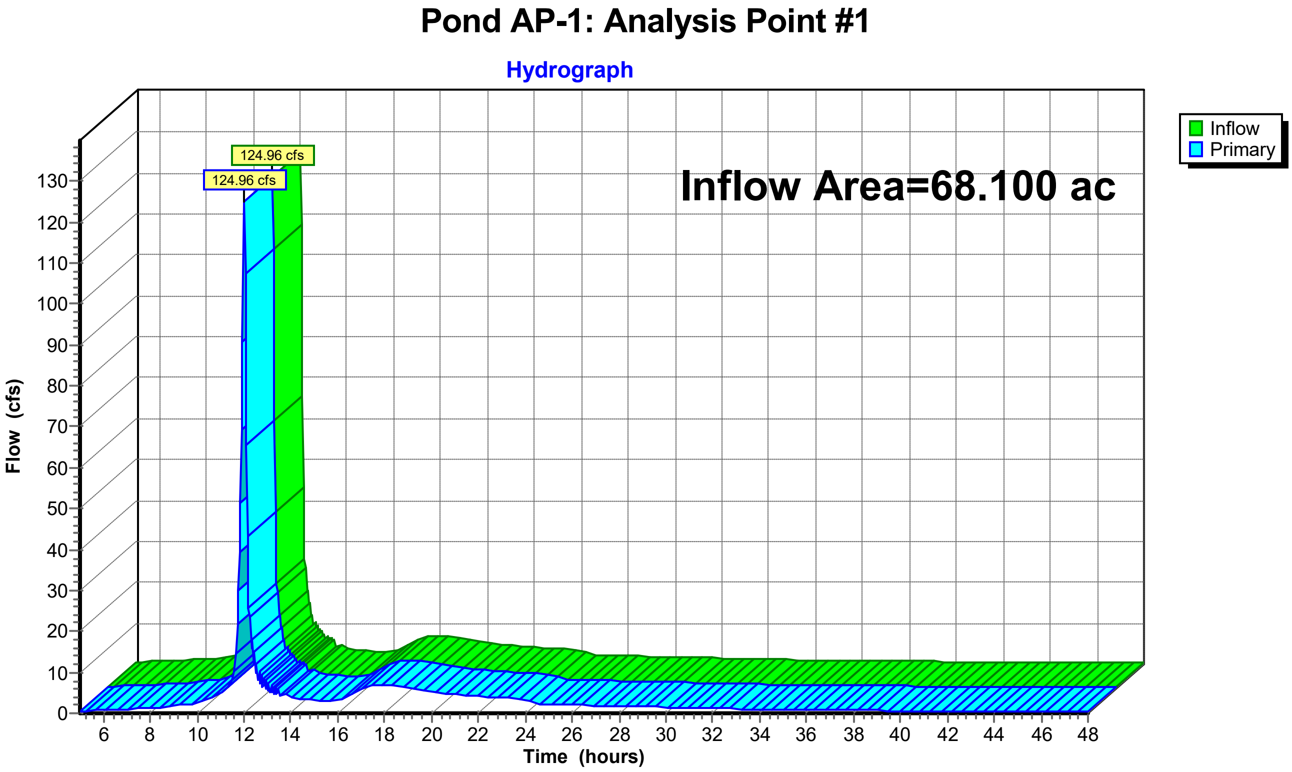


Summary for Pond AP-1: Analysis Point #1

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 68.100 ac, 20.36% Impervious, Inflow Depth > 1.94" for 10-Year event
Inflow = 124.96 cfs @ 12.01 hrs, Volume= 11.009 af
Primary = 124.96 cfs @ 12.01 hrs, Volume= 11.009 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs



Summary for Reach 1W: Wetland #1 / Analysis Point 1A

[91] Warning: Storage range exceeded by 0.14'
[55] Hint: Peak inflow is 246% of Manning's capacity

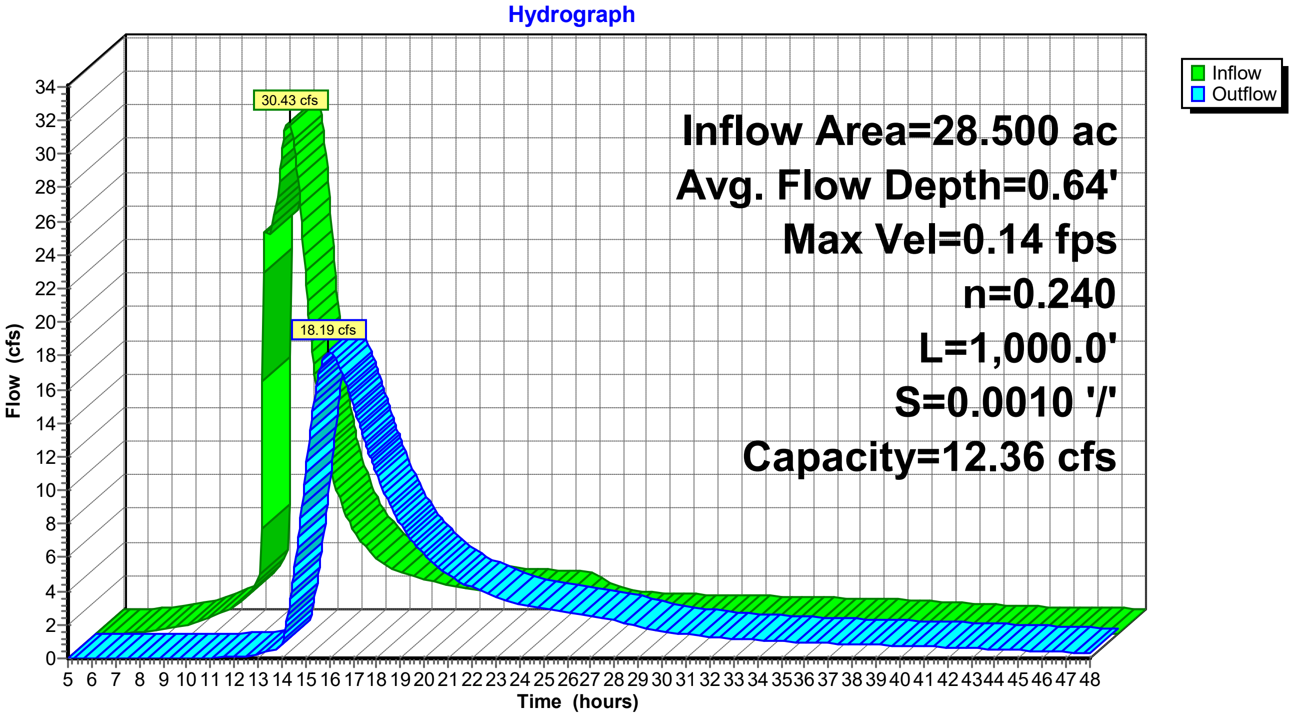
```
Inflow Area =       28.500 ac,  13.16% Impervious,  Inflow Depth >   4.12"    for  100-Year event
Inflow      =       30.43 cfs @   13.13 hrs,  Volume=          9.785 af
Outflow     =       18.19 cfs @   15.98 hrs,  Volume=          9.518 af,  Atten= 40%,  Lag= 170.8 min
    Routed to Reach 3R : Outlet Pipe
```

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.14 fps, Min. Travel Time= 118.7 min
Avg. Velocity = 0.05 fps, Avg. Travel Time= 313.6 min

Peak Storage= 129,558 cf @ 14.00 hrs
Average Depth at Peak Storage= 0.64' , Surface Width= 203.85'
Bank-Full Depth= 0.50' Flow Area= 100.8 sf, Capacity= 12.36 cfs

200.00' x 0.50' deep channel, n= 0.240
Side Slope Z-value= 3.0 '/' Top Width= 203.00'
Length= 1,000.0' Slope= 0.0010 '/'
Inlet Invert= 6.00', Outlet Invert= 5.00'

Reach 1W: Wetland #1 / Analysis Point 1A



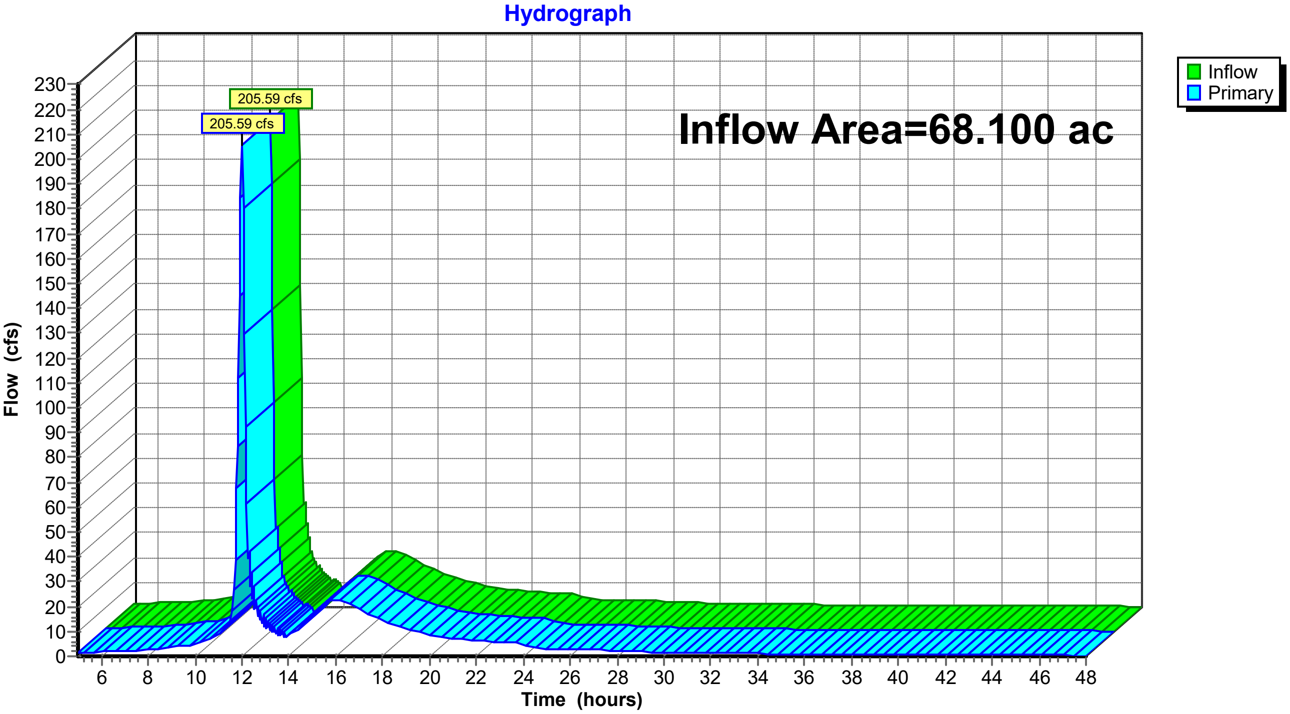
Summary for Pond AP-1: Analysis Point #1

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 68.100 ac, 20.36% Impervious, Inflow Depth > 3.95" for 100-Year event
Inflow = 205.59 cfs @ 12.00 hrs, Volume= 22.424 af
Primary = 205.59 cfs @ 12.00 hrs, Volume= 22.424 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

Pond AP-1: Analysis Point #1



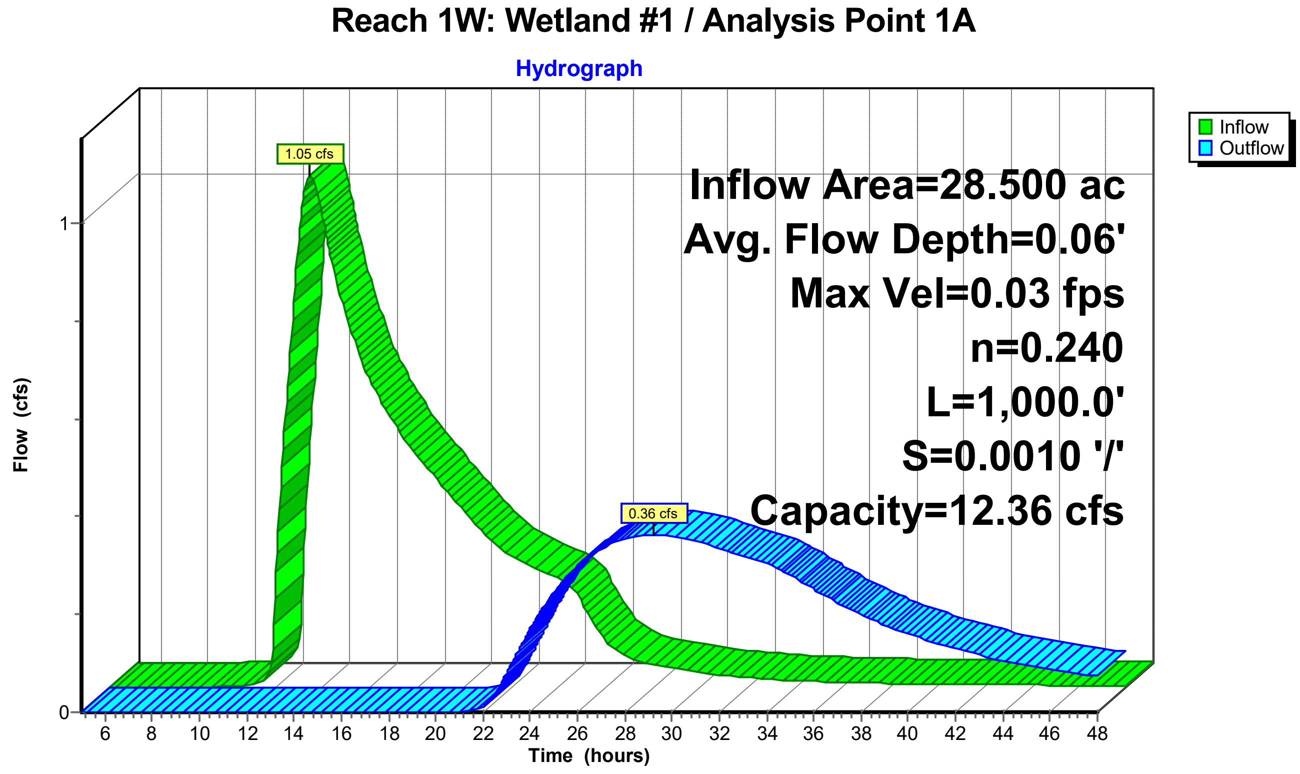
Summary for Reach 1W: Wetland #1 / Analysis Point 1A

Inflow Area = 28.500 ac, 13.16% Impervious, Inflow Depth > 0.24" for WQv event
Inflow = 1.05 cfs @ 13.51 hrs, Volume= 0.576 af
Outflow = 0.36 cfs @ 29.22 hrs, Volume= 0.465 af, Atten= 65%, Lag= 942.6 min
Routed to Reach 3R : Outlet Pipe

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.03 fps, Min. Travel Time= 554.0 min
Avg. Velocity = 0.02 fps, Avg. Travel Time= 872.3 min

Peak Storage= 12,066 cf @ 19.98 hrs
Average Depth at Peak Storage= 0.06' , Surface Width= 200.36'
Bank-Full Depth= 0.50' Flow Area= 100.8 sf, Capacity= 12.36 cfs

200.00' x 0.50' deep channel, n= 0.240
Side Slope Z-value= 3.0 '/' Top Width= 203.00'
Length= 1,000.0' Slope= 0.0010 '/'
Inlet Invert= 6.00', Outlet Invert= 5.00'

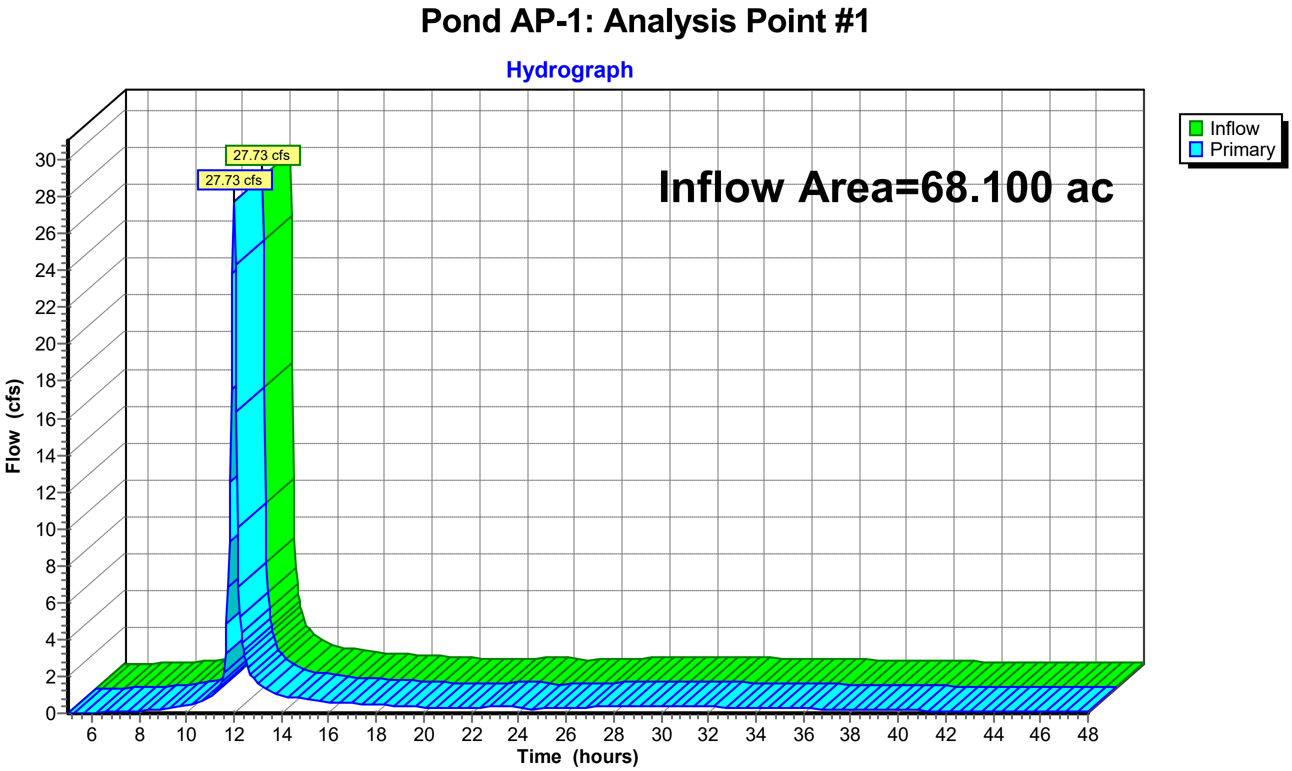

Reach 1W: Wetland #1 / Analysis Point 1A

Summary for Pond AP-1: Analysis Point #1

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area = 68.100 ac, 20.36% Impervious, Inflow Depth > 0.35" for WQv event
Inflow = 27.73 cfs @ 12.00 hrs, Volume= 1.975 af
Primary = 27.73 cfs @ 12.00 hrs, Volume= 1.975 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs



DRAINAGE DESIGN REPORT

FOR

MARMEN-WELCON TOWER MANUFACTURING PLANT

TOWN OF BETHLEHEM
ALBANY COUNTY
NEW YORK

FINAL SUBMISSION
JUNE 20, 2022

CREATED FOR:

ALBANY PORT DISTRICT COMMISSION
106 Smith Boulevard
Albany, NY 12202
518-463-8763
www.portofalbany.us

CREATED BY:

McFarland Johnson

60 Railroad Place, Suite 402
Saratoga Springs, NY 12866
518-580-9380
www.mjinc.com

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I. General Information

A. Project Description

This Stormwater Management Report has been developed for a Supplemental Environmental Impact Statement (SEIS) regarding a proposed development at the Port of Albany. The proposed development is an offshore wind (OSW) manufacturing facility that will produce wind turbine tower components. The Project is situated on 81.62 acres of land at the Beacon Island site, located at the confluence of the Normans Kill and Hudson River. The project also includes development within 4.4 acres of the adjoining parcel owned by National Grid, the extension and improvement of Normanskill Street, and widening of Rt. 144. The project owner, Albany Port District Commission (APDC), is proposing to develop the vacant parcels of land (tax parcels 98.00-2-10.23 and 98.01-2-1.0) to expand the existing Port of Albany in the Town of Bethlehem, Albany County, New York.

The proposed project will include development of an OSW tower manufacturing (Marmen-Welcon) facility consisting of five (5) separate buildings totaling up to 625,539+/- square feet of floor space. The following is a breakdown of the function and size of each building:

- Building A Plate Preparation & Welding (299,250 SF)
- Building B Welding Finishing (111,023 SF)
- Building C Blast Metallization Plant (131,968 SF)
- Building D Internal Assembly Finishing (61,550 SF)
- Building E Material Receiving (21,748 SF)

Tower production will occur within four (4) buildings (Buildings A-D) at the main facility on the Port Expansion property located in the Town of Bethlehem. The 5th building (Building E) will be located at 700 Smith Boulevard within the existing Port District in the City of Albany. A proposed gated bridge over the Normans Kill will provide a truck transportation route in and out of the main facility, by connecting Beacon Island and the 14.7-acre offsite parcel at 700 Smith Boulevard. In conjunction with the proposed bridge, Normanskill Street is to be extended from its existing end point to the bridge. The existing pavement will be improved to accommodate the proposed trucking route. River Road (Rt. 144) will be widened to accommodate the employee entrance. Employee parking will be situated on the adjoining land owned by National Grid with access from River Road. A proposed 500 LF wharf and associated dredging along the Hudson River will be used to load and ship completed tower sections. A separate stormwater analysis and SWPPP has been prepared for the 14.7-acre Building E site at 700 Smith Boulevard and the portion of Normanskill Street located in the City of Albany, as the sites are separated by approximately 1-mile and are under separate MS4 jurisdictions.

Historically, the Port Expansion site was composed of small islands and river channels subject to natural shifts due to flows associated with the Hudson River and the former Island Creek, a side channel of the Hudson River. Island Creek historically flowed along the western side of the site through the current power line corridor and discharged to the Hudson River at the southern end of the site. Based on available mapping, sometime between 1936 and 1961, Island Creek channel was diverted at the north end of the site directly to the Hudson River, whereupon it was referred to solely as Normans Kill, the main tributary to this former channel. The site was subject to historic filling operations to create usable lands and a portion of the site was operated as a coal ash (fly ash) disposal site by Niagara Mohawk from approximately 1952 to 1970. As such, there are large areas of fly ash deposits on the site that must be considered during the design and construction of the site infrastructure and stormwater management facilities. Excavated fly ash material will need to be appropriately handled and properly disposed of as discussed in Section B below. A soil management plan has been developed and will require a cap over the site.

The purpose of this report is to assess the stormwater quality, quantity, and erosion and sediment control for the development of the site. This report has been developed in accordance with the New York State Department of Environmental Conservation (NYSDEC) State Pollution Discharge Elimination System (SPDES) General Permit for Stormwater Discharges from Construction Activity, GP-0-20-001 (Permit) and the NYSDEC Stormwater Management Design Manual (The Manual). The project site is located within the Town of Bethlehem, Albany County, New York, which is an MS4 community, requiring this report and project to receive approval from the Town. A separate stormwater analysis and SWPPP has been prepared for the 14.7-acre Building E site at 700 Smith Boulevard and the portion of Normanskill Street located in the City of Albany, as the sites are separated by approximately 1-mile and are under separate MS4 jurisdictions.

The overall project limits analyzed in this Drainage Report are broken up into three (3) areas, hereafter referred to as (1) “Expansion Site”, (2) “Normanskill Street Improvements”, (3) “Offsite Improvements”. See Existing Conditions Map (Appendix A) for the location of each of these areas. The Expansion Site is the portion of the project area that is located on Beacon Island. The Normanskill Street Improvement portion begins on the north end of the proposed bridge over the Normans Kill and extends north to the border of the Town of Bethlehem and City of Albany. The Offsite Improvements portion refers to the widening of Rt. 144 adjacent to the employee entrance.

B. Soil Classification

According to the Natural Resources Conservation Service (NRCS) web soil survey, there are five (5) mapped soil units identified within the project boundary (see Appendix E). The majority of the soil at the expansion site falls within the hydrologic soil group B/D. The first letter corresponds to drained soil’s properties under drained conditions and the second to saturated conditions. Group B soils have moderate infiltration and runoff rates while group D have a low infiltration rate and a high runoff rate. The soils with dual group identifiers have been modeled with the more conservative of the two, in this case a D soils group. Most of the soil adjacent to Normanskill Street Improvements is within soil group A. Group A soils have a high infiltration rate.

The complete list of soils found on the project site is identified in the table below (see Appendix E for NRCS Soils Report).

Table I – Soils Summary

Symbol	Soil Name	Hydrologic Soil Group
HuE	Hudson silt loam, 25 to 45 Percent slopes	C/D
NrD	Nassau very channery silt loam, hilly, very rocky	D
Ug	Udorthents, loamy	A
Ur	Urban land	-
Wo	Wayland soils complex, non-calcareous substratum, 0 to 3 percent slopes, frequently flooded	B/D

Geotechnical studies have been undertaken to evaluate the subsurface conditions of the site. These investigations have been summarized in the following reports:

- *Preliminary Geotechnical Evaluation and Interpretive Report*, CME Associates, Inc., April 5, 2017
- *Supplemental Geotechnical Report*, Dente Group, July 20, 2017

Copies of these reports were included in the TOWN OF BETHLEHEM PLANNING BOARD, DRAFT GENERIC ENVIRONMENTAL IMPACT STATEMENT For ALBANY PORT DISTRICT COMMISSION PORT OF ALBANY EXPANSION PROJECT, Appendix E.

- *Draft Geotechnical Engineering Report,* Terracon, October 15, 2021

A copy of this reports is included in the TOWN OF BETHLEHEM PLANNING BOARD, SUPPLEMENTAL ENVIRONMENTAL IMPACT STATEMENT For ALBANY PORT DISTRICT COMMISSION PORT OF ALBANY EXPANSION PROJECT.

Based on these previous investigations, the subsurface conditions of the Expansion Site portion of the project site are generally characterized by historic fills of various depths overlying, in sequence with depth; river sediments, alluvial sands, glaciolacustrine silt/ clay, glacial till, and shale bedrock. The fill was noted at specific boring locations ranging from 6 to 23 feet below existing grade. The fill material is characterized as a random landfill deposit containing natural and solid waste deposits such as, but not limited to, foundry sand waste, sand, silt, coal ash, gravel, and organic matter. A predominant component of the fill was reported as coal ash.

Shale bedrock was found beneath the glacial till soils at select boring locations. The depth to rock ranged from approximately 61 feet below grade near the northwest portion of the site, to greater than 148 feet at the southeast portion of the site. The rock depths appear shallowest on the north and west sides of the site and increase to the east towards the Hudson River and in a south direction across the site. Based on the New York State Museum and Science Service's Geologic Map of New York: State Hudson-Mohawk Sheet, and the geotechnical rock core samples, the bedrock appears to be consistent with the Normans kill Shale Formation.

According to the geotechnical reports, shallow groundwater was observed at depths ranging from approximately 1.5 to 13.7 feet below existing grade. However, due to the subsurface conditions, the shallower observations could be representative of perched groundwater zones due to discontinuous impermeable layers. Shallow groundwater fluctuations should be expected to occur at this site depending on several factors such as rainfall, seasonal changes, prevailing climate, ambient weather conditions, and the tidal influences of the Hudson River.

A soil management plan (SMP) has been prepared in accordance with the NYSDEC regulations. The SMP is included in SWPPP. The SMP pertains only to the Expansion Site portion of the project.

II. Hydrology

A. Existing Conditions

The existing drainage area totals 108.4 +/- acres, separated by the Normans Kill. The drainage area is bordered by the Hudson River to the east. At the south boundary there is a Public Service Energy Group (PSEG) power plant, and to the west a parcel owned by National Grid that conveys overhead electric transmission lines as well as an underground gas main. The Port Expansion site consists primarily of brush and trees with a small gravel area as well as abandoned railroad tracks. The Normanskill Street Improvement site consists of an existing road as well as brush and trees to the west. The Offsite Improvements consist of the existing roadway as well as brush and trees to the east and west.

The existing drainage condition is split up into seven (7) drainage areas. Drainage area DR-A drains to analysis point #1A, drainage areas DR-B and DR-F drain to analysis point #1, drainage areas DR-D and DR-E drain to analysis point #2. Drainage area DR-G drains to analysis point #3. Drainage area DR-C drains to a self-contained depression for storage. See Appendix A for the Existing Conditions Drainage Map.

Runoff from DR-A travels via sheet and shallow concentrated flow directly to a wetland located in the northwest corner of the site (Wetland 1). During large storm events the wetland overflows into an existing 40" pipe with direct outlet to the Normans Kill. Analysis of the existing capacity of the outlet pipe is provided in section IV below. Runoff from areas DR-B, DR-D, DR-E, and DR-F travel via sheet and concentrated flow to low areas with eventual outfall directly to the Normans Kill and Hudson River. An approximately 30-acre internal portion of the site (DR-C) was determined to be self-contained within the site capable of storing and infiltrating the 100-year storm event. Runoff from area DR-G sheet flows to the west side of River Road and travels to a low spot adjacent to the roadway where it is stored and eventually infiltrated.

A wetland delineation was conducted in April 2019 by McFarland Johnson for the FGEIS. The results of the delineation indicated that there are 8 freshwater wetlands located within the project limits. These wetlands are hereafter referred to as Wetlands 1, 3, 4, 5, 6, 7, 8, and 9. Wetlands within the original study area totaled approximately 2.33 acres.

A Supplemental Wetland Delineation was performed by MJ in April 2021 of the 18.22 acres on the National Grid Parcel. One contiguous wetland, comprising of approximately 7.13 acres, was delineated within the 18.22-acre area. The delineated wetland represents an extension of the 2019 wetland delineation and previously identified as Wetland 1. Wetland 1 drains in a northerly direction into a 40-inch corrugated metal pipe (CMP) which discharges directly to the Normans Kill.

The existing site falls within the Normans Kill watershed of the Middle Hudson Sub-Basin for the Lower Hudson River Basin (HUC10: 0202000602, Water Index No H-221-4) which is listed as a Class C water. Neither the Normans Kill nor the Hudson River are listed in the Manual’s Appendix C as a watershed where enhanced phosphorus removal standards are required. Additionally, neither are listed in the Manual’s Appendix E as a watershed impaired by pollutants related to construction activity.

B. Proposed Conditions

The proposed Port Expansion Site development includes 603,791 +/- square feet of OSW manufacturing facility space spread out over four (4) separate buildings. Ancillary impervious areas include parking for automobiles and trucks, a roadway, bridge, and a maritime wharf. The remainder of the site will be used for tower storage and be made up of dense graded aggregate. There will also be small pervious areas of grass and unaltered brush and trees.

The Normanskill Street Improvements are along a 0.52 mile stretch of roadway within the Town of Bethlehem. A new portion of Normanskill Street is to be constructed from the proposed vehicle bridge north to the existing roadway. This extension is approximately 925 feet long and will be dense graded aggregate. The existing portion of Normanskill Street will be widened on the west side of the roadway. The remaining portion of the existing roadway will be re-paved and not disturbed, as subbase will remain. A top course mill and fill of 1.11 acres is proposed. The improvements also include the required corresponding stormwater conveyance and treatment.

The Offsite Improvements consist of widening an approximately 600 LF stretch of Rt. 144 adjacent to the expansion site employee entrance. The roadway is to be widened by 7 +/- ft on the east side. The new impervious area is 0.14 acres. Corresponding grading is also part of the offsite improvements.

The total post-development drainage area will be 108.6 acres. The post-development drainage area is larger than the pre-development area by 0.2 acres due to the proposed bridge over the Normans Kill. The total disturbance for construction of the site will be approximately 72.7 +/-

acres.

The proposed drainage condition is split up into seventeen (17) drainage areas. Drainage Areas DR-8, DR-9, and DR-10 drain to analysis point #1A. Drainage Areas DR-5, DR-6, DR-7, DR-12, DR-13, DR-14, DR-15, and DR-16 drain directly to analysis point #1. Drainage areas DR-1, DR-2, DR-3, DR-4, and DR-11 drain to analysis point #2. Drainage area DR-17 drains to analysis point #3. Each analysis point remains the same in the pre- and post-development condition for comparison. See Appendix B for the Proposed Conditions Drainage Map.

On the Expansion Site, runoff from the proposed impervious areas will travel via sheet and shallow concentrated flow to one of seven (7) closed drainage networks. Drainage networks 1-7 will be conveyed through a NYSDEC approved stormwater filtering system and/or an off-line infiltration chamber system which will provide water quality volume treatment and runoff reduction prior to being discharged into the Normans Kill or Hudson River.

Runoff from DR-8 and DR-9 will be conveyed via vegetated swales to Micropool Extended Detention Ponds (Type P-1 per the Manual). The ponds will provide water quality volume treatment. The portion of the water held above the wet pool will be slowly discharged to the surrounding area over a 24-hour period. The ponds will also hold larger storm events up to the 10-year storm. During the 10-year storm and larger, emergency spillways outlet to the surrounding vegetated area, eventually flowing to Wetland #1.

Drainage Areas DR-10, DR-11, and DR-12 maintain their existing drainage patterns.

Drainage areas DR-13 and DR-14 correspond to sections of new Normanskill Street roadway. In each area, stormwater is collected via roadside swales and directed into a sedimentation basin which overflows into Infiltration Basin #1. The basins are designed to infiltrate the WQv as well as smaller storm events. During large storm events water will overflow to the Normans Kill.

Drainage area DR-15 corresponds to a section of Normanskill Street that is being expanded to the west. Stormwater is collected via a roadside swale with a dry swale at the end. In large storm events, water will flow through the dry swale to an overflow trench to be discharged to the surrounding vegetated area, eventually flowing into the Normans Kill. Drainage Area DR-16 will not be disturbed in the development of this project and will continue to drain into the Normans Kill.

Drainage area DR-17 will collect runoff on Rt. 144 via a roadside swale with a dry swale at the end. In large storm events, water will flow through the dry swale to an overflow trench to be discharged to the surrounding area. The surround area is self-contained and eventually infiltrates stormwater runoff.

The overall drainage plan incorporates multiple separate systems with outlets to the Normans Kill and/or Hudson River to avoid a more concentrated larger outlet for the site. See Appendix B for proposed conditions plans and watershed mapping.

III. Stormwater Management & SPDES Requirements

The Proposed Development Project will have land disturbance of more than 1-acre, a full SPDES permit will be required, and a Stormwater Pollution Prevention Plan (SWPPP) will be developed in accordance with the Permit regulations and MS4 requirements as part of the Town of Bethlehem site plan approval process. A 5-acre waiver will also be requested in order to disturb more than 5 acres at one time.

Due to the presence of fly ash on the Expansion Site, in addition to a NYSDEC SPDES, a Site Management Plan (SMP) has been prepared in accordance with 6 NYCRR Part 375 and DER Technical Guidance for Site Investigation and Remediation and submitted to the NYSDEC, Division of Environmental Remediation and the NYSDOH. The SMP includes: a Health and Safety Plan (HASP), to inform and protect the contractor and their work force; a Community Air Monitoring Plan (CAMP), to monitor and protect the surrounding communities; and an Excavation Work Plan (EWP), to direct the activities of the contractor during construction. The EWP includes a detailed description of the work to be performed, the anticipated environmental conditions, and engineering controls to mitigate the movement of fly ash. The SMP has been included in the SEIS and SWPPP.

The SWPPP will be prepared in coordination with the Manual and meet the following criteria as the principal objectives contained in an approved SWPPP:

- Reduction or elimination of erosion and sediment loading to waterbodies during construction activities. Controls will be designed in accordance with the NYSDEC's New York State Standards and Specifications for Erosion and Sediment Control.
- Mitigate the impact of stormwater runoff on the water quality of the receiving waters.
- Maintenance of stormwater controls during and after completion of construction.

These objectives will be accomplished by incorporating design criteria outlined within the Technical Guidelines provided by The Manual and summarized below.

A. Methodology

To analyze the hydrologic impacts of the proposed development, a storm water management model was developed in accordance with the Manual. HydroCAD™, by HydroCAD Software Solutions LLC was used to model both the existing and proposed conditions: soil data from the NRCS Web Soil Survey was entered into the software; land coverage areas were estimated using aerial photography and site visits; watershed areas were developed using the surveyed topography; time of concentrations were estimated using USDA, Urban Hydrology for Small Watersheds, TR-55 (TR-55) methodology; and finally runoff and routing calculations were performed using the SCS Unit Hydrograph method.

The following rainfall depths were utilized in the analysis of the 1, 10, and 100-year storm events:

Table II – Hydrologic Analysis Data

Storm Event	Rainfall Depth (in.)
1-year	2.20
10-year	3.63
100-year	6.11

Rainfall depths were determined using the Northeast Regional Climate Center (NRCC) data for Albany County. The rainfall intensity utilized is the Type II-24 hour storm. This data is pre-programed in the HydroCAD software.

Green Infrastructure practices were designed in accordance with the Manual using the NYSDEC Runoff Reduction Worksheets available through the NYSDEC's Construction Stormwater Toolbox, available on their website.

The following general steps are followed when conducting a stormwater design:

1. **Site Planning:** The existing natural resource areas and drainage patterns including wetlands, waterways, floodplains, and soils are identified. Conservation of natural resources are maximized given the proposed site.
2. **Pre and Post-Development Conditions Analysis:** The pre and post-development stormwater runoff conditions for the 1, 10, and 100-year storm events are determined using HydroCAD (detailed HydroCAD reports for this project can be found in Appendices A and B).
3. **Water Quality:** The Water Quality Volume and Runoff Reduction Volume are calculated using Chapter 4 of the Manual and Green Infrastructure Worksheets (provided in Appendix C).
4. **Water Quantity:** Peak runoff and stormwater retention/detention are evaluated using the Manual.

B. Water Quality Volume (WQv) / Runoff Reduction Volume (RRv)

Section 4.2 of the Manual states that Water Quality Volume (WQv) is intended to improve the water quality by capturing and treating runoff from small, frequent storm events that contain higher pollutant levels created through the increase of impervious surfaces. Impervious surfaces accumulate pollutants that quickly wash off and rapidly enter downstream waters as well as prevent natural groundwater recharge.

The WQv required for the proposed site is based upon the 90% rainfall event number, percent of impervious cover, and the total site area. Calculations were done using the Green Infrastructure worksheets and can be found in Appendix C. The total WQv required is 273,874 cubic feet.

Runoff Reduction Volume (RRv) is the reduction of the total WQv by application of green infrastructure techniques and stormwater management practices to replicate pre-development hydrology more closely. The intent of RRv is to recognize the water quality benefits of certain site design practices to address flow as a pollutant of concern.

According to Section 4.3 of the Manual, RRv may be calculated based on three methods:

1. Reduction of the practice contributing area in WQv
2. Reduction of runoff volume by storage capacity of the practice
3. Reduction using standard SMPs with runoff reduction capacity

The minimum RRv required by the proposed site is based on the total area of new impervious cover and the Hydrologic Soil Group (HSG) Specific Reduction Factor (S). The specific reduction factor is based on the HSGs present at the existing site. Calculations were done using the NYSDEC Green Infrastructure worksheets and can be found in Appendix C. The minimum RRv was determined to be 57,313 cubic feet.

Due to the level of contamination present in the existing soils across the Expansion Site, stormwater infiltration practices were located only in areas along the Hudson River and Normans Kill where contamination is not expected to be present. However, the Normanskill Street extension project area does not contain contaminated soils, so all treatment practices selected utilize infiltration and therefore include RRv. Infiltration testing has been performed at all proposed infiltration practices. Test results are included as Appendix F.

The following stormwater treatment practices were designed to meet the WQv and RRv requirements of the Manual:

Stormwater Ponds

Two stormwater quality ponds (Pond #1 and Pond #2) have been designed as Micropool Extended Detention Ponds (P-1) in accordance with the Manual. Ponds #1 and #2 treat stormwater runoff from drainage areas DR-8 and DR-9 respectively. Runoff from these areas sheet flow to a vegetated swale and outlet into the forebay of the pond. As required by the manual, the permanent pool volume is a minimum 20% of the WQv. Any stormwater held above the permanent pool elevation will be slowly discharged from the pond over a period of 24 hours. In larger storm events, the ponds will provide a “first flush” treatment for up to a 10-year storm event with stabilized emergency spillways to direct flow from larger event greater than a 10-year event to the surrounding area. Due to the topography of the surrounding undisturbed area, water will flow toward Wetland #1. A pre- and post-development analysis of the inflow to Wetland #1 (Analysis Point #1A) has been included in Section IV below. The post-development runoff going to Wetland #1 does not exceed the pre-development condition. Detailed design of the stormwater ponds can be found on page GR-14 of the Expansion Site plan set.

Manufactured Stormwater Filtering Units

Six stormwater filtering systems have been designed to treat runoff from drainage areas DR-1, DR-2, DR-3, DR-4, DR-6, and DR-7. Runoff from these areas sheet flow to its respective closed drainage system and is treated through a filtering manhole unit(s) before the outfall. Details of the proposed systems are located on sheet GR-15 of the Expansion Site plan set. All systems meet the minimum criteria as defined in Chapter 4 of the Manual and are certified by Washington State Department of Ecology (TAPE) the Maryland Department of the Environment. The systems provide 89% TSS removal and 40% TP removal, which exceed the performance requirements defined in section 3.3.2 of the Manual. Usage of the manufactured stormwater systems is documented in section 9 of the SWPPP. The design and sizing calculations for the manufactured stormwater filtering units is provided in the GI Worksheets (Appendix C).

Infiltration Chamber Systems

Three infiltration chamber systems have been designed to provide runoff reduction volume. For drainage areas DR-1 and DR-4 the infiltration chambers work in conjunction with Stormwater Filtering Units in order to treat the entire water quality volume. For drainage area DR-5 the infiltration chamber system provides sufficient volume to provide runoff reduction as well as treat the entire water quality volume. After flowing through the stormwater filtering units, runoff flows into the infiltration chambers and is stored until infiltrated into the ground. During large storm events, a diversion manhole allows runoff to bypass the infiltration chambers and flow to the Hudson River or Normans Kill. The design and sizing calculations for the chambers is provided in the GI Worksheets under the Infiltration Basin tab (Appendix C). Specifications for the chamber systems utilized are provided in Appendix D.

Infiltration Basins

Two infiltration basins (Basin #1 and Basin #2) have been designed in accordance with the manual. Basin #1 and #2 treat stormwater runoff from drainage areas DR-13 and DR-14 respectively. Runoff from these areas sheet flow to an open roadside swale which outlets into the forebay of the pond. Both ponds have been designed to infiltrate the WQv as required by each catchment area. Because both ponds outlet directly to the Normans Kill, detention of large storm events is not required (see section III.C through III.E below). The basins do not have additional capacity for the Channel Protection Volume (CPv), Overbank Flood Control (Qp), or Extreme Flood Control (Qf). In large storm events, the basin will provide a “first flush” treatment with stabilized emergency spillways to direct flow to the Normans Kill. All stormwater within the basin will be infiltrated within 48 hours of a rain event. Design of the infiltration basins is provided in the GI Worksheets (Appendix C).

Dry Swale

Two dry swales have been designed to treat the new impervious area within DR-15 and DR-17 respectively. Runoff from each area will sheet flow to a roadside swale with the end segment constructed as a dry swale. In large storm events, water not infiltrated into the swale will overflow to a stabilized outlet which drains to the surrounding area. Dry swale #2 has been designed without infiltration, and therefore will only take credit for WQv. Design of the dry swales is provided in the GI Worksheets (Appendix C).

Conservation of Natural Areas

Drainage Area DR-11 will be deed restricted to ensure the perpetual protection of the proposed area to remain vegetated along the Hudson River. Therefore, this area qualifies under the "Conservation of Natural Areas" volume reduction practice.

The RRv provided in the designed practices are summarized in Table III below:

Table III – Practices Providing Runoff Reduction

Drainage Area	Stormwater Practice	RRv (cf)
DR-1	Infiltration Chamber	21,248
DR-4	Infiltration Chamber	18,464
DR-5	Infiltration Chamber	18,426
DR-11	Conservation of Natural Areas	868
DR-13	Infiltration Basin #1	1,995
DR-14	Infiltration Basin #2	2,245
DR-15	Dry Swale #1	87
	Total RRv	**63,333**

The WQv provided for each drainage area is summarized in Table IV below:

Table IV – Practices Providing Water Quality Volume

Drainage Area	Stormwater Practice	WQv (cf)
DR-1	Filter Type 2	18,005
	Infiltration Chamber	2,802
DR-2	Filter Type 1	21,971
DR-3	Filter Type 1	43,939
DR-4	Filter Type 3	14,989
	Infiltration Chamber	2,052
DR-5	Infiltration Chamber	2,047
DR-6	Filter Type 1	48,060
DR-7	Filter Type 2	34,826
DR-8	Stormwater Pond #1	8,437
DR-9	Stormwater Pond #2	13,361
DR-14	Infiltration Basin #2	141
DR-15	Dry Swale #1	129
DR-17	Dry Swale #2	636
	Total WQv	**211,395**

C. Channel Protection Volume (CPv)

Stream Channel Protection Volume Requirements (CPv) are designed to protect stream channels from erosion. The Manual was used to determine the water quantity requirements of CPv; specifically, providing 24-hour extended detention for the 1-year storm event or discharging directly to tidal waters.

According to Section 4.4 of the Manual, the Stream Channel Protection Volume (CPV) requirement does not apply when the site discharges to a tidal waterbody.
The CPv requirement does not apply in certain conditions, including the following:

- Reduction of the entire CPv volume is achieved at a site through green infrastructure or infiltration systems.
- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams.

The Hudson River and Normans Kill are classified as tidal waters at the project site. Therefore, 24-hour extended detention of the 1-year storm event is not required for all drainage areas that outlet directly to the Hudson River or Normans Kill.

Drainage areas DR-8 and DR-9 convey large storm events to Wetland #1, and therefore require water quantity controls. A pre-and post-development analysis of the inflow to Wetland #1 was performed and analyzed as Analysis Point #1A. The existing 40” outlet pipe from the existing wetland 1 was also analyzed to confirm that adequate capacity was present for the proposed drainage conditions prior to being discharged to the Normans Kill. See Section IV.

The change in hydrology for the 1-year storm event from existing to proposed is shown in the HydroCAD Report printouts provided in Appendix A and B and summarized in Table VI.

D. Overbank Flood Control (Qp)

The primary purpose of the overbank flood control sizing criterion is to prevent an increase in the frequency and magnitude of out-of-bank flooding generated by urban development. The Manual was used to determine the water quantity requirements of Qp; specifically, providing sufficient retention volume to discharge all runoff from the proposed 10-year storm event at a rate equal to or less than the existing peak 10-year runoff rate or discharging directly to tidal waters.

According to Section 4.5 of the Manual, the Overbank Flood Control Criteria (Qp) requirement does not apply when the site discharges to a tidal waterbody.

The overbank flood control requirement (Qp) does not apply in certain conditions, including:

- The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams. Refer to Section 4.3 of the Manual for instructions.

The Hudson River and Normans Kill are classified as tidal waters at the project site. Therefore, retention the 10-year storm event is not required for all drainage areas that outlet directly to the Hudson River or Normans Kill.

Drainage areas DR-8 and DR-9 convey large storm events to Wetland #1, and therefore require water quantity controls A pre-and post-development analysis of the inflow to Wetland #1 was performed and analyzed as Analysis Point #1A. The existing 40" outlet pipe from the existing wetland 1 was also analyzed to confirm that adequate capacity was present for the proposed drainage conditions prior to being discharged to the Normans Kill. See Section IV.

The change in hydrology for the 10-year storm event from existing to proposed is shown in the HydroCAD Report printouts provided in Appendix A and B and summarized in Table VI.

E. Extreme Flood Control (Qf)

The intent of the extreme flood criteria is to prevent the increased risk of flood damage from large storm events, maintain the boundaries of the predevelopment 100-year floodplain, and protect the physical integrity of stormwater management practices. The Manual was used to determine the water quantity requirements of Qf; specifically, providing sufficient retention volume to discharge all runoff from the proposed 100-year storm event at a rate equal to or less than the existing peak 100-year runoff rate or discharging directly to tidal waters.

According to Section 4.6 of the Manual, the Extreme Flood Control Criteria (Qf) requirement does not apply when the site discharges to a tidal waterbody.

> The 100-year storm control requirement can be waived if:
> - The site discharges directly tidal waters or fifth order (fifth downstream) or larger streams. Refer to Section 4.3 of the Manual for instructions.

The Hudson River and Normans Kill are classified as tidal waters at the project site. Therefore, retention the 100-year storm event is not required for all drainage areas that outlet directly to the Hudson River or Normans Kill.

Drainage areas DR-8 and DR-9 convey large storm events to Wetland #1, and therefore require water quantity controls. A pre-and post-development analysis of the inflow to Wetland #1 was performed and analyzed as Analysis Point #1A. The existing 40" outlet pipe from the existing wetland 1 was also analyzed to confirm that adequate capacity was present for the proposed drainage conditions prior to being discharged to the Normans Kill. See Section IV.

The change in hydrology for the 100-year storm event from existing to proposed is shown in the HydroCAD Report printouts provided in Appendix A and B and summarized in Table VI.

IV. Summary of Findings

A. Summary of Results

Table V lists the required and provided RRv and WQv for the project. As shown, the project meets both the total water quality volume and runoff reduction volume required by the Manual.

Table V – Stormwater Management Practice Summary

Drainage Area	Stormwater Practice	RRv (cf)	WQv (cf)	Total (RRv + WQv)
DR-1	Filter Type 2	-	18,005	42,055
	Infiltration Chamber	21,248	2,802	
DR-2	Filter Type 1	-	21,971	21,971
DR-3	Filter Type 1	-	43,939	43,939
DR-4	Filter Type 3	-	14,989	35,505
	Infiltration Chamber	18,464	2,052	
DR-5	Infiltration Chamber	18,426	2,047	20,473
DR-6	Filter Type 1	-	48,060	48,060
DR-7	Filter Type 2	-	34,826	34,826
DR-8	Stormwater Pond #1	-	8,437	8,437
DR-9	Stormwater Pond #2	-	13,361	13,361
DR-11	Conservation of Natural Areas	868	-	868
DR-13	Infiltration Basin #1	1,995	-	1,995
DR-14	Infiltration Basin #2	2,245	141	2,386
DR-15	Dry Swale #1	87	129	216
DR-17	Dry Swale #2	-	636	636
Totals	-	**63,333**	211,395	**274,728**
Required	-	57,313	-	273,874

Table VI below depicts the peak discharge in the existing and proposed conditions for the 1-year, 10-year and 100-year design storms. The peak discharge for all storm events draining to Analysis Point #1A is decreased in the post-development condition. The peak discharge for all storm events for the remaining analysis points exceed the existing value; however, as described in Sections III, C through E above, this requirement does not apply to analysis points 1 (Hudson River) and 2 (Normans Kill).

Table VI – Peak Discharge Storm Analysis

Analysis Point	Storm Event	Existing (cfs)	Proposed (cfs)
1A	1-year	27.32	4.77
	10-year	73.24	12.46
	100-year	163.60	30.97
1	1-year	3.25	58.54
	10-year	14.96	124.96
	100-year	43.20	205.59
2	1-year	7.17	82.79
	10-year	20.65	166.00
	100-year	48.06	290.19
3	1-year	0.60	1.20
	10-year	1.84	2.62
	100-year	4.39	5.25

Analysis Point #1A analyzes the peak discharge at Wetland #1. In large storm events, stormwater quality ponds #1 and #2 will overflow to an emergency spillway that outlets to the area surrounding Wetland #1. In the post-development condition, Analysis Point #1A has a peak discharge less than the pre-development condition during all storm events, therefore the required water quantity controls are met.

In the post-development condition, Analysis Point #1 has a total drainage area of 0.12 square miles (75.28 acres). This point drains to the Normans Kill with a drainage area of 162 square miles (103,680 acres). The project area makes up approximately 0.07% of the total drainage area of the Normans Kill. With an overall project time of concentration of around 10 minutes, the proposed project will have a negligible impact on the total Normans Kill hydrology as the site-produced runoff will be conveyed prior to the Normans Kill peak and will not impact the overall flood conditions of the Normans Kill.

In the post-development condition, Analysis Point #2 has a total drainage area of 0.04 square miles (23.6 acres). This point drains to the Hudson River with a drainage area of 8,090 square miles (5,177,600 acres). The project area makes up approximately 0.0005% of the total drainage area of the Hudson. With an overall project time of concentration of around 10 minutes, the proposed project will have a negligible impact on the total Hudson River hydrology, as the site-produced runoff will be conveyed prior to the Hudson River peak and will not impact the overall flood conditions of the Hudson River.

In the post-development condition, Analysis Point #3 discharge rates are higher than the pre-development condition. However, analysis point #3 drains to the surrounding area which stores runoff to be gradually infiltrated. Runoff from this analysis point does not flow to a stream or wetland.

The existing 40” outlet pipe from Wetland #1 was also analyzed for capacity. The analysis is summarized in table VII below.

Table VII – Outlet Pipe Capacity Comparison

Storm Event	Existing (cfs)	Proposed (cfs)	Capacity (cfs)
1-year	3.19	1.61	70.83
10-year	14.53	5.17	70.83
100-year	41.52	18.15	70.83

B. Deviation from NYS Stormwater Management Design Manual

The proposed stormwater management design deviates from The Manual by utilizing manufactured stormwater filtering systems for new development.

The need for alternative stormwater management practices is rooted in the extremely limited space available as well as the current site conditions. The proposed Offshore Wind Manufacturing Facility requires 85 acres of usable manufacturing and storage space along the Hudson River. It also requires close proximity to an existing port. Such requirements narrow the available project locations to a select few plots of unoccupied land in the entire state and this site was selected through a solicitation process by the state for off-shore wind development. This site was chosen given it is located adjacent to the existing Port of Albany and is directly on the Hudson River. However, the usable portion of the site adjacent to the Hudson River, is only 66-acres. Therefore, the entirety of the site is needed for the OSW manufacturing process, with an ancillary receiving site located at 700 Smith Boulevard.

The Expansion Site extends also onto the adjacent National Grid property from which APDC is leasing approximately 4.4 acres. However, National Grid has prohibited the installation of permanent stormwater practices within their property.

In addition to space limitations, the existing soils conditions prevent infiltration from being utilized as a stormwater management practice over most of the site. The existing soils are classified as Hydrologic Group D and B/D which provide little to no infiltration and are underlaid by fly ash. Infiltration practices were utilized in the areas where greater than 0.5 in/hr infiltration rates were achieved.

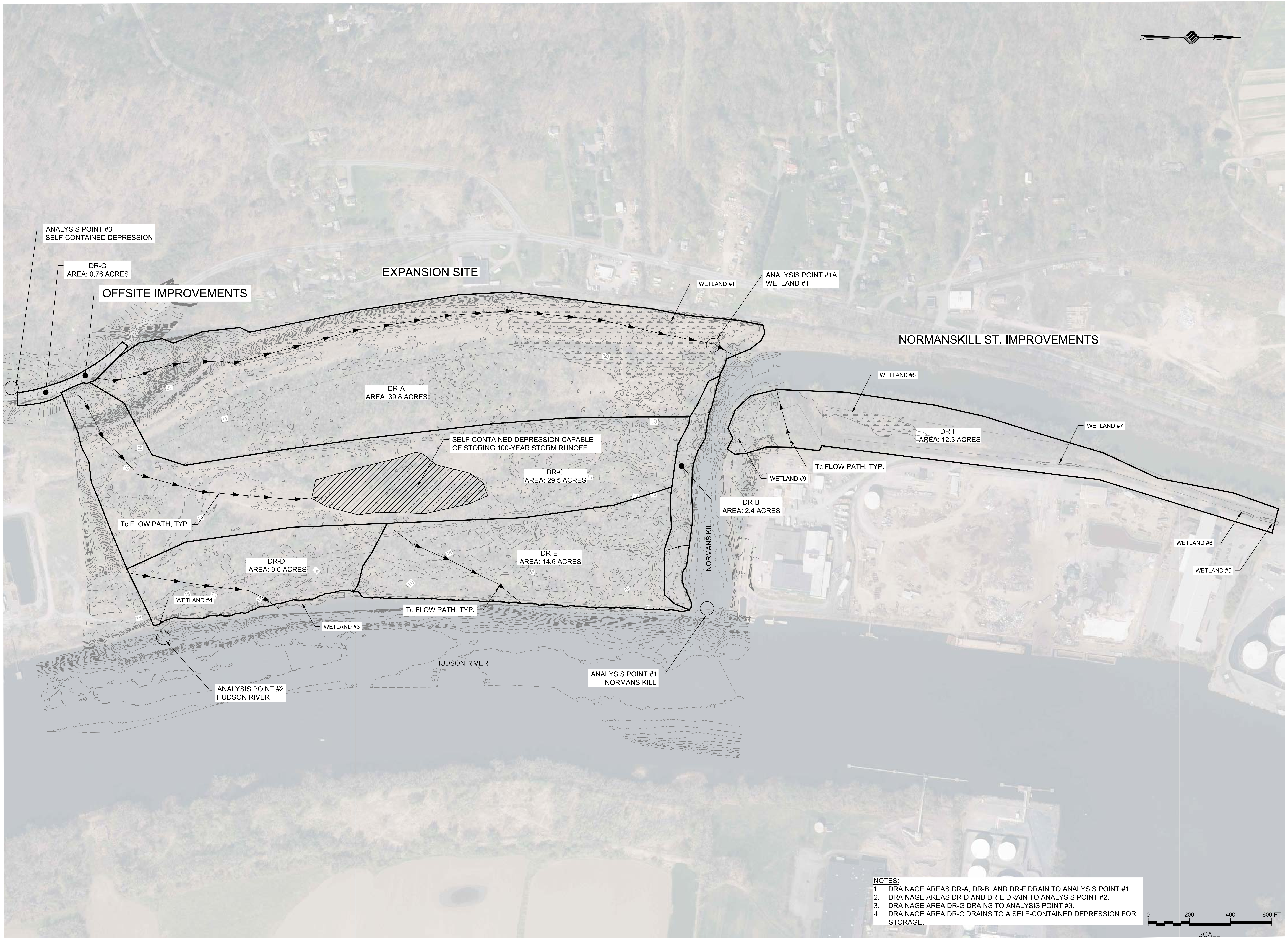
To adequately satisfy the WQv requirements of the Manual, manufactured systems are needed. The Contech Jellyfish units designed meet both the performance and sizing requirements of Chapter 4 of the Manual. The units are also certified by Washington State Department of Ecology (TAPE) and the Maryland Department of the Environment, adequate sources accepted by the NYSDEC. Specifications for the proposed units are provided in Appendix D.

C. Conclusion

Based upon the analysis provided in this report, the proposed development can meet the sizing and performance requirements as defined in Chapter 4 of the Manual. During construction, Erosion and Sediment Control activities will be designed and enforced in accordance with the NYSDEC New York State Standards and Specifications for Erosion and Sediment Control. Stormwater management practices can provide the required RRv and WQv for the proposed conditions. The elements of the Manual and the SPDES Permit that relate to stormwater quantity controls, specifically CPv (1-year), Qp (10-year), and Qf (100-year), are not applicable for portions of the site that discharge directly to a tidal water. For the remaining areas, the peak discharge for the 1-year, 10-year, and 100-year storm events are reduced in the post-development condition. A downstream analysis was completed for the existing Wetland #1 (Analysis Point #1A) and its outlet pipe to confirm adequate capacity prior to discharging into the tidal waters of the Normans Kill. All elements of the closed drainage system have been designed to be non-erosive during a 2-year storm event and capable of conveying a 10-year storm event. Analysis of all closed drainage pipe networks is included in Appendix C. After construction, a maintenance and operation report program and agreement will be made between the site operator and MS4 (Town of Bethlehem) to ensure all stormwater management practices are maintained over the life of the site’s operations.

Appendix A

Existing Conditions Drainage Map and HydroCAD Report



NOTES:
1. DRAINAGE AREAS DR-A, DR-B, AND DR-F DRAIN TO ANALYSIS POINT #1.
2. DRAINAGE AREAS DR-D AND DR-E DRAIN TO ANALYSIS POINT #2.
3. DRAINAGE AREA DR-G DRAINS TO ANALYSIS POINT #3.
4. DRAINAGE AREA DR-C DRAINS TO A SELF-CONTAINED DEPRESSION FOR STORAGE.

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SaratogaROM@mjinc.com

PROJECT MILESTONE
NFC

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY SITE INFRASTRUCTURE IMPROVEMENTS

DRAWN	NSO
DESIGNED	NSO
CHECKED	AJF
SCALE	1" = 200'
DATE	JANUARY 2022
PROJECT	18641.00

DRAWING TITLE

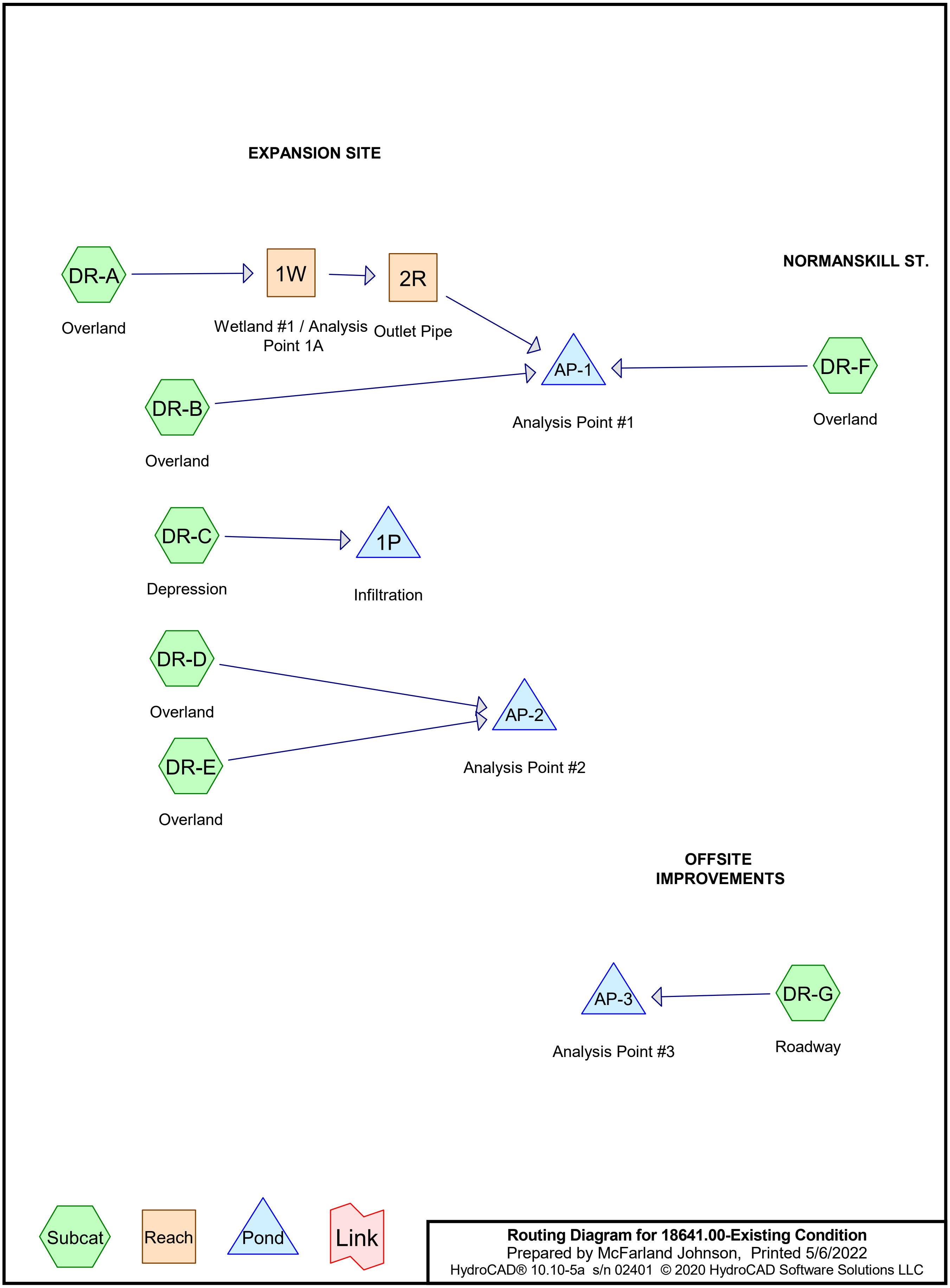
PRE-DEVELOPMENT
SITE DRAINAGE
AREAS

DRAWING NUMBER

DR-EX

01 OF 01

N:\18641.00 ALBANY PORT EXPANSION\REPORTS\DRAINAGE REPORT\18641.00-EXISTING CONDITION.DWG



Routing Diagram for 18641.00-Existing Condition
Prepared by McFarland Johnson, Printed 5/6/2022
HydroCAD® 10.10-5a s/n 02401 © 2020 HydroCAD Software Solutions LLC

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.300	39	>75% Grass cover, Good, HSG A (DR-G)
3.970	77	Brush, Fair, HSG D (DR-C)
1.100	98	Existing Railroad (DR-A)
2.500	96	Gravel surface, HSG D (DR-C)
1.100	98	Pavement (DR-F)
0.460	98	Roadway (DR-G)
87.730	79	Woods, Fair, HSG D (DR-A, DR-B, DR-C, DR-D, DR-E)
11.200	43	Woods/grass comb., Fair, HSG A (DR-F)
108.360	**76**	**TOTAL AREA**

Summary for Subcatchment DR-A: Overland

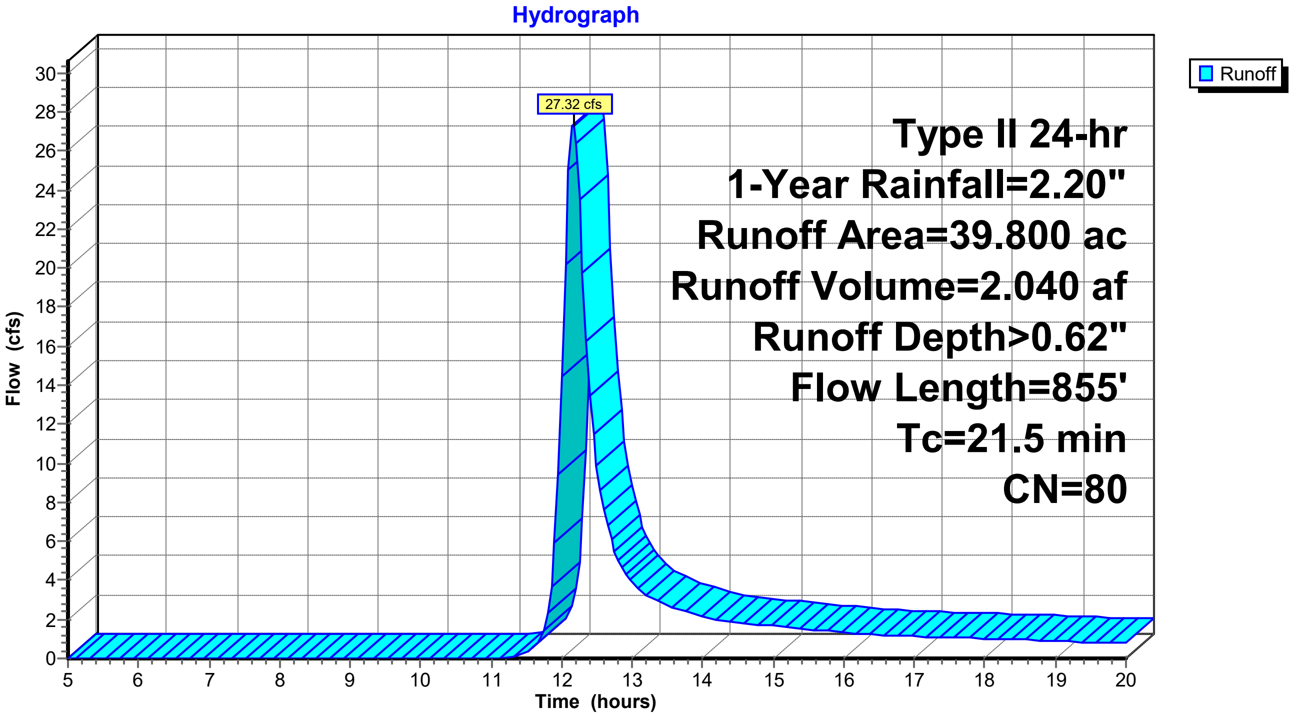
Runoff = 27.32 cfs @ 12.16 hrs, Volume= 2.040 af, Depth> 0.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
	38.700	79	Woods, Fair, HSG D
*	1.100	98	Existing Railroad
	39.800	80	Weighted Average
	38.700		97.24% Pervious Area
	1.100		2.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.5	150	0.1500	0.17		**Sheet Flow, Sheet Flow** Woods: Light underbrush n= 0.400 P2= 2.67"
4.8	575	0.1600	2.00		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
2.2	130	0.0400	1.00		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
21.5	855	Total			

Subcatchment DR-A: Overland



Summary for Subcatchment DR-B: Overland

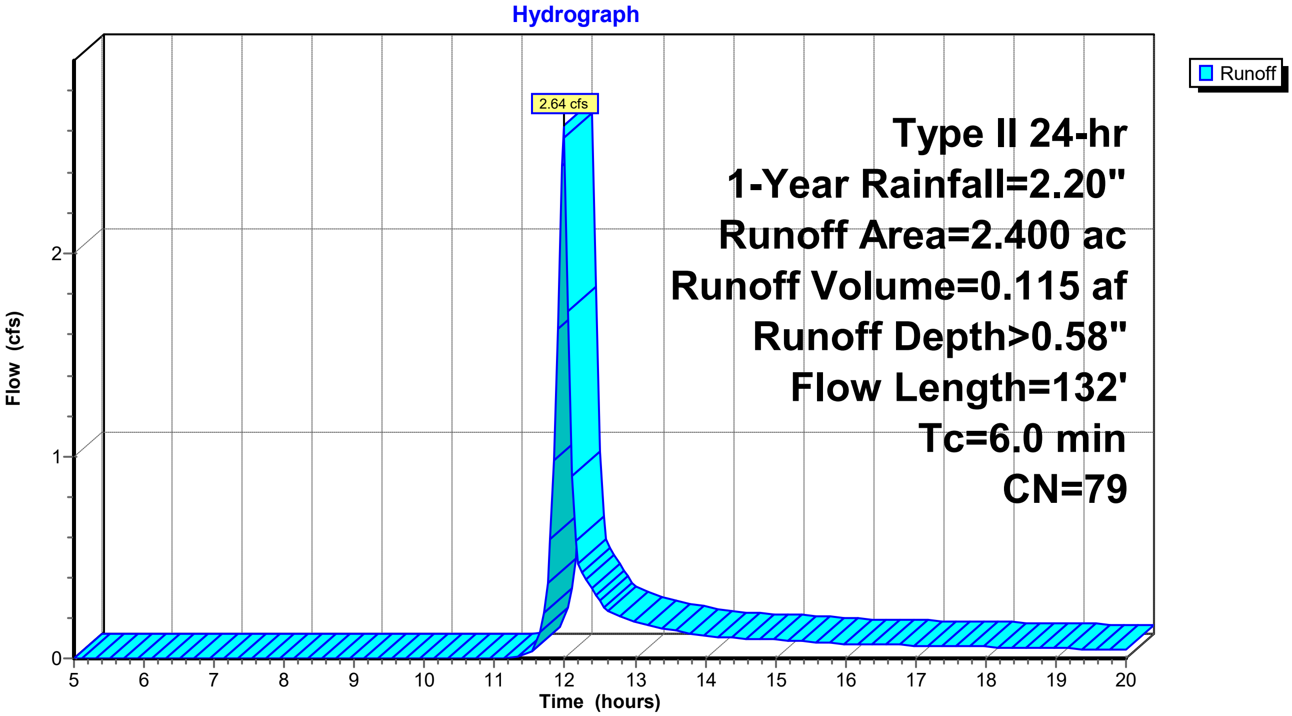
Runoff = 2.64 cfs @ 11.98 hrs, Volume= 0.115 af, Depth> 0.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

Area (ac)	CN	Description
2.400	79	Woods, Fair, HSG D
2.400		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	132		0.37		**Direct Entry, Sheet Flow**

Subcatchment DR-B: Overland



Summary for Subcatchment DR-C: Depression

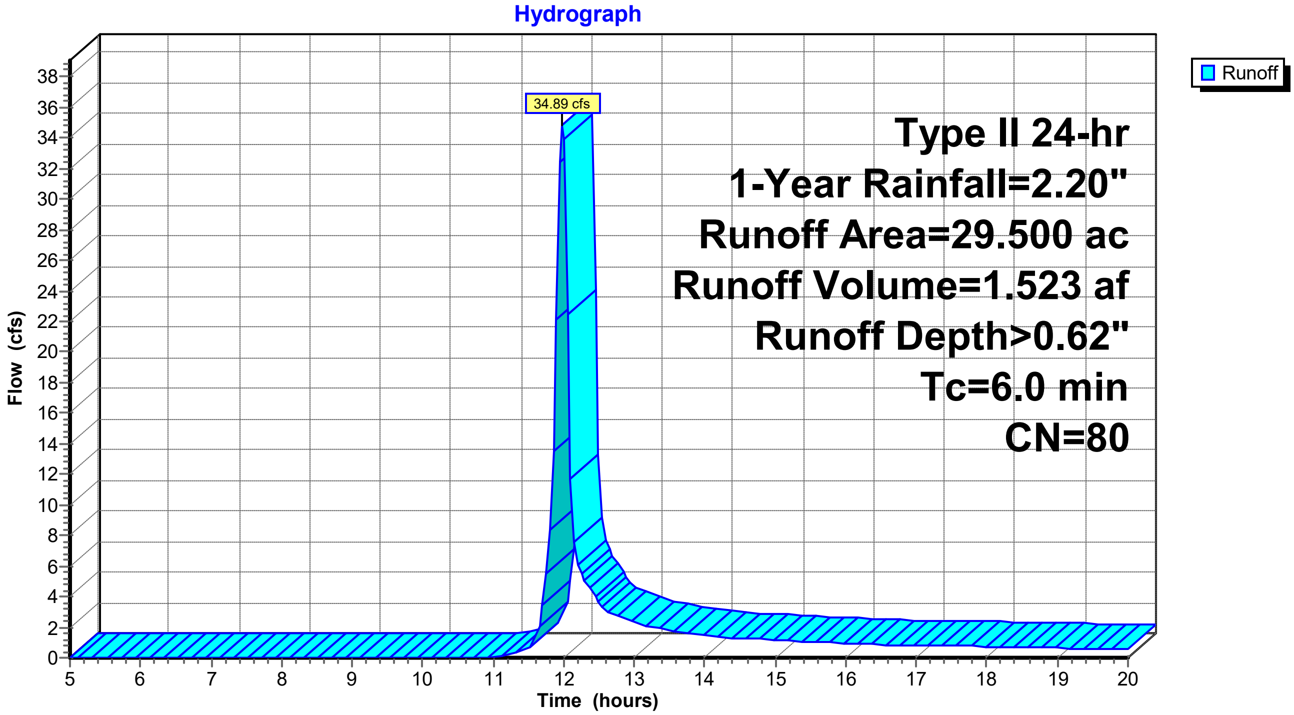
Runoff = 34.89 cfs @ 11.98 hrs, Volume= 1.523 af, Depth> 0.62"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

Area (ac)	CN	Description
3.970	77	Brush, Fair, HSG D
2.500	96	Gravel surface, HSG D
23.030	79	Woods, Fair, HSG D
29.500	80	Weighted Average
29.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-C: Depression



Summary for Subcatchment DR-D: Overland

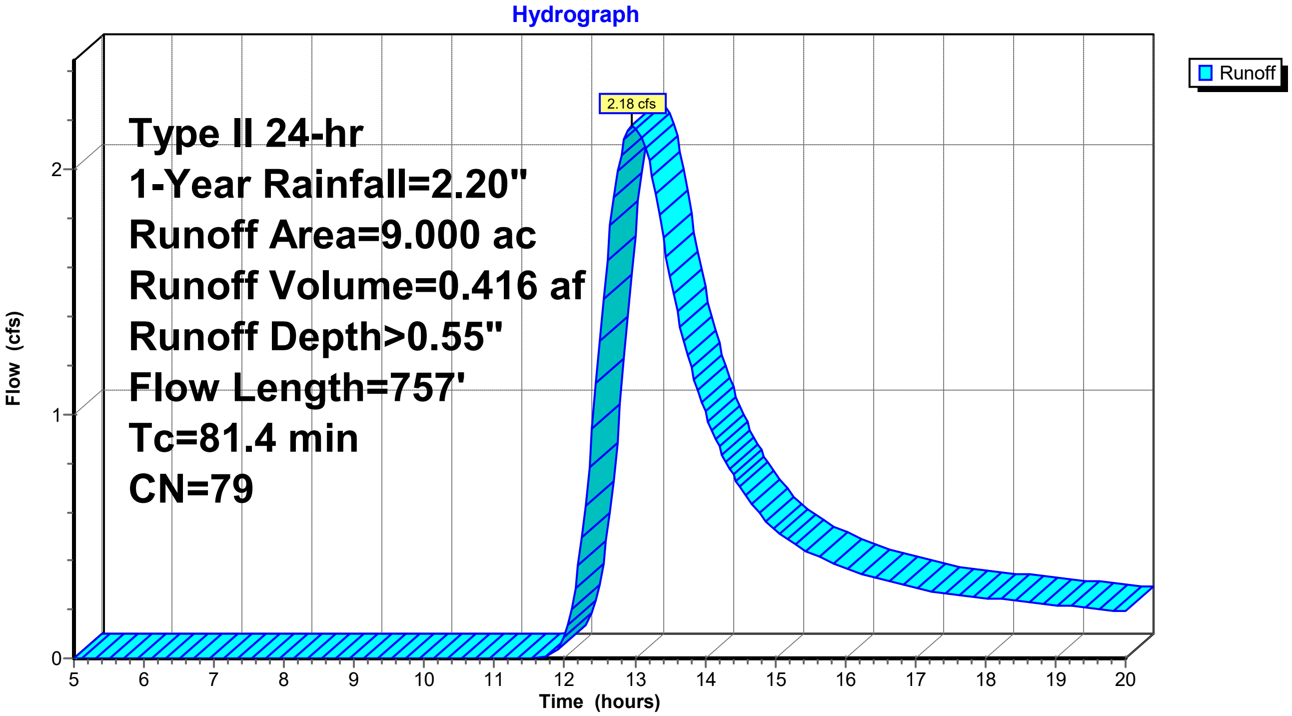
Runoff = 2.18 cfs @ 12.96 hrs, Volume= 0.416 af, Depth> 0.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

Area (ac)	CN	Description
9.000	79	Woods, Fair, HSG D
9.000		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
56.6	150	0.0050	0.04		**Sheet Flow, Sheet Flow** Woods: Light underbrush n= 0.400 P2= 2.67"
23.9	507	0.0050	0.35		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
0.9	100	0.1300	1.80		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
81.4	757	Total			

Subcatchment DR-D: Overland



Summary for Subcatchment DR-E: Overland

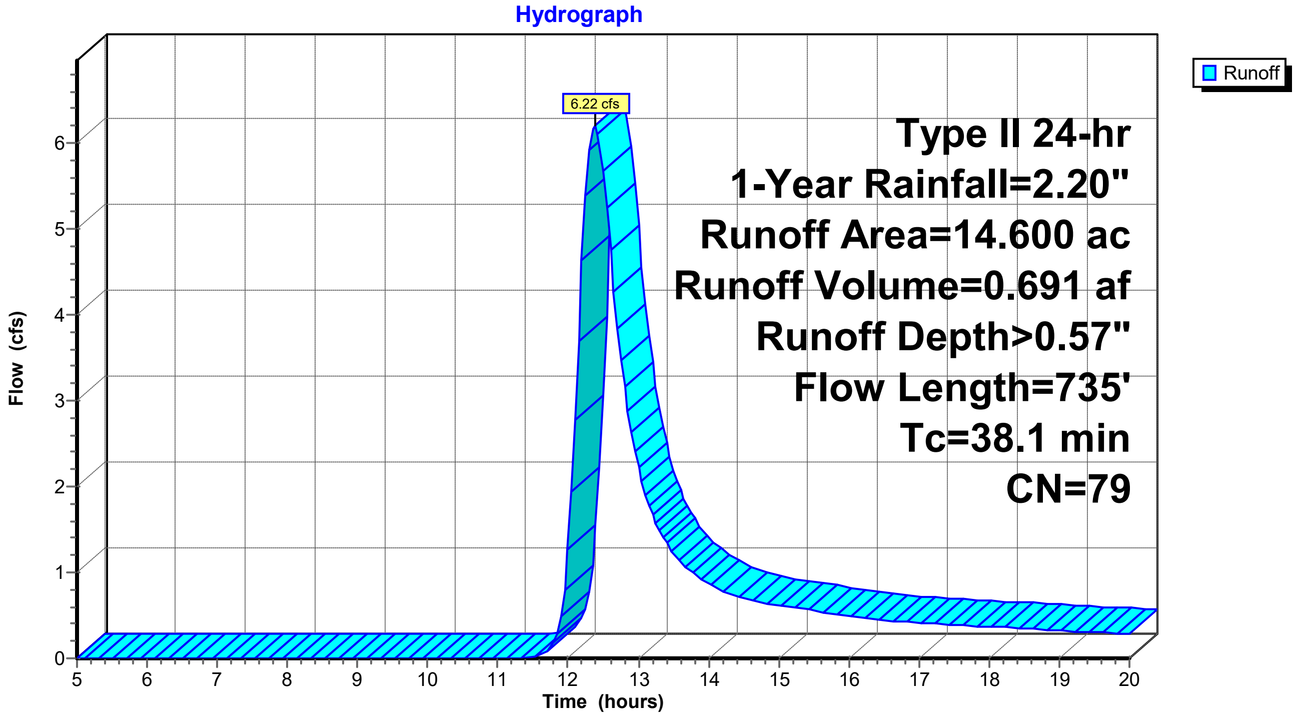
Runoff = 6.22 cfs @ 12.38 hrs, Volume= 0.691 af, Depth> 0.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

Area (ac)	CN	Description
14.600	79	Woods, Fair, HSG D
14.600		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6	150	0.0300	0.09		**Sheet Flow, Sheet Flow** Woods: Light underbrush n= 0.400 P2= 2.67"
9.8	510	0.0300	0.87		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
0.7	75	0.1200	1.73		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
38.1	735	Total			

Subcatchment DR-E: Overland



Summary for Subcatchment DR-F: Overland

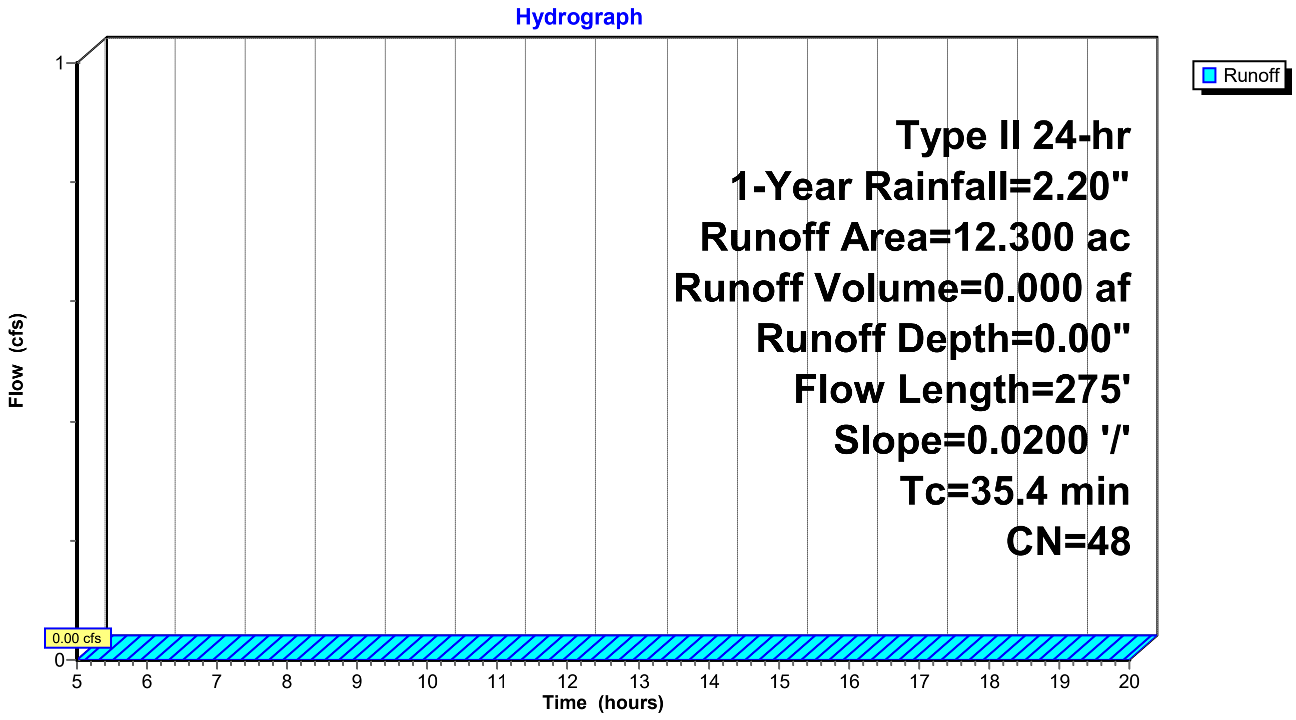
Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	1.100	98	Pavement
	11.200	43	Woods/grass comb., Fair, HSG A
	12.300	48	Weighted Average
	11.200		91.06% Pervious Area
	1.100		8.94% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.5	150	0.0200	0.08		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.67"
2.9	125	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
35.4	275	Total			

Subcatchment DR-F: Overland



Summary for Subcatchment DR-G: Roadway

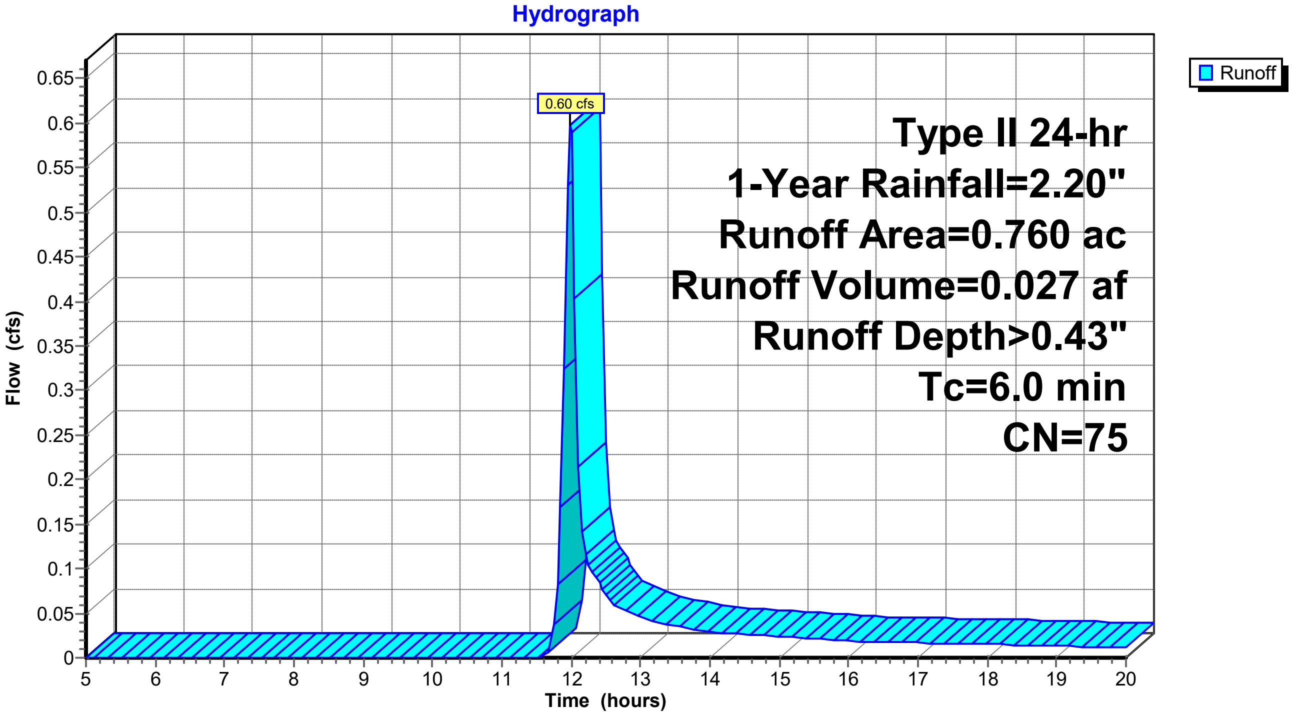
Runoff = 0.60 cfs @ 11.99 hrs, Volume= 0.027 af, Depth> 0.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	0.460	98	Roadway
	0.300	39	>75% Grass cover, Good, HSG A
	0.760	75	Weighted Average
	0.300		39.47% Pervious Area
	0.460		60.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-G: Roadway



Summary for Reach 1W: Wetland #1 / Analysis Point 1A

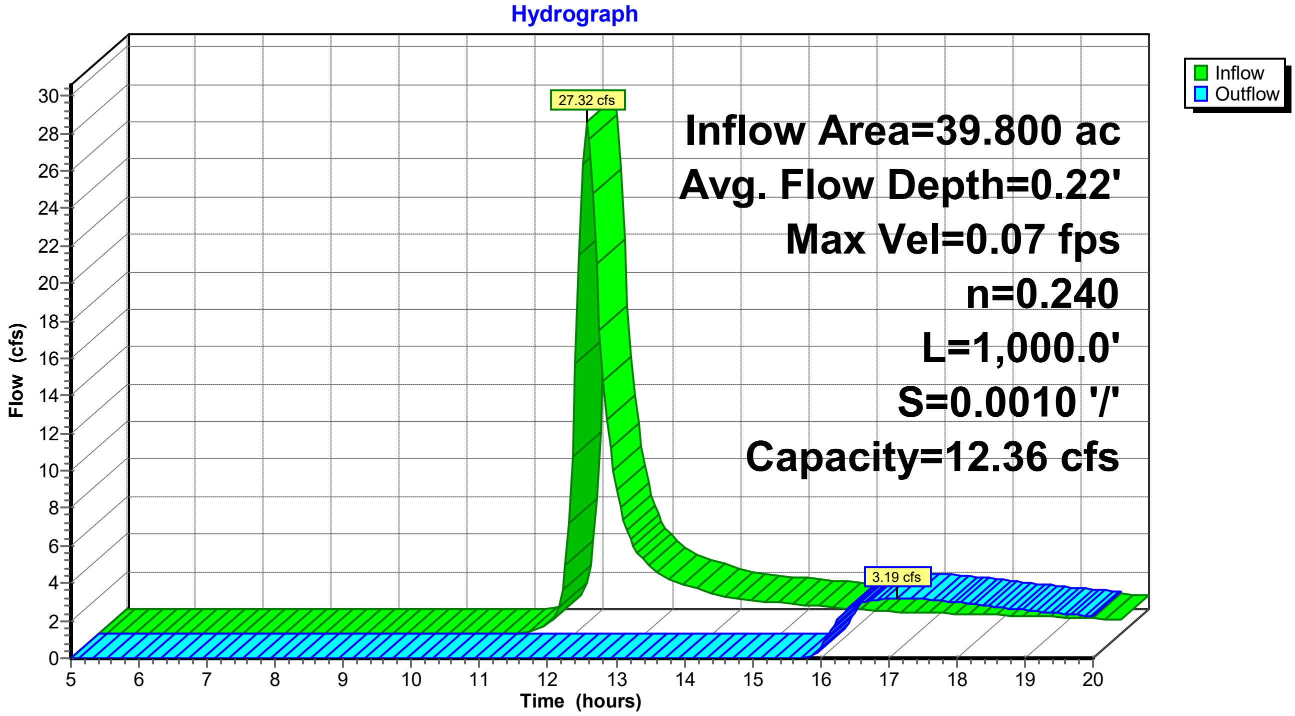
Inflow Area = 39.800 ac, 2.76% Impervious, Inflow Depth > 0.62" for 1-Year event
Inflow = 27.32 cfs @ 12.16 hrs, Volume= 2.040 af
Outflow = 3.19 cfs @ 17.12 hrs, Volume= 0.898 af, Atten= 88%, Lag= 297.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.07 fps, Min. Travel Time= 232.8 min
Avg. Velocity = 0.05 fps, Avg. Travel Time= 304.0 min

Peak Storage= 44,519 cf @ 13.24 hrs
Average Depth at Peak Storage= 0.22' , Surface Width= 201.33'
Bank-Full Depth= 0.50' Flow Area= 100.8 sf, Capacity= 12.36 cfs

200.00' x 0.50' deep channel, n= 0.240 Sheet flow over Dense Grass
Side Slope Z-value= 3.0 '/' Top Width= 203.00'
Length= 1,000.0' Slope= 0.0010 '/'
Inlet Invert= 6.00', Outlet Invert= 5.00'

Reach 1W: Wetland #1 / Analysis Point 1A



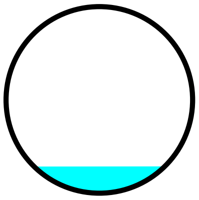
Summary for Reach 2R: Outlet Pipe

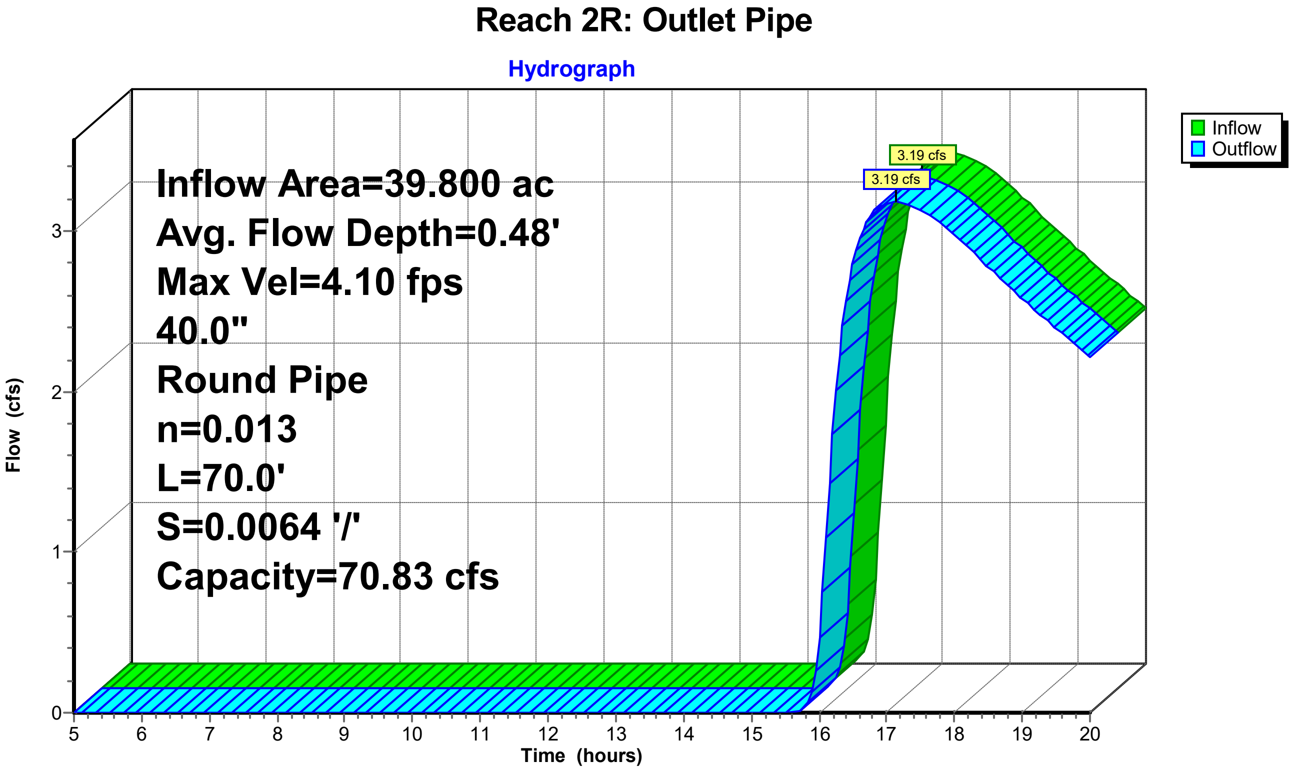
Inflow Area = 39.800 ac, 2.76% Impervious, Inflow Depth > 0.27" for 1-Year event
Inflow = 3.19 cfs @ 17.12 hrs, Volume= 0.898 af
Outflow = 3.19 cfs @ 17.13 hrs, Volume= 0.896 af, Atten= 0%, Lag= 0.5 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.10 fps, Min. Travel Time= 0.3 min
Avg. Velocity = 3.35 fps, Avg. Travel Time= 0.3 min

Peak Storage= 54 cf @ 17.12 hrs
Average Depth at Peak Storage= 0.48' , Surface Width= 2.34'
Bank-Full Depth= 3.33' Flow Area= 8.7 sf, Capacity= 70.83 cfs

40.0" Round Pipe
n= 0.013 Corrugated PE, smooth interior
Length= 70.0' Slope= 0.0064 '/'
Inlet Invert= 4.25', Outlet Invert= 3.80'





Summary for Pond 1P: Infiltration

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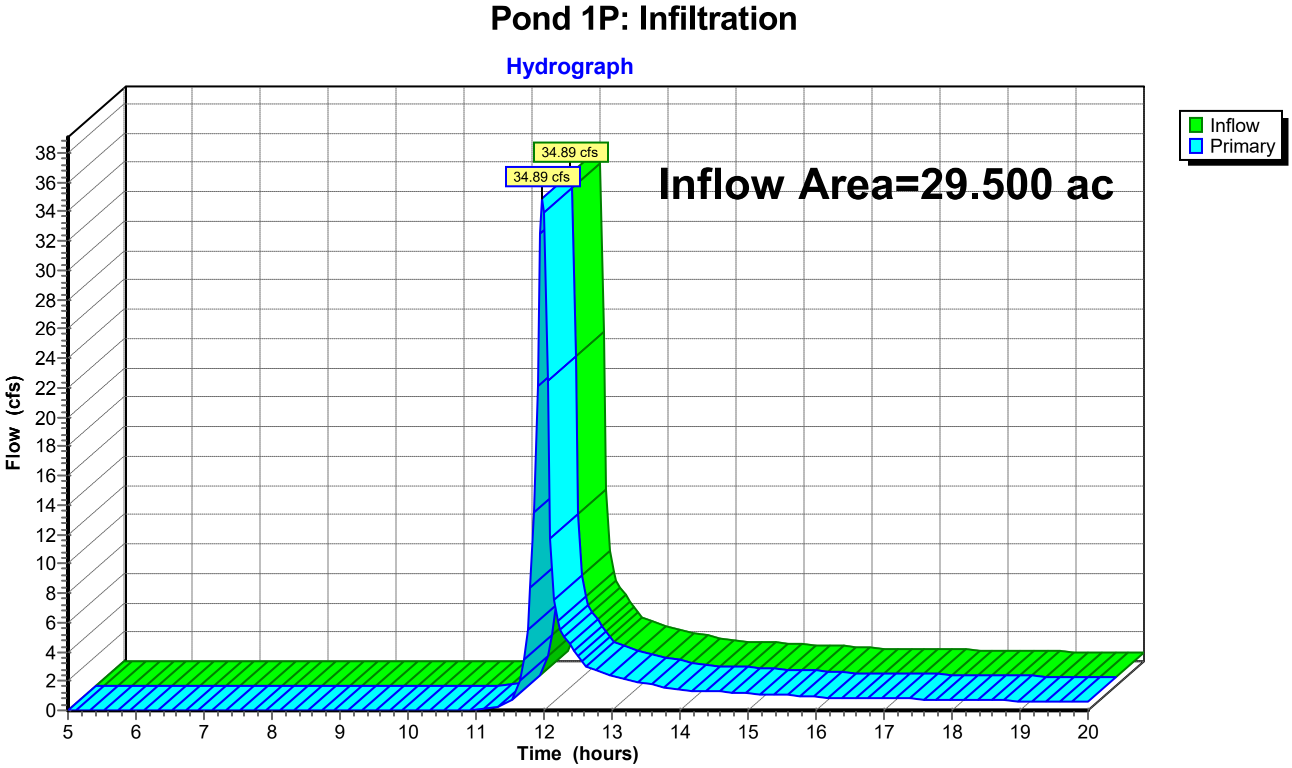
Inflow Area =       29.500 ac,   0.00% Impervious,  Inflow Depth >   0.62"    for  1-Year event
Inflow      =      34.89 cfs @   11.98 hrs,  Volume=          1.523 af
Primary     =      34.89 cfs @   11.98 hrs,  Volume=          1.523 af,  Atten= 0%,  Lag= 0.0 min
```

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

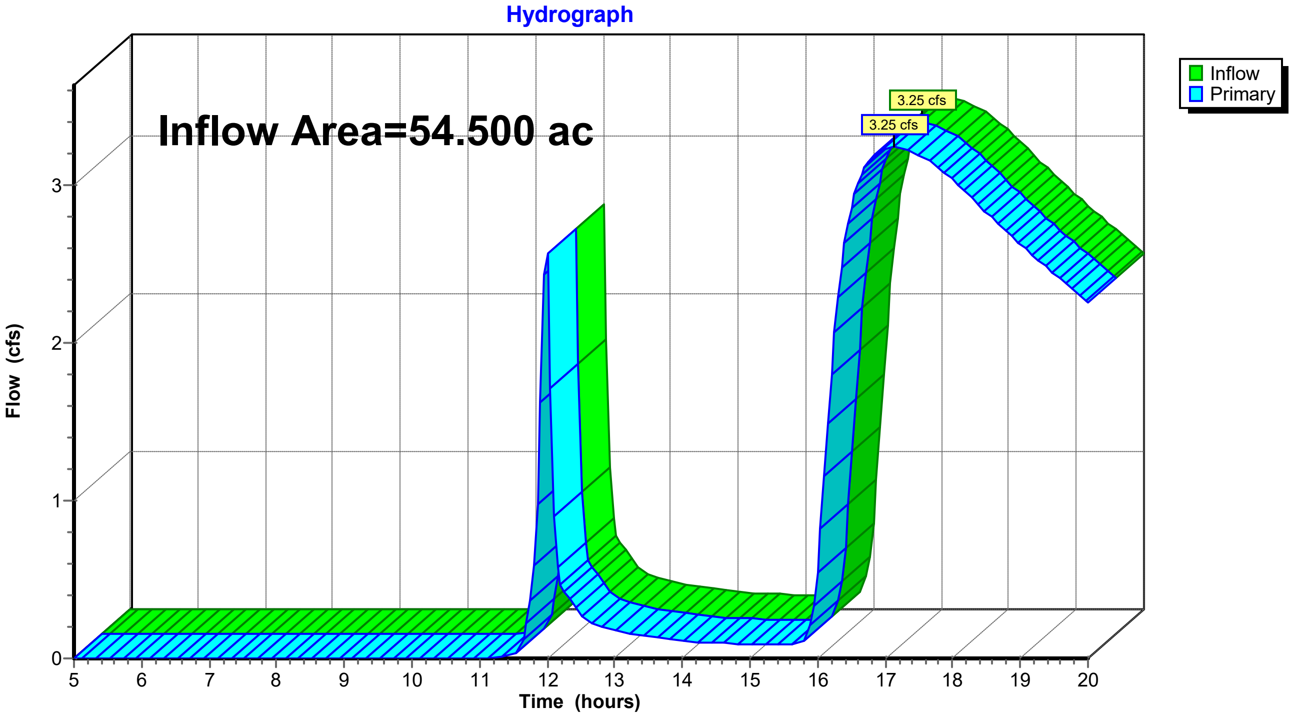


Summary for Pond AP-1: Analysis Point #1

Inflow Area = 54.500 ac, 4.04% Impervious, Inflow Depth > 0.22" for 1-Year event
Inflow = 3.25 cfs @ 17.12 hrs, Volume= 1.012 af
Primary = 3.25 cfs @ 17.12 hrs, Volume= 1.012 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Pond AP-1: Analysis Point #1

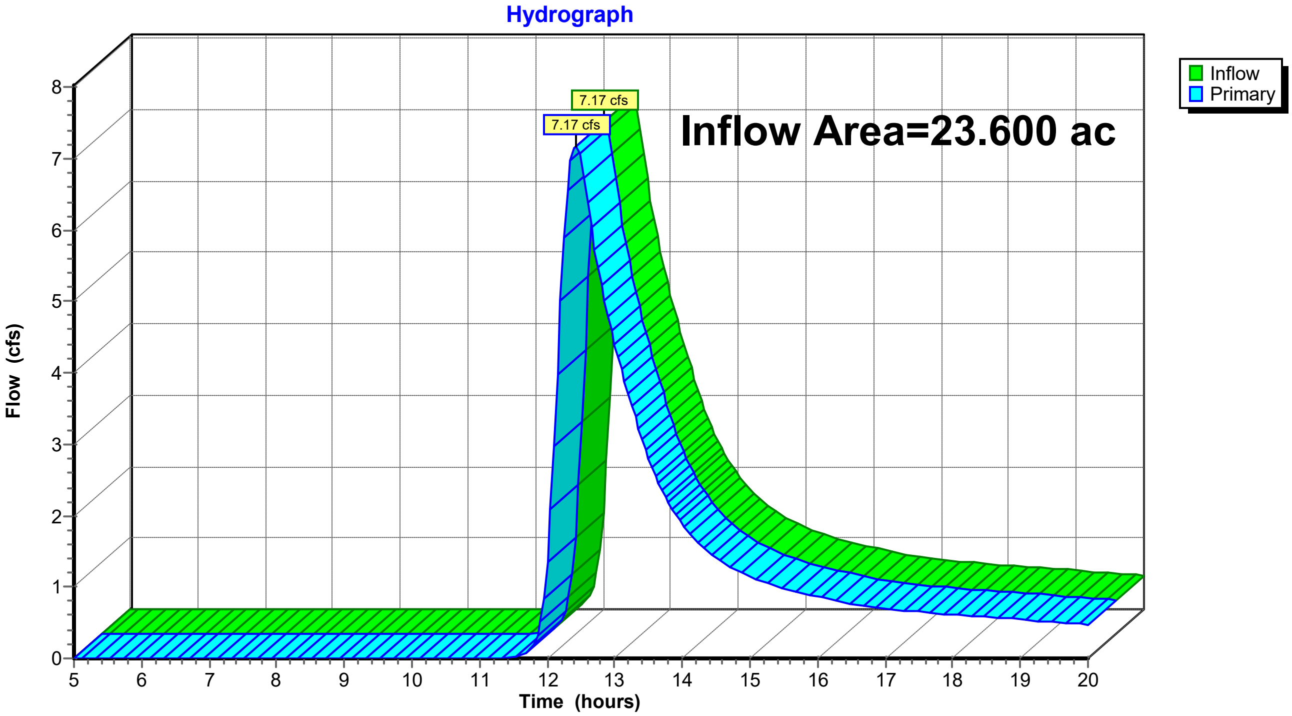


Summary for Pond AP-2: Analysis Point #2

Inflow Area =	23.600 ac,	0.00% Impervious,	Inflow Depth > 0.56"	for 1-Year event
Inflow =	7.17 cfs @	12.42 hrs,	Volume=	1.107 af
Primary =	7.17 cfs @	12.42 hrs,	Volume=	1.107 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Pond AP-2: Analysis Point #2

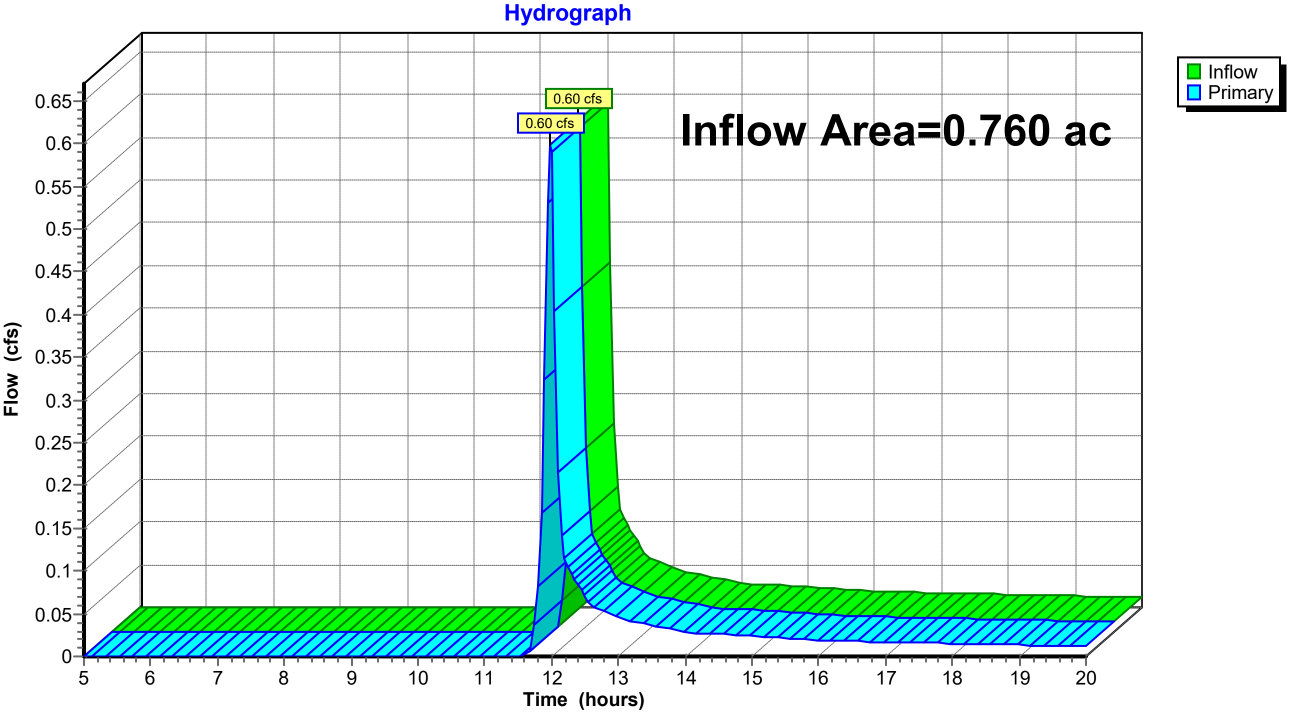


Summary for Pond AP-3: Analysis Point #3

Inflow Area = 0.760 ac, 60.53% Impervious, Inflow Depth > 0.43" for 1-Year event
Inflow = 0.60 cfs @ 11.99 hrs, Volume= 0.027 af
Primary = 0.60 cfs @ 11.99 hrs, Volume= 0.027 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Pond AP-3: Analysis Point #3



Summary for Subcatchment DR-A: Overland

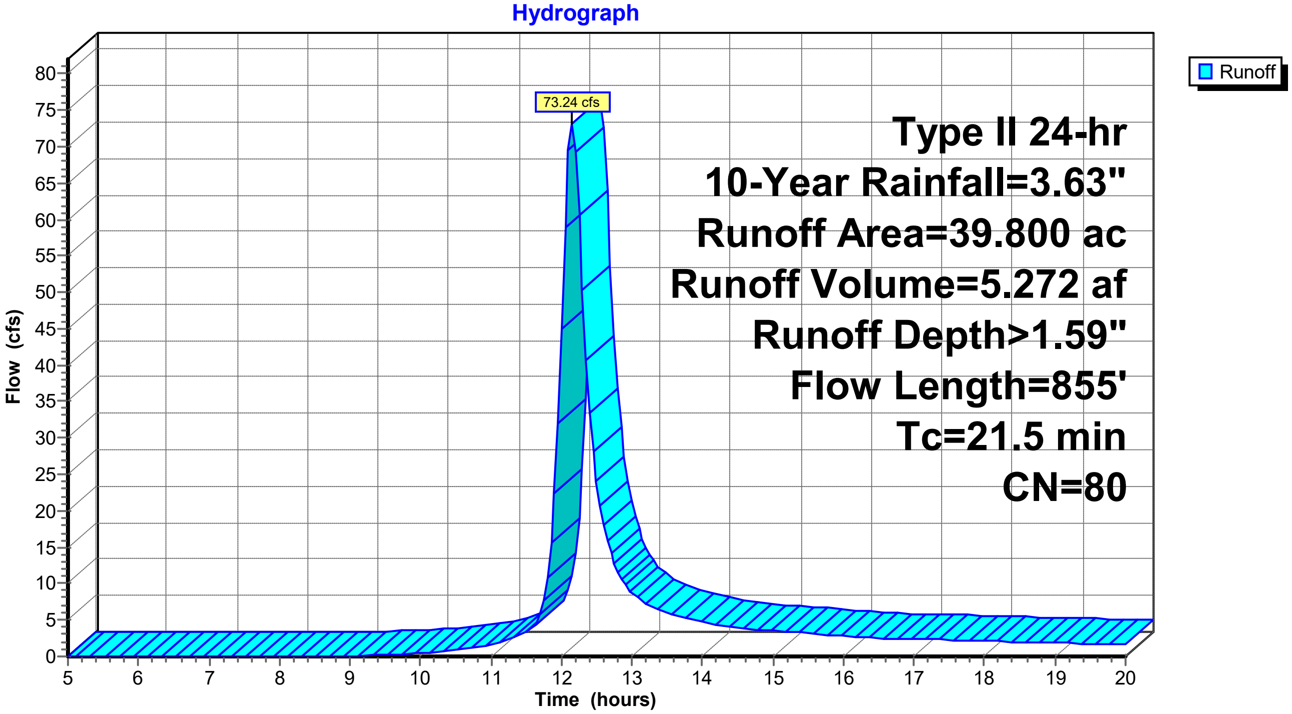
Runoff = 73.24 cfs @ 12.15 hrs, Volume= 5.272 af, Depth> 1.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
	38.700	79	Woods, Fair, HSG D
*	1.100	98	Existing Railroad
	39.800	80	Weighted Average
	38.700		97.24% Pervious Area
	1.100		2.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.5	150	0.1500	0.17		**Sheet Flow, Sheet Flow** Woods: Light underbrush n= 0.400 P2= 2.67"
4.8	575	0.1600	2.00		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
2.2	130	0.0400	1.00		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
21.5	855	Total			

Subcatchment DR-A: Overland



Summary for Subcatchment DR-B: Overland

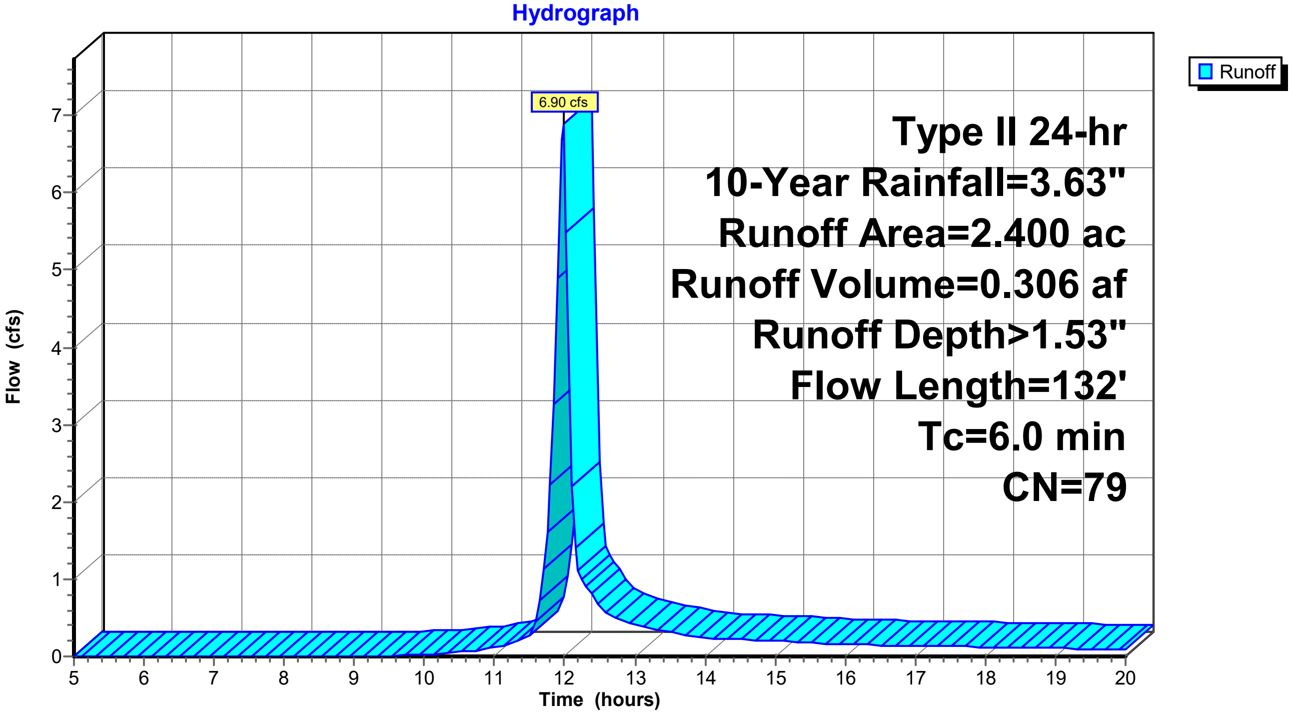
Runoff = 6.90 cfs @ 11.97 hrs, Volume= 0.306 af, Depth> 1.53"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

Area (ac)	CN	Description
2.400	79	Woods, Fair, HSG D
2.400		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	132		0.37		**Direct Entry, Sheet Flow**

Subcatchment DR-B: Overland



Summary for Subcatchment DR-C: Depression

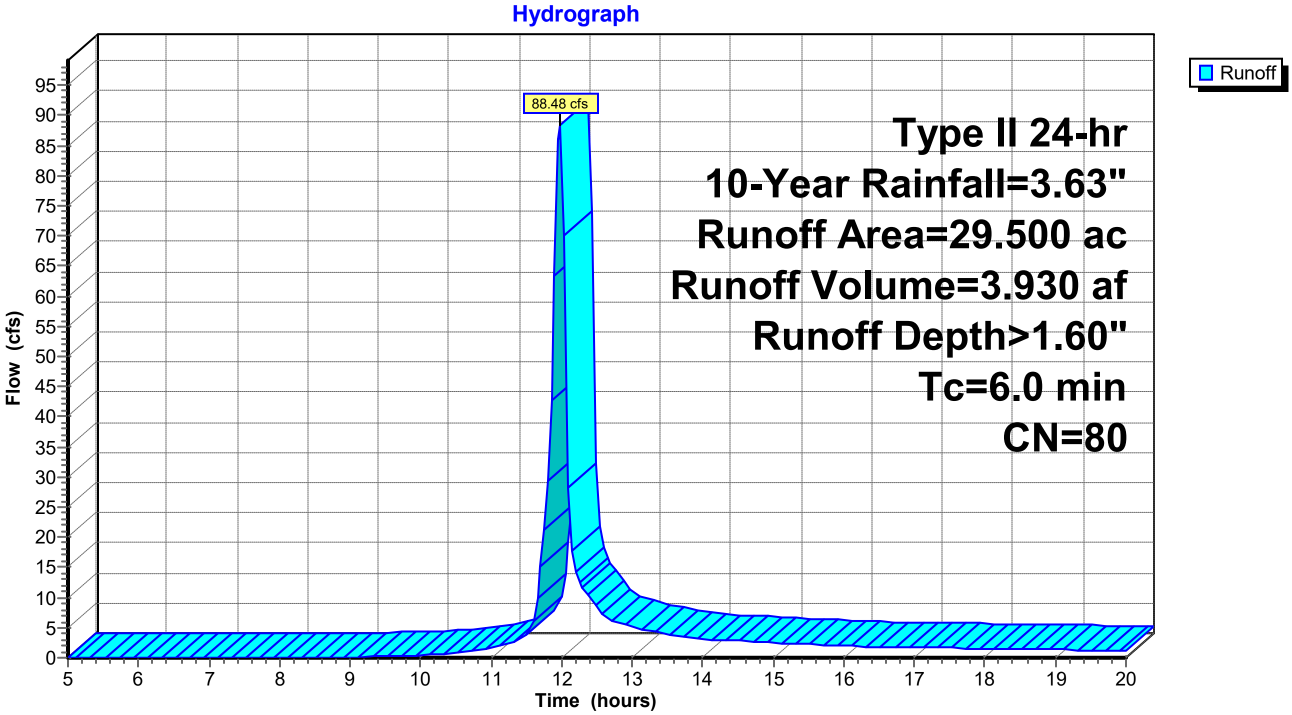
Runoff = 88.48 cfs @ 11.97 hrs, Volume= 3.930 af, Depth> 1.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

Area (ac)	CN	Description
3.970	77	Brush, Fair, HSG D
2.500	96	Gravel surface, HSG D
23.030	79	Woods, Fair, HSG D
29.500	80	Weighted Average
29.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-C: Depression



Summary for Subcatchment DR-D: Overland

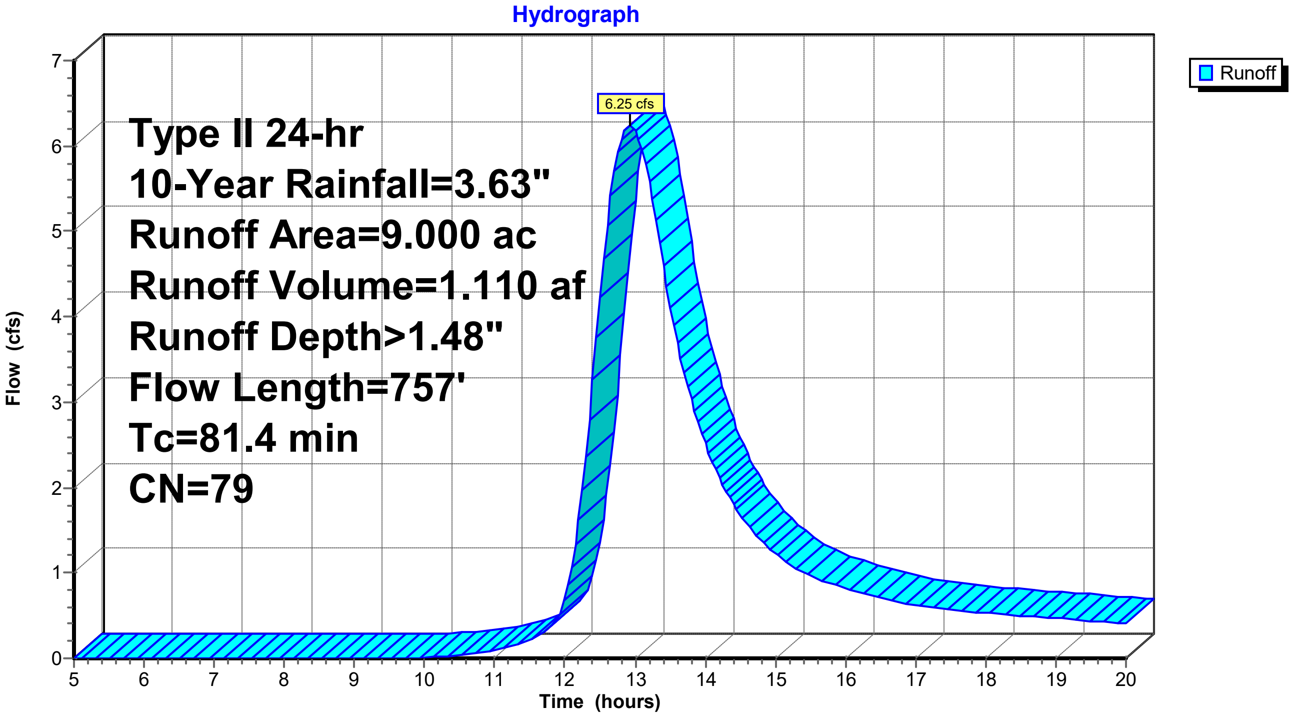
Runoff = 6.25 cfs @ 12.93 hrs, Volume= 1.110 af, Depth> 1.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

Area (ac)	CN	Description
9.000	79	Woods, Fair, HSG D
9.000		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
56.6	150	0.0050	0.04		**Sheet Flow, Sheet Flow** Woods: Light underbrush n= 0.400 P2= 2.67"
23.9	507	0.0050	0.35		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
0.9	100	0.1300	1.80		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
81.4	757	Total			

Subcatchment DR-D: Overland



Summary for Subcatchment DR-E: Overland

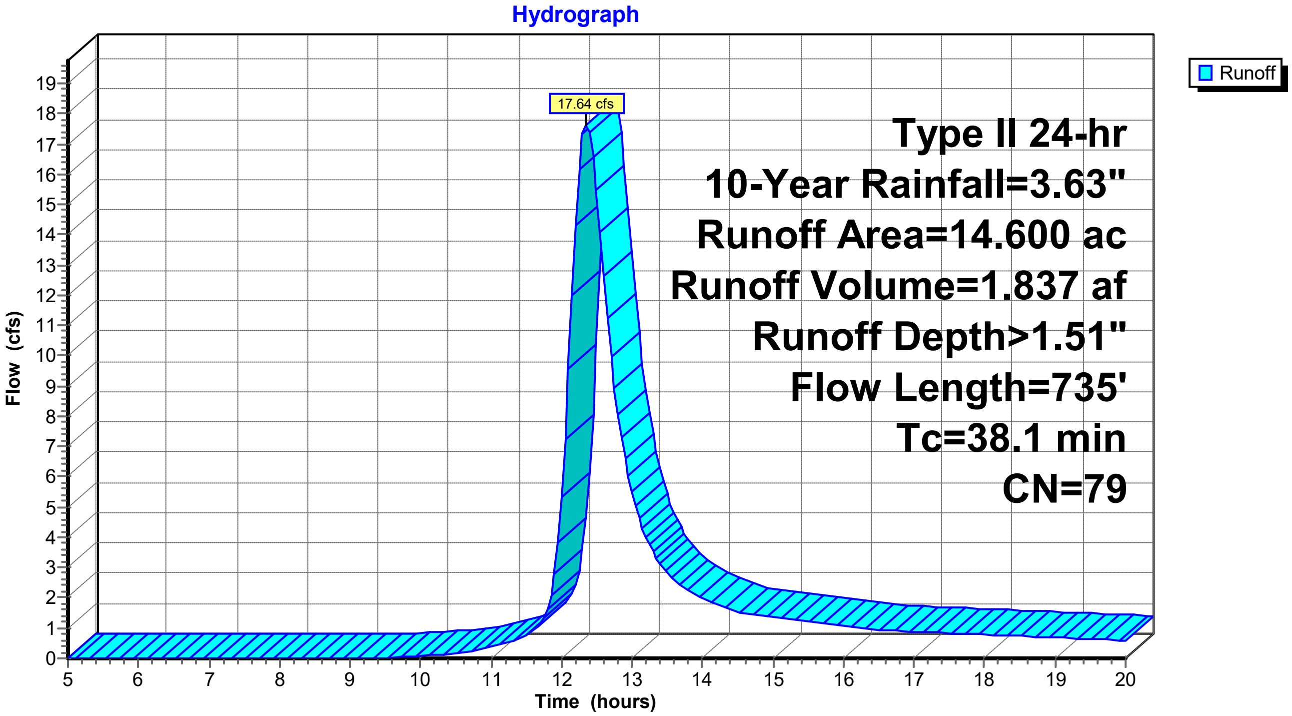
Runoff = 17.64 cfs @ 12.35 hrs, Volume= 1.837 af, Depth> 1.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

Area (ac)	CN	Description
14.600	79	Woods, Fair, HSG D
14.600		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6	150	0.0300	0.09		**Sheet Flow, Sheet Flow** Woods: Light underbrush n= 0.400 P2= 2.67"
9.8	510	0.0300	0.87		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
0.7	75	0.1200	1.73		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
38.1	735	Total			

Subcatchment DR-E: Overland



Summary for Subcatchment DR-F: Overland

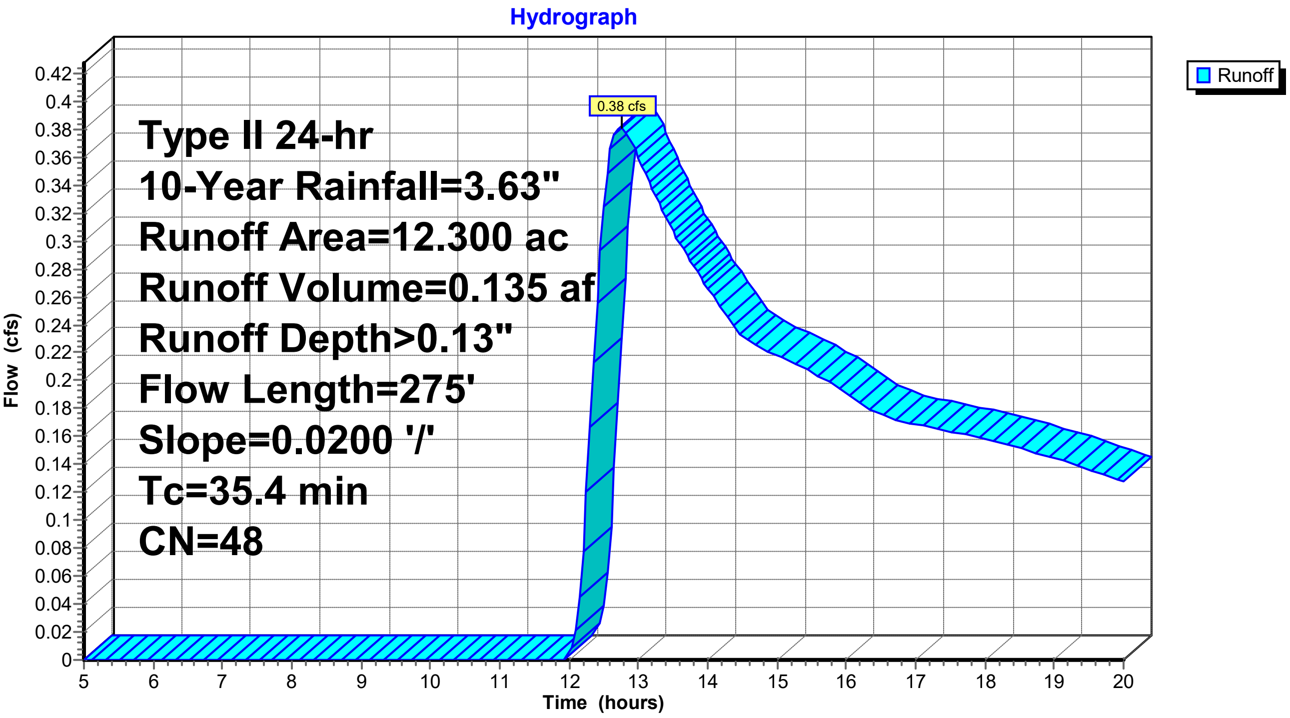
Runoff = 0.38 cfs @ 12.75 hrs, Volume= 0.135 af, Depth> 0.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	1.100	98	Pavement
	11.200	43	Woods/grass comb., Fair, HSG A
	12.300	48	Weighted Average
	11.200		91.06% Pervious Area
	1.100		8.94% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.5	150	0.0200	0.08		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.67"
2.9	125	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
35.4	275	Total			

Subcatchment DR-F: Overland



Summary for Subcatchment DR-G: Roadway

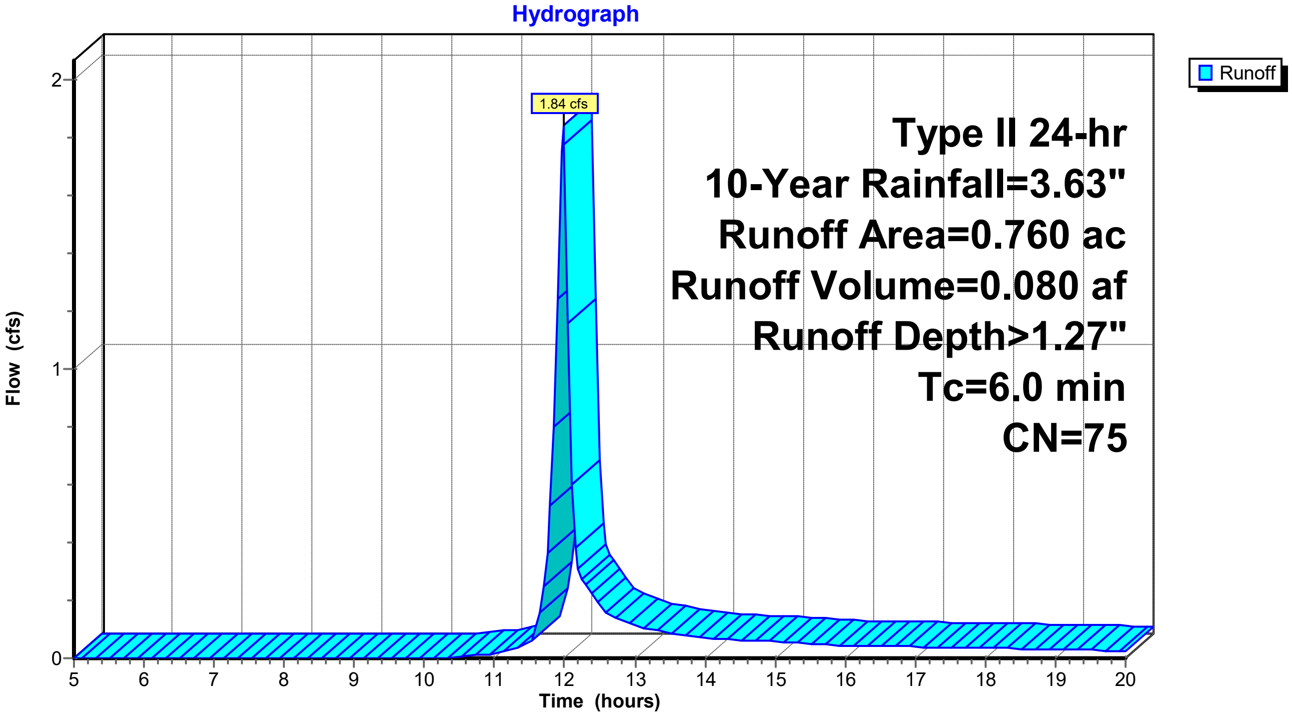
Runoff = 1.84 cfs @ 11.98 hrs, Volume= 0.080 af, Depth> 1.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	0.460	98	Roadway
	0.300	39	>75% Grass cover, Good, HSG A
	0.760	75	Weighted Average
	0.300		39.47% Pervious Area
	0.460		60.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-G: Roadway



Summary for Reach 1W: Wetland #1 / Analysis Point 1A

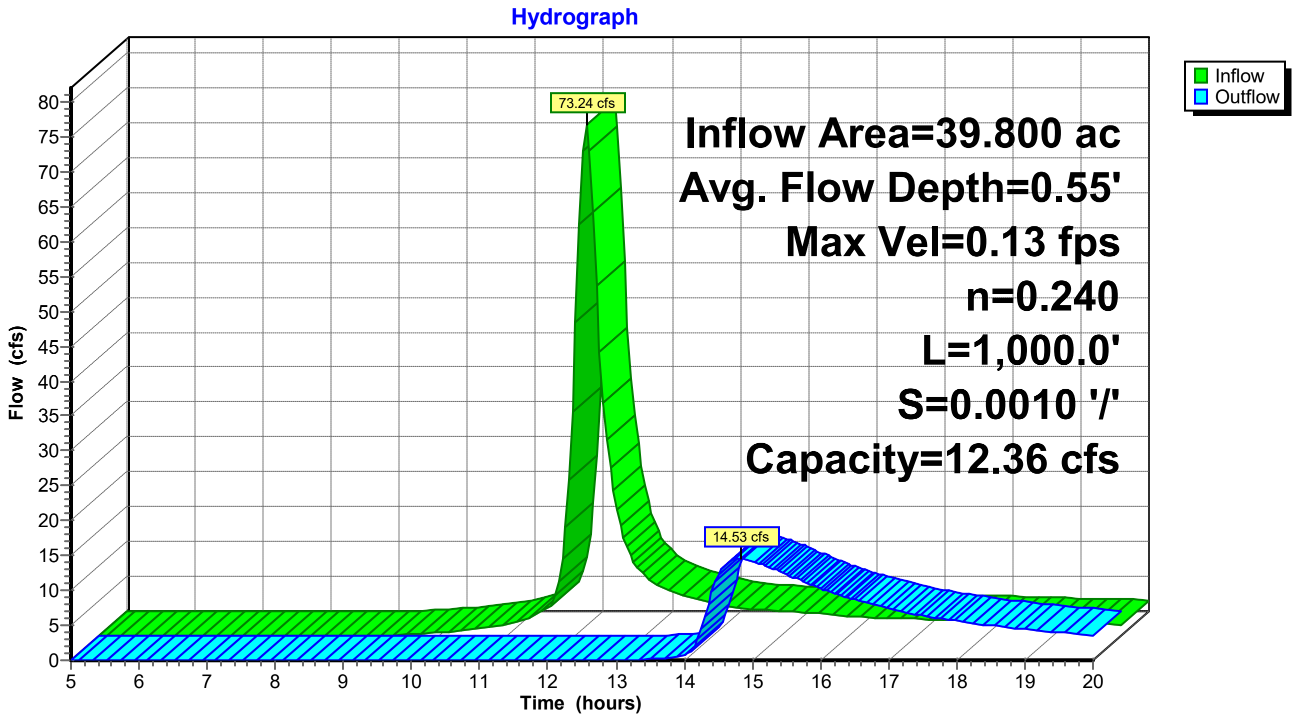
Inflow Area = 39.800 ac, 2.76% Impervious, Inflow Depth > 1.59" for 10-Year event
Inflow = 73.24 cfs @ 12.15 hrs, Volume= 5.272 af
Outflow = 14.53 cfs @ 14.82 hrs, Volume= 3.841 af, Atten= 80%, Lag= 160.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.13 fps, Min. Travel Time= 127.8 min
Avg. Velocity = 0.07 fps, Avg. Travel Time= 239.5 min

Peak Storage= 111,494 cf @ 12.69 hrs
Average Depth at Peak Storage= 0.55' , Surface Width= 203.32'
Bank-Full Depth= 0.50' Flow Area= 100.8 sf, Capacity= 12.36 cfs

200.00' x 0.50' deep channel, n= 0.240 Sheet flow over Dense Grass
Side Slope Z-value= 3.0 '/' Top Width= 203.00'
Length= 1,000.0' Slope= 0.0010 '/'
Inlet Invert= 6.00', Outlet Invert= 5.00'

Reach 1W: Wetland #1 / Analysis Point 1A



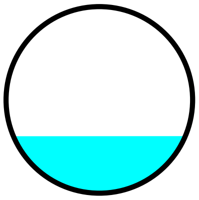
Summary for Reach 2R: Outlet Pipe

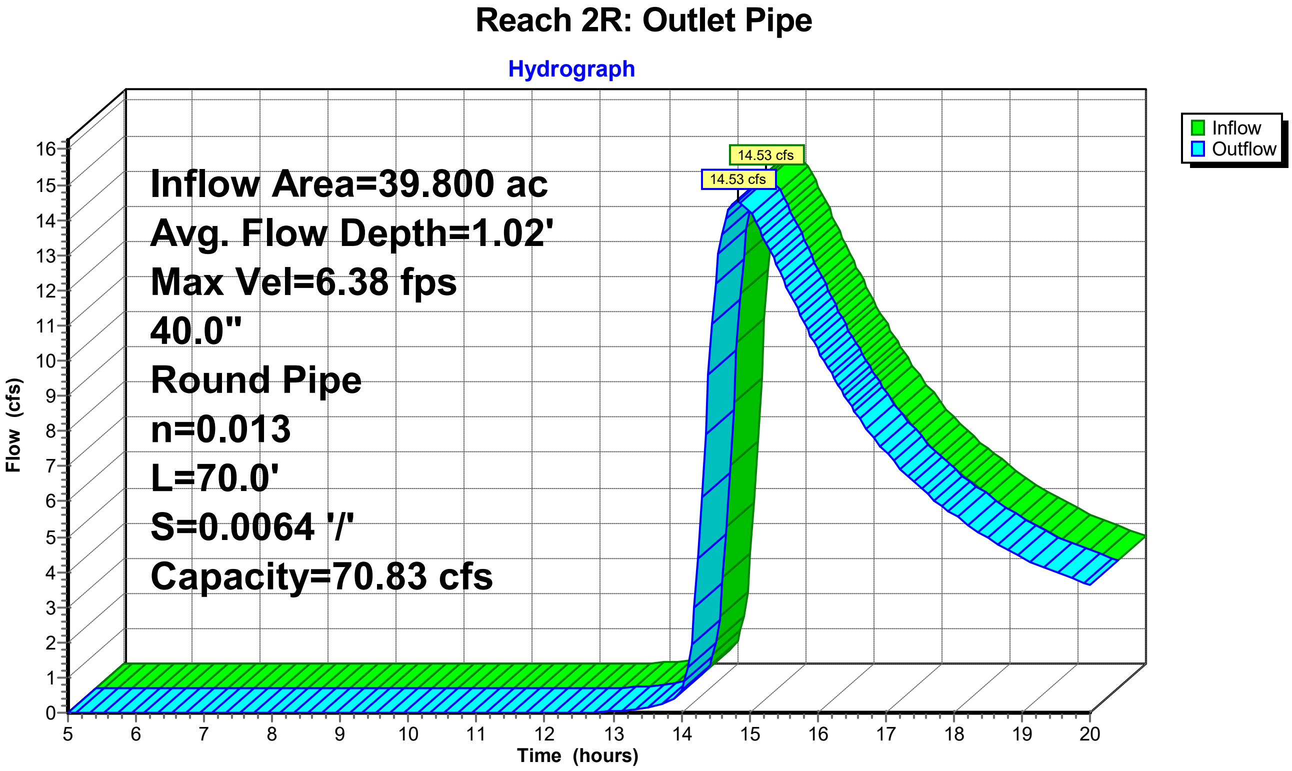
Inflow Area = 39.800 ac, 2.76% Impervious, Inflow Depth > 1.16" for 10-Year event
Inflow = 14.53 cfs @ 14.82 hrs, Volume= 3.841 af
Outflow = 14.53 cfs @ 14.83 hrs, Volume= 3.838 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.38 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 3.89 fps, Avg. Travel Time= 0.3 min

Peak Storage= 159 cf @ 14.82 hrs
Average Depth at Peak Storage= 1.02' , Surface Width= 3.08'
Bank-Full Depth= 3.33' Flow Area= 8.7 sf, Capacity= 70.83 cfs

40.0" Round Pipe
n= 0.013 Corrugated PE, smooth interior
Length= 70.0' Slope= 0.0064 '/'
Inlet Invert= 4.25', Outlet Invert= 3.80'



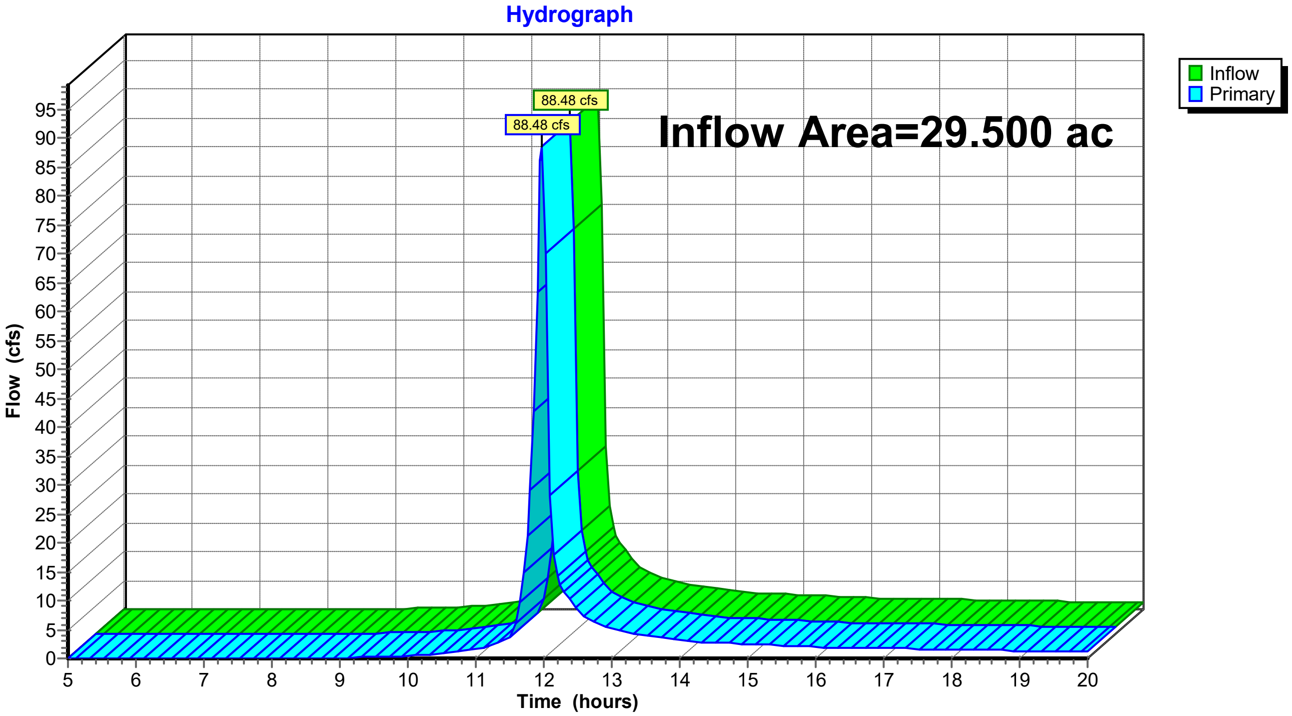


Summary for Pond 1P: Infiltration

Inflow Area =	29.500 ac,	0.00% Impervious,	Inflow Depth > 1.60"	for 10-Year event	
Inflow =	88.48 cfs @	11.97 hrs,	Volume=	3.930 af	
Primary =	88.48 cfs @	11.97 hrs,	Volume=	3.930 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Pond 1P: Infiltration

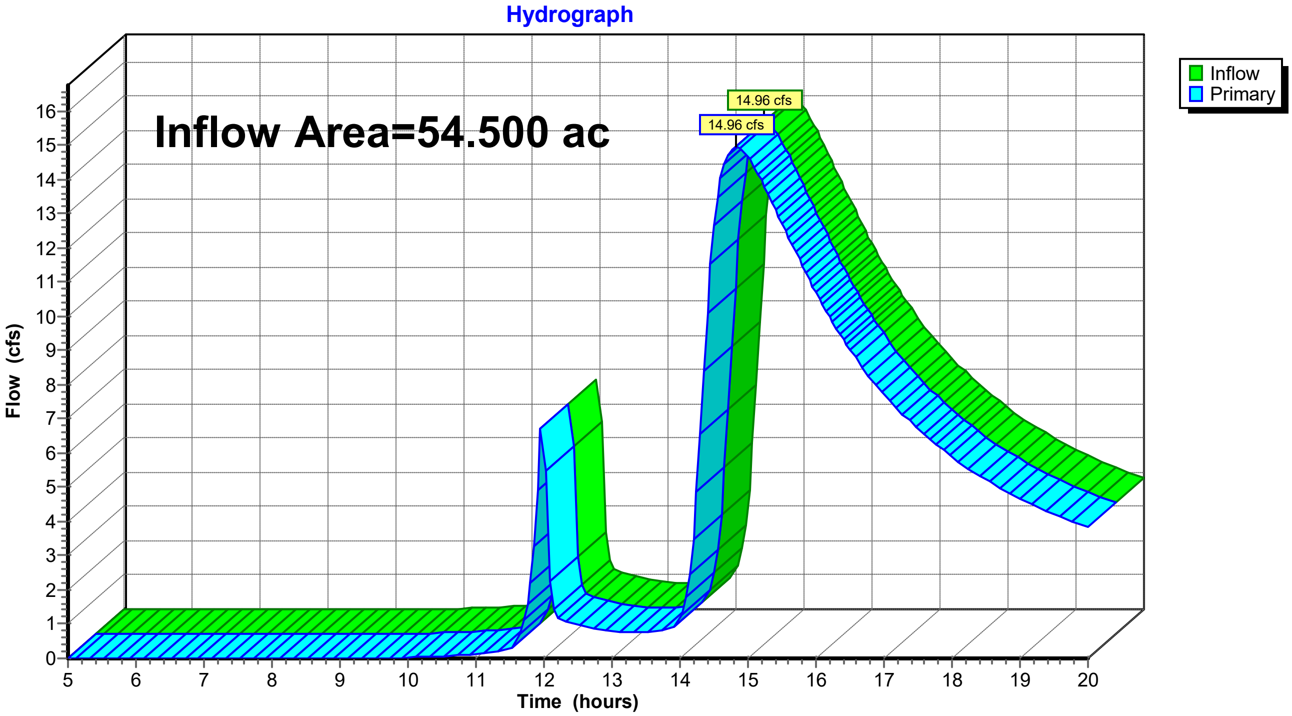


Summary for Pond AP-1: Analysis Point #1

Inflow Area = 54.500 ac, 4.04% Impervious, Inflow Depth > 0.94" for 10-Year event
Inflow = 14.96 cfs @ 14.82 hrs, Volume= 4.279 af
Primary = 14.96 cfs @ 14.82 hrs, Volume= 4.279 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Pond AP-1: Analysis Point #1

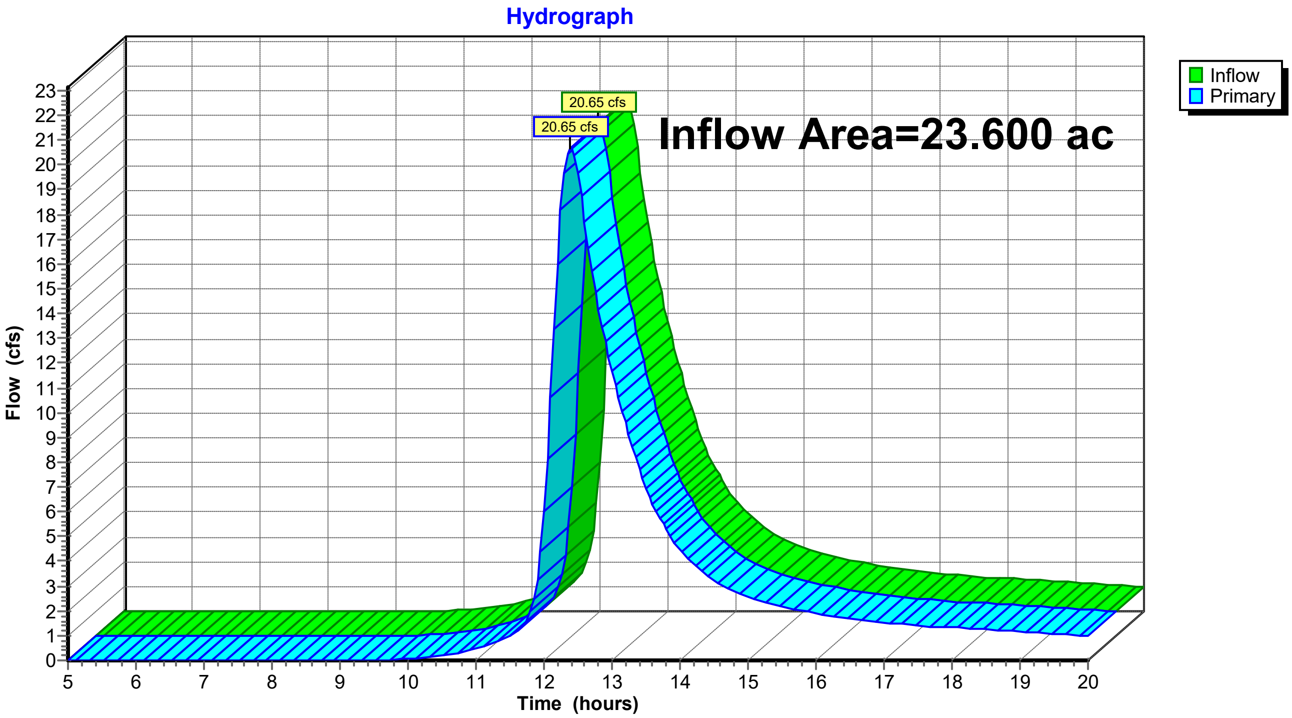


Summary for Pond AP-2: Analysis Point #2

```
Inflow Area =       23.600 ac,    0.00% Impervious,  Inflow Depth >   1.50"    for  10-Year event
Inflow      =       20.65 cfs @   12.40 hrs,  Volume=           2.947 af
Primary     =       20.65 cfs @   12.40 hrs,  Volume=           2.947 af,  Atten= 0%,  Lag= 0.0 min
```

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

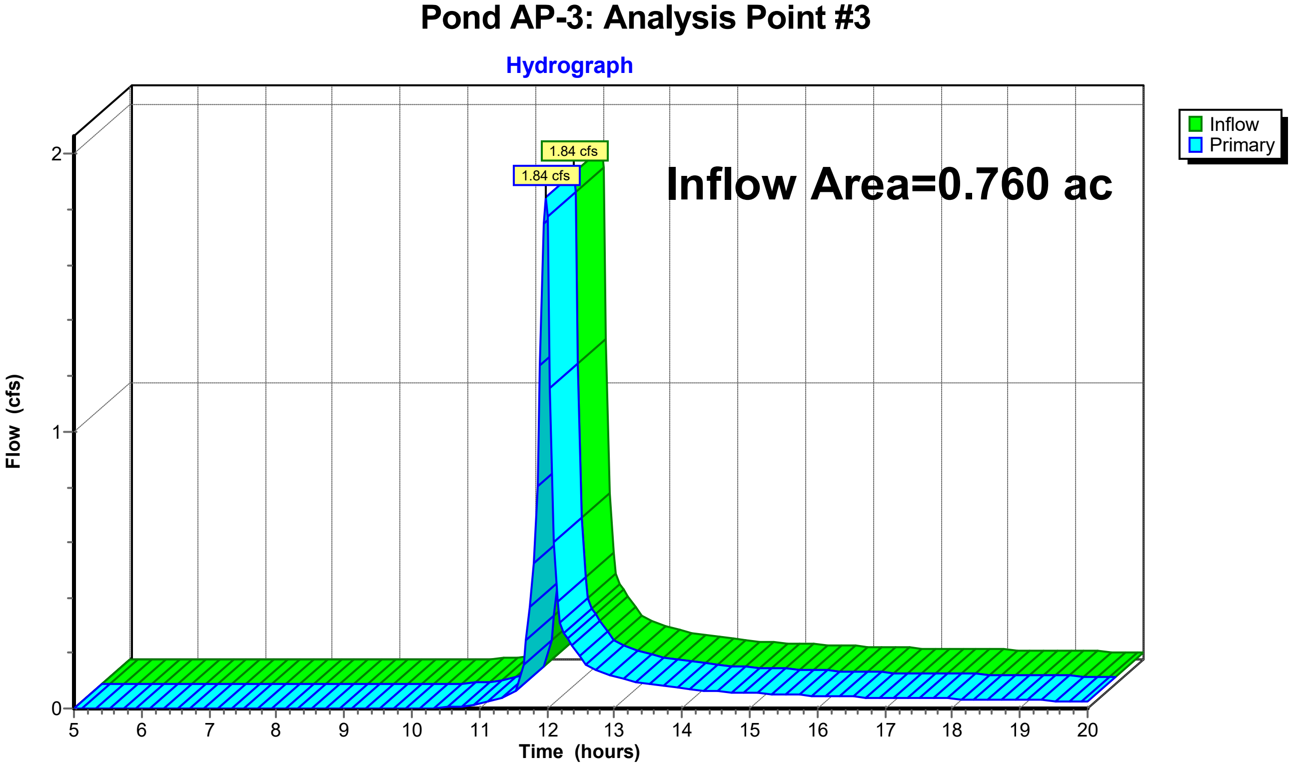
Pond AP-2: Analysis Point #2



Summary for Pond AP-3: Analysis Point #3

Inflow Area = 0.760 ac, 60.53% Impervious, Inflow Depth > 1.27" for 10-Year event
Inflow = 1.84 cfs @ 11.98 hrs, Volume= 0.080 af
Primary = 1.84 cfs @ 11.98 hrs, Volume= 0.080 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Summary for Subcatchment DR-A: Overland

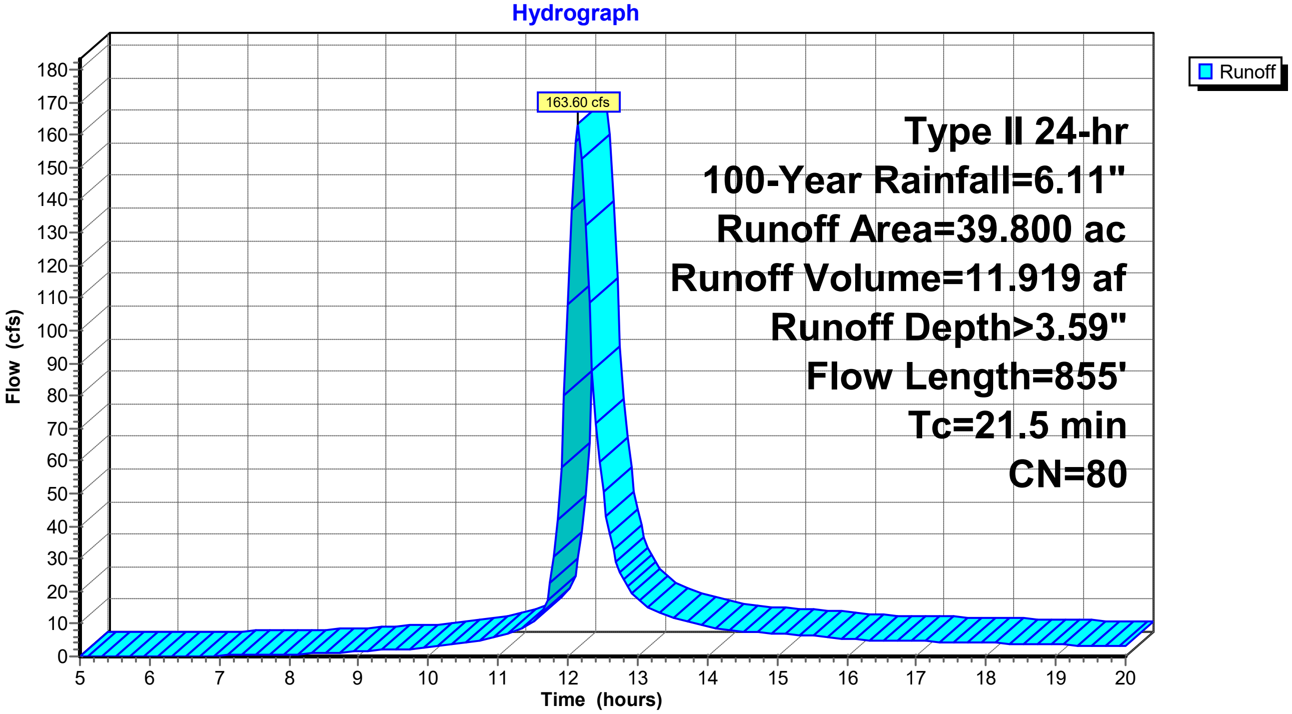
Runoff = 163.60 cfs @ 12.14 hrs, Volume= 11.919 af, Depth> 3.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
	38.700	79	Woods, Fair, HSG D
*	1.100	98	Existing Railroad
	39.800	80	Weighted Average
	38.700		97.24% Pervious Area
	1.100		2.76% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
14.5	150	0.1500	0.17		**Sheet Flow, Sheet Flow** Woods: Light underbrush n= 0.400 P2= 2.67"
4.8	575	0.1600	2.00		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
2.2	130	0.0400	1.00		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
21.5	855	Total			

Subcatchment DR-A: Overland



Summary for Subcatchment DR-B: Overland

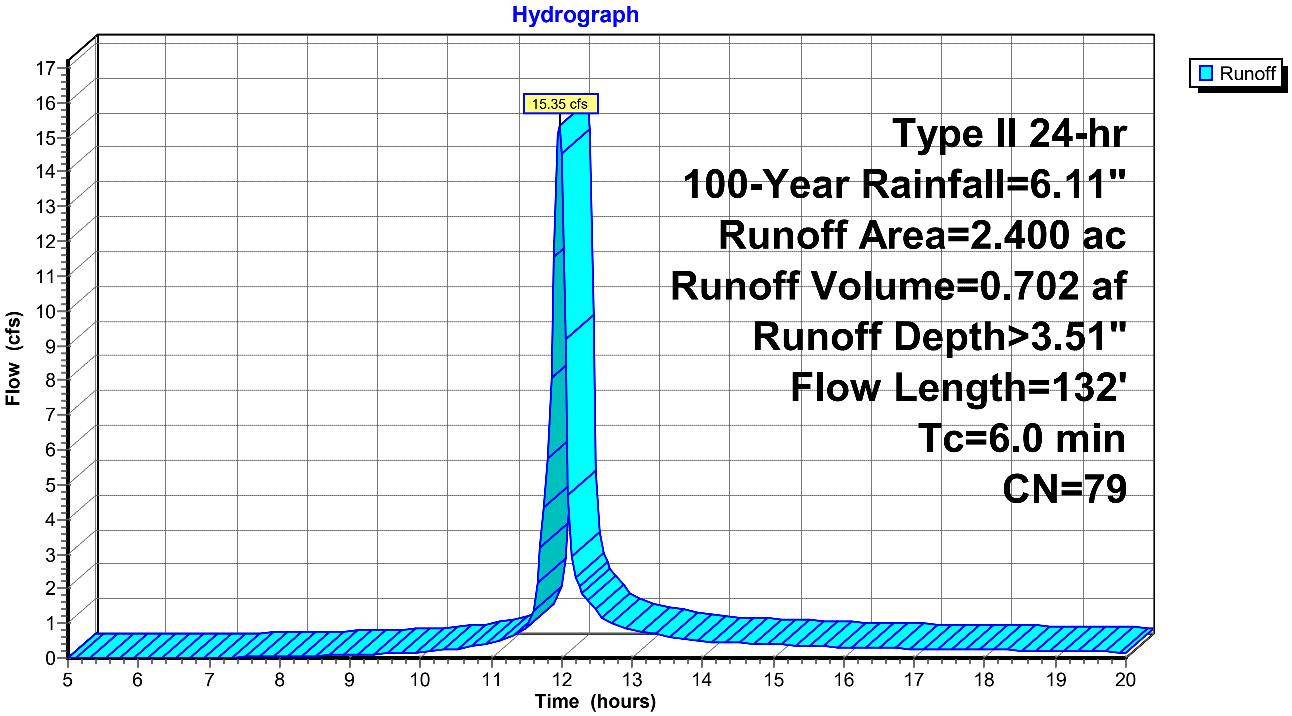
Runoff = 15.35 cfs @ 11.97 hrs, Volume= 0.702 af, Depth> 3.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

Area (ac)	CN	Description
2.400	79	Woods, Fair, HSG D
2.400		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	132		0.37		**Direct Entry, Sheet Flow**

Subcatchment DR-B: Overland



APPENDIX B-3B

RFP # 2025-12

BEACON ISLAND PHASE 3
Packaged Wastewater Treatment Plant
and
Fire Pump House and Marine Inlet

REFERENCE DOCUMENTS (2 of 5)

Reference Documents

Beacon Island Phase 3 Program

- NYSDEC Article 11 and 15 Permits. Dated 11/10/22 - Page 1
- State Pollutant Discharge Elimination System (SPDES) Permit. Dated 10/1/23 - Page 21
- Stormwater Pollution Protection Plan (SWPPP). Dated 6/20/22 - Page 59
- Soil Management Plan. Dated 10/23/22 - Page 564
- Landfill Closure Certification Report. Dated 10/21/24 - Page 634
- Geotechnical Engineering Report. Dated 2/2/2023 - Page 659
- Army Corps of Engineers Permit. Date 4/10/23 - Page 779
- Community Air Monitoring Plan (CAMP). Dated 10/23/22 – Page 806

Summary for Subcatchment DR-C: Depression

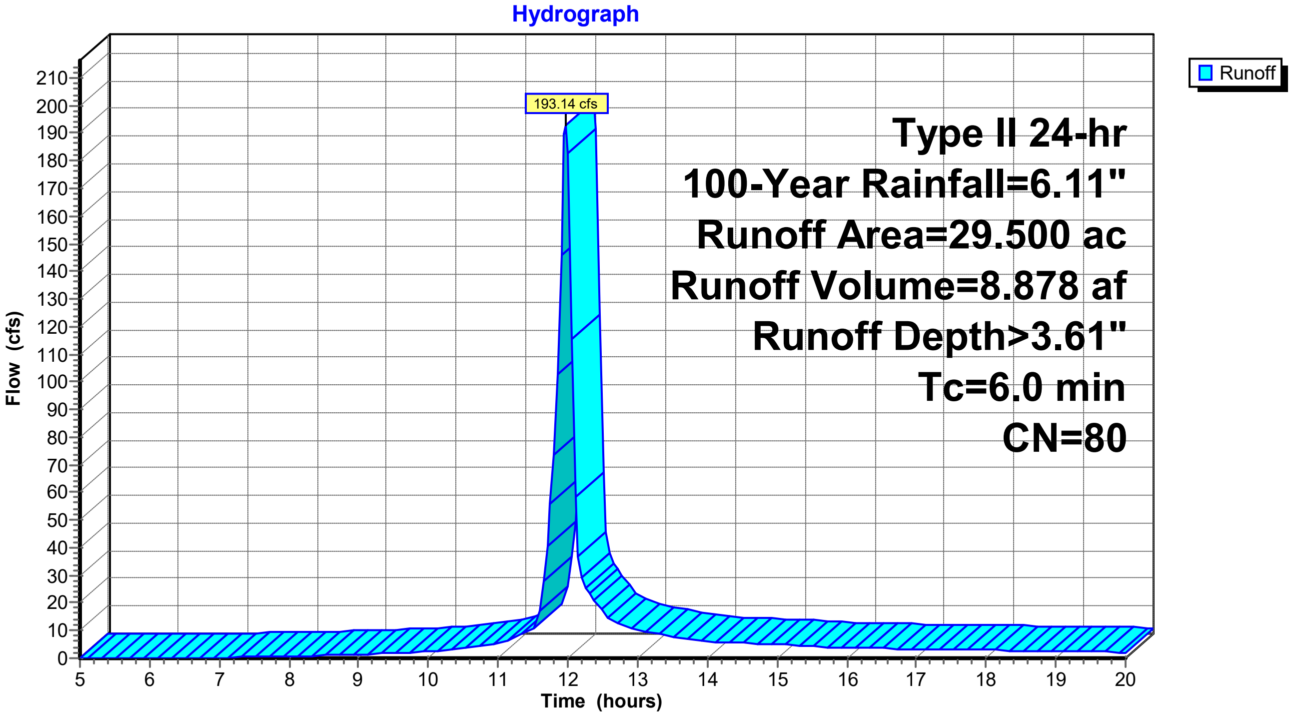
Runoff = 193.14 cfs @ 11.97 hrs, Volume= 8.878 af, Depth> 3.61"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

Area (ac)	CN	Description
3.970	77	Brush, Fair, HSG D
2.500	96	Gravel surface, HSG D
23.030	79	Woods, Fair, HSG D
29.500	80	Weighted Average
29.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-C: Depression



Summary for Subcatchment DR-D: Overland

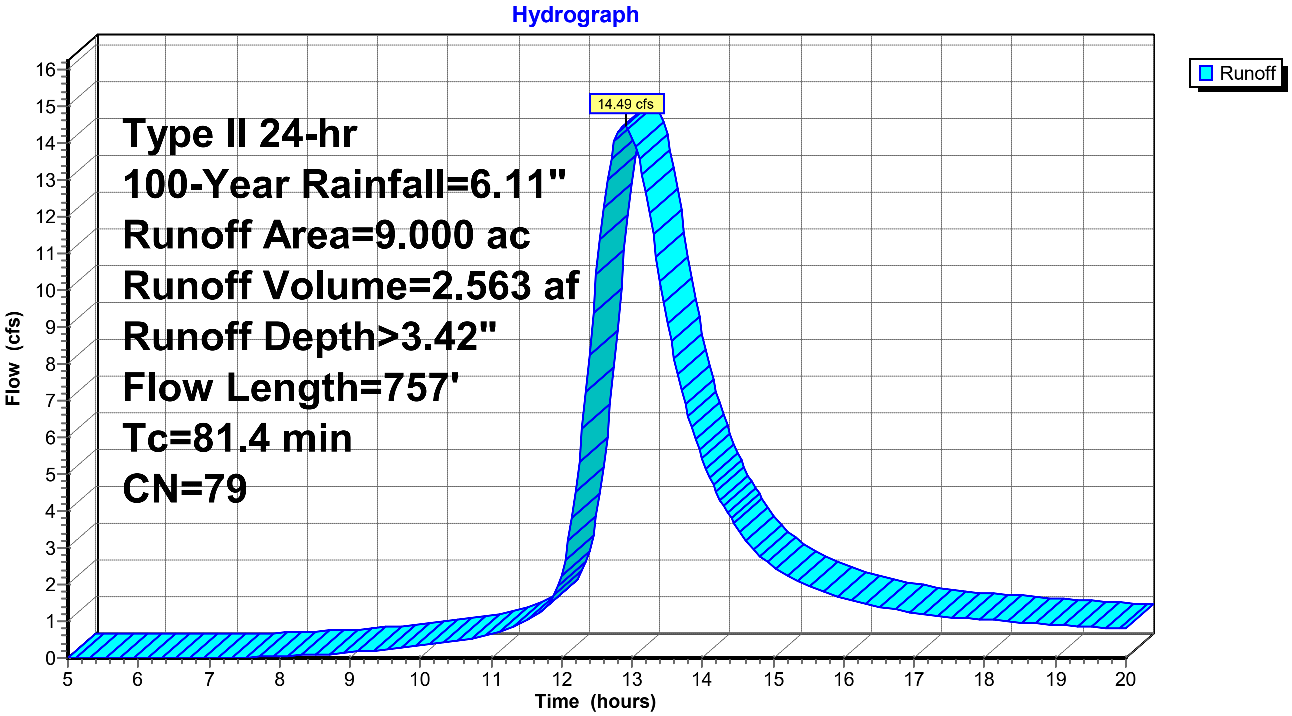
Runoff = 14.49 cfs @ 12.90 hrs, Volume= 2.563 af, Depth> 3.42"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

Area (ac)	CN	Description
9.000	79	Woods, Fair, HSG D
9.000		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
56.6	150	0.0050	0.04		**Sheet Flow, Sheet Flow** Woods: Light underbrush n= 0.400 P2= 2.67"
23.9	507	0.0050	0.35		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
0.9	100	0.1300	1.80		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
81.4	757	Total			

Subcatchment DR-D: Overland



Summary for Subcatchment DR-E: Overland

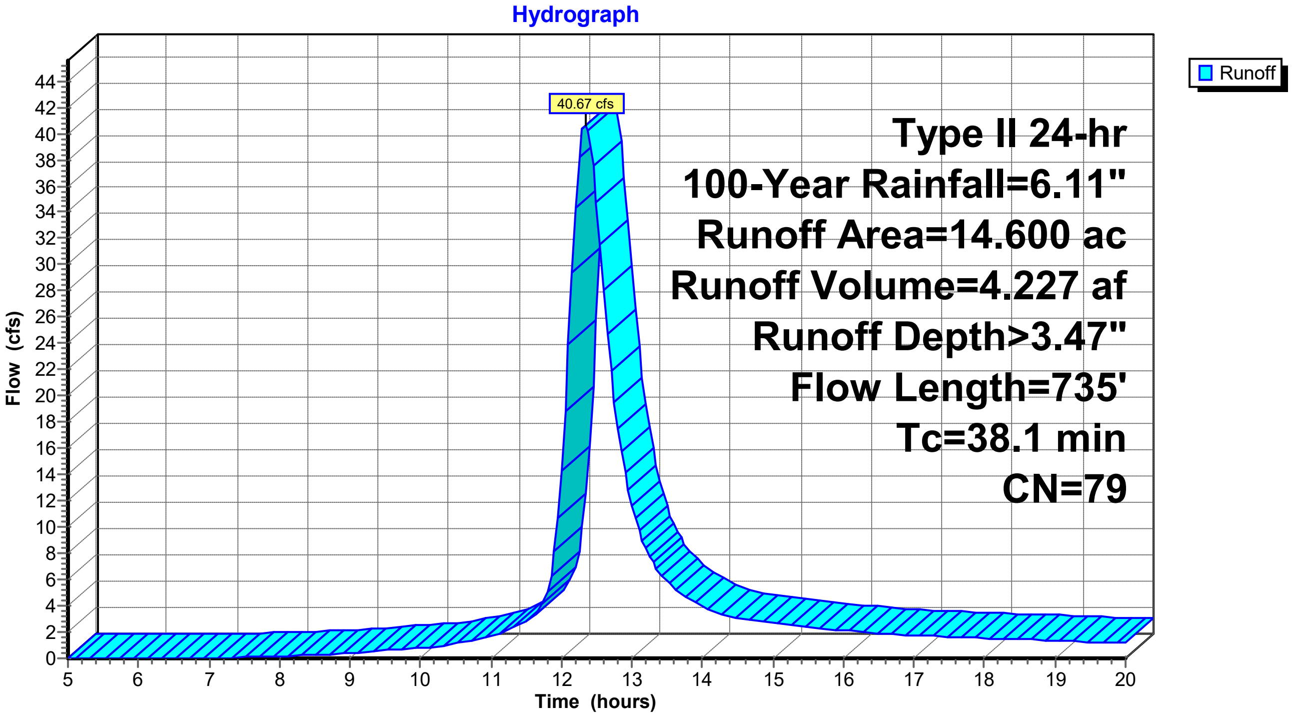
Runoff = 40.67 cfs @ 12.34 hrs, Volume= 4.227 af, Depth> 3.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

Area (ac)	CN	Description
14.600	79	Woods, Fair, HSG D
14.600		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
27.6	150	0.0300	0.09		**Sheet Flow, Sheet Flow** Woods: Light underbrush n= 0.400 P2= 2.67"
9.8	510	0.0300	0.87		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
0.7	75	0.1200	1.73		**Shallow Concentrated Flow, Shallow Concentrated** Woodland Kv= 5.0 fps
38.1	735	Total			

Subcatchment DR-E: Overland



Summary for Subcatchment DR-F: Overland

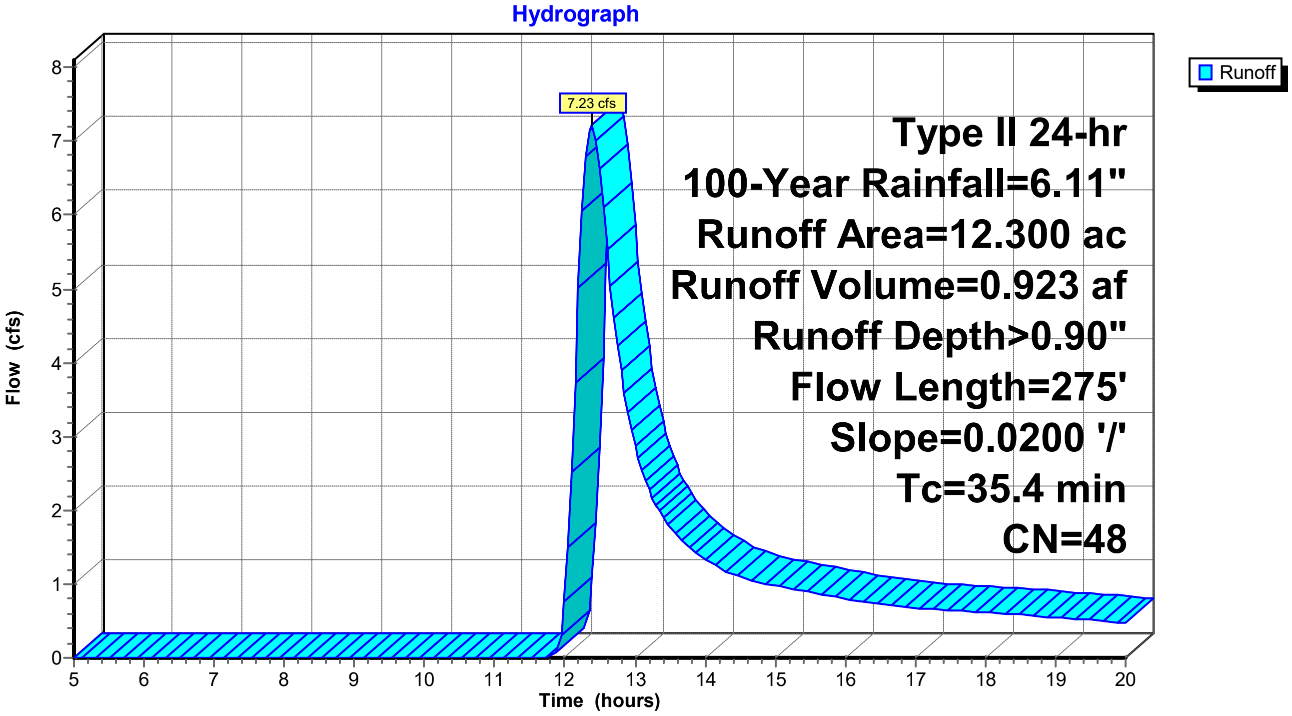
Runoff = 7.23 cfs @ 12.39 hrs, Volume= 0.923 af, Depth> 0.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	1.100	98	Pavement
	11.200	43	Woods/grass comb., Fair, HSG A
	12.300	48	Weighted Average
	11.200		91.06% Pervious Area
	1.100		8.94% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
32.5	150	0.0200	0.08		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.67"
2.9	125	0.0200	0.71		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
35.4	275	Total			

Subcatchment DR-F: Overland



Summary for Subcatchment DR-G: Roadway

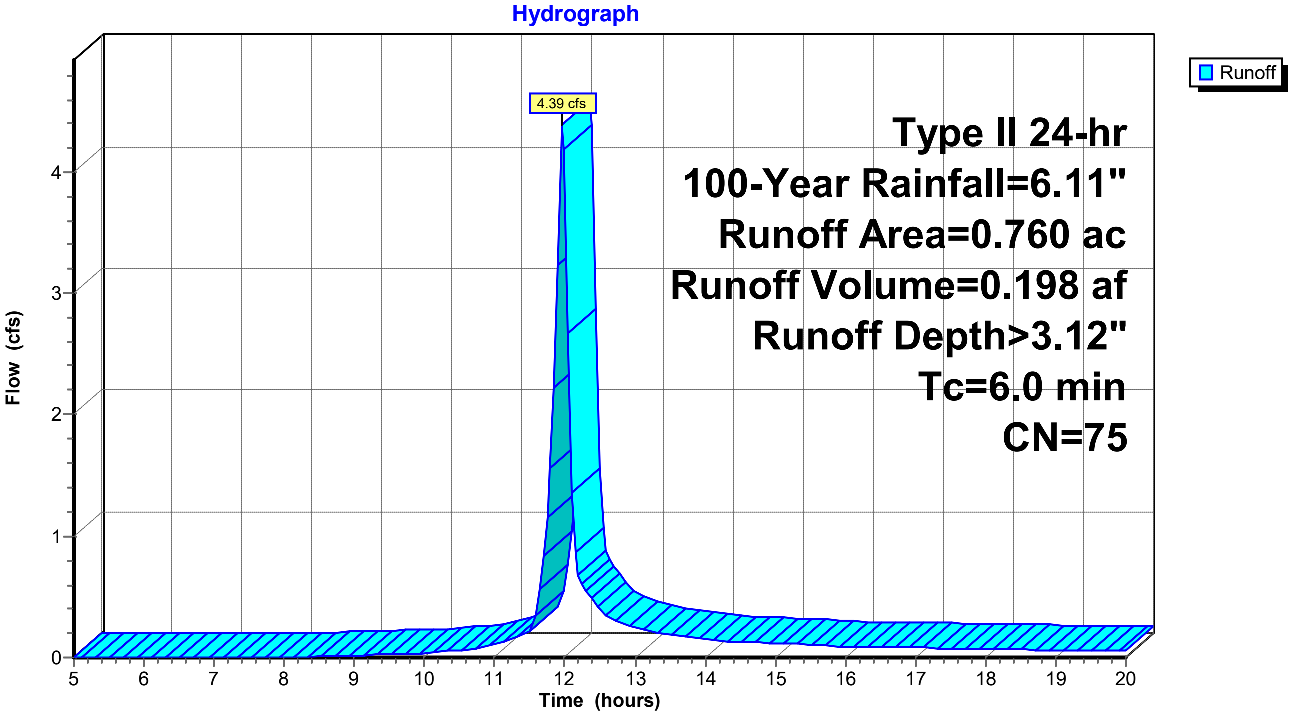
Runoff = 4.39 cfs @ 11.97 hrs, Volume= 0.198 af, Depth> 3.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	0.460	98	Roadway
	0.300	39	>75% Grass cover, Good, HSG A
	0.760	75	Weighted Average
	0.300		39.47% Pervious Area
	0.460		60.53% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-G: Roadway



Summary for Reach 1W: Wetland #1 / Analysis Point 1A

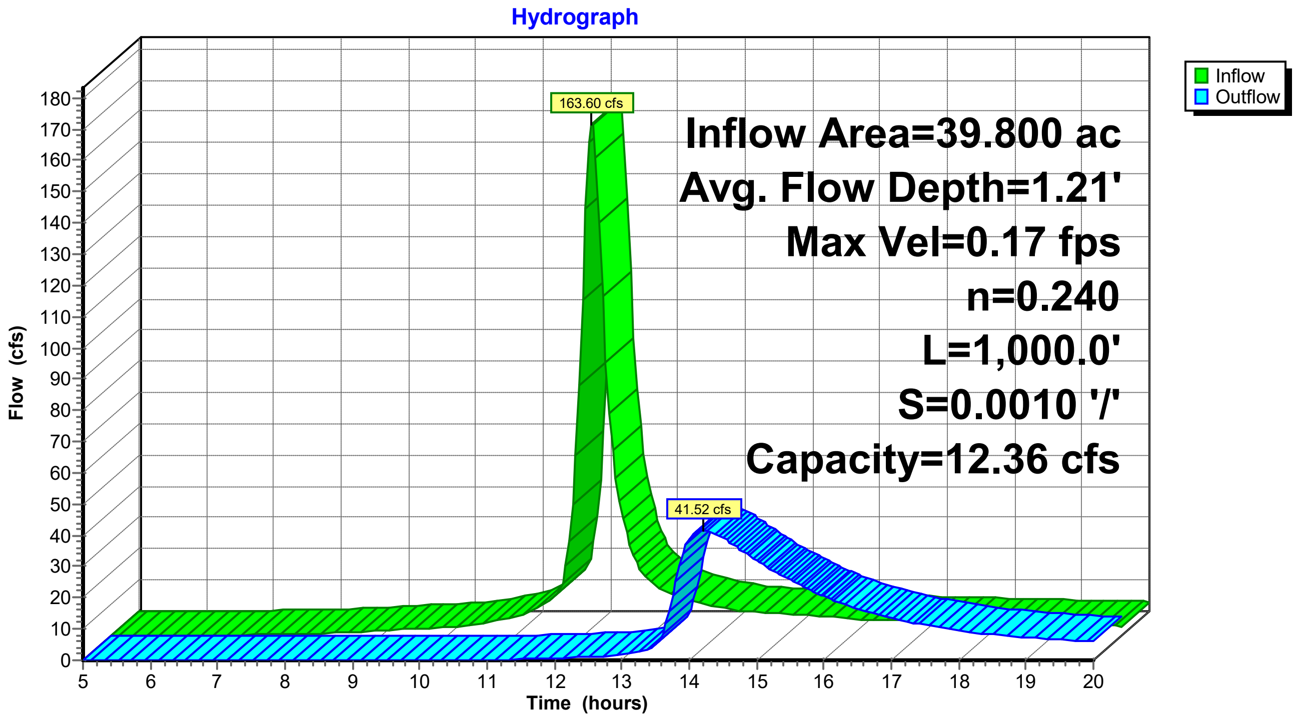
Inflow Area =	39.800 ac,	2.76% Impervious,	Inflow Depth > 3.59"	for 100-Year event
Inflow =	163.60 cfs @	12.14 hrs,	Volume=	11.919 af
Outflow =	41.52 cfs @	14.21 hrs,	Volume=	9.967 af, Atten= 75%, Lag= 123.8 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.17 fps, Min. Travel Time= 98.2 min
Avg. Velocity = 0.08 fps, Avg. Travel Time= 203.6 min

Peak Storage= 244,871 cf @ 12.57 hrs
Average Depth at Peak Storage= 1.21' , Surface Width= 207.26'
Bank-Full Depth= 0.50' Flow Area= 100.8 sf, Capacity= 12.36 cfs

200.00' x 0.50' deep channel, n= 0.240 Sheet flow over Dense Grass
Side Slope Z-value= 3.0 '/' Top Width= 203.00'
Length= 1,000.0' Slope= 0.0010 '/'
Inlet Invert= 6.00', Outlet Invert= 5.00'

Reach 1W: Wetland #1 / Analysis Point 1A



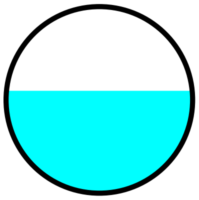
Summary for Reach 2R: Outlet Pipe

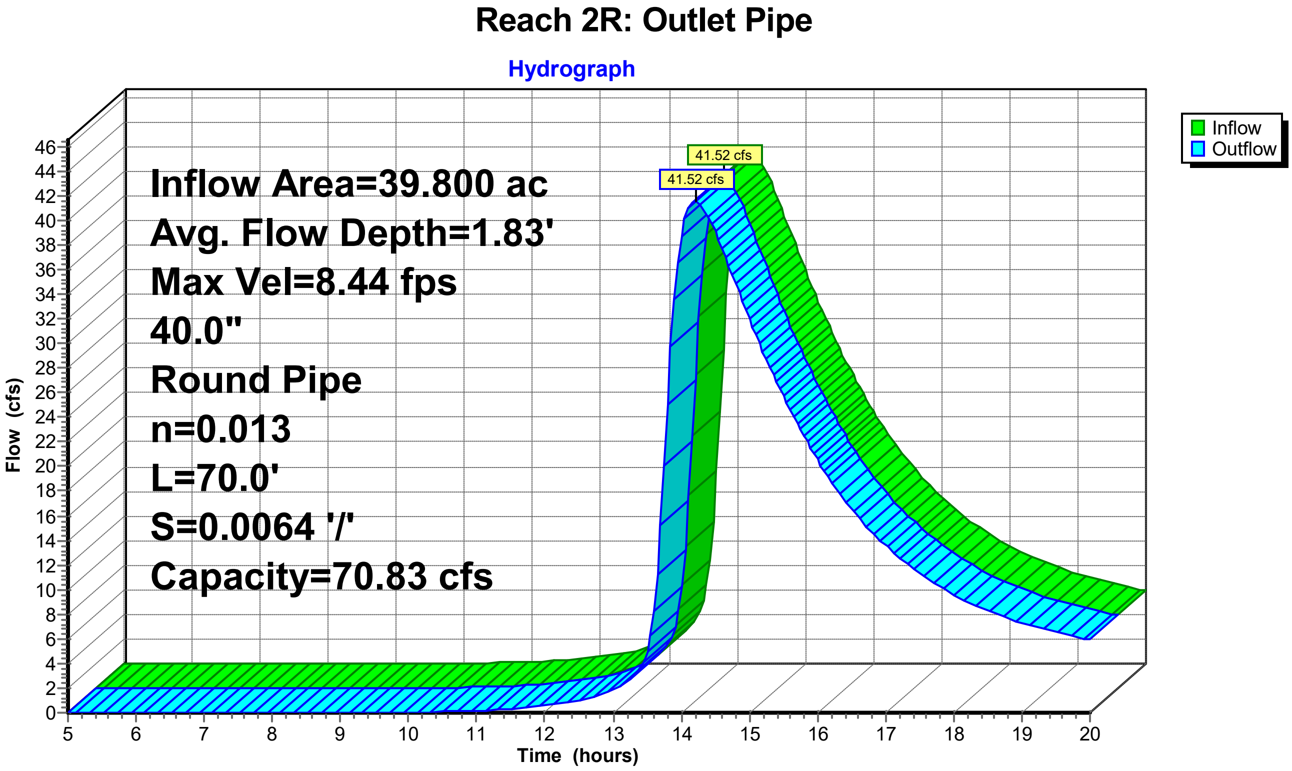
Inflow Area = 39.800 ac, 2.76% Impervious, Inflow Depth > 3.01" for 100-Year event
Inflow = 41.52 cfs @ 14.21 hrs, Volume= 9.967 af
Outflow = 41.52 cfs @ 14.21 hrs, Volume= 9.964 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Max. Velocity= 8.44 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 4.38 fps, Avg. Travel Time= 0.3 min

Peak Storage= 345 cf @ 14.21 hrs
Average Depth at Peak Storage= 1.83' , Surface Width= 3.32'
Bank-Full Depth= 3.33' Flow Area= 8.7 sf, Capacity= 70.83 cfs

40.0" Round Pipe
n= 0.013 Corrugated PE, smooth interior
Length= 70.0' Slope= 0.0064 '/'
Inlet Invert= 4.25', Outlet Invert= 3.80'



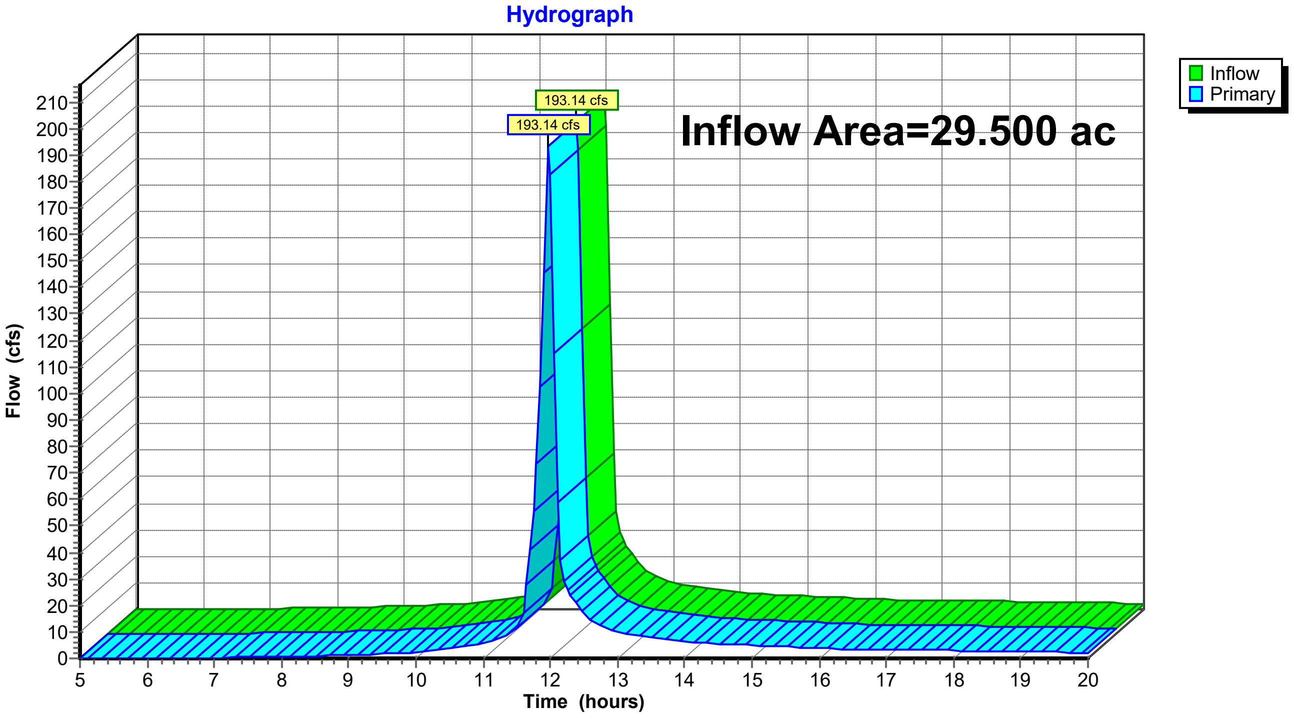


Summary for Pond 1P: Infiltration

```
Inflow Area =      29.500 ac,   0.00% Impervious,  Inflow Depth >   3.61"    for  100-Year event
Inflow      =     193.14 cfs @  11.97 hrs,  Volume=           8.878 af
Primary     =     193.14 cfs @  11.97 hrs,  Volume=           8.878 af,  Atten= 0%,  Lag= 0.0 min
```

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Pond 1P: Infiltration

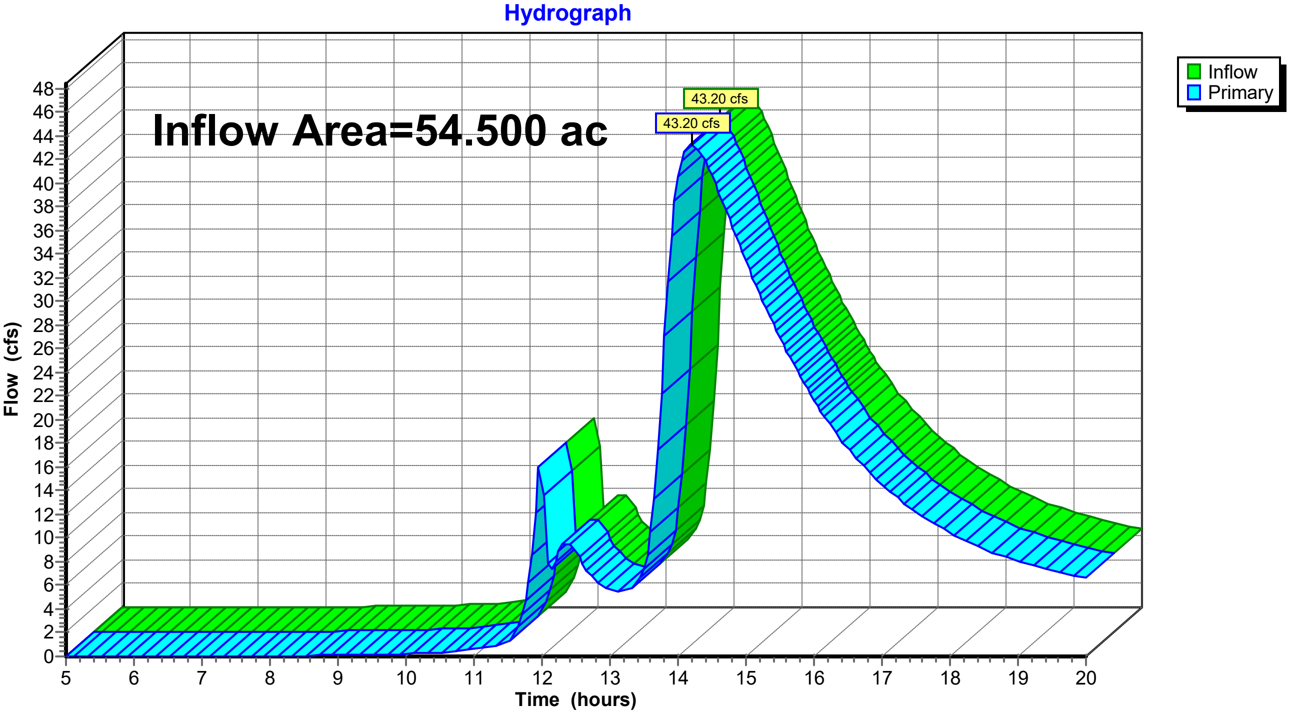


Summary for Pond AP-1: Analysis Point #1

Inflow Area = 54.500 ac, 4.04% Impervious, Inflow Depth > 2.55" for 100-Year event
Inflow = 43.20 cfs @ 14.20 hrs, Volume= 11.589 af
Primary = 43.20 cfs @ 14.20 hrs, Volume= 11.589 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Pond AP-1: Analysis Point #1

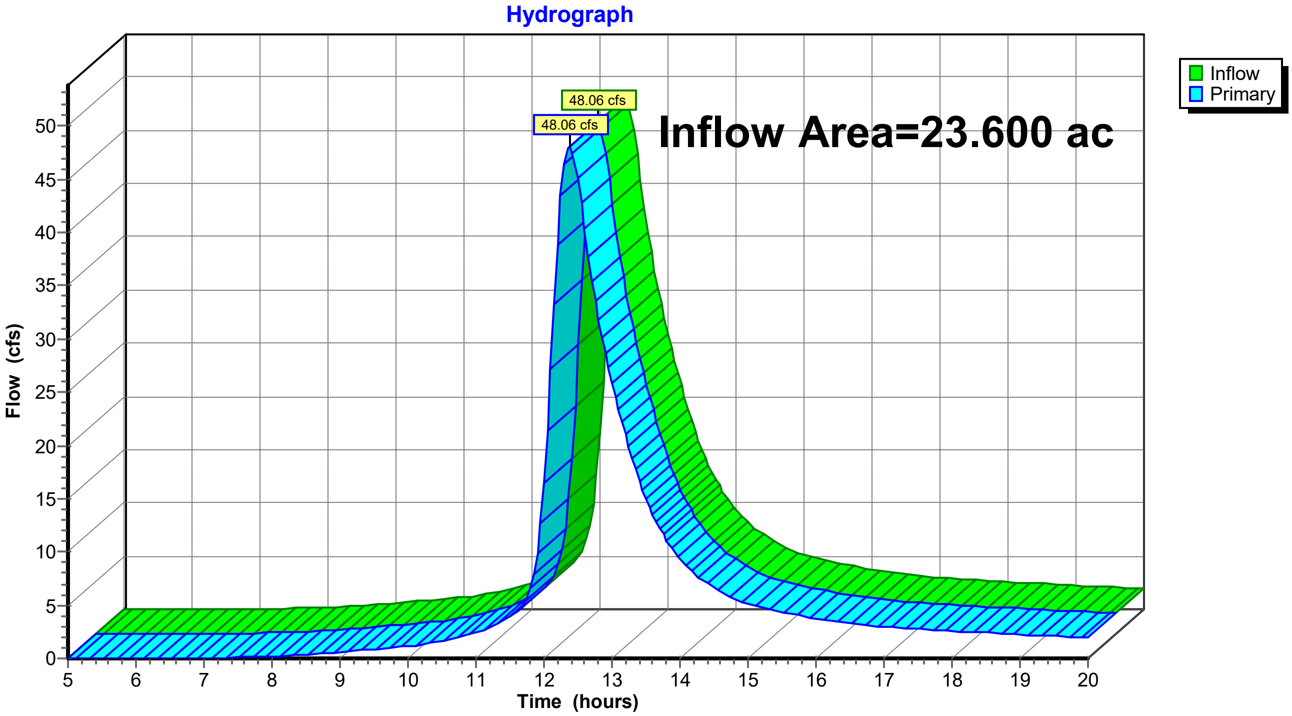


Summary for Pond AP-2: Analysis Point #2

```
Inflow Area =        23.600 ac,    0.00% Impervious,  Inflow Depth >   3.45"    for  100-Year event
Inflow      =       48.06 cfs @   12.38 hrs,  Volume=            6.789 af
Primary     =       48.06 cfs @   12.38 hrs,  Volume=            6.789 af,  Atten= 0%,  Lag= 0.0 min
```

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

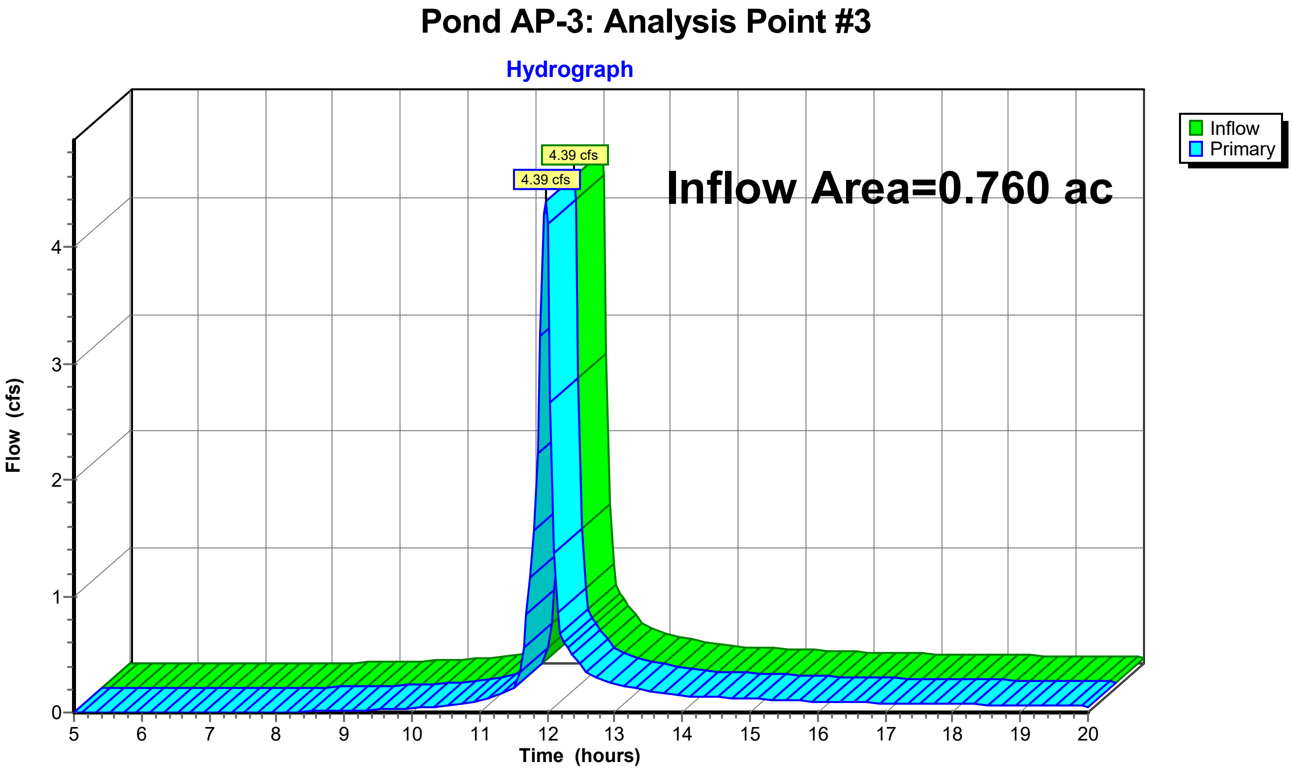
Pond AP-2: Analysis Point #2



Summary for Pond AP-3: Analysis Point #3

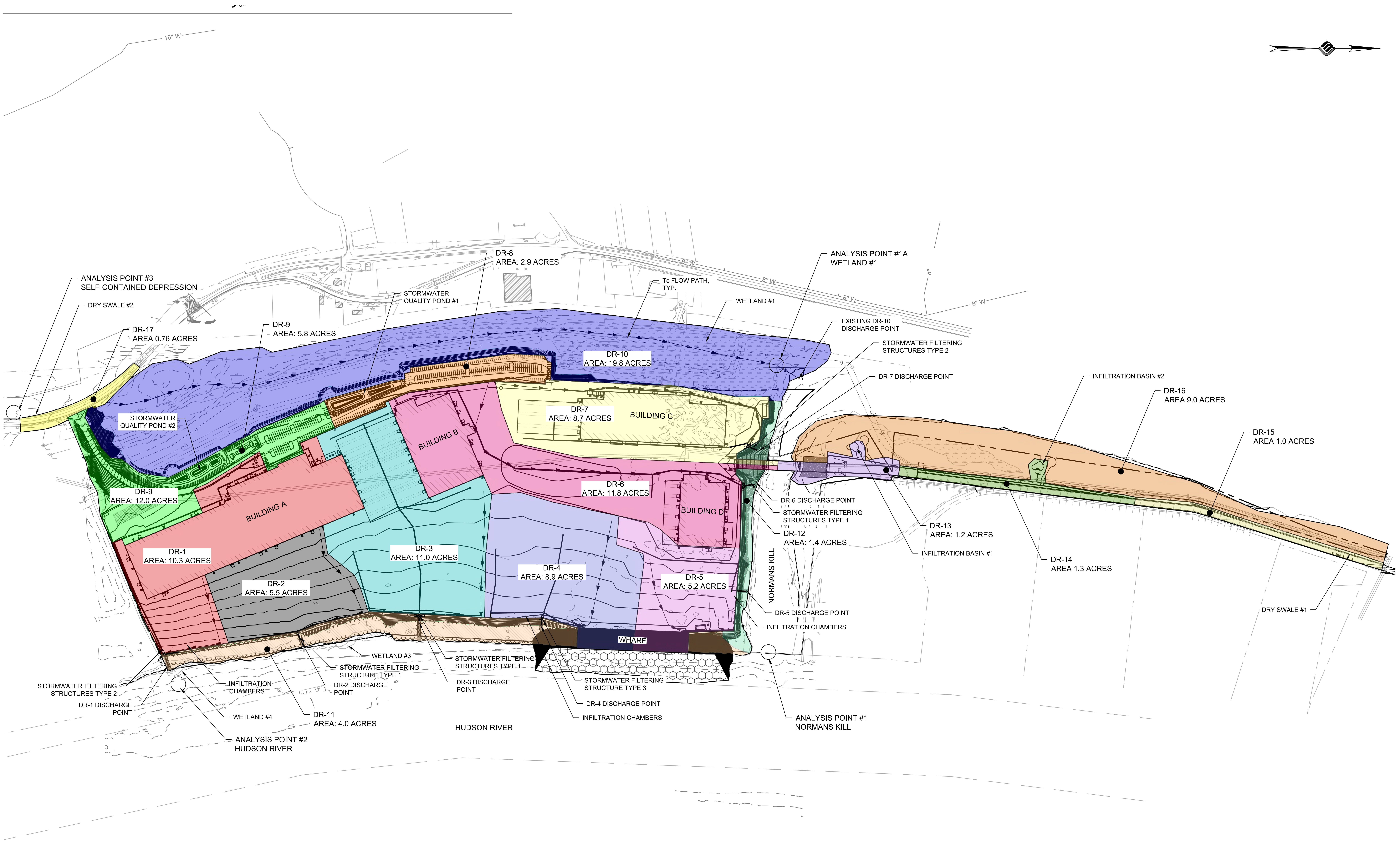
Inflow Area = 0.760 ac, 60.53% Impervious, Inflow Depth > 3.12" for 100-Year event
Inflow = 4.39 cfs @ 11.97 hrs, Volume= 0.198 af
Primary = 4.39 cfs @ 11.97 hrs, Volume= 0.198 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Appendix B

Proposed Conditions Drainage Map and HydroCAD Report



LEGEND

▨	DA-1	▨	DA-10
▨	DA-2	▨	DA-11
▨	DA-3	▨	DA-12
▨	DA-4	▨	DA-13
▨	DA-5	▨	DA-14
▨	DA-6	▨	DA-15
▨	DA-7	▨	DA-16
▨	DA-8	▨	DA-17
▨	DA-9		

NOTE:
1. DRAINAGE AREAS DR-5, DR-6, DR-7, DR-8, DR-9, DR-10, DR-12, DR-13, DR-14, DR-15 AND DR-16 DRAIN TO ANALYSIS POINT #1.
2. DRAINAGE AREAS DR-1, DR-2, DR-3, DR-4 AND DR-11 DRAIN TO ANALYSIS POINT #2.
3. DRAINAGE AREA DR-17 DRAINS TO ANALYSIS POINT #3.

0 200 400 600 FT
SCALE

N:\18641.00 ALBANY PORT EXPANSION\REPORTS\DRAINAGE REPORT\18641.00-PROPOSED CONDITION.DWG

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
NFC

NO.	DATE	DESCRIPTION
1	05/20/22	TOWN COMMENTS

CLIENT:
ALBANY PORT DISTRICT COMMISSION
TOWN OF BETHLEHEM, NEW YORK

PROJECT:
PORT OF ALBANY SITE INFRASTRUCTURE IMPROVEMENTS

DRAWN	NSO
DESIGNED	NSO
CHECKED	AJF
SCALE	1" = 200'
DATE	MAY 2022
PROJECT	18641.00

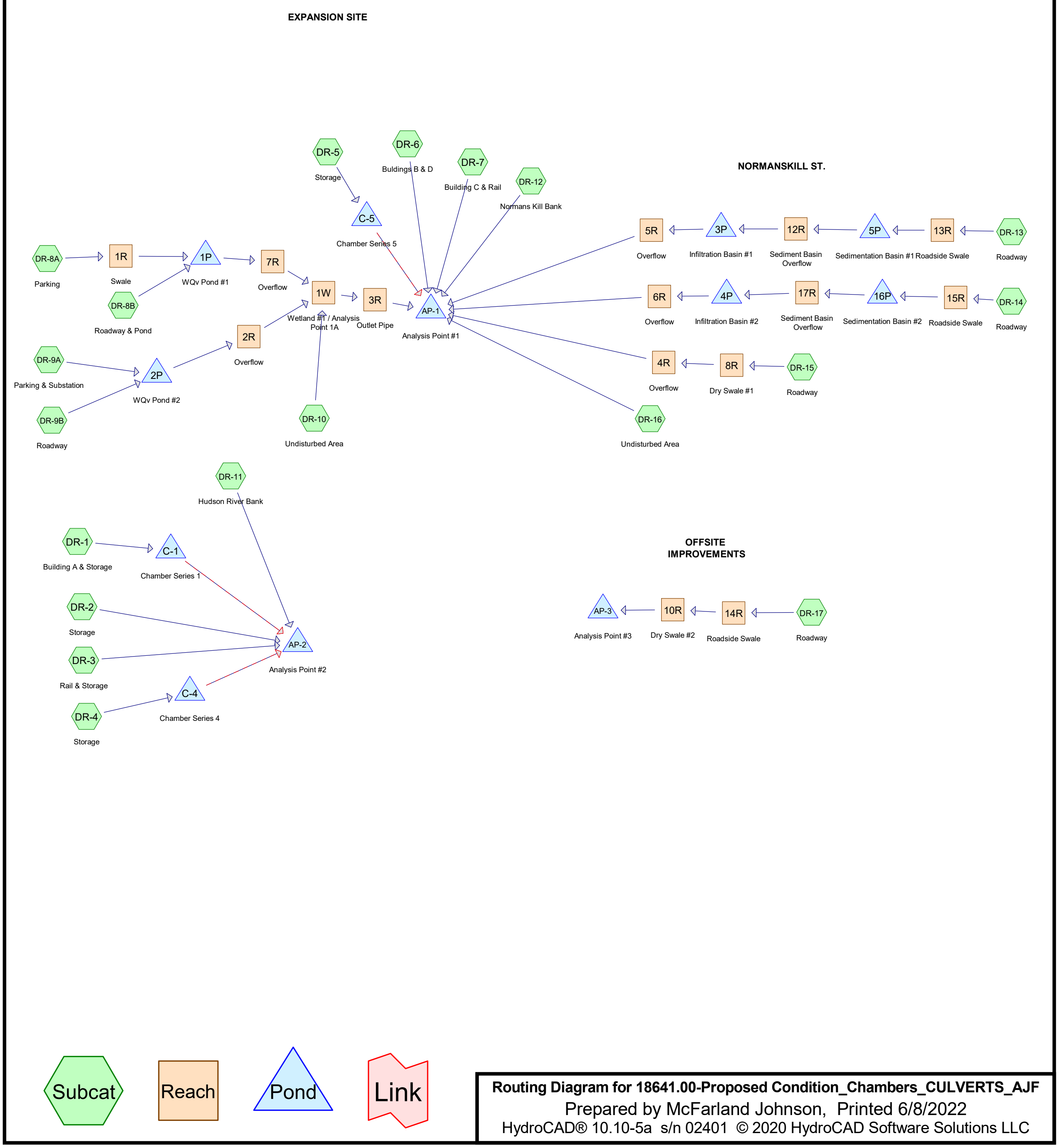
DRAWING TITLE

POST-DEVELOPMENT
SITE DRAINAGE
AREAS

DRAWING NUMBER

DR-PR

01 OF 01



Routing Diagram for 18641.00-Proposed Condition_Chambers_CULVERTS_AJF
Prepared by McFarland Johnson, Printed 6/8/2022
HydroCAD® 10.10-5a s/n 02401 © 2020 HydroCAD Software Solutions LLC

Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-Year	Type II 24-hr		Default	24.00	1	2.20	2
2	10-Year	Type II 24-hr		Default	24.00	1	3.63	2
3	100-Year	Type II 24-hr		Default	24.00	1	6.11	2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
1.510	39	>75% Grass cover, Good, HSG A (DR-13, DR-14, DR-15, DR-17)
5.500	80	>75% Grass cover, Good, HSG D (DR-1, DR-2, DR-3, DR-4, DR-5, DR-6, DR-7, DR-8A, DR-8B, DR-9A, DR-9B)
6.870	98	Building A (DR-1)
2.549	98	Building B (DR-6)
3.030	98	Building C (DR-7)
1.413	98	Building D (DR-6)
16.900	95	Compacted Gravel (DR-3, DR-4)
0.200	92	Compacted Gravel (DR-9A)
26.618	95	Dense Graded Aggregate (DR-1, DR-2, DR-5, DR-6, DR-7, DR-9B)
0.970	96	Gravel surface, HSG D (DR-12)
1.100	98	Mill & Fill of Old Pavement (DR-14, DR-15)
0.600	98	New Pavement (DR-14, DR-15)
1.600	98	Parking (DR-8A)
1.200	98	Parking and Road (DR-9A)
0.450	98	Pavement (DR-13)
3.270	98	Rail (DR-3, DR-7)
0.140	98	Road Widening (DR-17)
1.240	98	Roadway (DR-17, DR-8B, DR-9B)
0.170	98	Substation (DR-9A)
24.230	79	Woods, Fair, HSG D (DR-10, DR-11, DR-12)
9.000	43	Woods/grass comb., Fair, HSG A (DR-16)
108.560	**86**	**TOTAL AREA**

Summary for Subcatchment DR-1: Building A & Storage

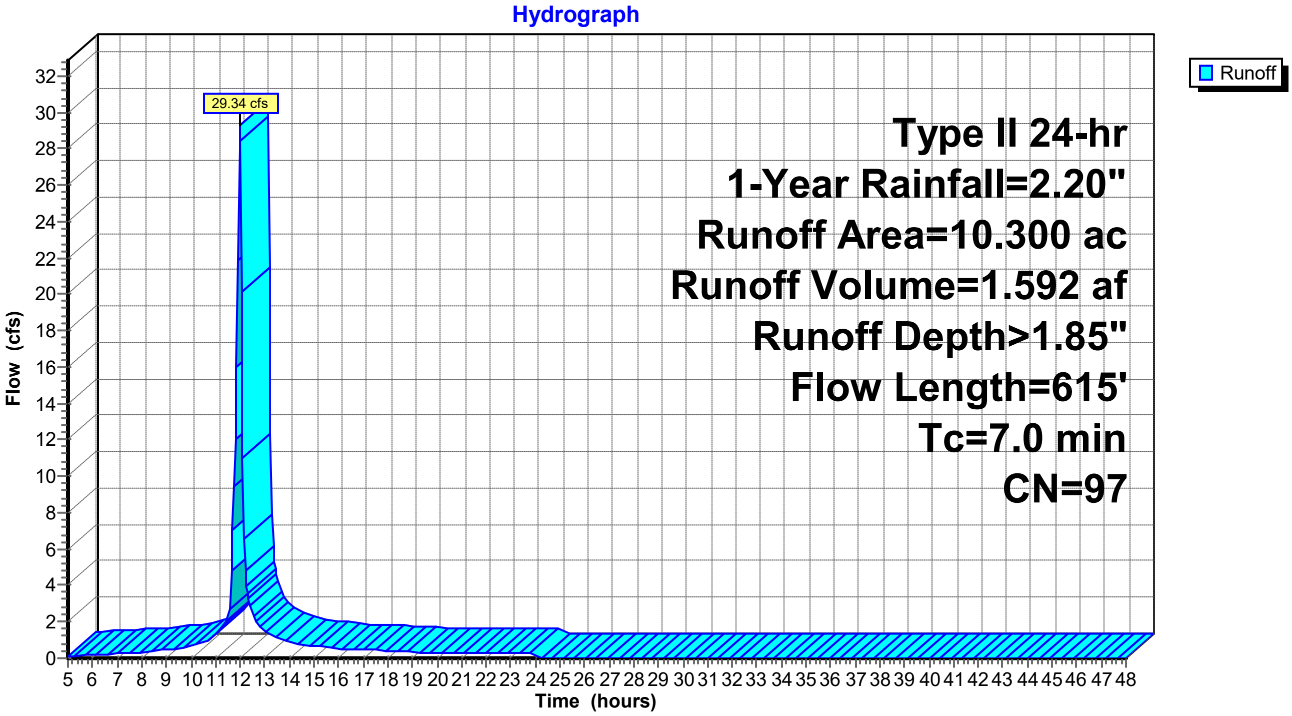
Runoff = 29.34 cfs @ 11.98 hrs, Volume= 1.592 af, Depth> 1.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	6.870	98	Building A
	0.100	80	>75% Grass cover, Good, HSG D
*	3.330	95	Dense Graded Aggregate
	10.300	97	Weighted Average
	3.430		33.30% Pervious Area
	6.870		66.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
3.1	300	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.6	215	0.0050	5.91	29.00	**Pipe Channel,** 30.0" Round Area= 4.9 sf Perim= 7.9' r= 0.63' n= 0.013
7.0	615	Total			

Subcatchment DR-1: Building A & Storage



Summary for Subcatchment DR-10: Undisturbed Area

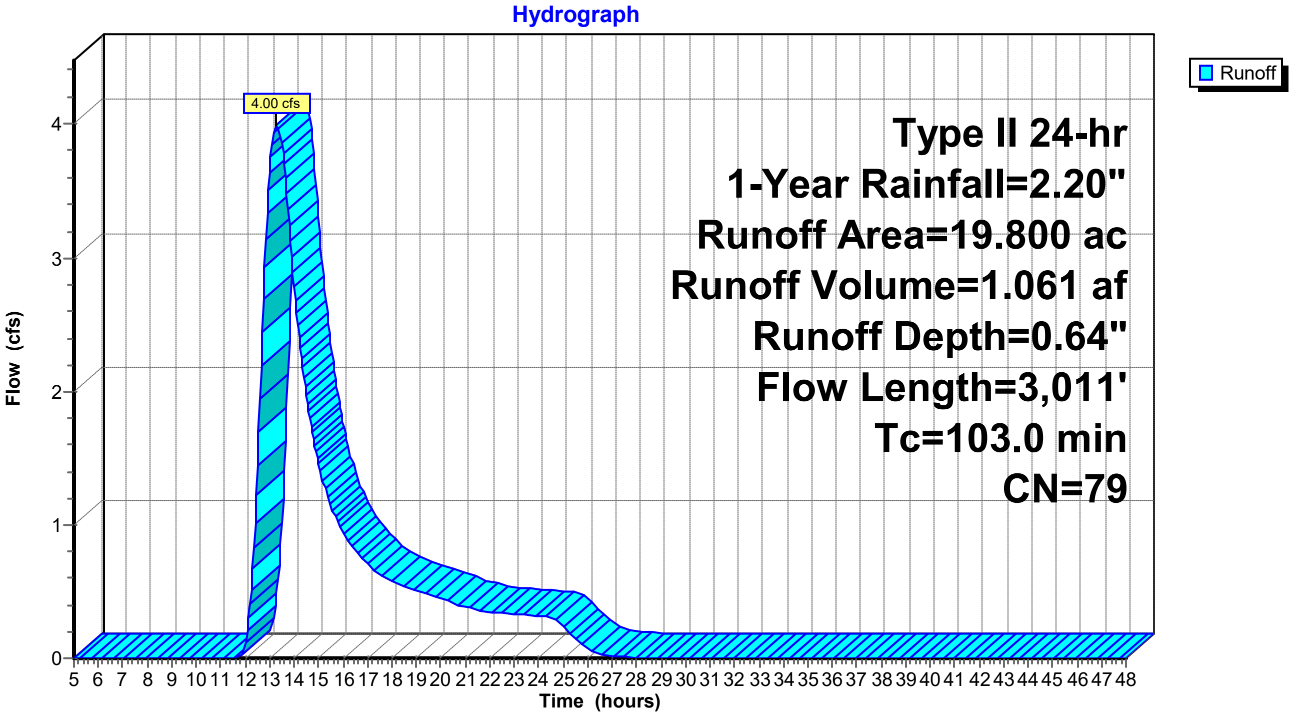
Runoff = 4.00 cfs @ 13.29 hrs, Volume= 1.061 af, Depth= 0.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

Area (ac)	CN	Description
19.800	79	Woods, Fair, HSG D
19.800		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.7	150	0.0800	0.13		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.40"
3.0	200	0.0500	1.12		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
1.6	250	0.2600	2.55		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
78.7	2,361	0.0100	0.50		**Shallow Concentrated Flow, Wetland Flow** Woodland Kv= 5.0 fps
0.0	50	0.0500	22.86	161.57	**Pipe Channel,** 36.0" Round Area= 7.1 sf Perim= 9.4' r= 0.75' n= 0.012 Corrugated PP, smooth interior
103.0	3,011	Total			

Subcatchment DR-10: Undisturbed Area



Summary for Subcatchment DR-11: Hudson River Bank

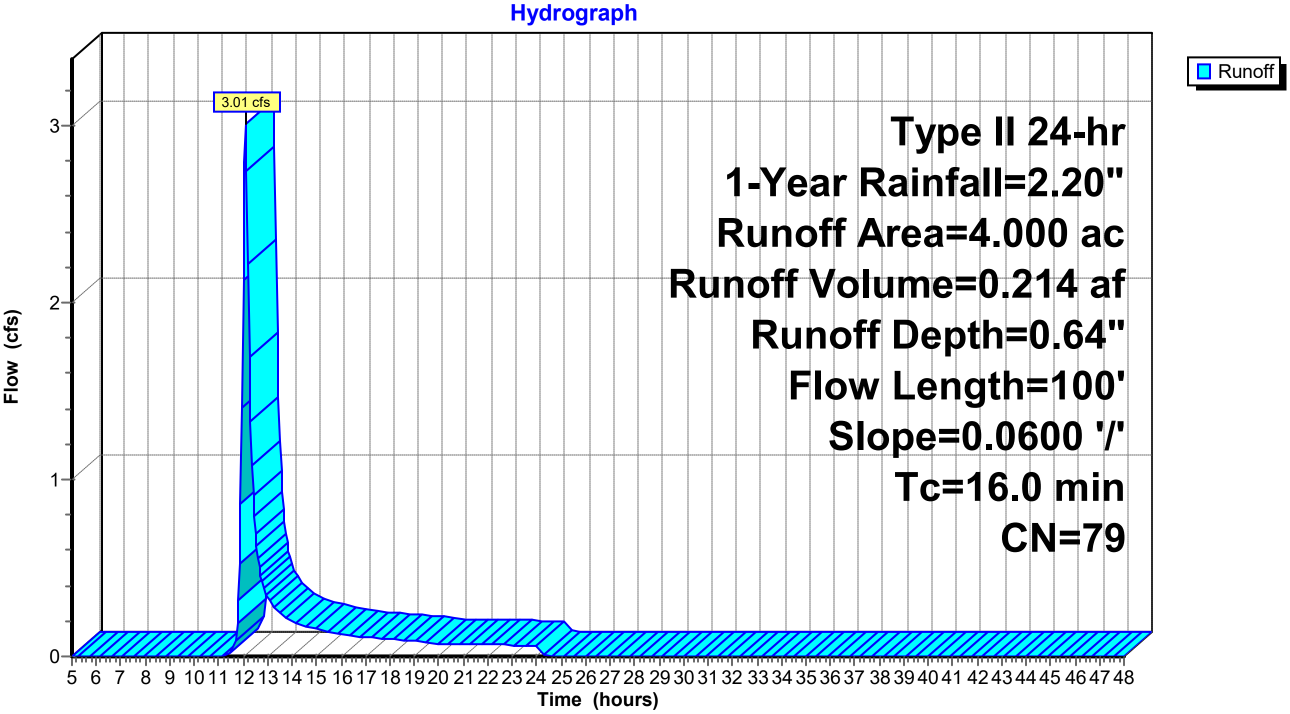
Runoff = 3.01 cfs @ 12.10 hrs, Volume= 0.214 af, Depth= 0.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

Area (ac)	CN	Description
4.000	79	Woods, Fair, HSG D
4.000		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.0	100	0.0600	0.10		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.40"

Subcatchment DR-11: Hudson River Bank



Summary for Subcatchment DR-12: Normans Kill Bank

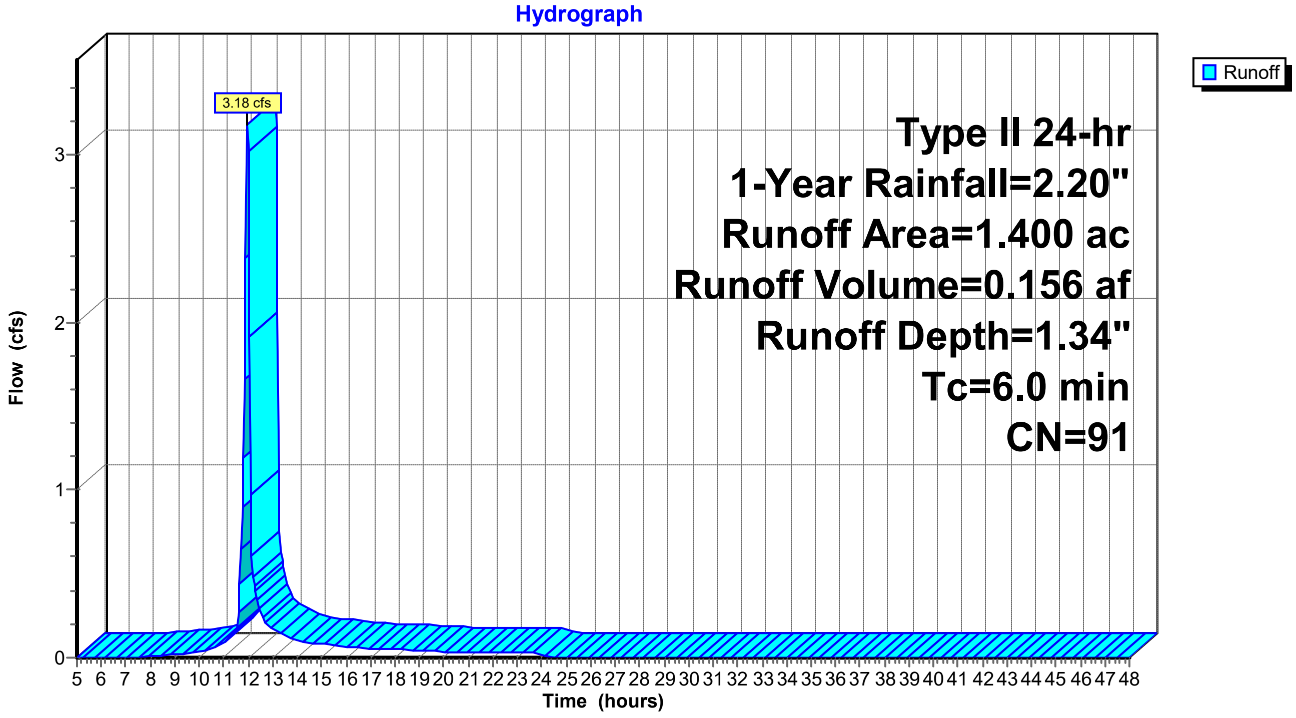
Runoff = 3.18 cfs @ 11.97 hrs, Volume= 0.156 af, Depth= 1.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

Area (ac)	CN	Description
0.430	79	Woods, Fair, HSG D
0.970	96	Gravel surface, HSG D
1.400	91	Weighted Average
1.400		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-12: Normans Kill Bank



Summary for Subcatchment DR-13: Roadway

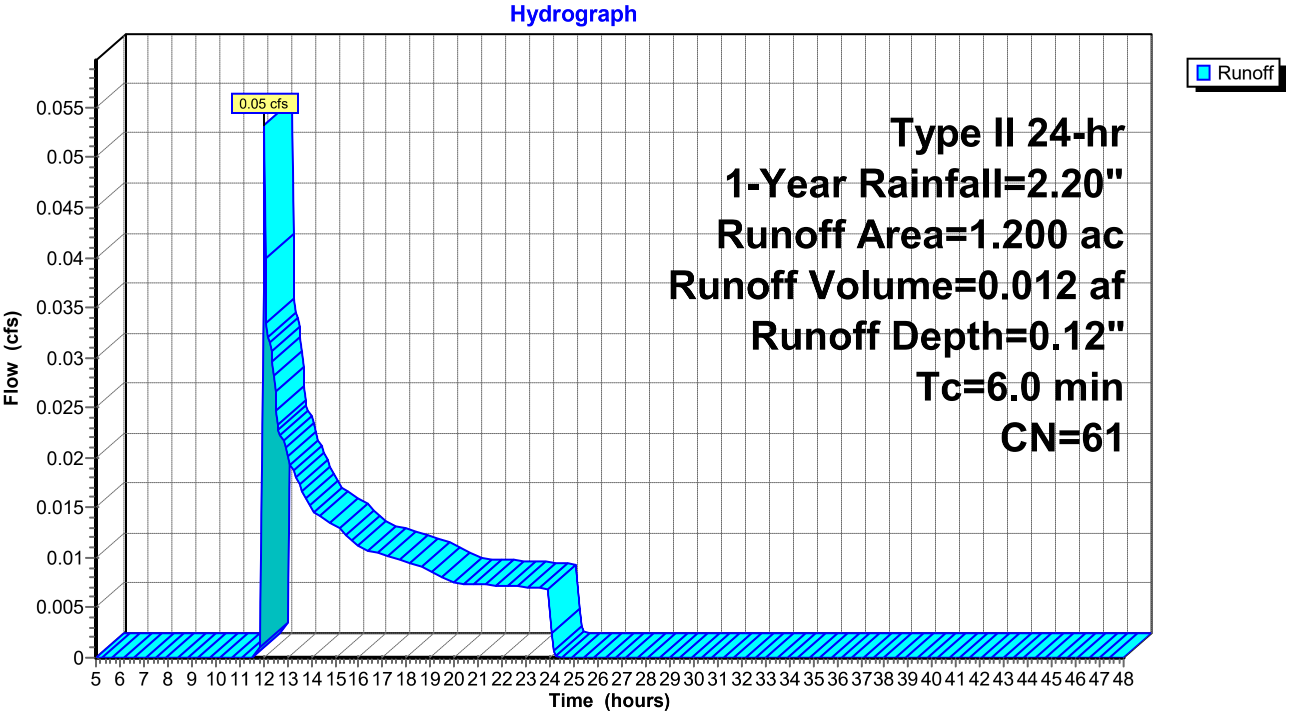
Runoff = 0.05 cfs @ 12.05 hrs, Volume= 0.012 af, Depth= 0.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	0.450	98	Pavement
	0.750	39	>75% Grass cover, Good, HSG A
	1.200	61	Weighted Average
	0.750		62.50% Pervious Area
	0.450		37.50% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-13: Roadway



Summary for Subcatchment DR-14: Roadway

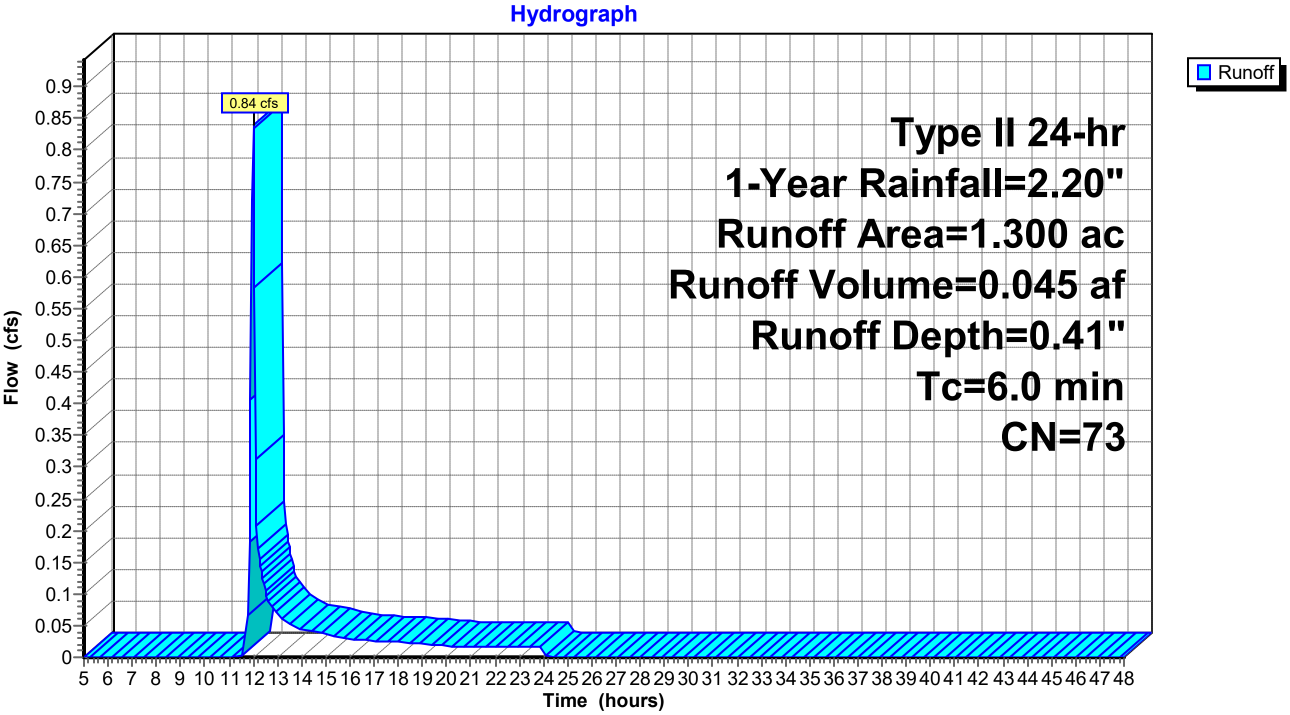
Runoff = 0.84 cfs @ 11.99 hrs, Volume= 0.045 af, Depth= 0.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	0.550	98	New Pavement
	0.550	39	>75% Grass cover, Good, HSG A
*	0.200	98	Mill & Fill of Old Pavement
	1.300	73	Weighted Average
	0.550		42.31% Pervious Area
	0.750		57.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-14: Roadway



Summary for Subcatchment DR-15: Roadway

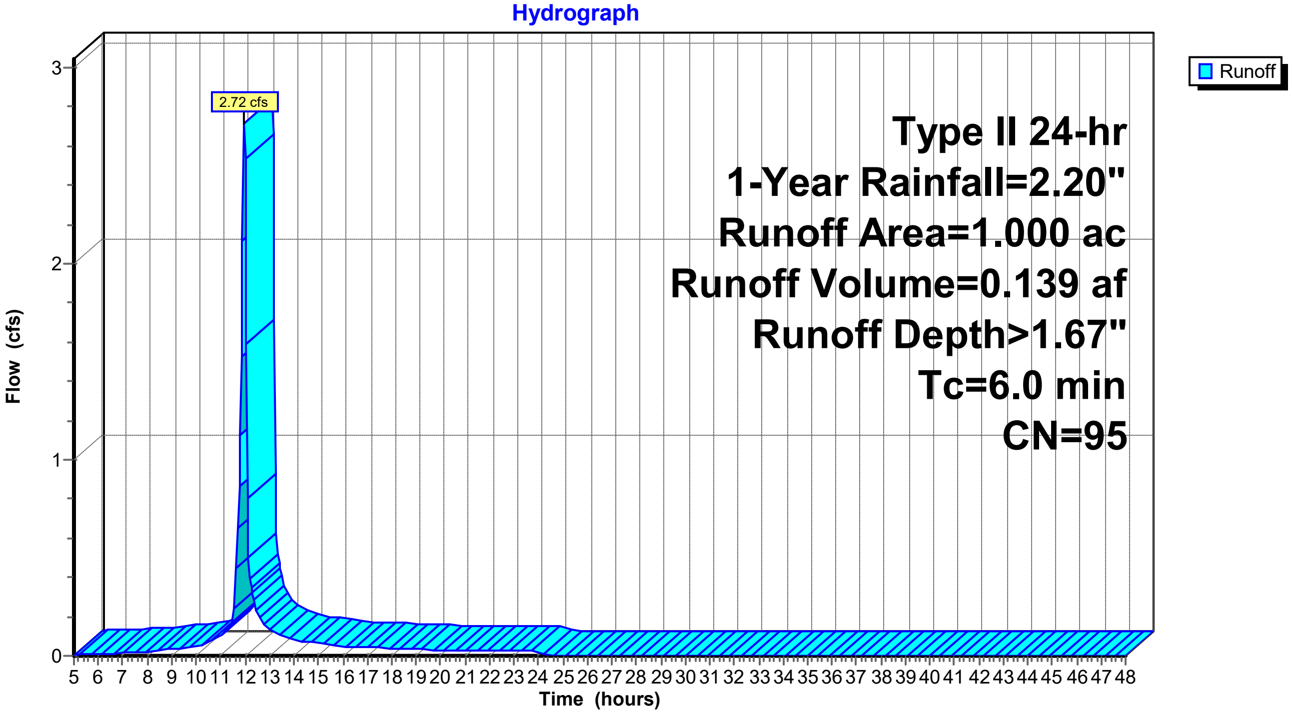
Runoff = 2.72 cfs @ 11.96 hrs, Volume= 0.139 af, Depth> 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	0.050	98	New Pavement
	0.050	39	>75% Grass cover, Good, HSG A
*	0.900	98	Mill & Fill of Old Pavement
	1.000	95	Weighted Average
	0.050		5.00% Pervious Area
	0.950		95.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-15: Roadway



Summary for Subcatchment DR-16: Undisturbed Area

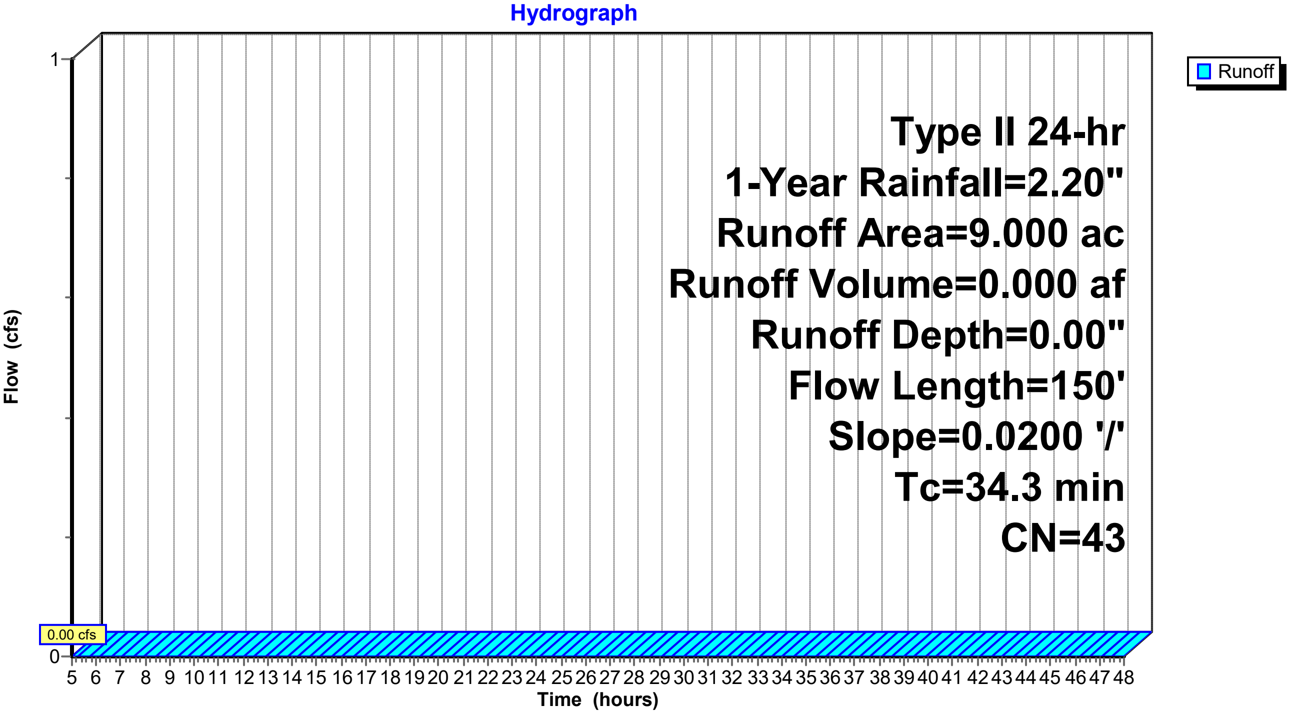
Runoff = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

Area (ac)	CN	Description
9.000	43	Woods/grass comb., Fair, HSG A
9.000		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
34.3	150	0.0200	0.07		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.40"

Subcatchment DR-16: Undisturbed Area



Summary for Subcatchment DR-17: Roadway

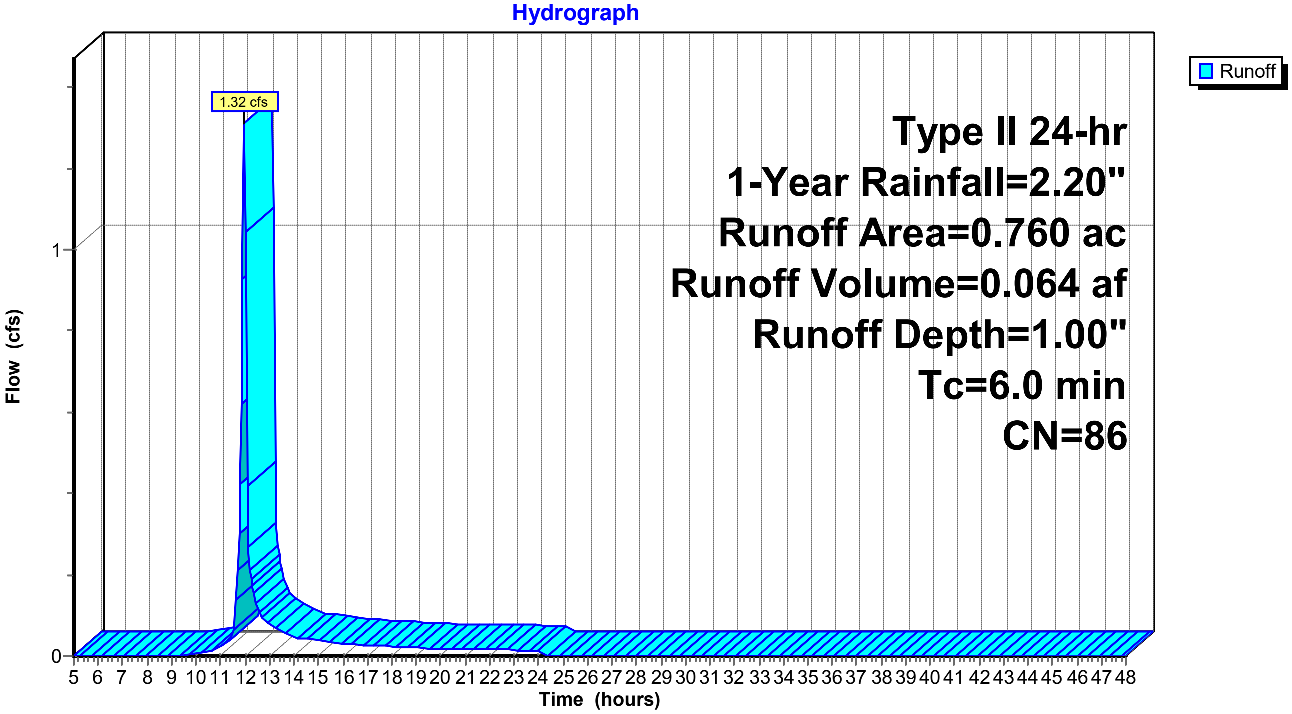
Runoff = 1.32 cfs @ 11.97 hrs, Volume= 0.064 af, Depth= 1.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	0.140	98	Road Widening
*	0.460	98	Roadway
	0.160	39	>75% Grass cover, Good, HSG A
	0.760	86	Weighted Average
	0.160		21.05% Pervious Area
	0.600		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-17: Roadway



Summary for Subcatchment DR-2: Storage

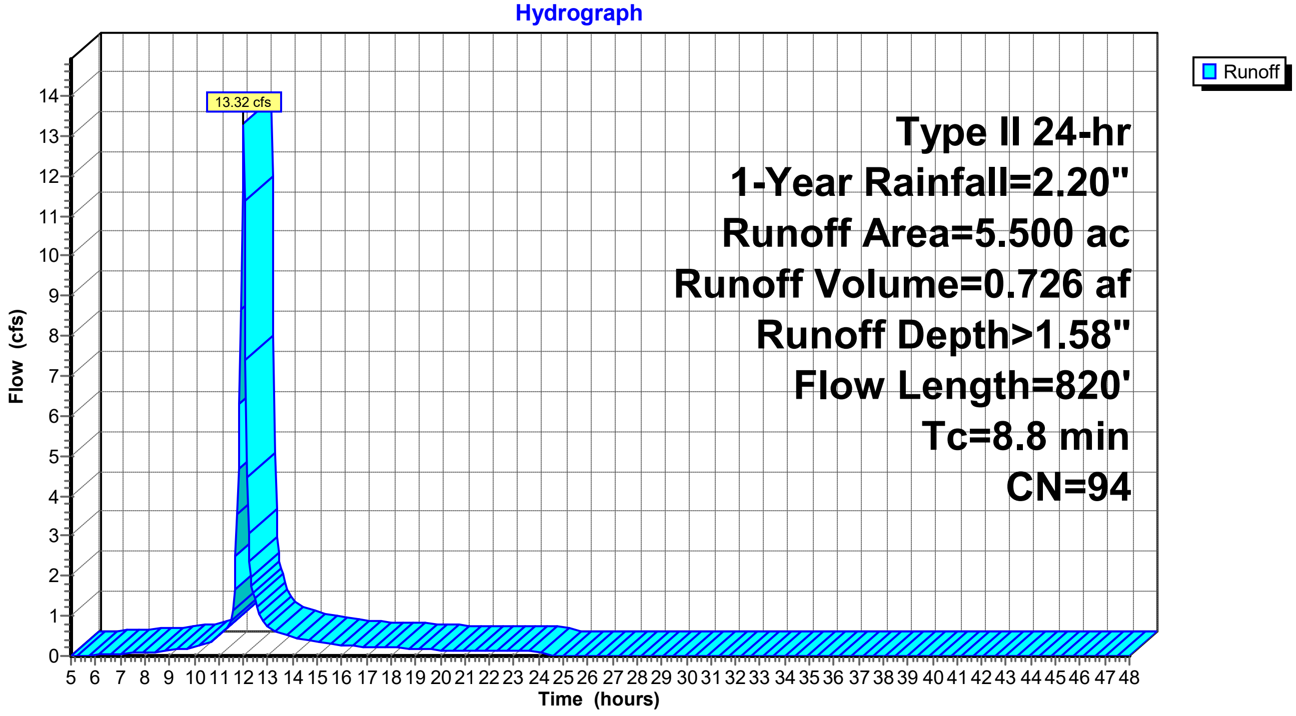
Runoff = 13.32 cfs @ 12.00 hrs, Volume= 0.726 af, Depth> 1.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	5.300	95	Dense Graded Aggregate
	0.200	80	>75% Grass cover, Good, HSG D
	5.500	94	Weighted Average
	5.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
4.9	470	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.6	250	0.0050	6.67	47.16	**Pipe Channel,** 36.0" Round Area= 7.1 sf Perim= 9.4' r= 0.75' n= 0.013
8.8	820	Total			

Subcatchment DR-2: Storage



Summary for Subcatchment DR-3: Rail & Storage

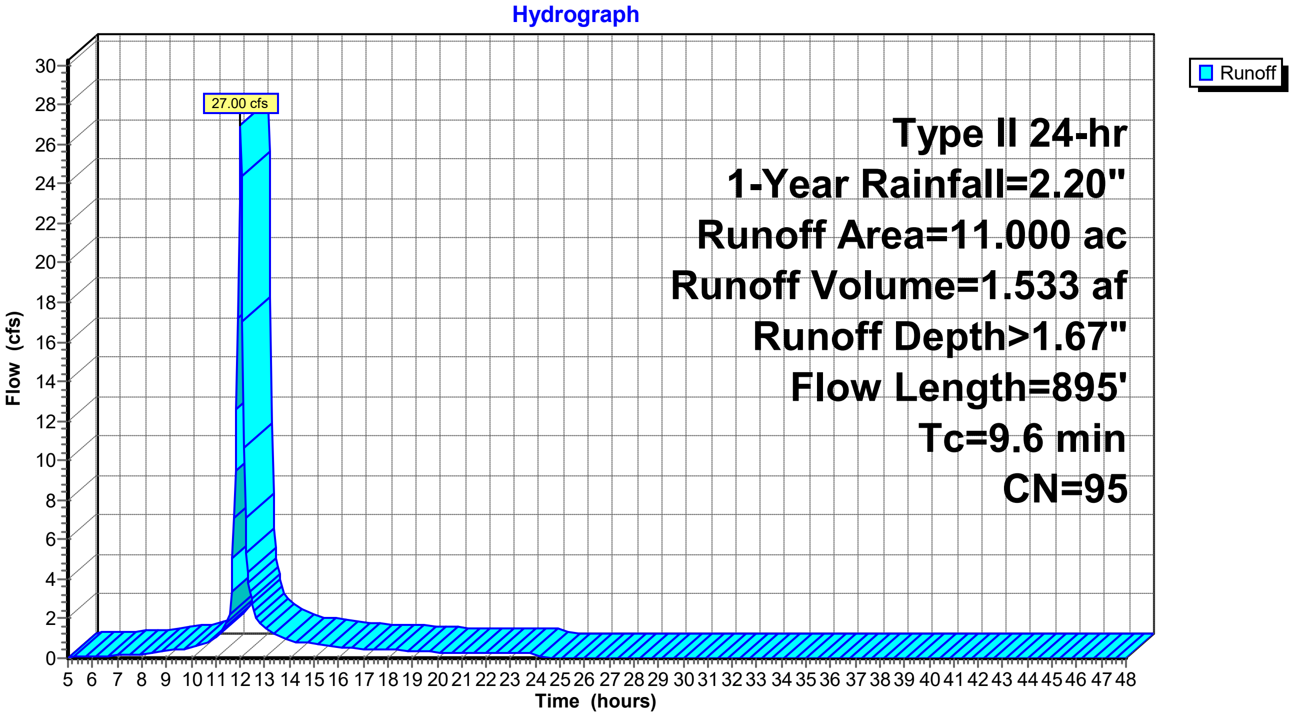
Runoff = 27.00 cfs @ 12.01 hrs, Volume= 1.533 af, Depth> 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	8.300	95	Compacted Gravel
	0.400	80	>75% Grass cover, Good, HSG D
*	2.300	98	Rail
	11.000	95	Weighted Average
	8.700		79.09% Pervious Area
	2.300		20.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
5.4	525	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.9	270	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
9.6	895	Total			

Subcatchment DR-3: Rail & Storage



Summary for Subcatchment DR-4: Storage

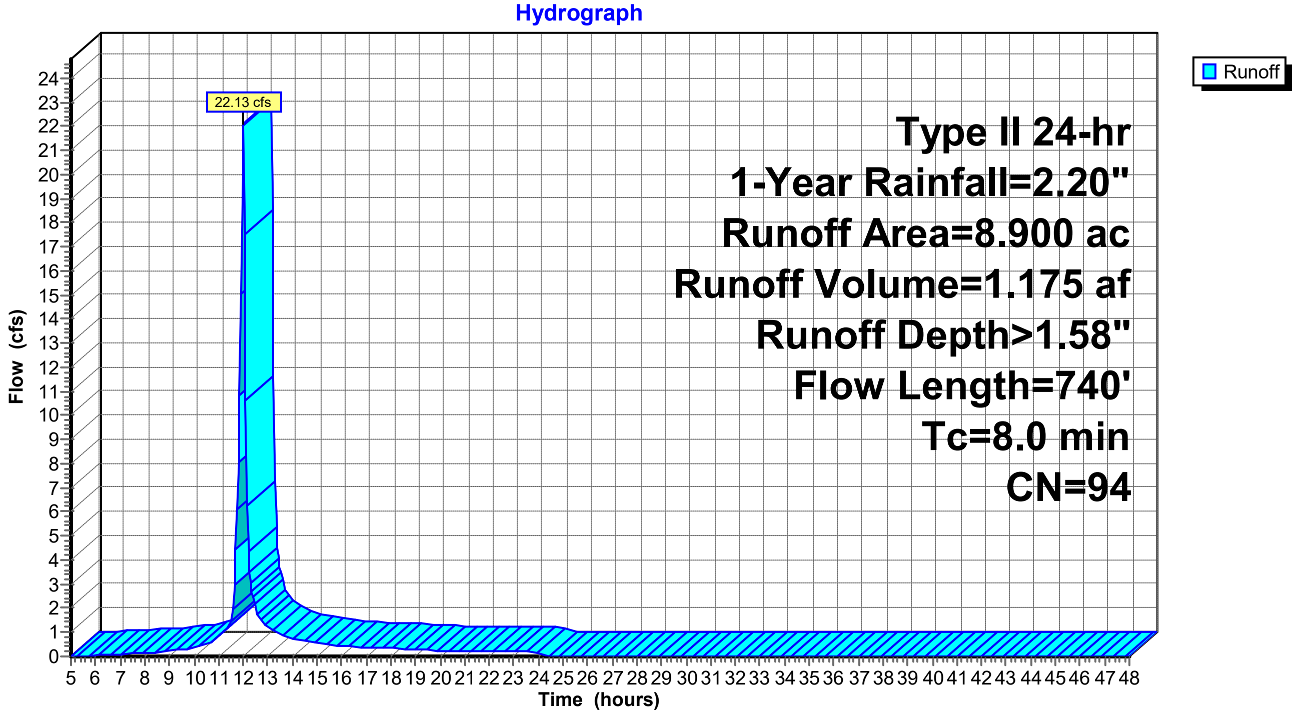
Runoff = 22.13 cfs @ 11.99 hrs, Volume= 1.175 af, Depth> 1.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	8.600	95	Compacted Gravel
	0.300	80	>75% Grass cover, Good, HSG D
	8.900	94	Weighted Average
	8.900		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
4.1	400	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.6	240	0.0050	6.67	47.16	**Pipe Channel,** 36.0" Round Area= 7.1 sf Perim= 9.4' r= 0.75' n= 0.013 Corrugated PE, smooth interior
8.0	740	Total			

Subcatchment DR-4: Storage



Summary for Subcatchment DR-5: Storage

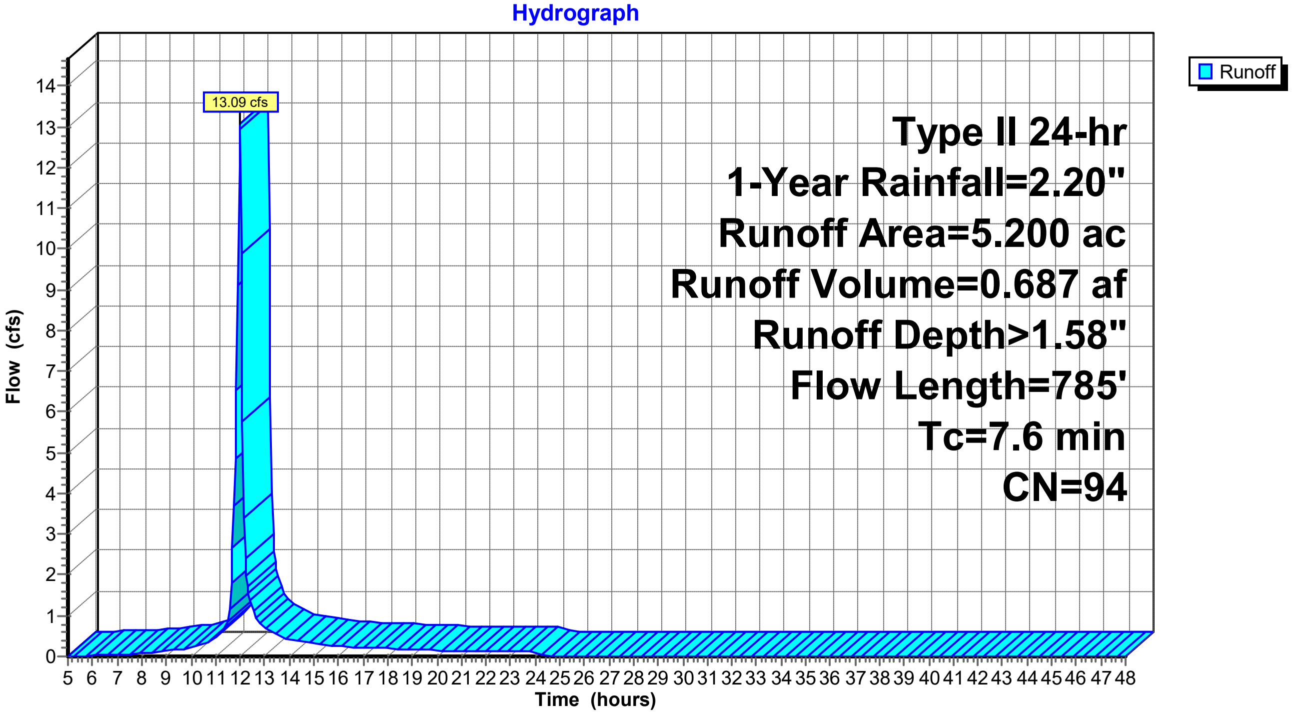
Runoff = 13.09 cfs @ 11.99 hrs, Volume= 0.687 af, Depth> 1.58"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	4.900	95	Dense Graded Aggregate
	0.300	80	>75% Grass cover, Good, HSG D
	5.200	94	Weighted Average
	5.200		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
3.0	285	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
1.3	400	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
7.6	785	Total			

Subcatchment DR-5: Storage



Summary for Subcatchment DR-6: Buldings B & D

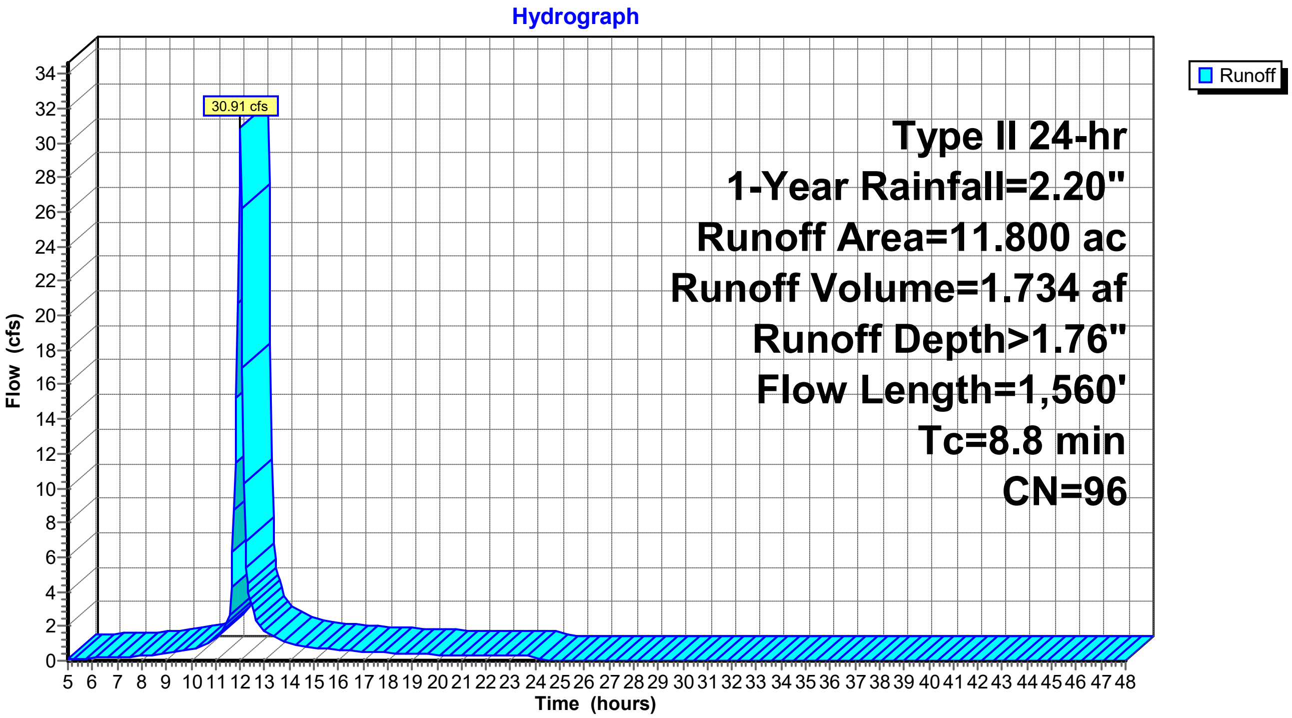
Runoff = 30.91 cfs @ 12.00 hrs, Volume= 1.734 af, Depth> 1.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	2.549	98	Building B
*	1.413	98	Building D
	0.200	80	>75% Grass cover, Good, HSG D
*	7.638	95	Dense Graded Aggregate
	11.800	96	Weighted Average
	7.838		66.42% Pervious Area
	3.962		33.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
1.0	100	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
4.5	1,360	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
8.8	1,560	Total			

Subcatchment DR-6: Buldings B & D



Summary for Subcatchment DR-7: Building C & Rail

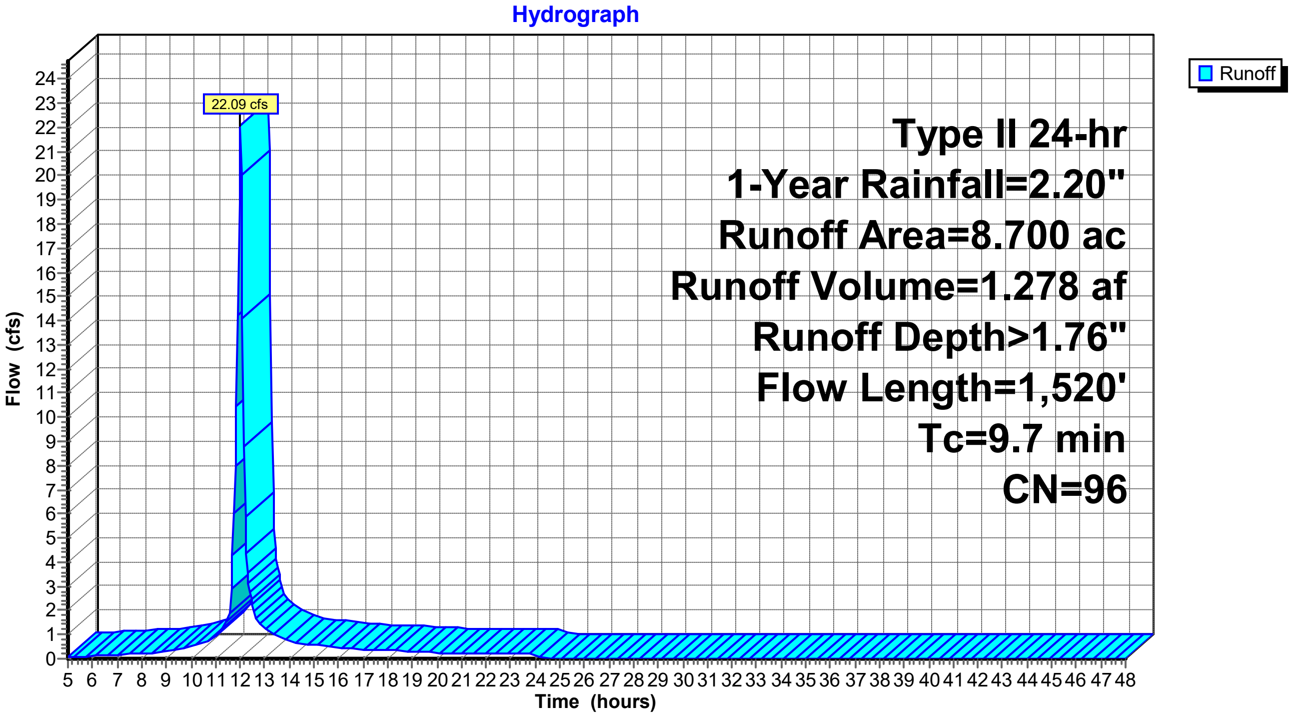
Runoff = 22.09 cfs @ 12.01 hrs, Volume= 1.278 af, Depth> 1.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	3.030	98	Building C
*	0.970	98	Rail
*	4.400	95	Dense Graded Aggregate
	0.300	80	>75% Grass cover, Good, HSG D
	8.700	96	Weighted Average
	4.700		54.02% Pervious Area
	4.000		45.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
2.6	250	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
3.8	1,170	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
9.7	1,520	Total			

Subcatchment DR-7: Building C & Rail



Summary for Subcatchment DR-8A: Parking

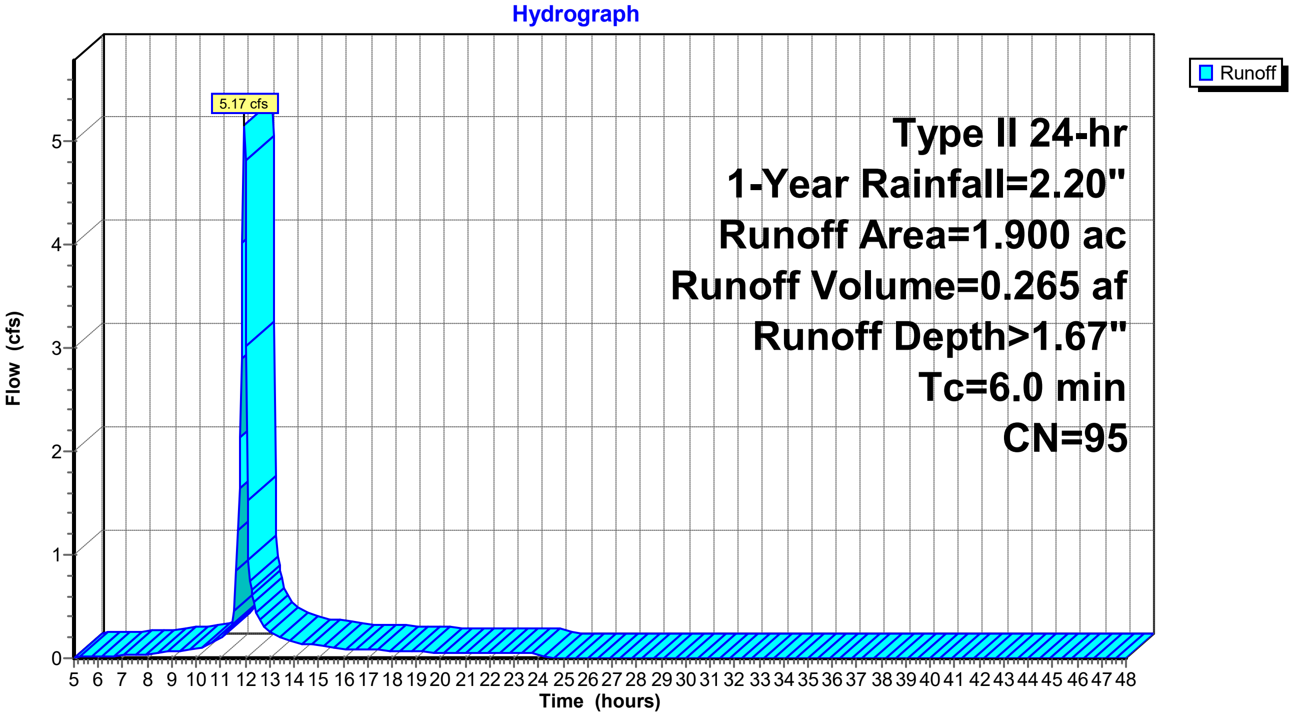
Runoff = 5.17 cfs @ 11.96 hrs, Volume= 0.265 af, Depth> 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	1.600	98	Parking
	0.300	80	>75% Grass cover, Good, HSG D
	1.900	95	Weighted Average
	0.300		15.79% Pervious Area
	1.600		84.21% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-8A: Parking



Summary for Subcatchment DR-8B: Roadway & Pond

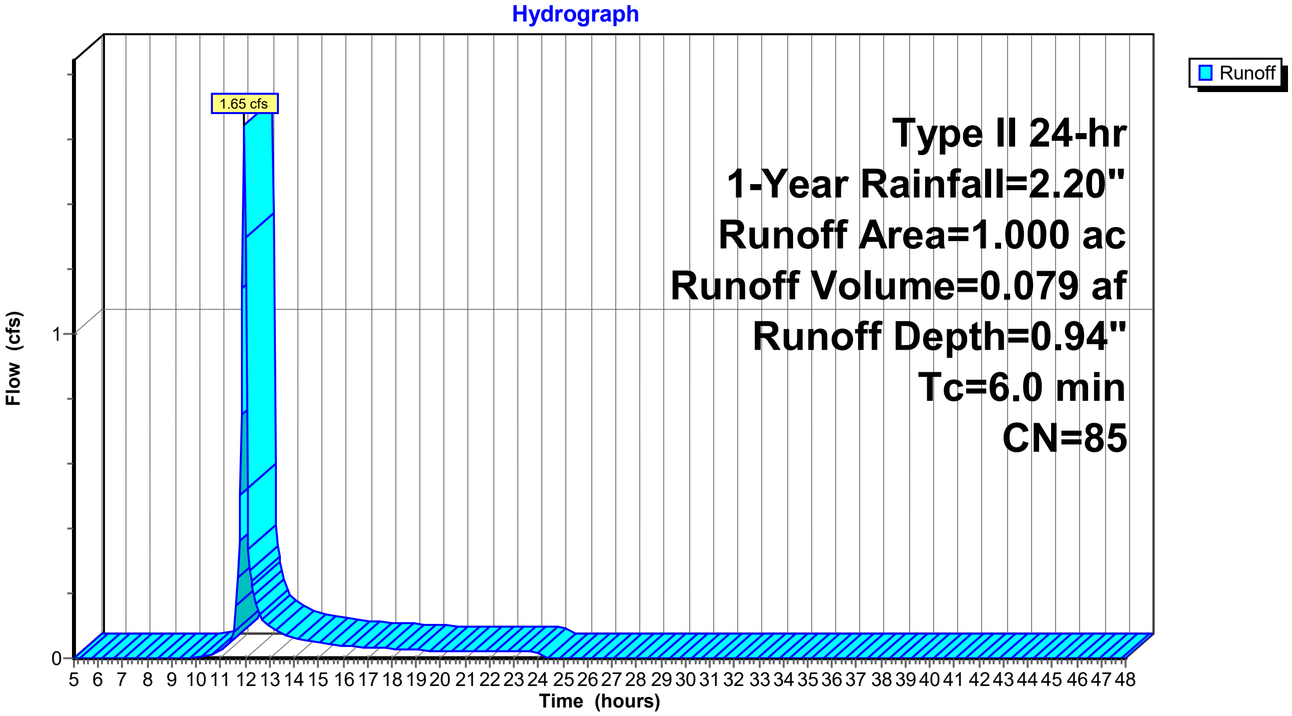
Runoff = 1.65 cfs @ 11.98 hrs, Volume= 0.079 af, Depth= 0.94"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	0.300	98	Roadway
	0.700	80	>75% Grass cover, Good, HSG D
	1.000	85	Weighted Average
	0.700		70.00% Pervious Area
	0.300		30.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-8B: Roadway & Pond



Summary for Subcatchment DR-9A: Parking & Substation

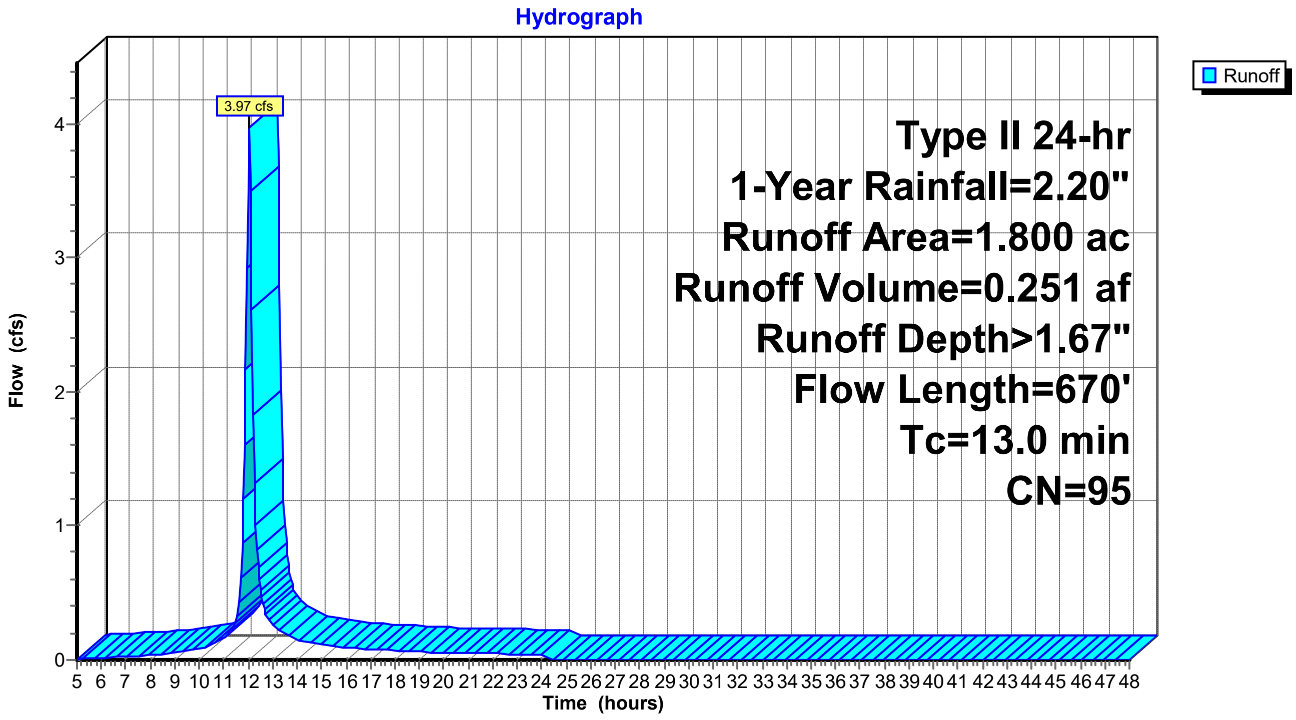
Runoff = 3.97 cfs @ 12.04 hrs, Volume= 0.251 af, Depth> 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
	0.230	80	>75% Grass cover, Good, HSG D
*	0.200	92	Compacted Gravel
*	1.200	98	Parking and Road
*	0.170	98	Substation
	1.800	95	Weighted Average
	0.430		23.89% Pervious Area
	1.370		76.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.4	100	0.0200	1.19		**Sheet Flow, Parking Lot Runoff** Smooth surfaces n= 0.011 P2= 2.40"
11.6	570	0.0030	0.82		**Shallow Concentrated Flow, Grass Lined Ditch to Pond** Grassed Waterway Kv= 15.0 fps
13.0	670	Total			

Subcatchment DR-9A: Parking & Substation



Summary for Subcatchment DR-9B: Roadway

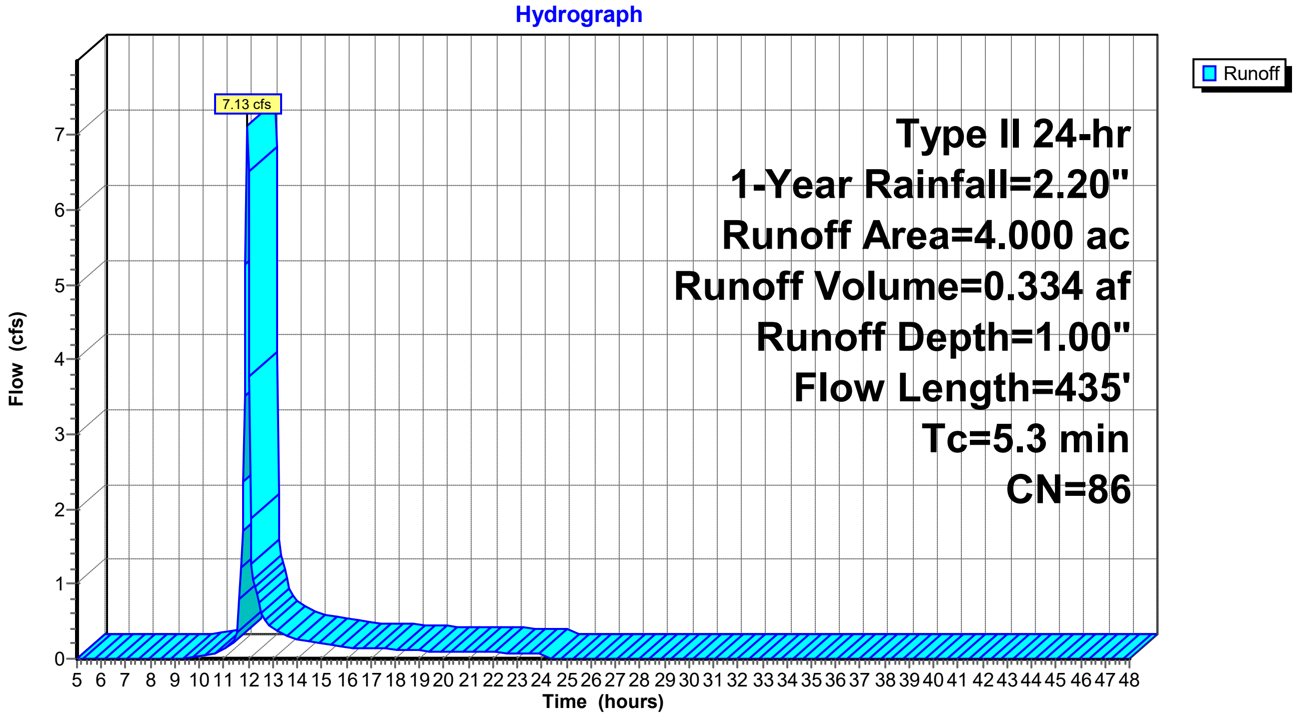
Runoff = 7.13 cfs @ 11.96 hrs, Volume= 0.334 af, Depth= 1.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 1-Year Rainfall=2.20"

	Area (ac)	CN	Description
*	1.050	95	Dense Graded Aggregate
*	0.480	98	Roadway
	2.470	80	>75% Grass cover, Good, HSG D
	4.000	86	Weighted Average
	3.520		88.00% Pervious Area
	0.480		12.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.6	100	0.0250	0.46		**Sheet Flow, Dense Graded Aggregate Yard** n= 0.040 P2= 2.40"
1.3	230	0.0100	3.07	9.20	**Channel Flow, Grass lined ditch** Area= 3.0 sf Perim= 4.0' r= 0.75' n= 0.040 Earth, cobble bottom, clean sides
0.4	105	0.0050	4.20	7.43	**Pipe Channel, driveway culvert** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013 Corrugated PE, smooth interior
5.3	435	Total			

Subcatchment DR-9B: Roadway



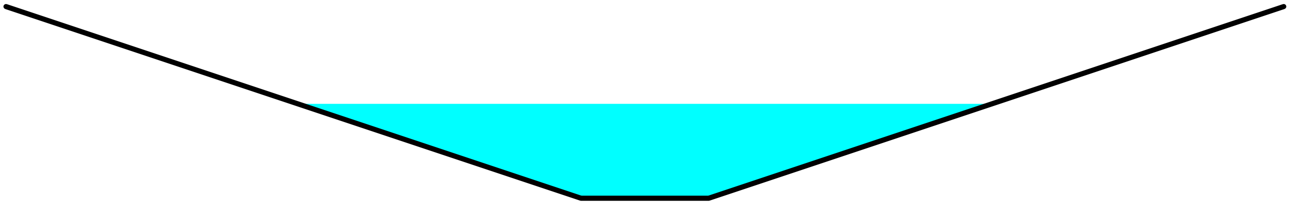
Summary for Reach 1R: Swale

Inflow Area = 1.900 ac, 84.21% Impervious, Inflow Depth > 1.67" for 1-Year event
Inflow = 5.17 cfs @ 11.96 hrs, Volume= 0.265 af
Outflow = 4.10 cfs @ 12.13 hrs, Volume= 0.265 af, Atten= 21%, Lag= 9.9 min

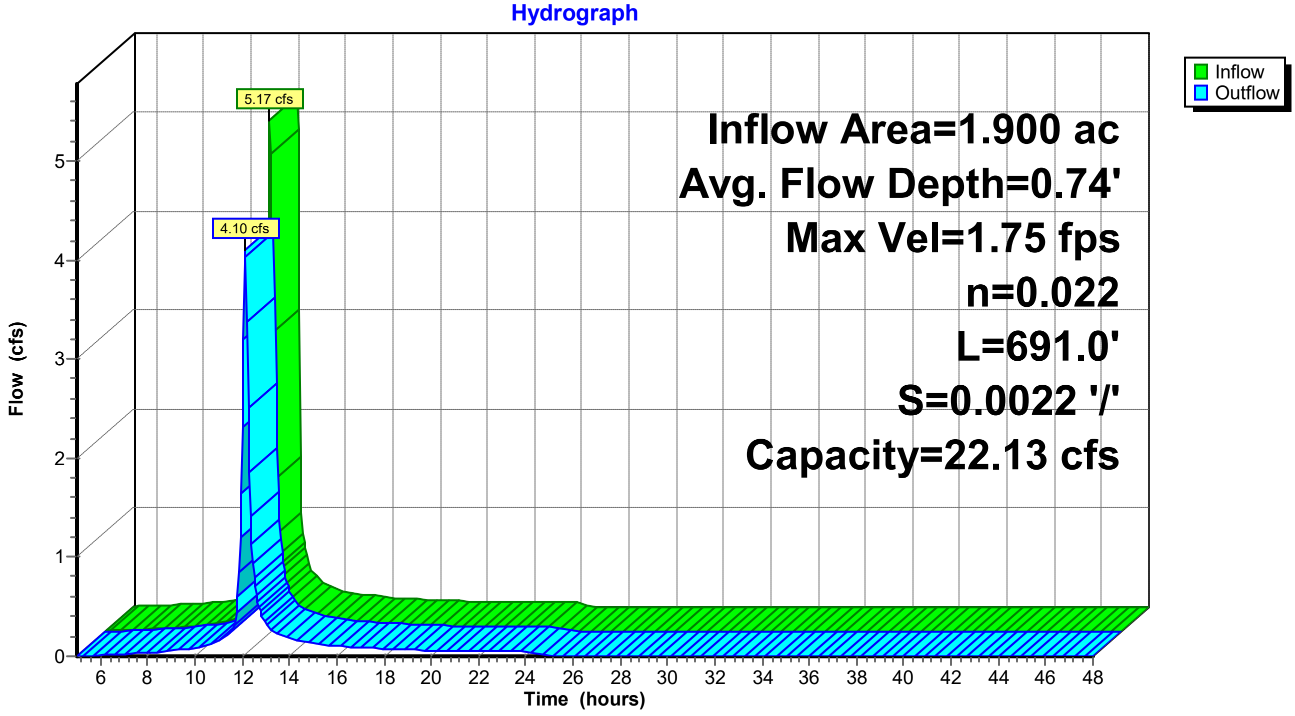
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.75 fps, Min. Travel Time= 6.6 min
Avg. Velocity = 0.50 fps, Avg. Travel Time= 22.9 min

Peak Storage= 1,638 cf @ 12.02 hrs
Average Depth at Peak Storage= 0.74' , Surface Width= 5.43'
Bank-Full Depth= 1.50' Flow Area= 8.3 sf, Capacity= 22.13 cfs

1.00' x 1.50' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 10.00'
Length= 691.0' Slope= 0.0022 '/'
Inlet Invert= 15.50', Outlet Invert= 14.00'



Reach 1R: Swale



Summary for Reach 1W: Wetland #1 / Analysis Point 1A

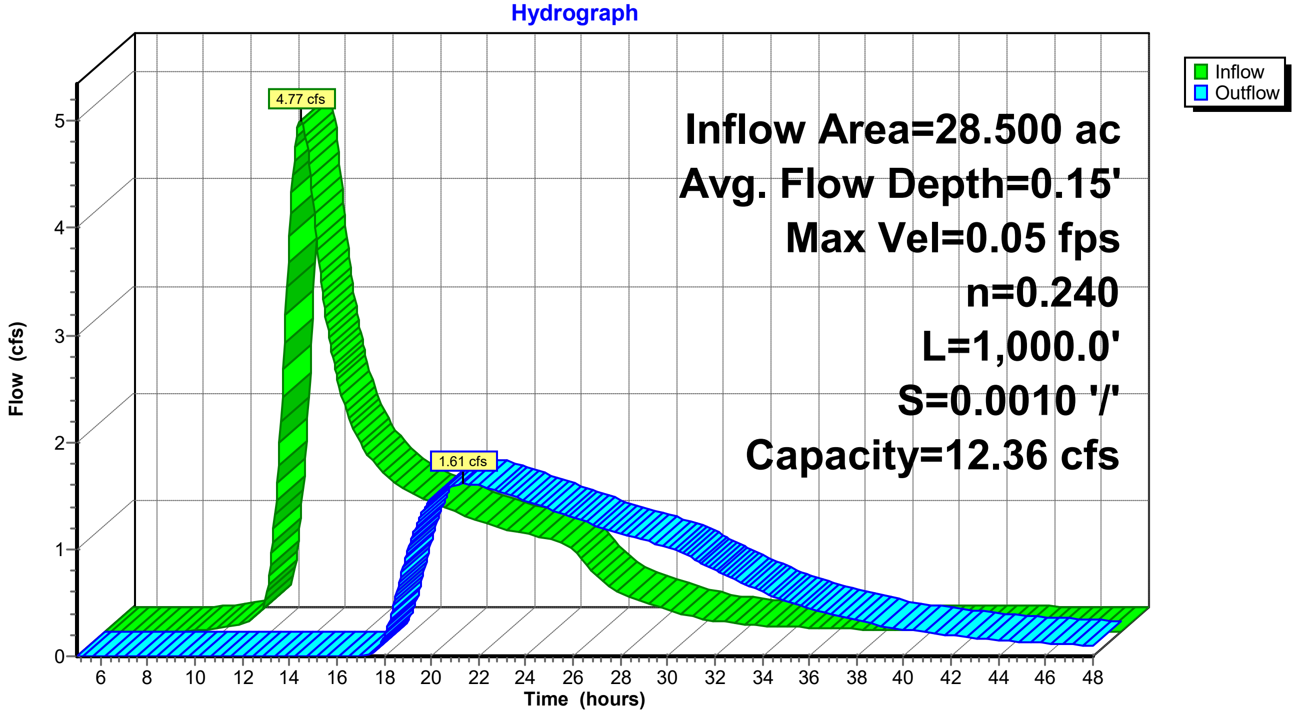
Inflow Area = 28.500 ac, 13.16% Impervious, Inflow Depth > 0.83" for 1-Year event
Inflow = 4.77 cfs @ 13.30 hrs, Volume= 1.982 af
Outflow = 1.61 cfs @ 21.37 hrs, Volume= 1.851 af, Atten= 66%, Lag= 484.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.05 fps, Min. Travel Time= 305.5 min
Avg. Velocity = 0.03 fps, Avg. Travel Time= 568.8 min

Peak Storage= 29,542 cf @ 16.28 hrs
Average Depth at Peak Storage= 0.15' , Surface Width= 200.88'
Bank-Full Depth= 0.50' Flow Area= 100.8 sf, Capacity= 12.36 cfs

200.00' x 0.50' deep channel, n= 0.240
Side Slope Z-value= 3.0 '/' Top Width= 203.00'
Length= 1,000.0' Slope= 0.0010 '/'
Inlet Invert= 6.00', Outlet Invert= 5.00'

Reach 1W: Wetland #1 / Analysis Point 1A



Summary for Reach 2R: Overflow

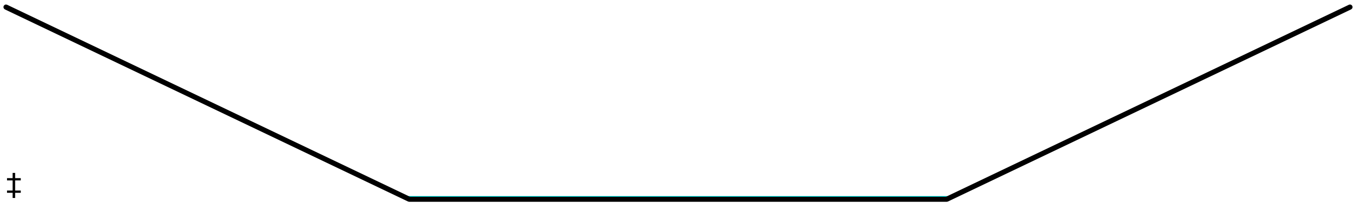
Inflow Area =	5.800 ac,	31.90% Impervious,	Inflow Depth > 1.20"	for 1-Year event	
Inflow =	0.48 cfs @	13.64 hrs,	Volume=	0.581 af	
Outflow =	0.48 cfs @	13.65 hrs,	Volume=	0.581 af,	Atten= 0%, Lag= 0.8 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.78 fps, Min. Travel Time= 0.5 min
Avg. Velocity = 1.38 fps, Avg. Travel Time= 0.6 min

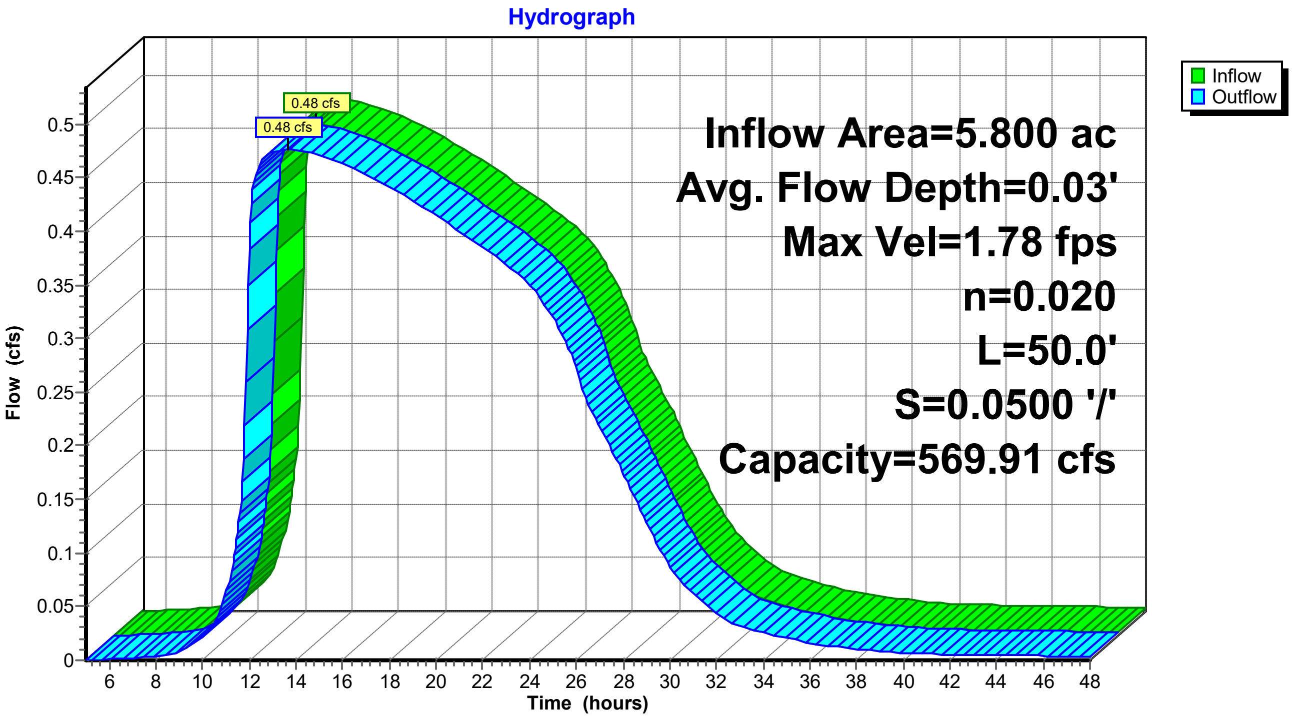
Peak Storage= 13 cf @ 13.64 hrs
Average Depth at Peak Storage= 0.03' , Surface Width= 8.20'
Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 569.91 cfs

8.00' x 2.00' deep channel, n= 0.020 Corrugated PE, corrugated interior
Side Slope Z-value= 3.0 '/' Top Width= 20.00'
Length= 50.0' Slope= 0.0500 '/'
Inlet Invert= 16.50', Outlet Invert= 14.00'

‡



Reach 2R: Overflow



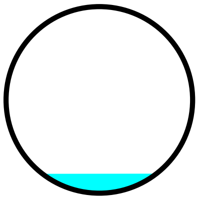
Summary for Reach 3R: Outlet Pipe

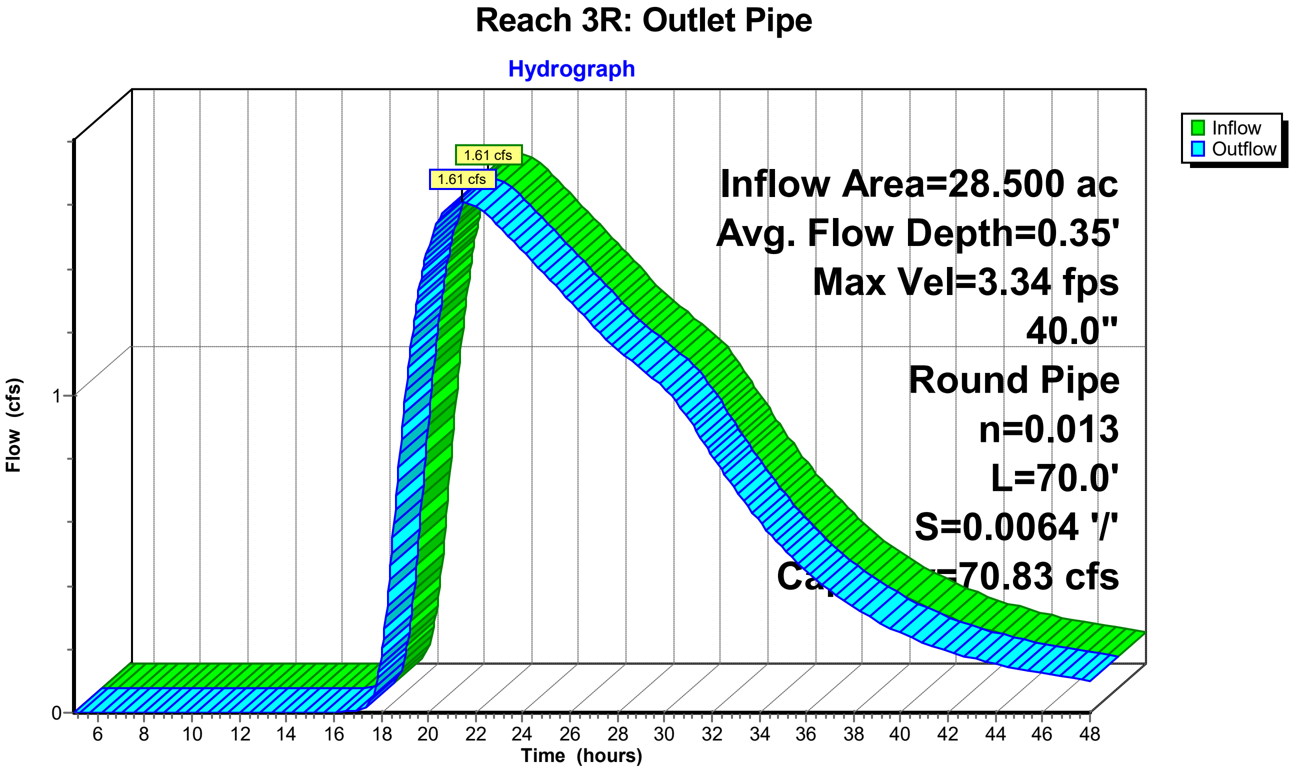
Inflow Area = 28.500 ac, 13.16% Impervious, Inflow Depth > 0.78" for 1-Year event
Inflow = 1.61 cfs @ 21.37 hrs, Volume= 1.851 af
Outflow = 1.61 cfs @ 21.38 hrs, Volume= 1.851 af, Atten= 0%, Lag= 0.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.34 fps, Min. Travel Time= 0.3 min
Avg. Velocity = 2.22 fps, Avg. Travel Time= 0.5 min

Peak Storage= 34 cf @ 21.38 hrs
Average Depth at Peak Storage= 0.35' , Surface Width= 2.04'
Bank-Full Depth= 3.33' Flow Area= 8.7 sf, Capacity= 70.83 cfs

40.0" Round Pipe
n= 0.013 Corrugated PE, smooth interior
Length= 70.0' Slope= 0.0064 '/'
Inlet Invert= 4.25', Outlet Invert= 3.80'





Summary for Reach 4R: Overflow

Inflow Area = 1.000 ac, 95.00% Impervious, Inflow Depth > 1.67" for 1-Year event
Inflow = 2.65 cfs @ 11.99 hrs, Volume= 0.139 af
Outflow = 2.56 cfs @ 12.00 hrs, Volume= 0.139 af, Atten= 3%, Lag= 1.0 min

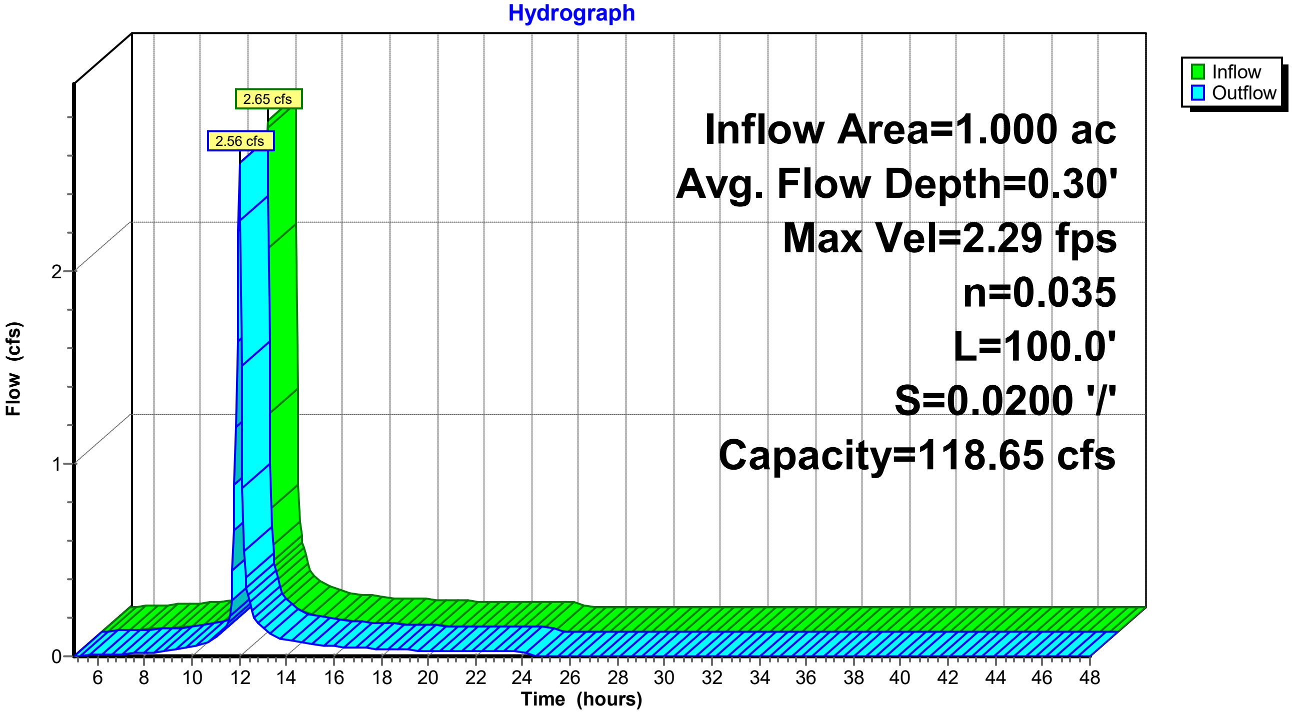
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.29 fps, Min. Travel Time= 0.7 min
Avg. Velocity = 0.58 fps, Avg. Travel Time= 2.9 min

Peak Storage= 115 cf @ 11.99 hrs
Average Depth at Peak Storage= 0.30' , Surface Width= 4.77'
Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 118.65 cfs

3.00' x 2.00' deep channel, n= 0.035 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 15.00'
Length= 100.0' Slope= 0.0200 '/'
Inlet Invert= 12.00', Outlet Invert= 10.00'

‡

Reach 4R: Overflow



Summary for Reach 5R: Overflow

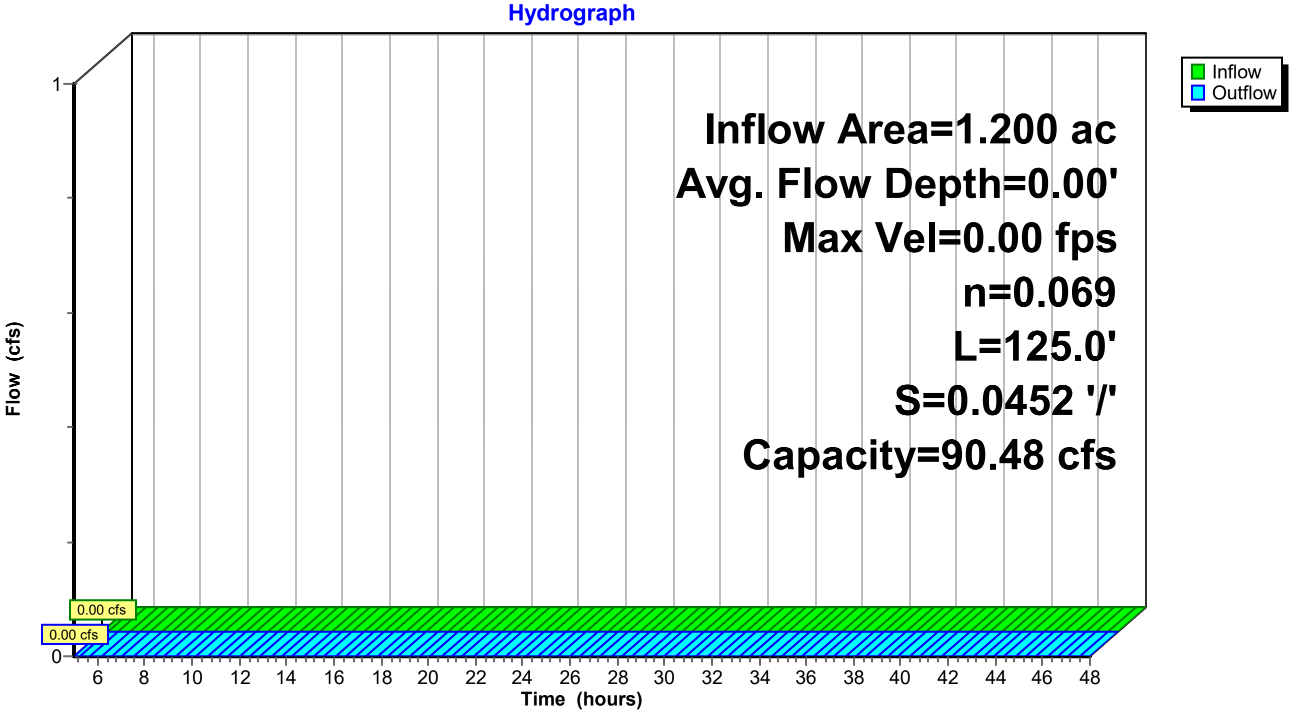
Inflow Area = 1.200 ac, 37.50% Impervious, Inflow Depth = 0.00" for 1-Year event
Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 90.48 cfs

3.00' x 2.00' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 15.00'
Length= 125.0' Slope= 0.0452 '/'
Inlet Invert= 11.65', Outlet Invert= 6.00'

Reach 5R: Overflow



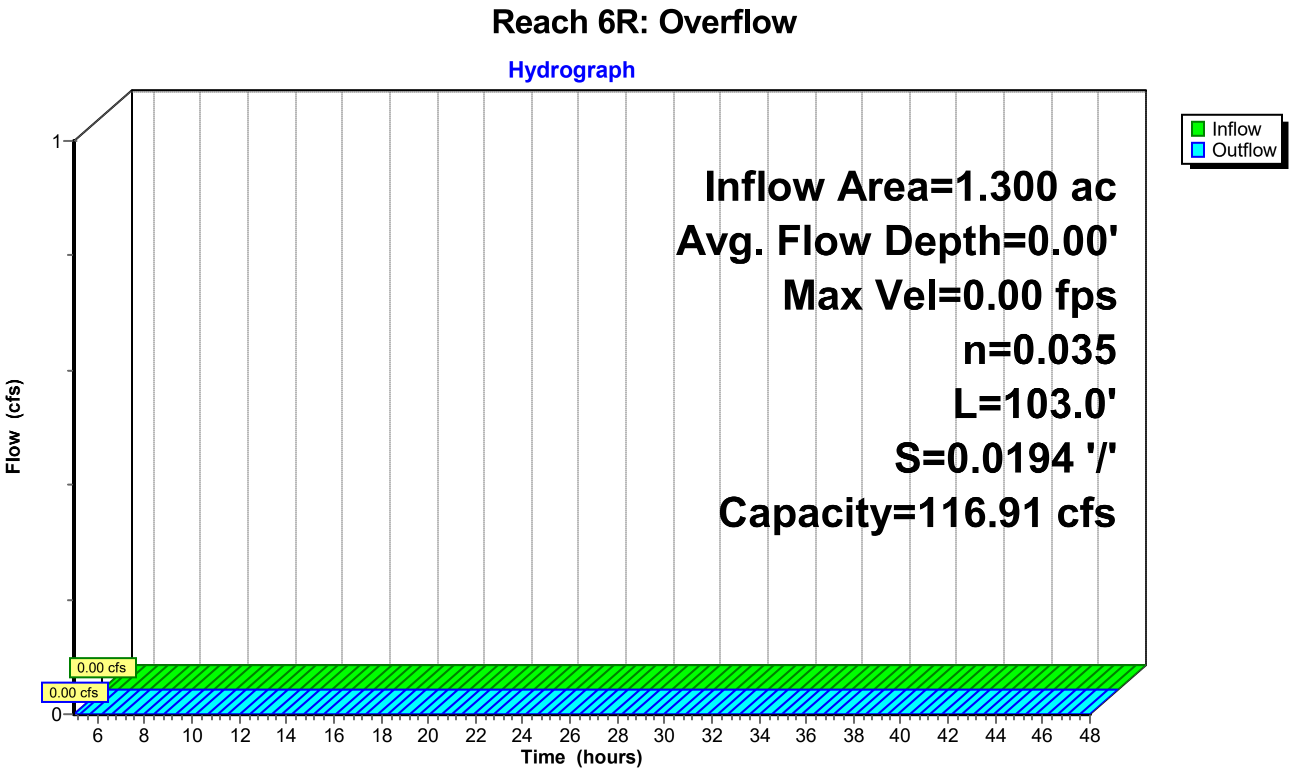
Summary for Reach 6R: Overflow

Inflow Area = 1.300 ac, 57.69% Impervious, Inflow Depth = 0.00" for 1-Year event
Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 116.91 cfs

3.00' x 2.00' deep channel, n= 0.035 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 15.00'
Length= 103.0' Slope= 0.0194 '/'
Inlet Invert= 8.50', Outlet Invert= 6.50'



Summary for Reach 7R: Overflow

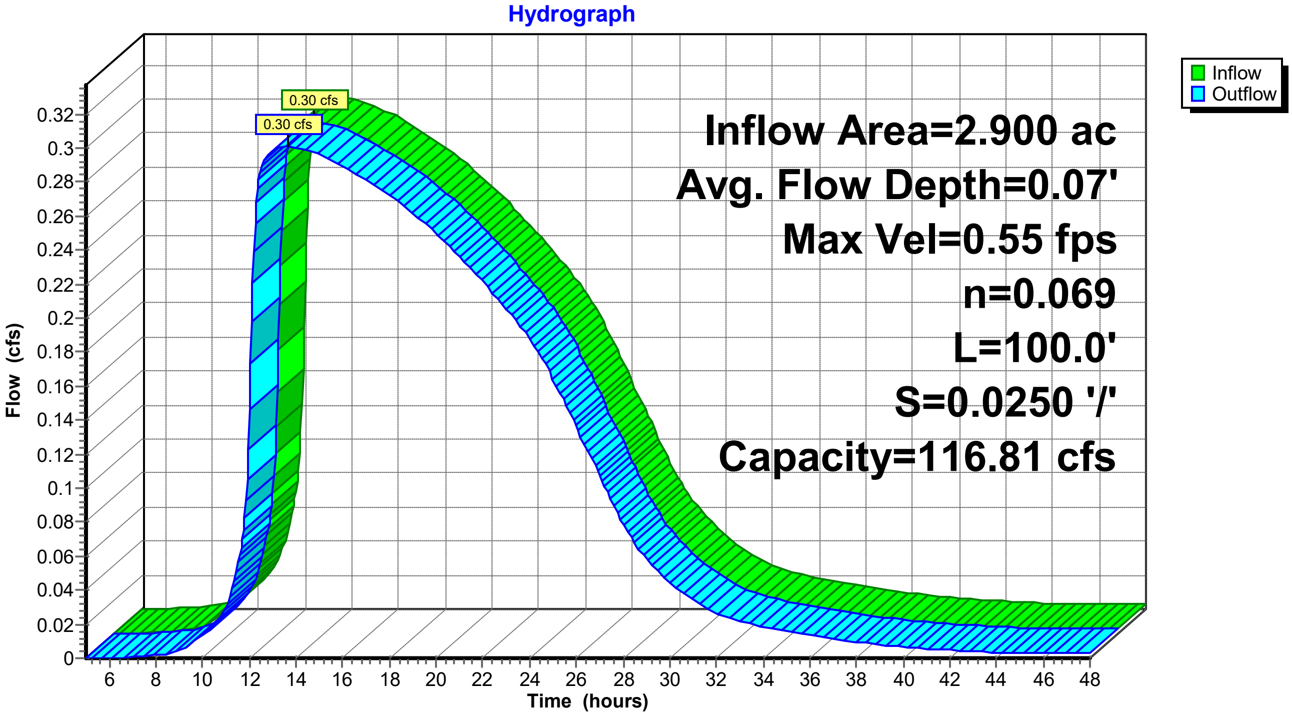
Inflow Area = 2.900 ac, 65.52% Impervious, Inflow Depth > 1.40" for 1-Year event
Inflow = 0.30 cfs @ 13.53 hrs, Volume= 0.339 af
Outflow = 0.30 cfs @ 13.61 hrs, Volume= 0.339 af, Atten= 0%, Lag= 4.9 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.55 fps, Min. Travel Time= 3.0 min
Avg. Velocity = 0.34 fps, Avg. Travel Time= 4.8 min

Peak Storage= 54 cf @ 13.56 hrs
Average Depth at Peak Storage= 0.07' , Surface Width= 8.40'
Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 116.81 cfs

8.00' x 2.00' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 20.00'
Length= 100.0' Slope= 0.0250 '/'
Inlet Invert= 14.50', Outlet Invert= 12.00'

Reach 7R: Overflow



Summary for Reach 8R: Dry Swale #1

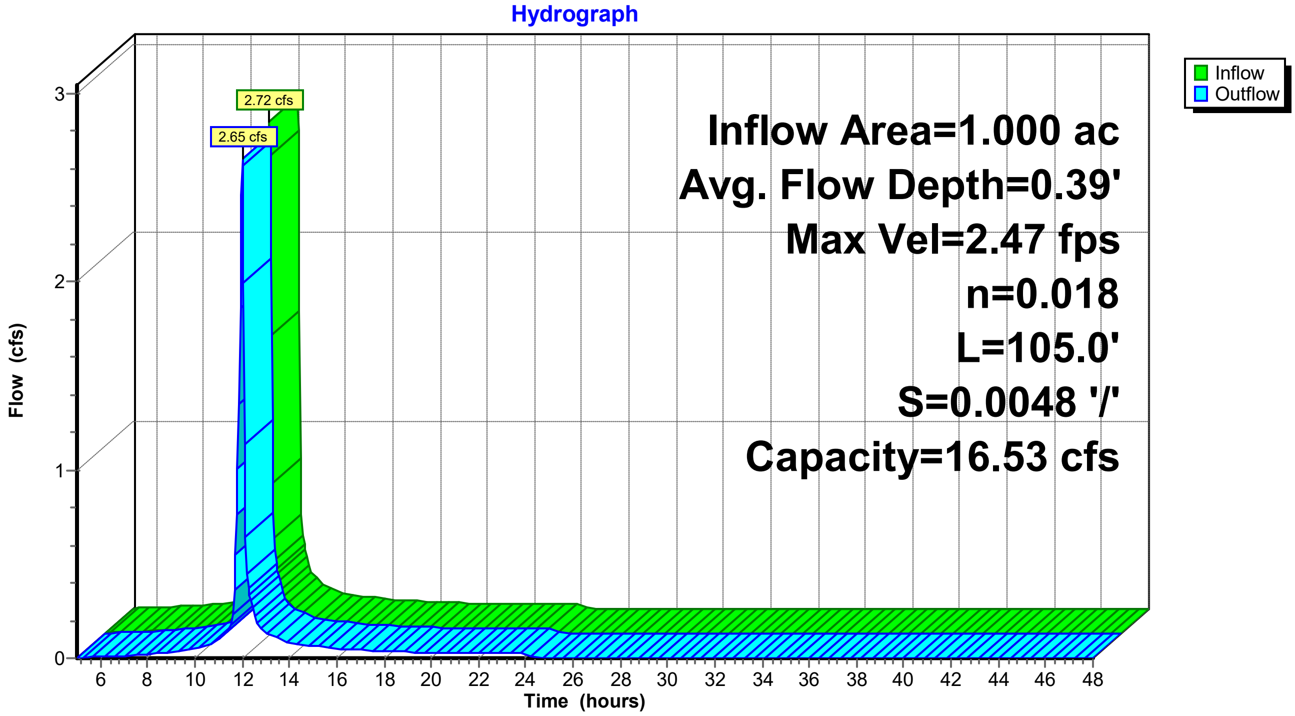
Inflow Area =	1.000 ac, 95.00% Impervious, Inflow Depth > 1.67"	for 1-Year event	
Inflow =	2.72 cfs @ 11.96 hrs, Volume=	0.139 af	
Outflow =	2.65 cfs @ 11.99 hrs, Volume=	0.139 af, Atten= 3%, Lag= 1.2 min	

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.47 fps, Min. Travel Time= 0.7 min
Avg. Velocity = 0.61 fps, Avg. Travel Time= 2.9 min

Peak Storage= 114 cf @ 11.98 hrs
Average Depth at Peak Storage= 0.39' , Surface Width= 3.57'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 16.53 cfs

2.00' x 1.00' deep channel, n= 0.018 Earth, clean & straight
Side Slope Z-value= 2.0 '/' Top Width= 6.00'
Length= 105.0' Slope= 0.0048 '/'
Inlet Invert= 10.00', Outlet Invert= 9.50'

Reach 8R: Dry Swale #1



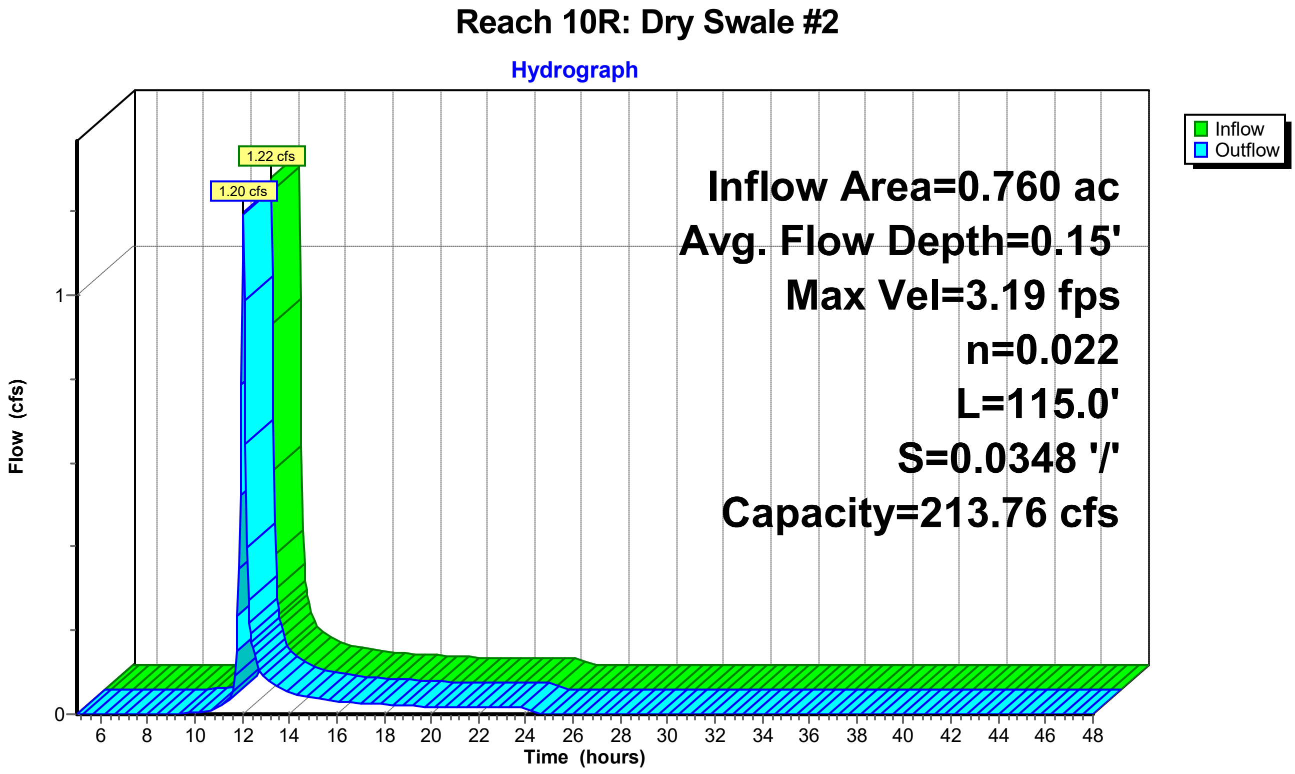
Summary for Reach 10R: Dry Swale #2

Inflow Area =	0.760 ac,	78.95% Impervious,	Inflow Depth = 1.00"	for 1-Year event
Inflow =	1.22 cfs @	12.03 hrs, Volume=	0.064 af	
Outflow =	1.20 cfs @	12.04 hrs, Volume=	0.064 af, Atten= 2%, Lag= 0.9 min	

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.19 fps, Min. Travel Time= 0.6 min
Avg. Velocity = 1.01 fps, Avg. Travel Time= 1.9 min

Peak Storage= 44 cf @ 12.03 hrs
Average Depth at Peak Storage= 0.15' , Surface Width= 2.93'
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 213.76 cfs

2.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 14.00'
Length= 115.0' Slope= 0.0348 '/'
Inlet Invert= 37.00', Outlet Invert= 33.00'



Summary for Reach 12R: Sediment Basin Overflow

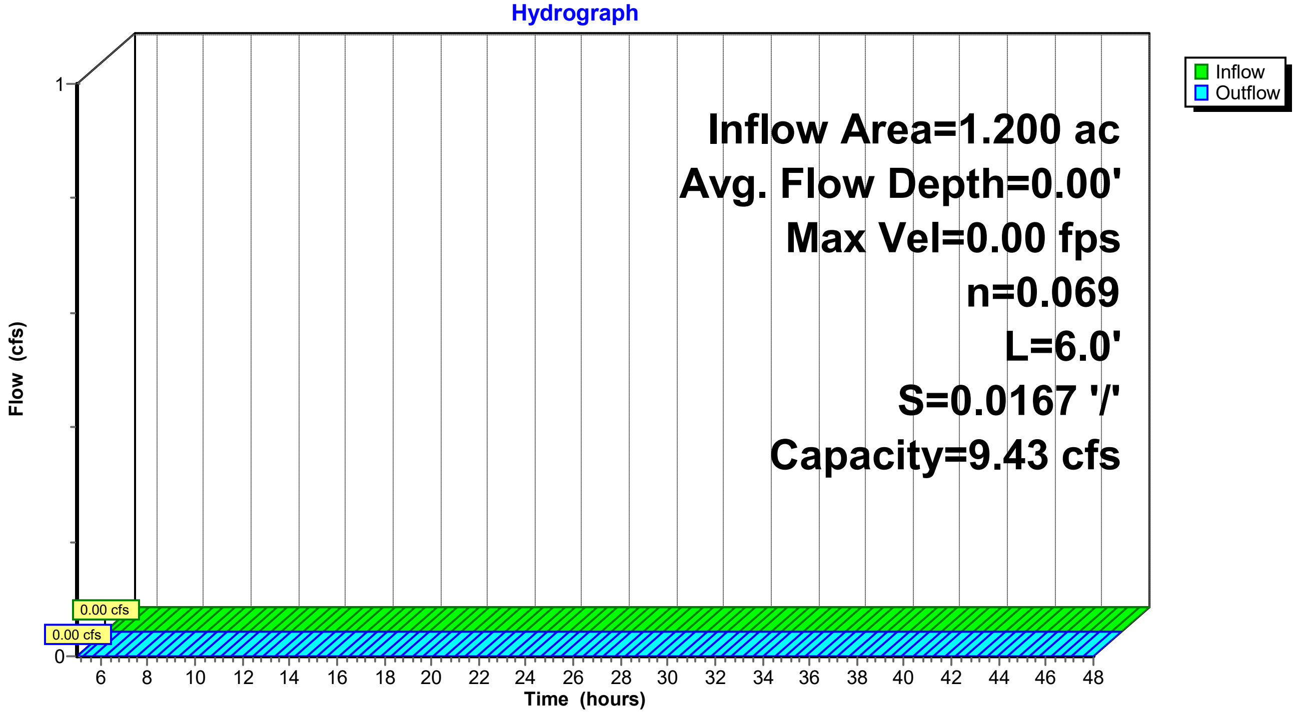
Inflow Area = 1.200 ac, 37.50% Impervious, Inflow Depth = 0.00" for 1-Year event
Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 0.50' Flow Area= 6.0 sf, Capacity= 9.43 cfs

10.00' x 0.50' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 4.0 '/' Top Width= 14.00'
Length= 6.0' Slope= 0.0167 '/'
Inlet Invert= 12.00', Outlet Invert= 11.90'

Reach 12R: Sediment Basin Overflow



Summary for Reach 13R: Roadside Swale

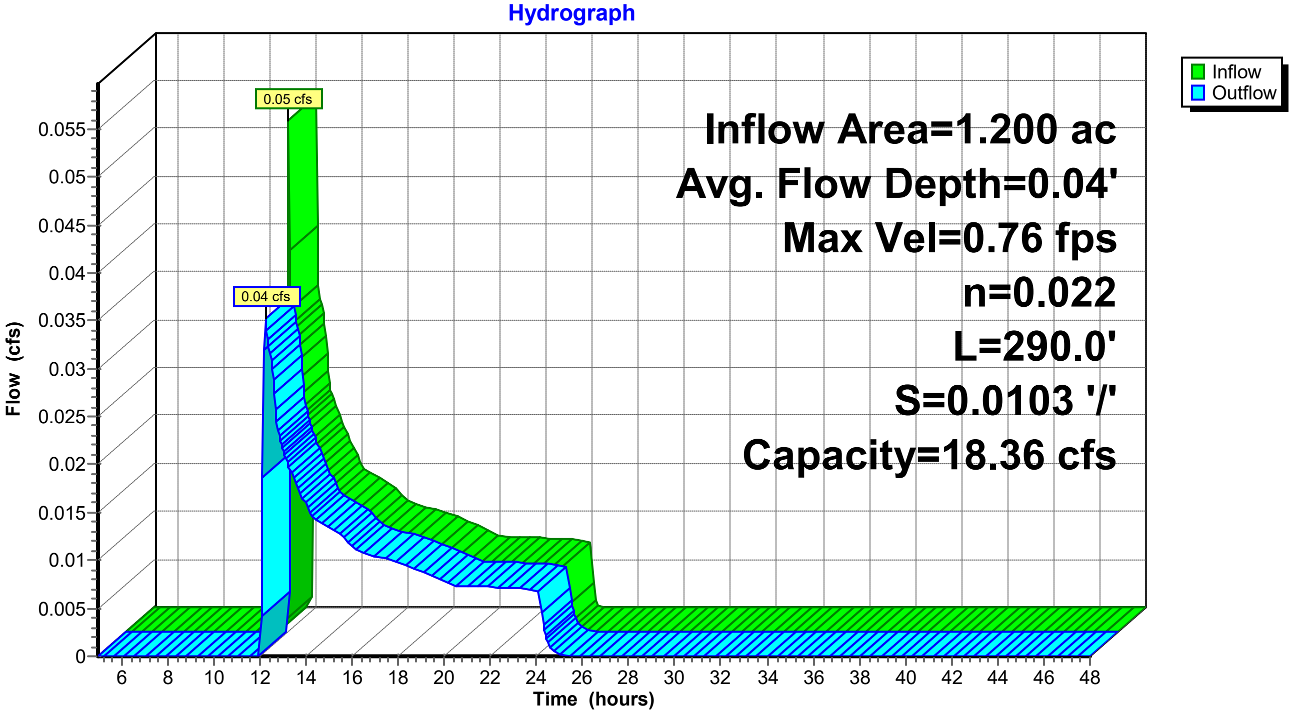
Inflow Area = 1.200 ac, 37.50% Impervious, Inflow Depth = 0.12" for 1-Year event
Inflow = 0.05 cfs @ 12.05 hrs, Volume= 0.012 af
Outflow = 0.04 cfs @ 12.26 hrs, Volume= 0.012 af, Atten= 34%, Lag= 12.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.76 fps, Min. Travel Time= 6.4 min
Avg. Velocity = 0.48 fps, Avg. Travel Time= 10.0 min

Peak Storage= 13 cf @ 12.15 hrs
Average Depth at Peak Storage= 0.04' , Surface Width= 1.25'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 18.36 cfs

1.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 7.00'
Length= 290.0' Slope= 0.0103 '/'
Inlet Invert= 15.00', Outlet Invert= 12.00'

Reach 13R: Roadside Swale



Summary for Reach 14R: Roadside Swale

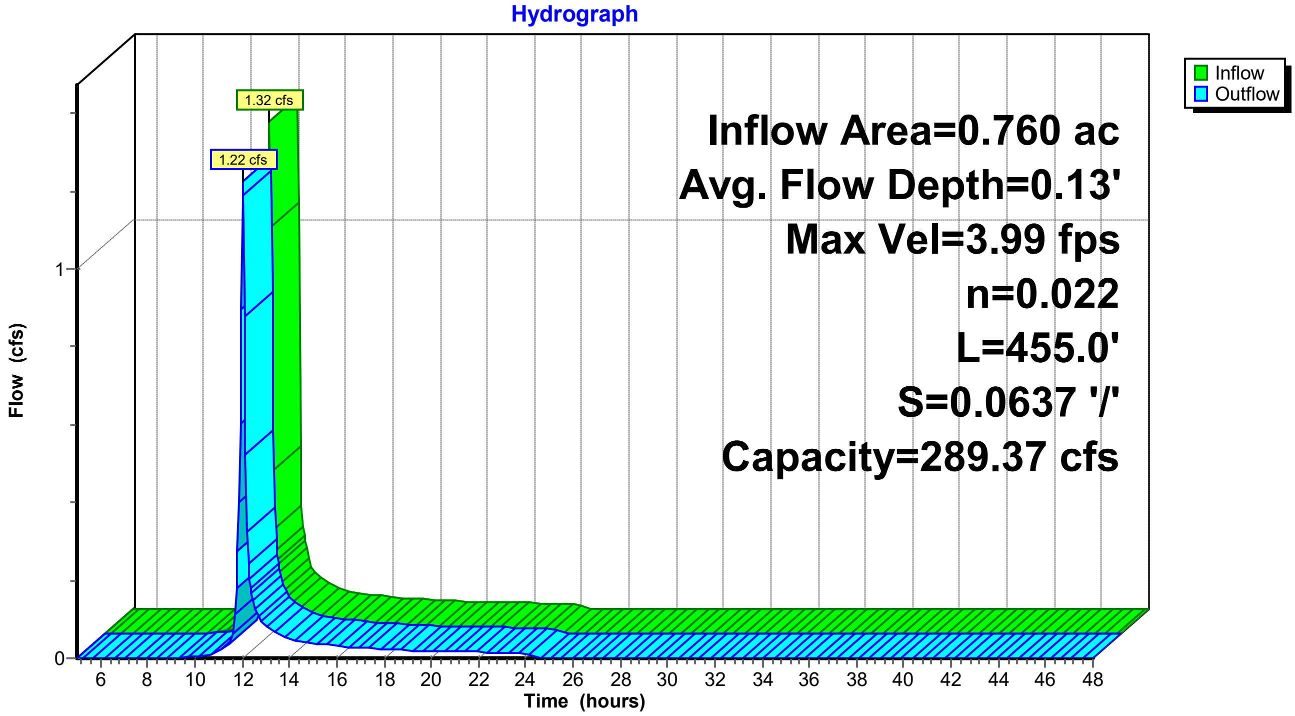
Inflow Area = 0.760 ac, 78.95% Impervious, Inflow Depth = 1.00" for 1-Year event
Inflow = 1.32 cfs @ 11.97 hrs, Volume= 0.064 af
Outflow = 1.22 cfs @ 12.03 hrs, Volume= 0.064 af, Atten= 7%, Lag= 3.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.99 fps, Min. Travel Time= 1.9 min
Avg. Velocity = 1.33 fps, Avg. Travel Time= 5.7 min

Peak Storage= 146 cf @ 11.99 hrs
Average Depth at Peak Storage= 0.13' , Surface Width= 2.80'
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 289.37 cfs

2.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 14.00'
Length= 455.0' Slope= 0.0637 '/'
Inlet Invert= 66.00', Outlet Invert= 37.00'

Reach 14R: Roadside Swale



Summary for Reach 15R: Roadside Swale

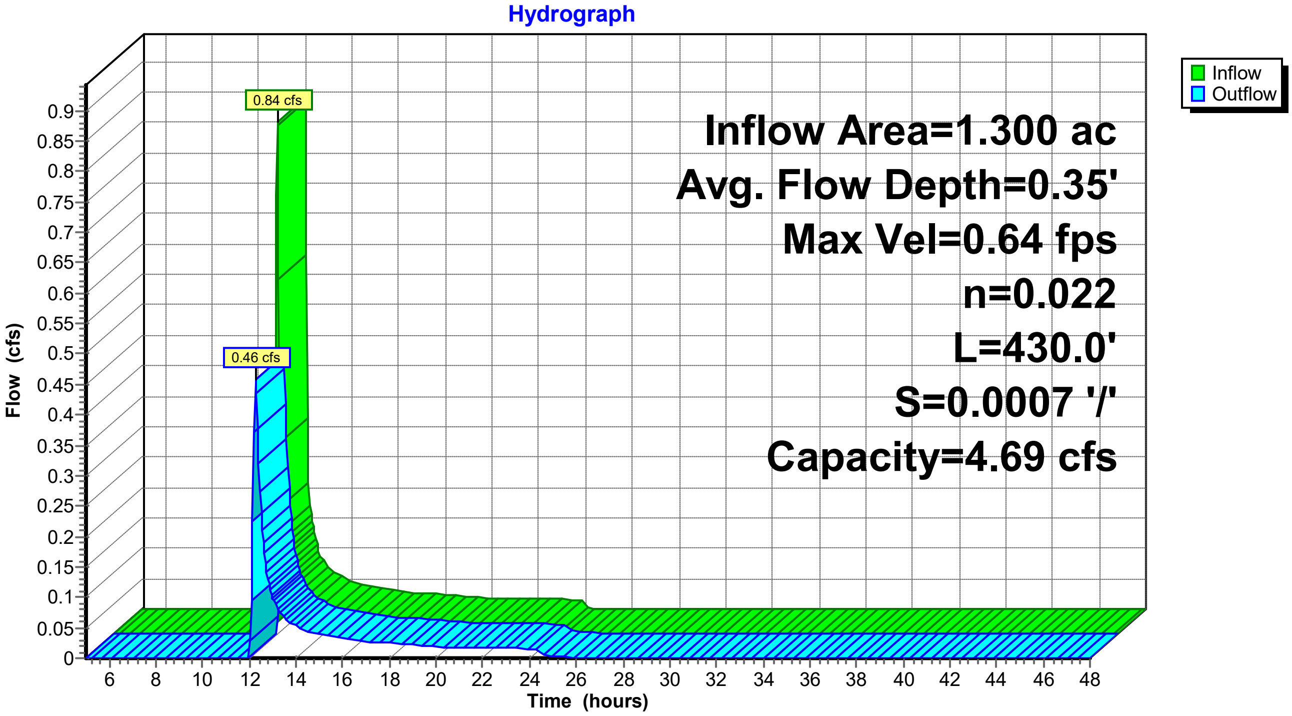
Inflow Area =	1.300 ac,	57.69% Impervious,	Inflow Depth = 0.41"	for 1-Year event	
Inflow =	0.84 cfs @	11.99 hrs, Volume=	0.045 af		
Outflow =	0.46 cfs @	12.27 hrs, Volume=	0.045 af,	Atten= 46%,	Lag= 16.5 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.64 fps, Min. Travel Time= 11.2 min
Avg. Velocity = 0.21 fps, Avg. Travel Time= 34.0 min

Peak Storage= 310 cf @ 12.07 hrs
Average Depth at Peak Storage= 0.35' , Surface Width= 3.11'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 4.69 cfs

1.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 7.00'
Length= 430.0' Slope= 0.0007 '/'
Inlet Invert= 10.00', Outlet Invert= 9.71'

Reach 15R: Roadside Swale



Summary for Reach 17R: Sediment Basin Overflow

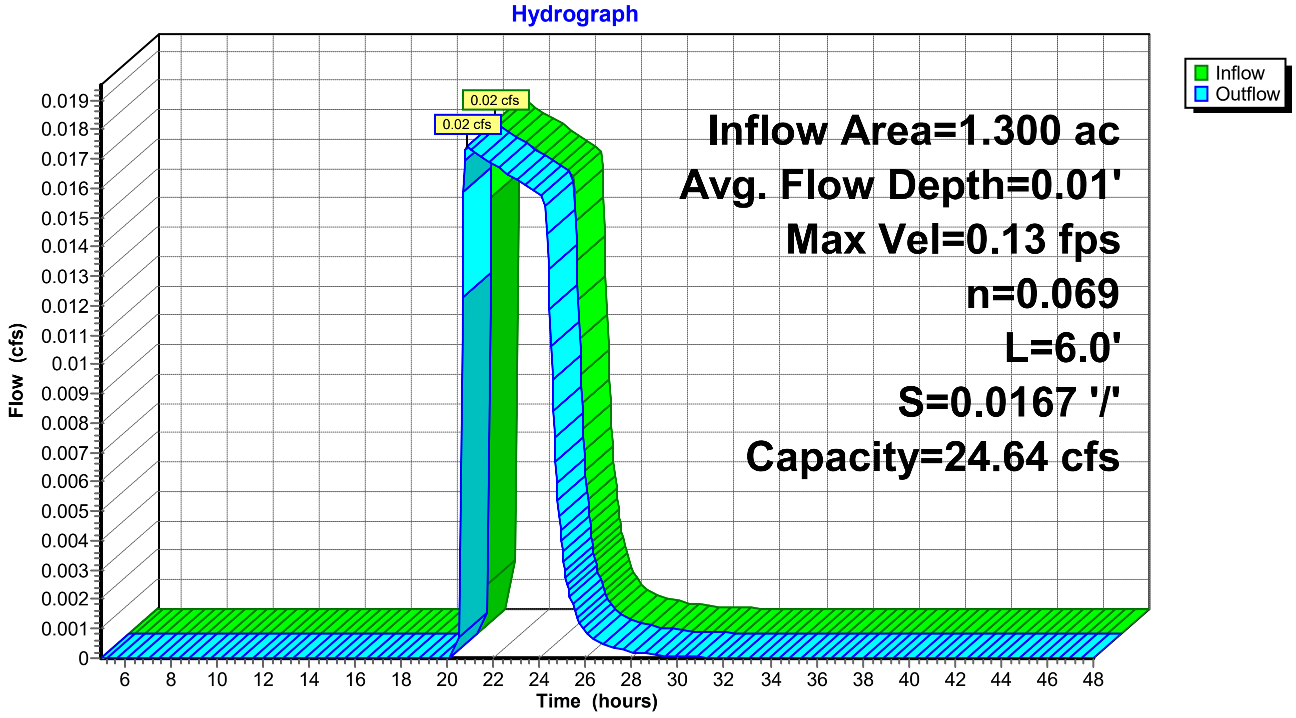
Inflow Area = 1.300 ac, 57.69% Impervious, Inflow Depth = 0.05" for 1-Year event
Inflow = 0.02 cfs @ 20.86 hrs, Volume= 0.006 af
Outflow = 0.02 cfs @ 20.88 hrs, Volume= 0.006 af, Atten= 0%, Lag= 1.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.13 fps, Min. Travel Time= 0.8 min
Avg. Velocity = 0.11 fps, Avg. Travel Time= 0.9 min

Peak Storage= 1 cf @ 20.86 hrs
Average Depth at Peak Storage= 0.01' , Surface Width= 15.07'
Bank-Full Depth= 0.70' Flow Area= 12.5 sf, Capacity= 24.64 cfs

15.00' x 0.70' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 4.0 '/' Top Width= 20.60'
Length= 6.0' Slope= 0.0167 '/'
Inlet Invert= 9.00', Outlet Invert= 8.90'

Reach 17R: Sediment Basin Overflow



Summary for Pond 1P: WQv Pond #1

Inflow Area =	2.900 ac, 65.52% Impervious, Inflow Depth = 1.42" for 1-Year event			
Inflow =	4.50 cfs @	12.11 hrs, Volume=	0.344 af	
Outflow =	0.30 cfs @	13.53 hrs, Volume=	0.339 af,	Atten= 93%, Lag= 85.1 min
Primary =	0.30 cfs @	13.53 hrs, Volume=	0.339 af	

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Starting Elev= 14.00' Surf.Area= 9,229 sf Storage= 19,003 cf
Peak Elev= 14.99' @ 13.53 hrs Surf.Area= 17,494 sf Storage= 27,569 cf (8,566 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= 363.1 min (1,180.3 - 817.1)

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	4,795 cf	**Forebay (Prismatic)** Listed below (Recalc)
#2	9.00'	57,882 cf	**Permanent Pool (Prismatic)** Listed below (Recalc)
		62,677 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	232	0	0
11.00	569	401	401
12.00	1,018	794	1,194
13.00	1,467	1,243	2,437
14.00	3,249	2,358	4,795

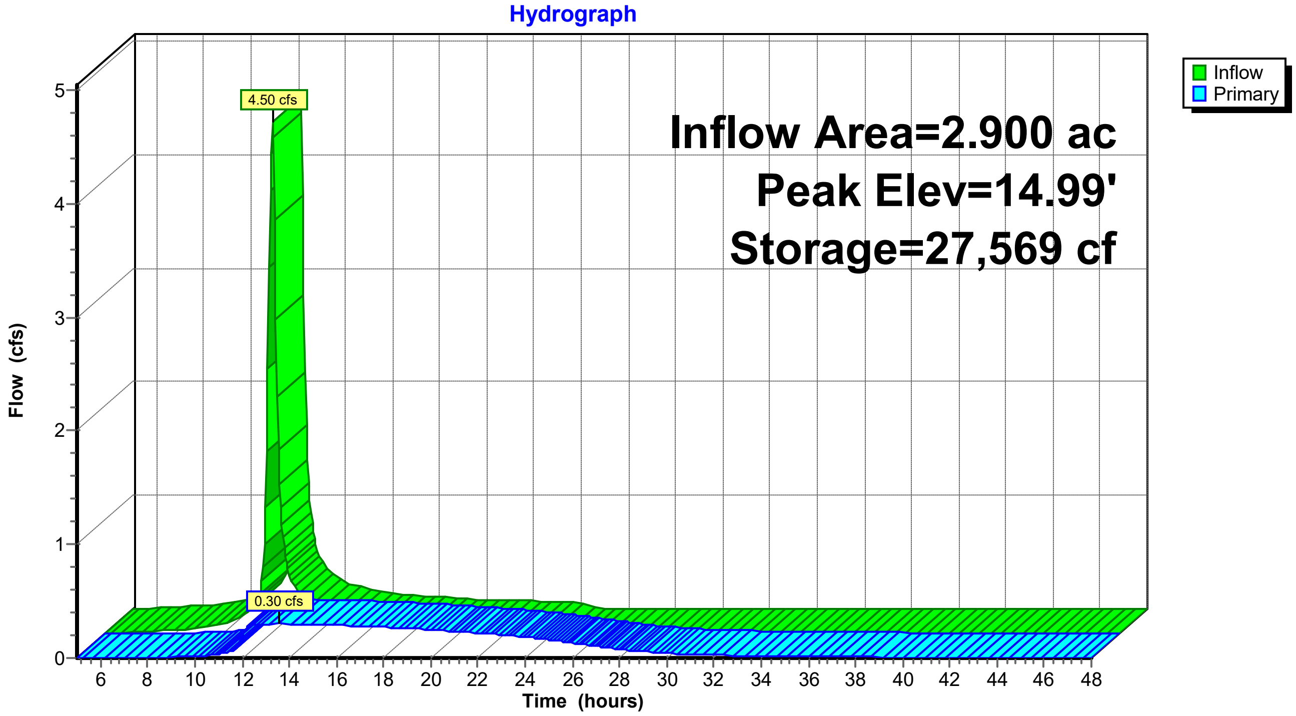
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.00	1,145	0	0
10.00	1,751	1,448	1,448
11.00	2,339	2,045	3,493
12.00	2,959	2,649	6,142
13.00	3,597	3,278	9,420
14.00	5,980	4,789	14,209
14.50	7,240	3,305	17,514
15.00	14,392	5,408	22,922
16.00	17,455	15,924	38,845
17.00	20,619	19,037	57,882

Device	Routing	Invert	Outlet Devices
#1	Primary	13.50'	**12.0" Round Culvert** L= 50.0' Box, headwall w/3 square edges, Ke= 0.500 Inlet / Outlet Invert= 13.50' / 13.23' S= 0.0054 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	16.00'	**24.0" x 24.0" Horiz. Outlet Structure Top Grate** C= 0.600 in 24.0" x 24.0" Grate (100% open area) Limited to weir flow at low heads
#3	Device 1	14.00'	**4.0" Round Reverse Slope Pipe** L= 40.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 9.00' / 14.00' S= -0.1250 '/' Cc= 0.900

			n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf
#4	Primary	16.25'	**6.0' long x 1.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Primary OutFlow Max=0.30 cfs @ 13.53 hrs HW=14.99' (Free Discharge)
1=Culvert (Passes 0.30 cfs of 3.14 cfs potential flow)
  2=Outlet Structure Top Grate (Controls 0.00 cfs)
  3=Reverse Slope Pipe (Inlet Controls 0.30 cfs @ 3.45 fps)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 1P: WQv Pond #1



Summary for Pond 2P: WQv Pond #2

Inflow Area = 5.800 ac, 31.90% Impervious, Inflow Depth > 1.21" for 1-Year event
Inflow = 10.43 cfs @ 11.98 hrs, Volume= 0.585 af
Outflow = 0.48 cfs @ 13.64 hrs, Volume= 0.581 af, Atten= 95%, Lag= 99.4 min
Primary = 0.48 cfs @ 13.64 hrs, Volume= 0.581 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Starting Elev= 14.00' Surf.Area= 5,917 sf Storage= 9,000 cf
Peak Elev= 15.45' @ 13.64 hrs Surf.Area= 20,156 sf Storage= 23,392 cf (14,393 cf above start)

Plug-Flow detention time= 671.8 min calculated for 0.374 af (64% of inflow)
Center-of-Mass det. time= 373.8 min (1,193.1 - 819.3)

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	4,020 cf	**Forebay #1 (Prismatic)** Listed below (Recalc)
#2	10.00'	2,575 cf	**Forebay #2 (Prismatic)** Listed below (Recalc)
#3	10.00'	58,093 cf	**Permanent Pool (Prismatic)** Listed below (Recalc)
		64,688 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	141	0	0
11.00	330	236	236
12.00	562	446	682
13.00	866	714	1,396
14.00	2,023	1,445	2,840
14.50	2,696	1,180	4,020

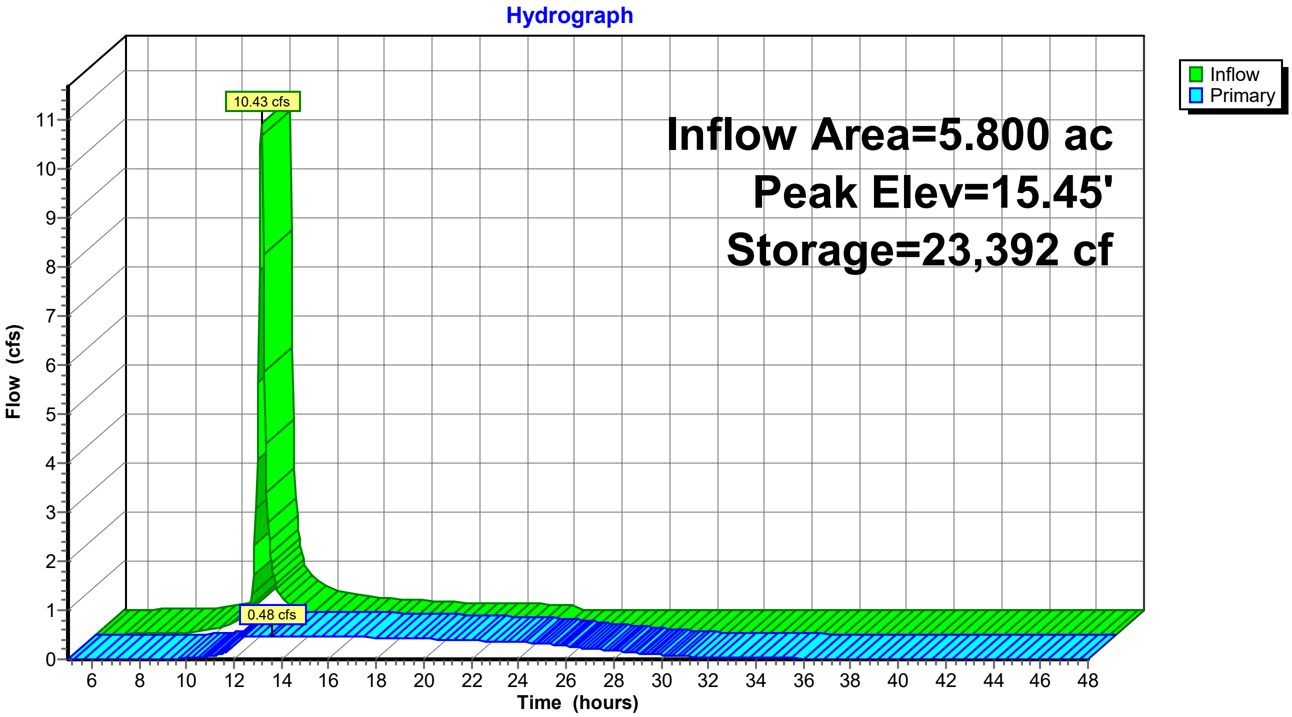
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	82	0	0
11.00	202	142	142
12.00	351	277	419
13.00	535	443	862
14.00	1,323	929	1,791
14.50	1,815	785	2,575

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	375	0	0
11.00	653	514	514
12.00	957	805	1,319
13.00	1,286	1,122	2,441
14.00	2,571	1,929	4,369
14.50	3,307	1,470	5,839
15.00	13,814	4,280	10,119
16.00	17,852	15,833	25,952
17.00	22,659	20,256	46,207
17.50	24,884	11,886	58,093

Device	Routing	Invert	Outlet Devices
#1	Device 3	16.50'	**24.0" x 24.0" Horiz. Orifice/Grate** C= 0.600 in 24.0" x 24.0" Grate (100% open area) Limited to weir flow at low heads
#2	Device 3	14.00'	**4.0" Vert. Reverse Slope Pipe** C= 0.600 Limited to weir flow at low heads
#3	Primary	14.00'	**12.0" Round Outlet Structure Discard Pipe** L= 46.0' Box, headwall w/3 square edges, Ke= 0.500 Inlet / Outlet Invert= 14.00' / 13.77' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#4	Primary	16.60'	**10.0' long x 0.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.48 cfs @ 13.64 hrs HW=15.45' (Free Discharge)
3=Outlet Structure Discard Pipe (Passes 0.48 cfs of 3.04 cfs potential flow)
  1=Orifice/Grate (Controls 0.00 cfs)
  2=Reverse Slope Pipe (Orifice Controls 0.48 cfs @ 5.46 fps)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 2P: WQv Pond #2



Summary for Pond 3P: Infiltration Basin #1

```
Inflow Area =        1.200 ac, 37.50% Impervious, Inflow Depth =   0.00"   for 1-Year event
Inflow      =        0.00 cfs @    5.00 hrs, Volume=          0.000 af
Outflow     =        0.00 cfs @    5.00 hrs, Volume=          0.000 af, Atten= 0%, Lag= 0.0 min
Discarded   =        0.00 cfs @    5.00 hrs, Volume=          0.000 af
Primary     =        0.00 cfs @    5.00 hrs, Volume=          0.000 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 8.10' @ 5.00 hrs Surf.Area= 109 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Storage	Storage Description
#1	8.10'	2,492 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

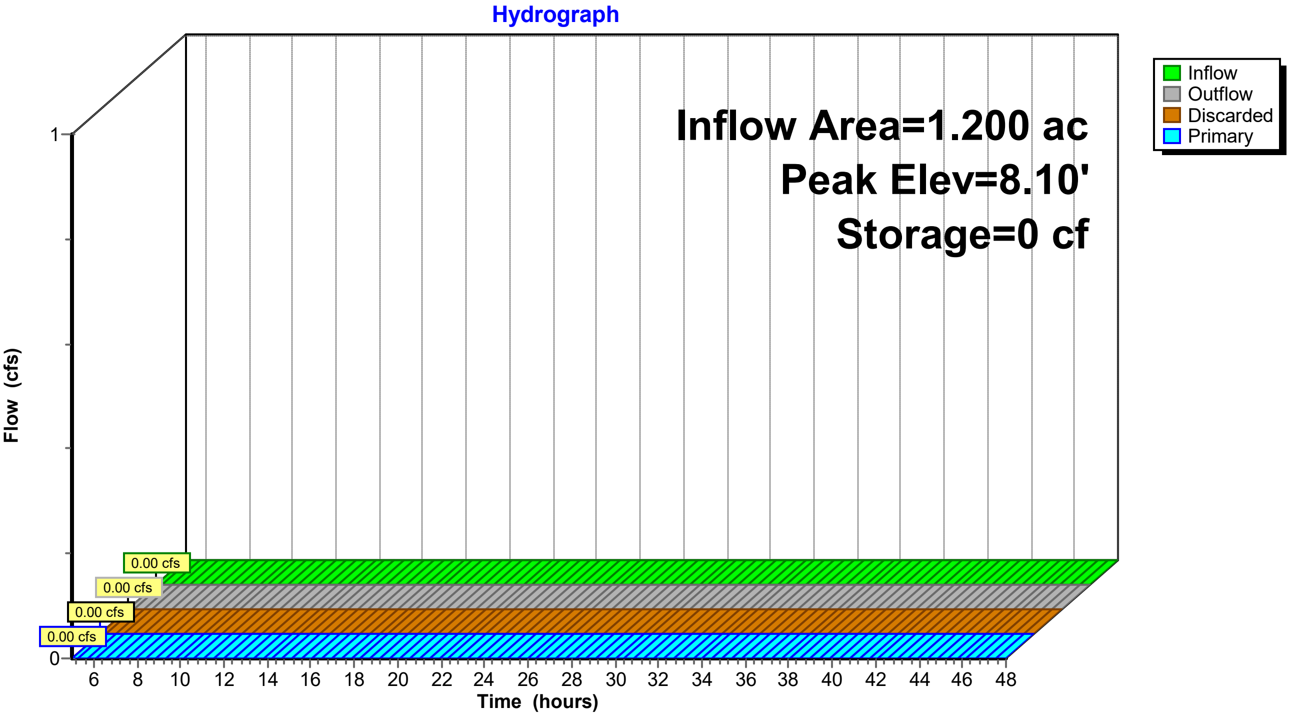
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
8.10	109	56.0	0	0	109
12.00	1,415	149.0	2,492	2,492	1,678

Device	Routing	Invert	Outlet Devices
#1	Primary	11.65'	**Channel/Reach** using Reach 5R: Overflow
#2	Discarded	8.10'	**0.500 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 4.00'

Discarded OutFlow Max=0.00 cfs @ 5.00 hrs HW=8.10' (Free Discharge)
↑—**2=Exfiltration** (Passes 0.00 cfs of 0.00 cfs potential flow)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=8.10' (Free Discharge)
↑—**1=Channel/Reach** (Controls 0.00 cfs)

Pond 3P: Infiltration Basin #1



Summary for Pond 4P: Infiltration Basin #2

Inflow Area = 1.300 ac, 57.69% Impervious, Inflow Depth = 0.05" for 1-Year event
Inflow = 0.02 cfs @ 20.88 hrs, Volume= 0.006 af
Outflow = 0.02 cfs @ 20.97 hrs, Volume= 0.006 af, Atten= 0%, Lag= 5.4 min
Discarded = 0.02 cfs @ 20.97 hrs, Volume= 0.006 af
Primary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 5.82' @ 20.97 hrs Surf.Area= 114 sf Storage= 3 cf

Plug-Flow detention time= 2.7 min calculated for 0.006 af (100% of inflow)
Center-of-Mass det. time= 2.7 min (1,369.3 - 1,366.6)

Volume	Invert	Avail.Storage	Storage Description
#1	5.80'	2,495 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

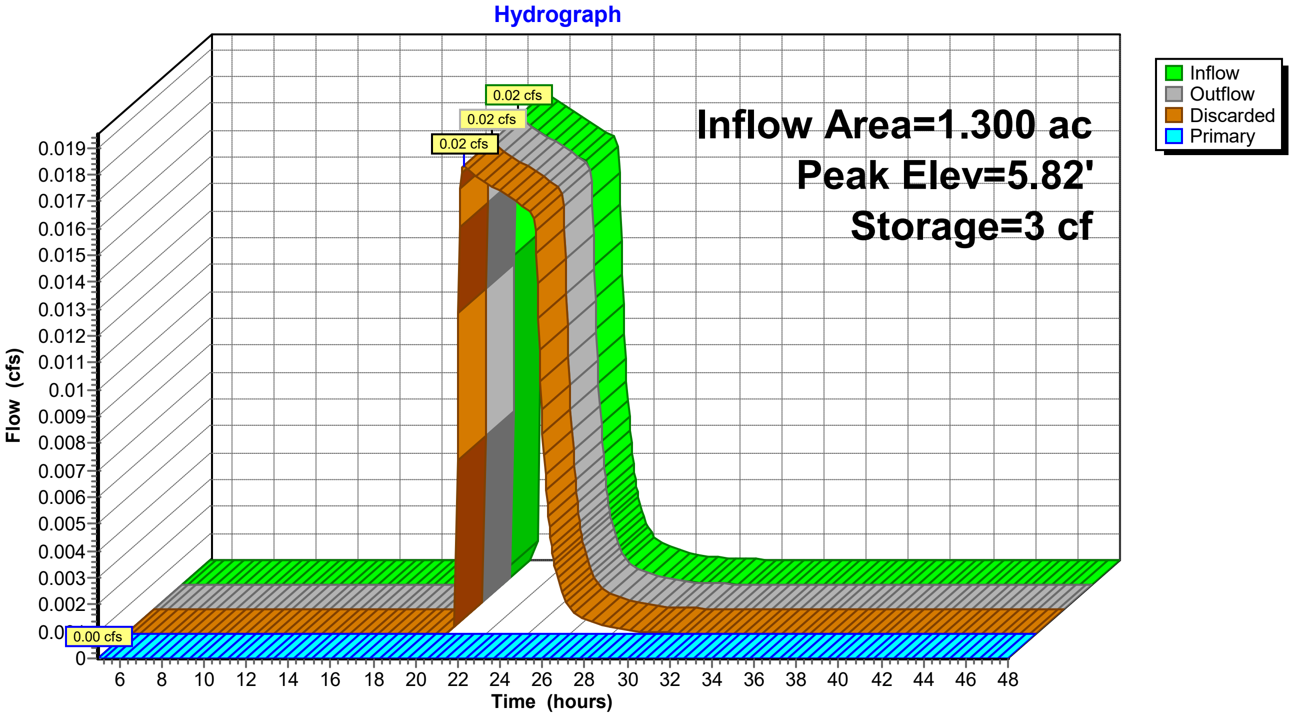
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
5.80	110	56.0	0	0	110
9.70	1,415	150.0	2,495	2,495	1,702

Device	Routing	Invert	Outlet Devices
#1	Primary	8.50'	**Channel/Reach** using Reach 6R: Overflow
#2	Discarded	5.80'	**12.000 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 3.00'

Discarded OutFlow Max=0.03 cfs @ 20.97 hrs HW=5.82' (Free Discharge)
↑—**2=Exfiltration** (Controls 0.03 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=5.80' (Free Discharge)
↑—**1=Channel/Reach** (Controls 0.00 cfs)

Pond 4P: Infiltration Basin #2



Summary for Pond 5P: Sedimentation Basin #1

```
Inflow Area =        1.200 ac, 37.50% Impervious, Inflow Depth =  0.12"   for 1-Year event
Inflow     =       0.04 cfs @  12.26 hrs, Volume=          0.012 af
Outflow    =       0.00 cfs @   5.00 hrs, Volume=          0.000 af, Atten= 100%, Lag= 0.0 min
Primary    =       0.00 cfs @   5.00 hrs, Volume=          0.000 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 11.50' @ 31.80 hrs Surf.Area= 427 sf Storage= 505 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow)
Center-of-Mass det. time= (not calculated: no outflow)

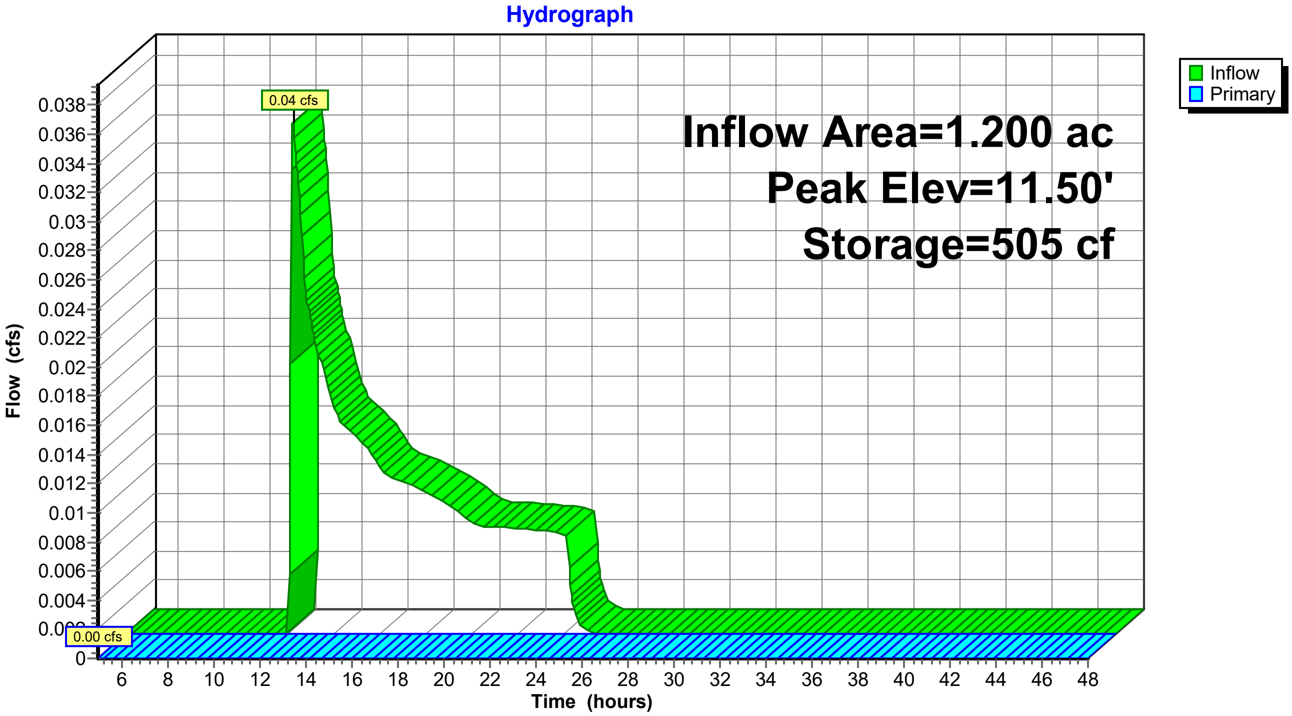
Volume	Invert	Avail.Storage	Storage Description
#1	9.00'	1,058 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
9.00	41	25.8	0	0	41
10.00	150	46.7	90	90	167
11.00	320	66.6	230	320	355
12.00	550	86.4	430	749	608
12.50	687	96.2	309	1,058	758

Device	Routing	Invert	Outlet Devices
#1	Primary	12.00'	**Channel/Reach** using Reach 12R: Sediment Basin Overflow

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=9.00' (Free Discharge)
↑—**1=Channel/Reach** (Controls 0.00 cfs)

Pond 5P: Sedimentation Basin #1



Summary for Pond 16P: Sedimentation Basin #2

Inflow Area = 1.300 ac, 57.69% Impervious, Inflow Depth = 0.41" for 1-Year event
Inflow = 0.46 cfs @ 12.27 hrs, Volume= 0.045 af
Outflow = 0.02 cfs @ 20.86 hrs, Volume= 0.006 af, Atten= 96%, Lag= 515.7 min
Primary = 0.02 cfs @ 20.86 hrs, Volume= 0.006 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 9.00' @ 20.86 hrs Surf.Area= 893 sf Storage= 1,703 cf

Plug-Flow detention time= 632.6 min calculated for 0.006 af (13% of inflow)
Center-of-Mass det. time= 440.5 min (1,365.0 - 924.5)

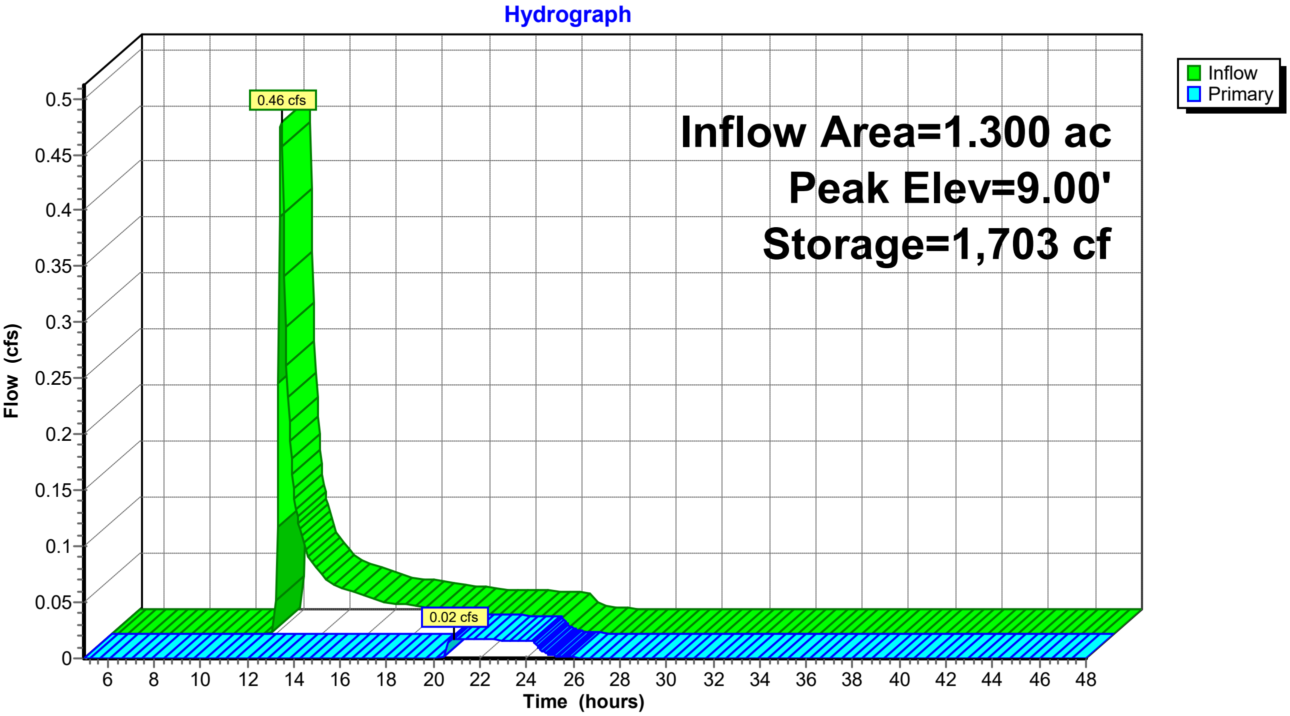
Volume	Invert	Avail.Storage	Storage Description
#1	5.80'	2,389 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
5.80	224	80.0	0	0	224
6.00	257	83.0	48	48	266
7.00	438	98.0	344	392	500
8.00	650	113.0	541	932	773
9.00	892	128.0	768	1,700	1,085
9.70	1,079	139.0	689	2,389	1,337

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	**Channel/Reach** using Reach 17R: Sediment Basin Overflow

Primary OutFlow Max=0.00 cfs @ 20.86 hrs HW=9.00' (Free Discharge)
└─**1=Channel/Reach** (Channel Controls 0.00 cfs @ 0.07 fps)

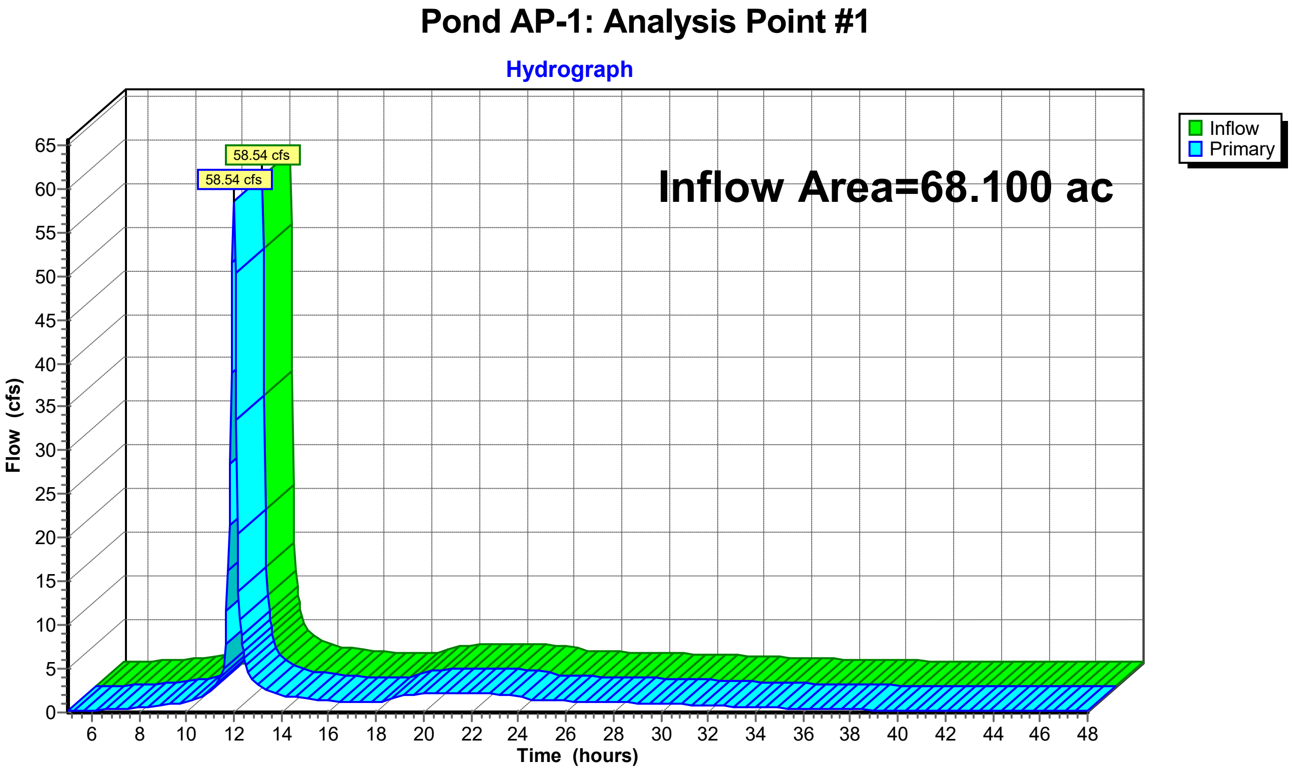
Pond 16P: Sedimentation Basin #2



Summary for Pond AP-1: Analysis Point #1

```
Inflow Area =        68.100 ac,  20.36% Impervious,  Inflow Depth >   0.92"    for  1-Year event
Inflow       =       58.54 cfs @   12.00 hrs,  Volume=           5.220 af
Primary      =       58.54 cfs @   12.00 hrs,  Volume=           5.220 af,  Atten= 0%,  Lag= 0.0 min
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

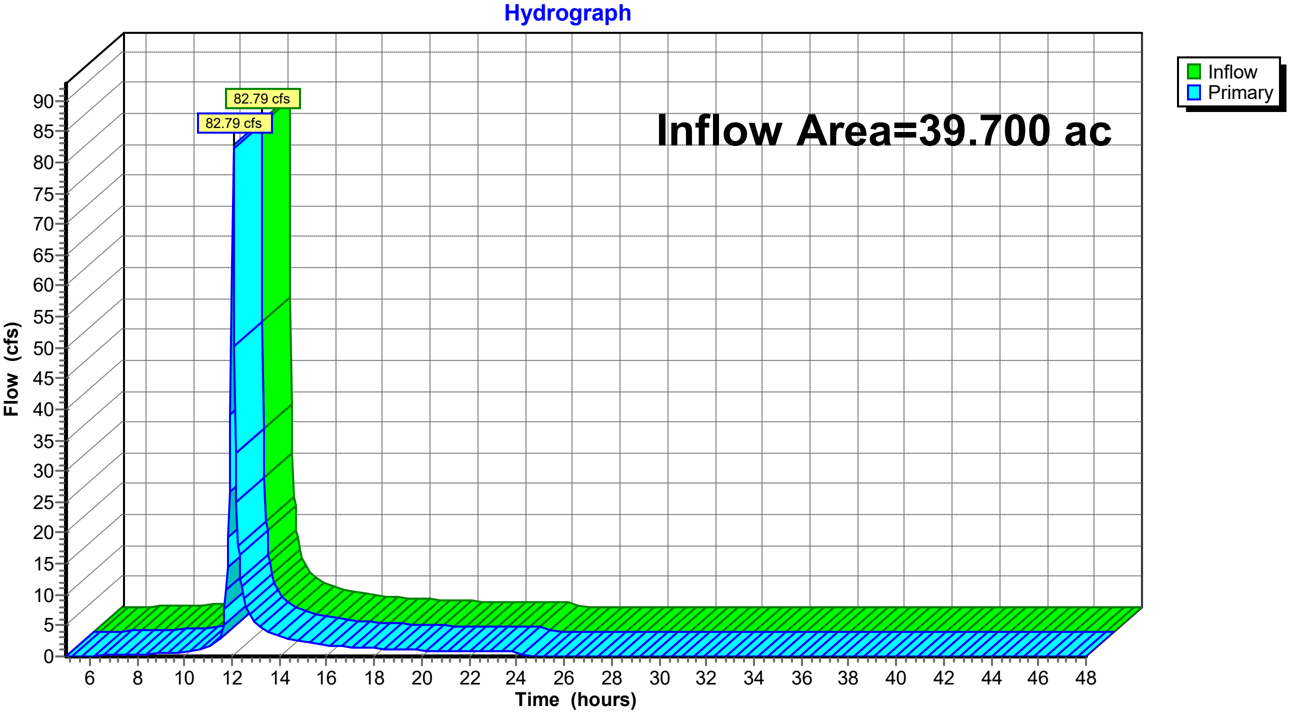


Summary for Pond AP-2: Analysis Point #2

Inflow Area = 39.700 ac, 23.10% Impervious, Inflow Depth > 1.20" for 1-Year event
Inflow = 82.79 cfs @ 12.04 hrs, Volume= 3.954 af
Primary = 82.79 cfs @ 12.04 hrs, Volume= 3.954 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

Pond AP-2: Analysis Point #2

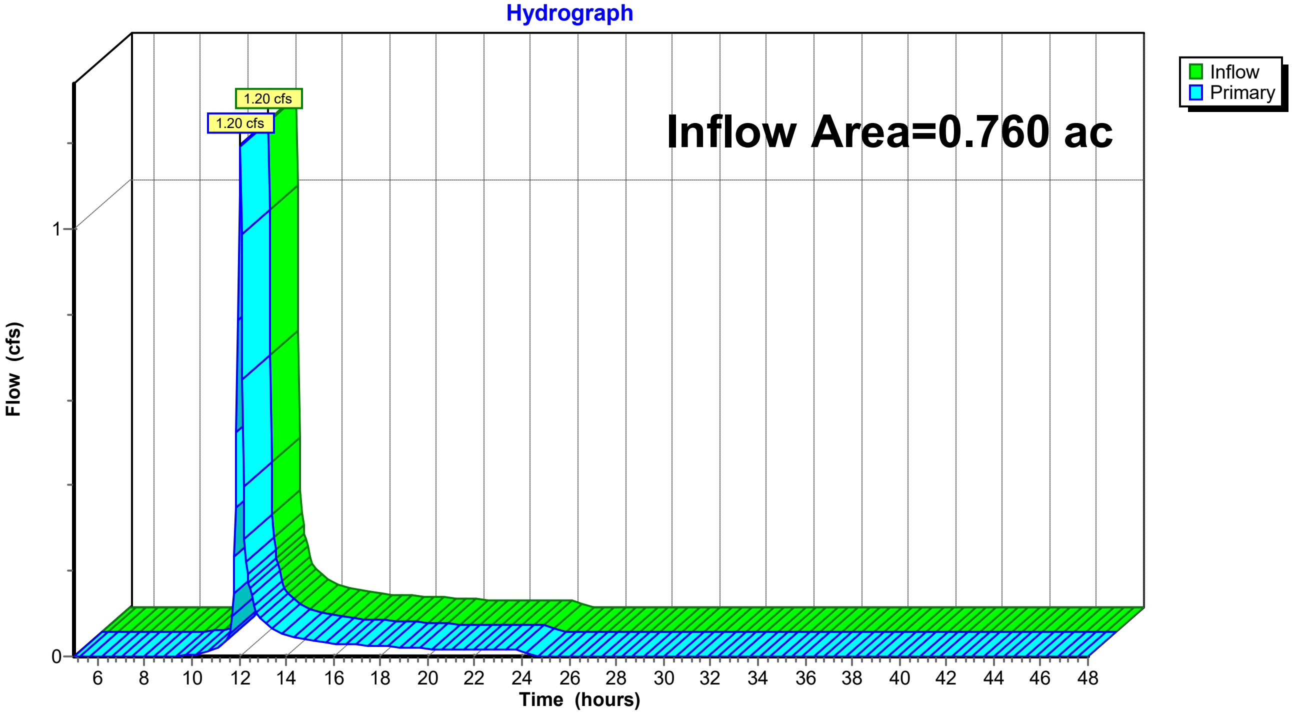


Summary for Pond AP-3: Analysis Point #3

Inflow Area = 0.760 ac, 78.95% Impervious, Inflow Depth = 1.00" for 1-Year event
Inflow = 1.20 cfs @ 12.04 hrs, Volume= 0.064 af
Primary = 1.20 cfs @ 12.04 hrs, Volume= 0.064 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

Pond AP-3: Analysis Point #3



Summary for Pond C-1: Chamber Series 1

```
Inflow Area =      10.300 ac, 66.70% Impervious, Inflow Depth >  1.85"   for 1-Year event
Inflow      =      29.34 cfs @  11.98 hrs, Volume=          1.592 af
Outflow     =      27.91 cfs @  12.04 hrs, Volume=          1.233 af, Atten= 5%, Lag= 3.7 min
Discarded  =        0.10 cfs @   5.80 hrs, Volume=          0.345 af
Primary     =      27.81 cfs @  12.04 hrs, Volume=          0.888 af
Secondary =         0.00 cfs @   5.00 hrs, Volume=          0.000 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 12.19' @ 12.04 hrs Surf.Area= 8,454 sf Storage= 28,913 cf

Plug-Flow detention time= 361.5 min calculated for 1.232 af (77% of inflow)
Center-of-Mass det. time= 279.3 min (1,056.9 - 777.6)

Volume	Invert	Avail.Storage	Storage Description
#1B	6.00'	11,722 cf	**37.08'W x 227.97'L x 5.50'H Field B** 46,496 cf Overall - 17,192 cf Embedded = 29,305 cf x 40.0% Voids
#2B	6.75'	17,192 cf	**ADS_StormTech MC-3500 d +Cap** x 155 Inside #1 Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap 155 Chambers in 5 Rows Cap Storage= +14.9 cf x 2 x 5 rows = 149.0 cf
#3	13.25'	6,100 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)
		35,013 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
13.25	600	0	0
13.75	4,900	1,375	1,375
14.00	8,200	1,638	3,013
14.25	16,500	3,088	6,100

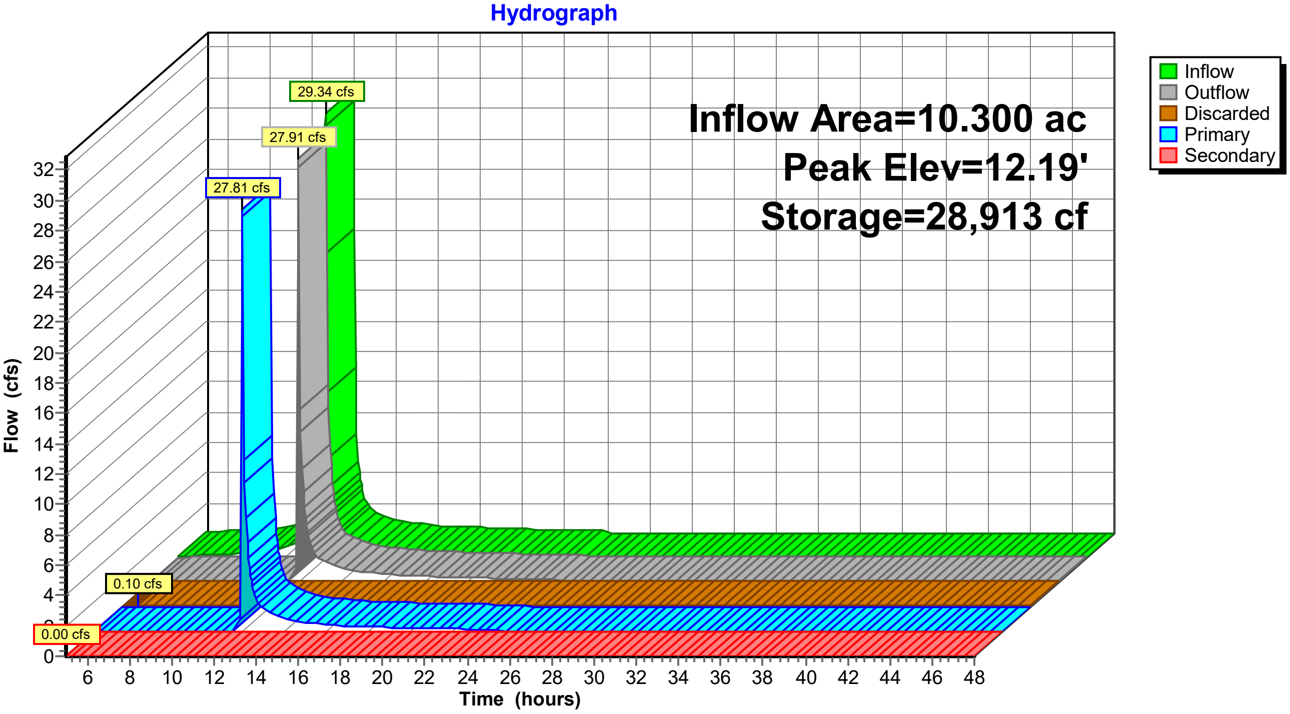
Device	Routing	Invert	Outlet Devices
#1	Discarded	6.00'	**0.500 in/hr Exfiltration over Surface area**
#2	Primary	10.00'	**36.0" Round Culvert** L= 55.9' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.00' / 7.59' S= 0.0431 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf
#3	Secondary	14.24'	**50.0' long x 0.7' breadth Concrete Curb** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 Coef. (English) 2.76 2.82 2.93 3.09 3.18 3.22 3.27 3.30 3.32 3.31 3.32

Discarded OutFlow Max=0.10 cfs @ 5.80 hrs HW=6.08' (Free Discharge)
└─**1=Exfiltration** (Exfiltration Controls 0.10 cfs)

Primary OutFlow Max=25.90 cfs @ 12.04 hrs HW=12.09' (Free Discharge)
└─**2=Culvert** (Inlet Controls 25.90 cfs @ 4.92 fps)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=6.00' (Free Discharge)
└─**3=Concrete Curb** (Controls 0.00 cfs)

Pond C-1: Chamber Series 1



Summary for Pond C-4: Chamber Series 4

Inflow Area = 8.900 ac, 0.00% Impervious, Inflow Depth > 1.58" for 1-Year event
Inflow = 22.13 cfs @ 11.99 hrs, Volume= 1.175 af
Outflow = 17.34 cfs @ 12.06 hrs, Volume= 0.845 af, Atten= 22%, Lag= 4.4 min
Discarded = 0.07 cfs @ 7.60 hrs, Volume= 0.254 af
Primary = 17.27 cfs @ 12.06 hrs, Volume= 0.592 af
Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 12.75' @ 12.07 hrs Surf.Area= 6,391 sf Storage= 21,785 cf

Plug-Flow detention time= 397.3 min calculated for 0.844 af (72% of inflow)
Center-of-Mass det. time= 304.6 min (1,103.4 - 798.9)

Volume	Invert	Avail.Storage	Storage Description
#1B	6.00'	8,911 cf	**29.92'W x 213.63'L x 5.50'H Field B** 35,151 cf Overall - 12,874 cf Embedded = 22,277 cf x 40.0% Voids
#2B	6.75'	12,874 cf	**ADS_StormTech MC-3500 d +Cap** x 116 Inside #1 Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap 116 Chambers in 4 Rows Cap Storage= +14.9 cf x 2 x 4 rows = 119.2 cf
#3	14.10'	8,015 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)
		29,800 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
14.10	400	0	0
14.60	2,400	700	700
14.80	6,300	870	1,570
15.10	10,000	2,445	4,015
15.50	10,000	4,000	8,015

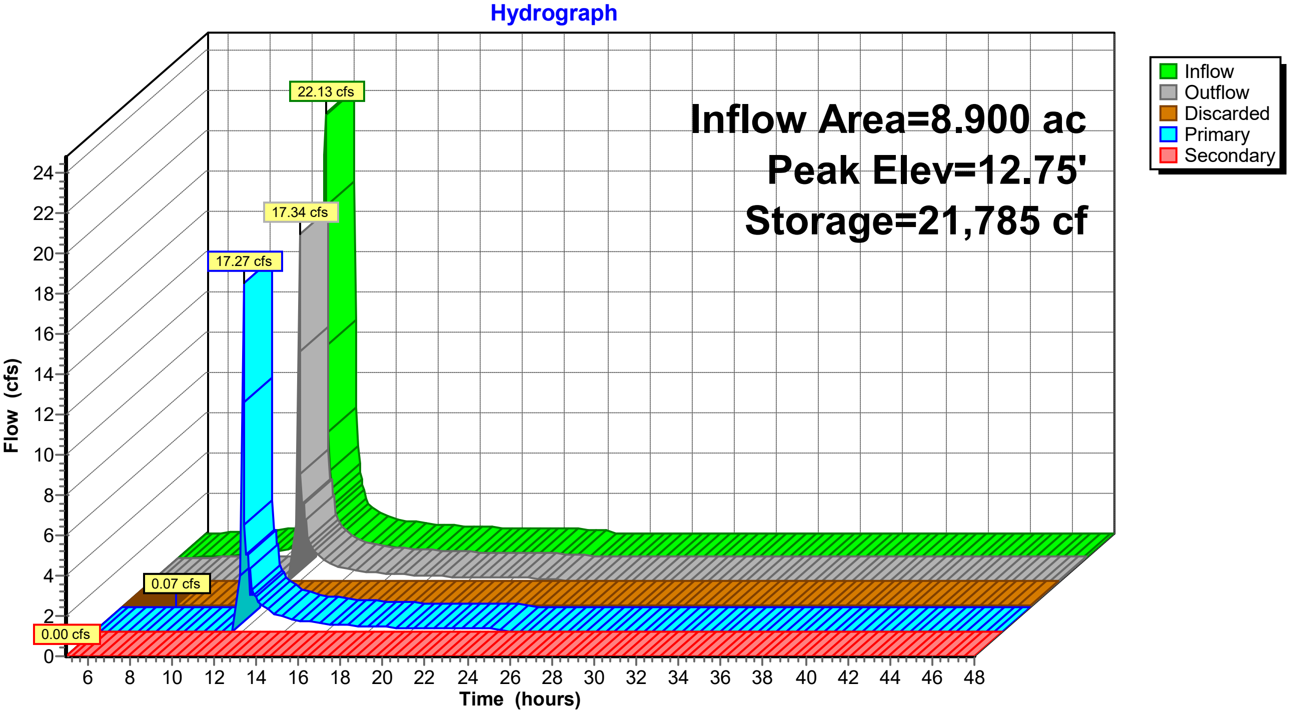
Device	Routing	Invert	Outlet Devices
#1	Discarded	6.00'	**0.500 in/hr Exfiltration over Surface area**
#2	Primary	11.00'	**36.0" Round Culvert** L= 24.3' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 11.00' / 9.42' S= 0.0650 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf
#3	Secondary	15.50'	**100.0' long x 0.5' breadth Wharf** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.07 cfs @ 7.60 hrs HW=6.10' (Free Discharge)
└─**1=Exfiltration** (Exfiltration Controls 0.07 cfs)

Primary OutFlow Max=15.10 cfs @ 12.06 hrs HW=12.52' (Free Discharge)
└─**2=Culvert** (Inlet Controls 15.10 cfs @ 4.20 fps)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=6.00' (Free Discharge)
└─**3=Wharf** (Controls 0.00 cfs)

Pond C-4: Chamber Series 4



Summary for Pond C-5: Chamber Series 5

```
Inflow Area =        5.200 ac,   0.00% Impervious,  Inflow Depth >   1.58"    for  1-Year event
Inflow      =      13.09 cfs @  11.99 hrs,  Volume=          0.687 af
Outflow     =       0.43 cfs @  14.80 hrs,  Volume=          0.434 af,  Atten= 97%,  Lag= 168.8 min
Discarded  =       0.11 cfs @   9.00 hrs,  Volume=          0.373 af
Primary     =       0.32 cfs @  14.80 hrs,  Volume=          0.062 af
Secondary =       0.00 cfs @   5.00 hrs,  Volume=          0.000 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 12.84' @ 14.80 hrs Surf.Area= 4,801 sf Storage= 20,773 cf

Plug-Flow detention time= 879.5 min calculated for 0.434 af (63% of inflow)
Center-of-Mass det. time= 777.3 min (1,575.8 - 798.5)

Volume	Invert	Avail.Storage	Storage Description
#1B	6.00'	7,757 cf	**28.50'W x 168.47'L x 6.75'H Field B** 32,409 cf Overall - 13,016 cf Embedded = 19,393 cf x 40.0% Voids
#2B	6.75'	13,016 cf	**ADS_StormTech MC-4500 b +Cap** x 120 Inside #1 Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 120 Chambers in 3 Rows Cap Storage= +39.5 cf x 2 x 3 rows = 237.0 cf
#3	14.60'	7,420 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)
		28,193 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
14.60	200	0	0
14.80	1,000	120	120
15.00	10,000	1,100	1,220
15.50	14,800	6,200	7,420

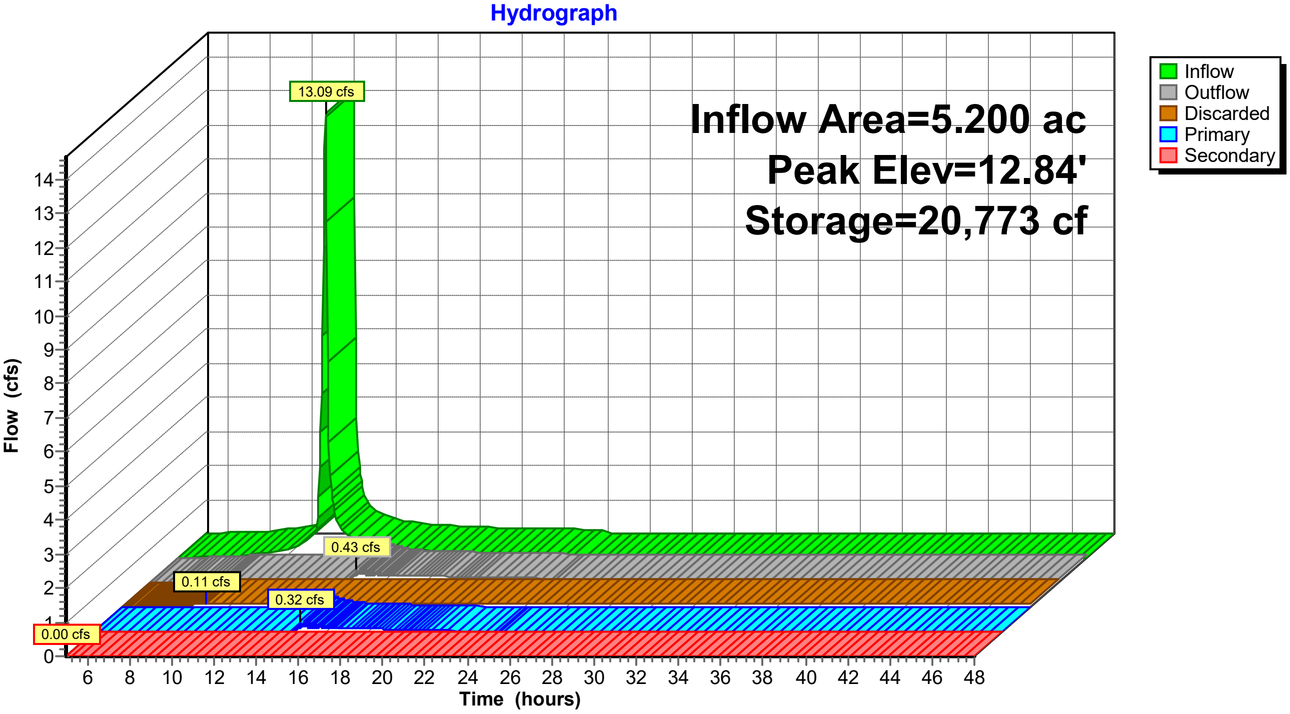
Device	Routing	Invert	Outlet Devices
#1	Discarded	6.00'	**1.000 in/hr Exfiltration over Surface area**
#2	Primary	12.60'	**36.0" Round Culvert** L= 62.8' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 12.60' / 12.29' S= 0.0049 '/' Cc= 0.900 n= 0.013 Cast iron, coated, Flow Area= 7.07 sf
#3	Secondary	15.50'	**100.0' long x 0.5' breadth Wharf** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.11 cfs @ 9.00 hrs HW=6.10' (Free Discharge)
↑—**1=Exfiltration** (Exfiltration Controls 0.11 cfs)

Primary OutFlow Max=0.31 cfs @ 14.80 hrs HW=12.84' (Free Discharge)
↑—**2=Culvert** (Barrel Controls 0.31 cfs @ 1.84 fps)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=6.00' (Free Discharge)
↑—**3=Wharf** (Controls 0.00 cfs)

Pond C-5: Chamber Series 5



Summary for Subcatchment DR-1: Building A & Storage

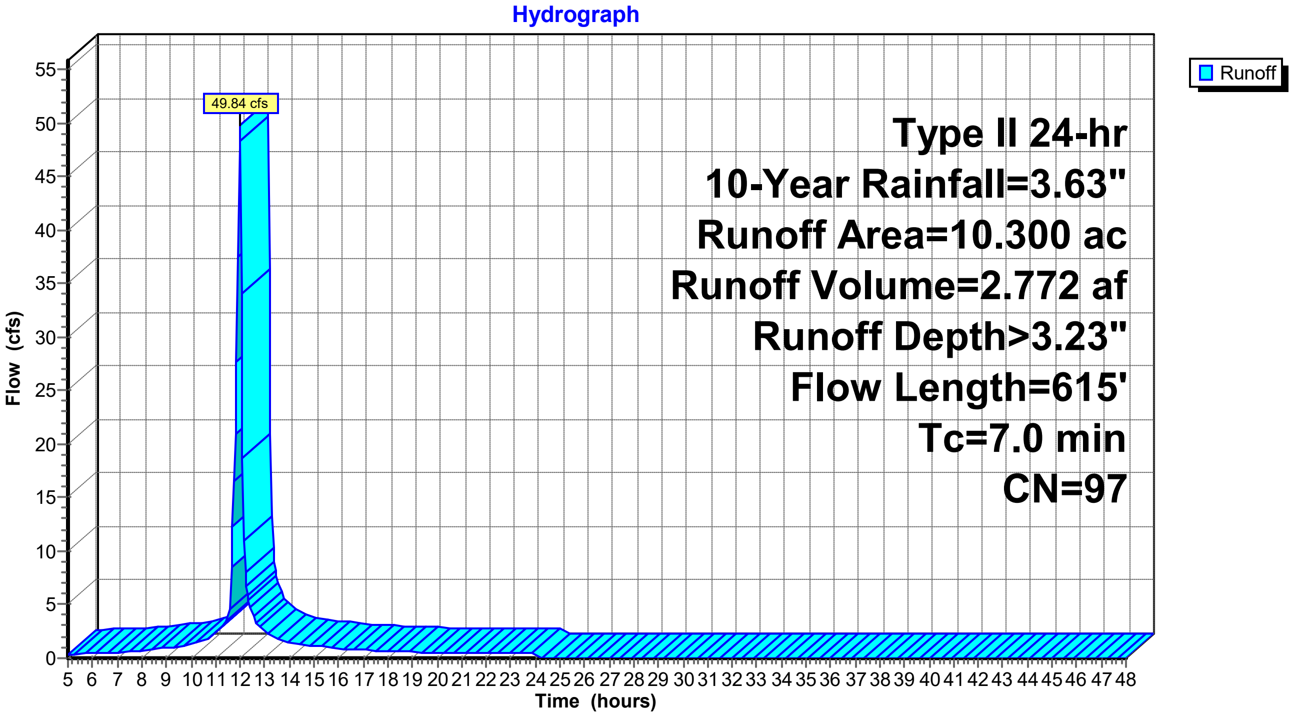
Runoff = 49.84 cfs @ 11.98 hrs, Volume= 2.772 af, Depth> 3.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	6.870	98	Building A
	0.100	80	>75% Grass cover, Good, HSG D
*	3.330	95	Dense Graded Aggregate
	10.300	97	Weighted Average
	3.430		33.30% Pervious Area
	6.870		66.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
3.1	300	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.6	215	0.0050	5.91	29.00	**Pipe Channel,** 30.0" Round Area= 4.9 sf Perim= 7.9' r= 0.63' n= 0.013
7.0	615	Total			

Subcatchment DR-1: Building A & Storage



Summary for Subcatchment DR-10: Undisturbed Area

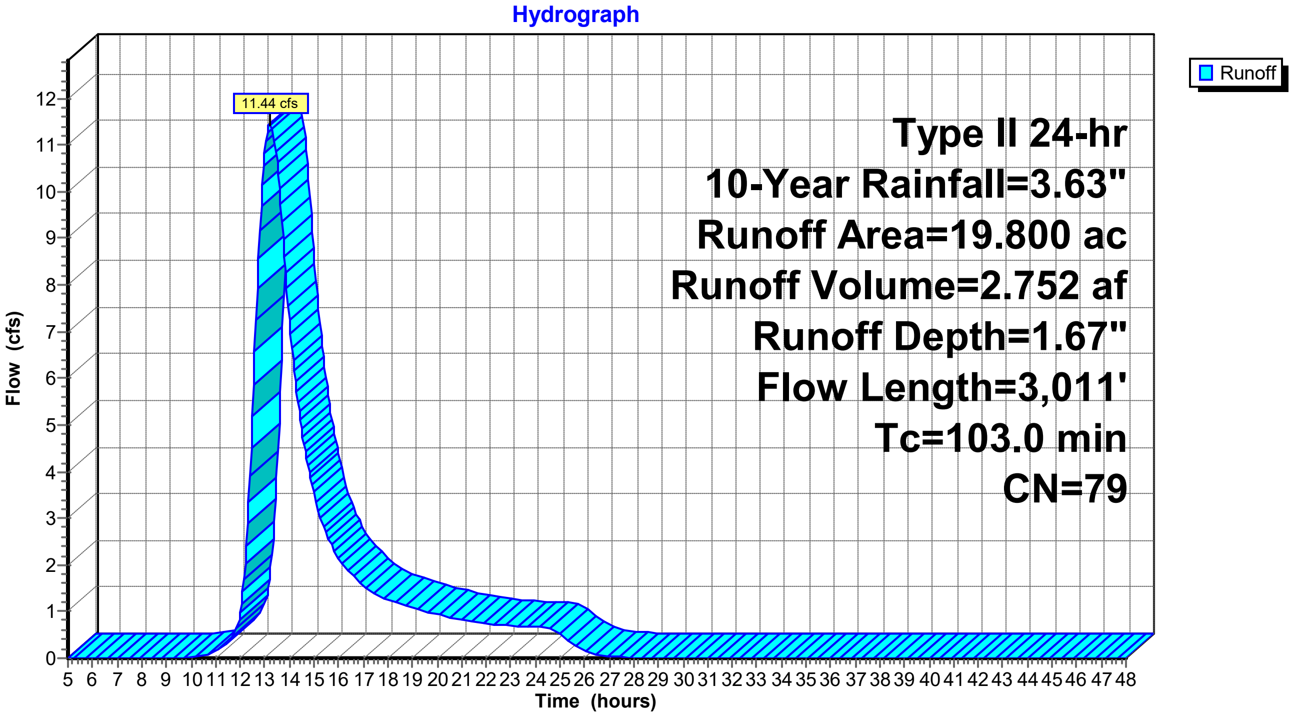
Runoff = 11.44 cfs @ 13.19 hrs, Volume= 2.752 af, Depth= 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

Area (ac)	CN	Description
19.800	79	Woods, Fair, HSG D
19.800		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.7	150	0.0800	0.13		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.40"
3.0	200	0.0500	1.12		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
1.6	250	0.2600	2.55		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
78.7	2,361	0.0100	0.50		**Shallow Concentrated Flow, Wetland Flow** Woodland Kv= 5.0 fps
0.0	50	0.0500	22.86	161.57	**Pipe Channel,** 36.0" Round Area= 7.1 sf Perim= 9.4' r= 0.75' n= 0.012 Corrugated PP, smooth interior
103.0	3,011	Total			

Subcatchment DR-10: Undisturbed Area



Summary for Subcatchment DR-11: Hudson River Bank

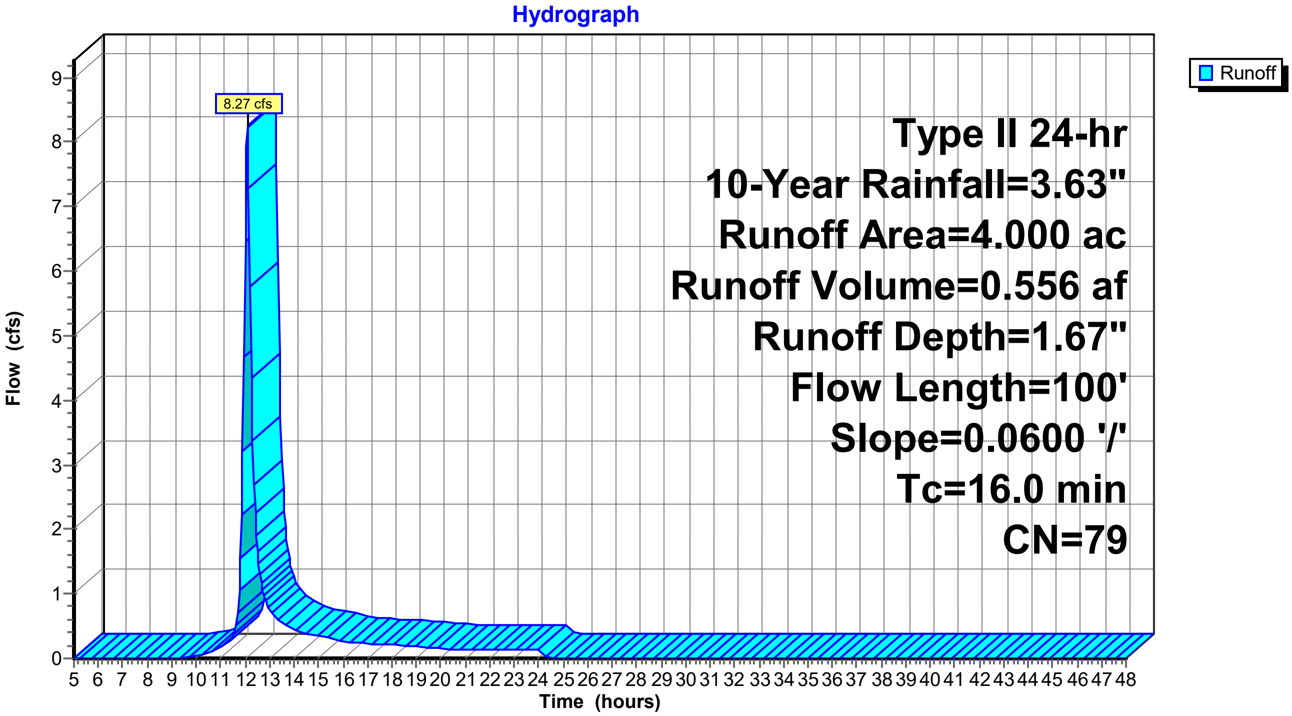
Runoff = 8.27 cfs @ 12.09 hrs, Volume= 0.556 af, Depth= 1.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

Area (ac)	CN	Description
4.000	79	Woods, Fair, HSG D
4.000		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.0	100	0.0600	0.10		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.40"

Subcatchment DR-11: Hudson River Bank



Summary for Subcatchment DR-12: Normans Kill Bank

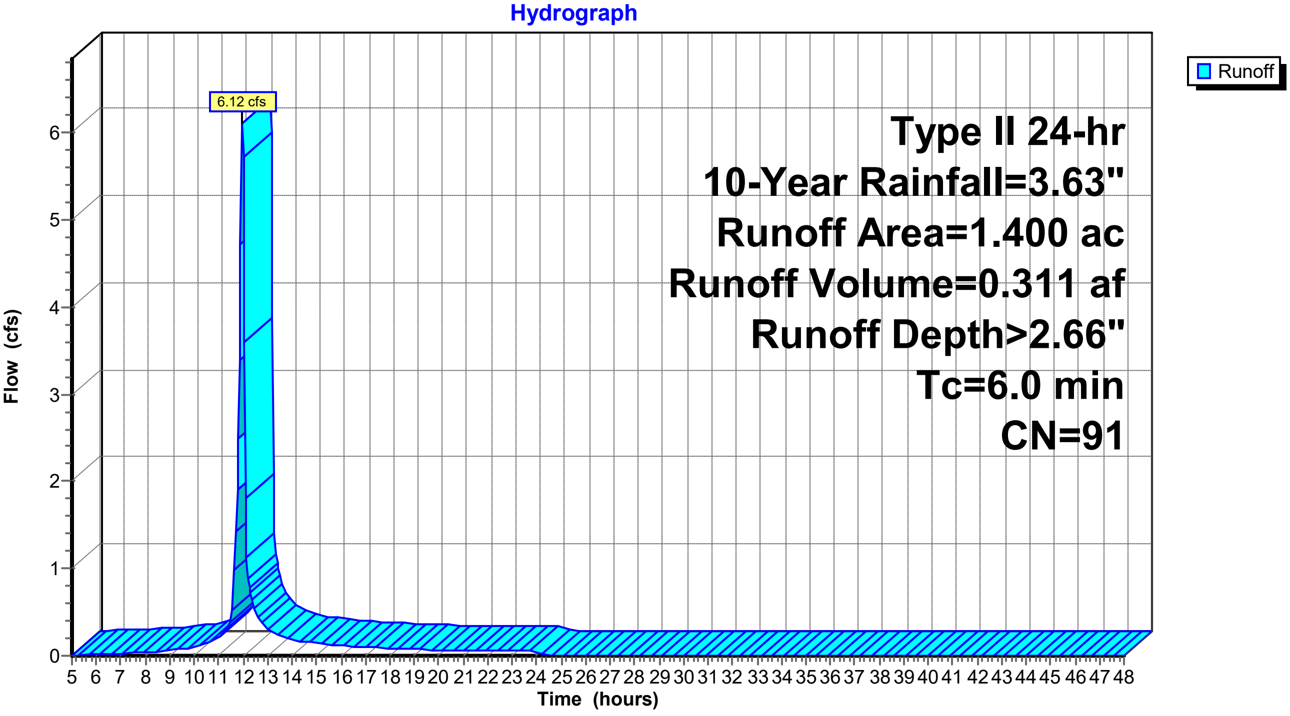
Runoff = 6.12 cfs @ 11.97 hrs, Volume= 0.311 af, Depth> 2.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

Area (ac)	CN	Description
0.430	79	Woods, Fair, HSG D
0.970	96	Gravel surface, HSG D
1.400	91	Weighted Average
1.400		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-12: Normans Kill Bank



Summary for Subcatchment DR-13: Roadway

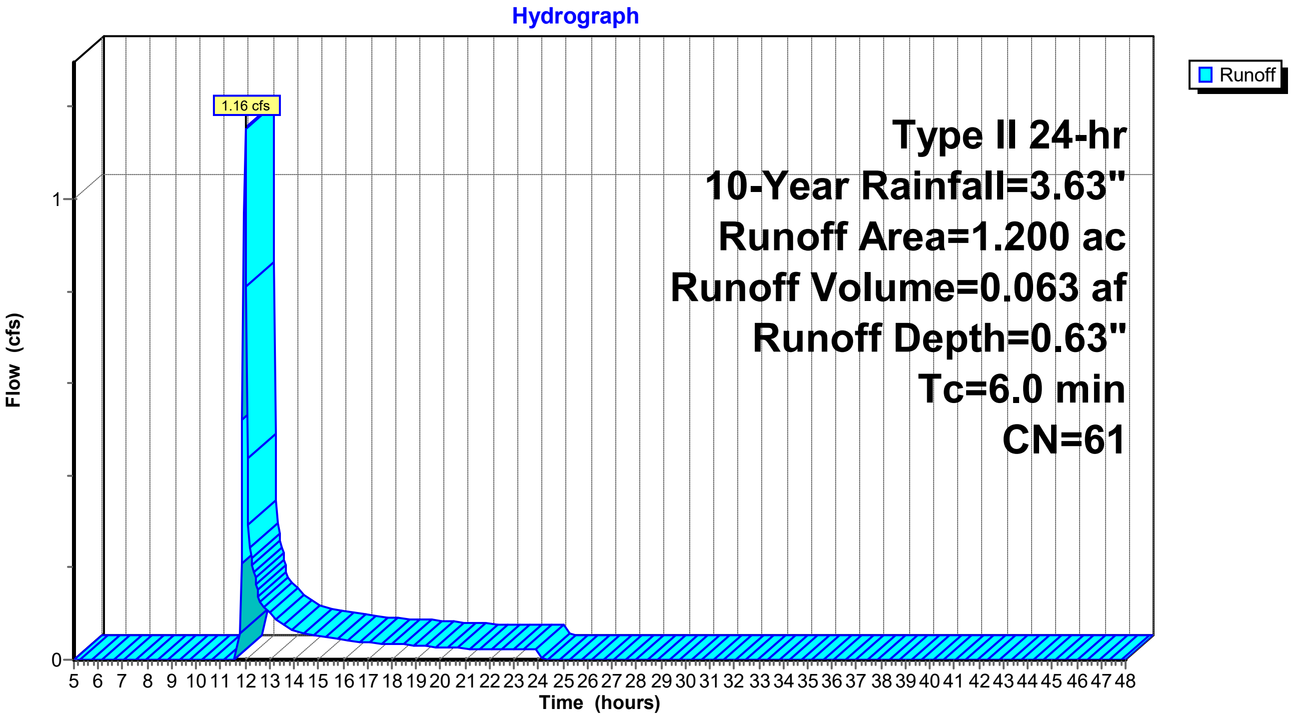
Runoff = 1.16 cfs @ 11.99 hrs, Volume= 0.063 af, Depth= 0.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	0.450	98	Pavement
	0.750	39	>75% Grass cover, Good, HSG A
	1.200	61	Weighted Average
	0.750		62.50% Pervious Area
	0.450		37.50% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-13: Roadway



Summary for Subcatchment DR-14: Roadway

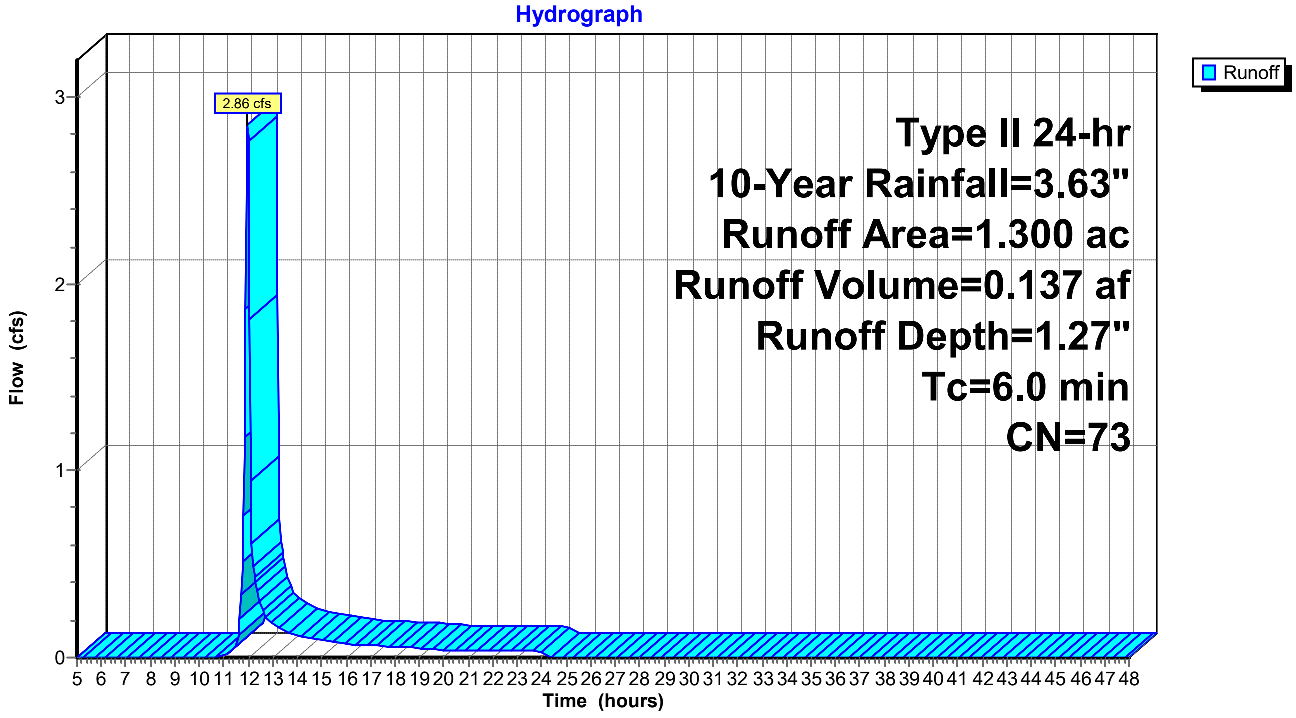
Runoff = 2.86 cfs @ 11.98 hrs, Volume= 0.137 af, Depth= 1.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	0.550	98	New Pavement
	0.550	39	>75% Grass cover, Good, HSG A
*	0.200	98	Mill & Fill of Old Pavement
	1.300	73	Weighted Average
	0.550		42.31% Pervious Area
	0.750		57.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-14: Roadway



Summary for Subcatchment DR-15: Roadway

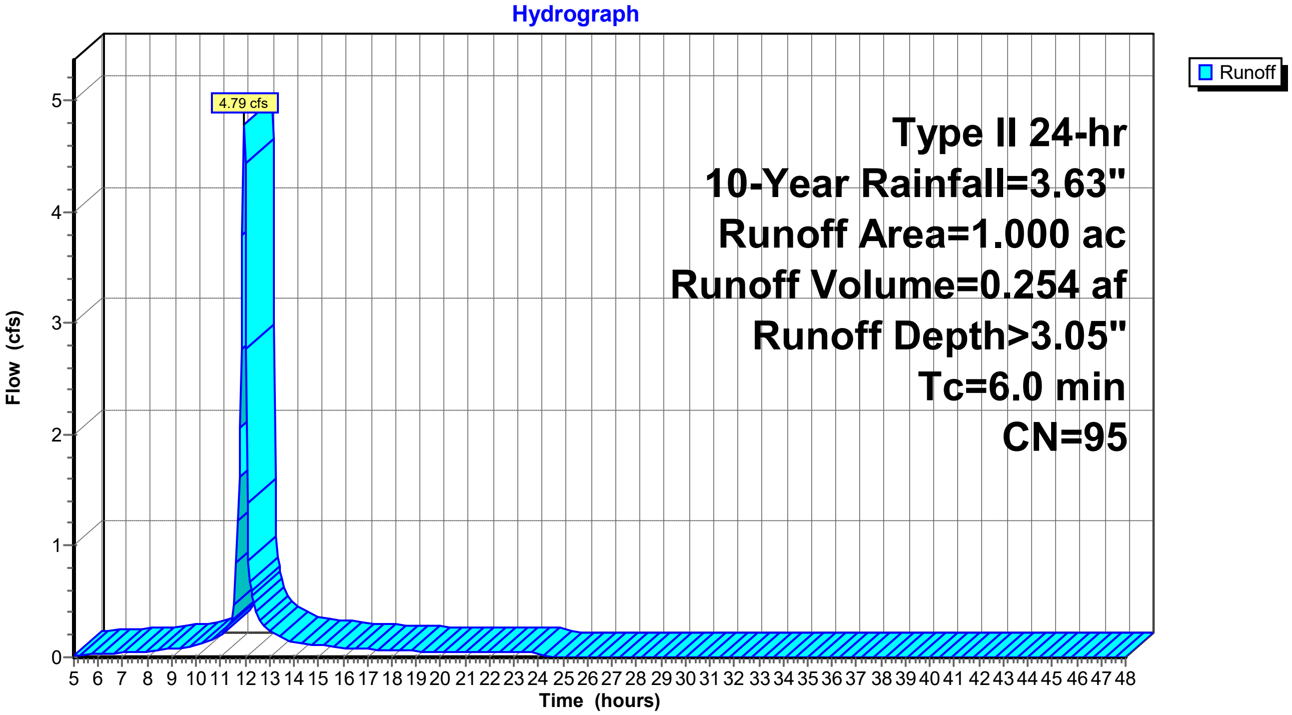
Runoff = 4.79 cfs @ 11.96 hrs, Volume= 0.254 af, Depth> 3.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	0.050	98	New Pavement
	0.050	39	>75% Grass cover, Good, HSG A
*	0.900	98	Mill & Fill of Old Pavement
	1.000	95	Weighted Average
	0.050		5.00% Pervious Area
	0.950		95.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-15: Roadway



Summary for Subcatchment DR-16: Undisturbed Area

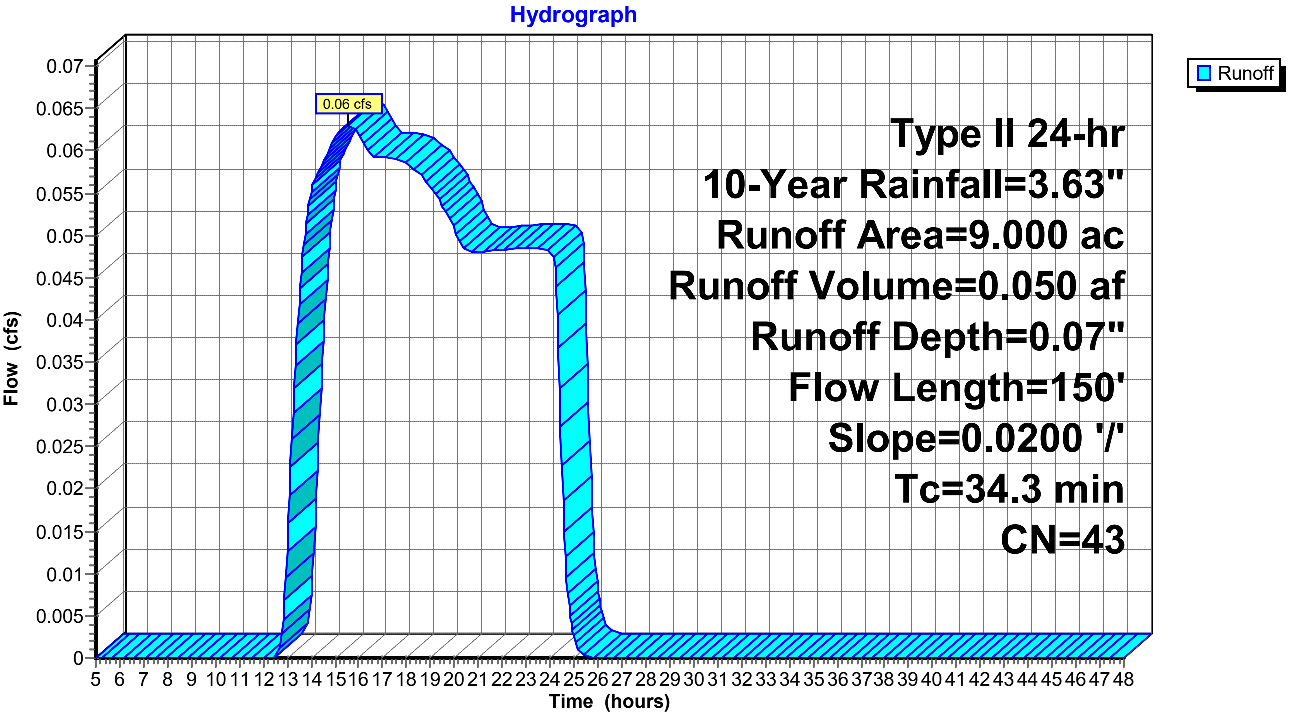
Runoff = 0.06 cfs @ 15.50 hrs, Volume= 0.050 af, Depth= 0.07"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

Area (ac)	CN	Description
9.000	43	Woods/grass comb., Fair, HSG A
9.000		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
34.3	150	0.0200	0.07		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.40"

Subcatchment DR-16: Undisturbed Area



Summary for Subcatchment DR-17: Roadway

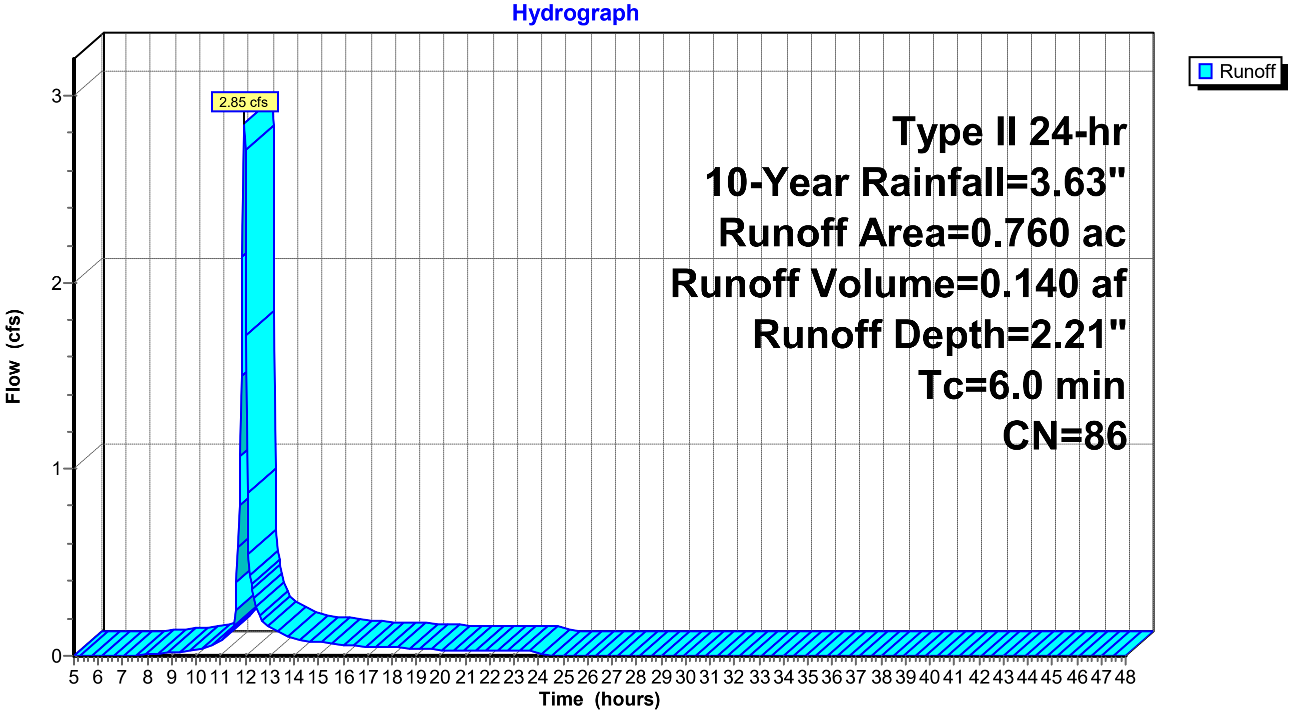
Runoff = 2.85 cfs @ 11.97 hrs, Volume= 0.140 af, Depth= 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	0.140	98	Road Widening
*	0.460	98	Roadway
	0.160	39	>75% Grass cover, Good, HSG A
	0.760	86	Weighted Average
	0.160		21.05% Pervious Area
	0.600		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-17: Roadway



Summary for Subcatchment DR-2: Storage

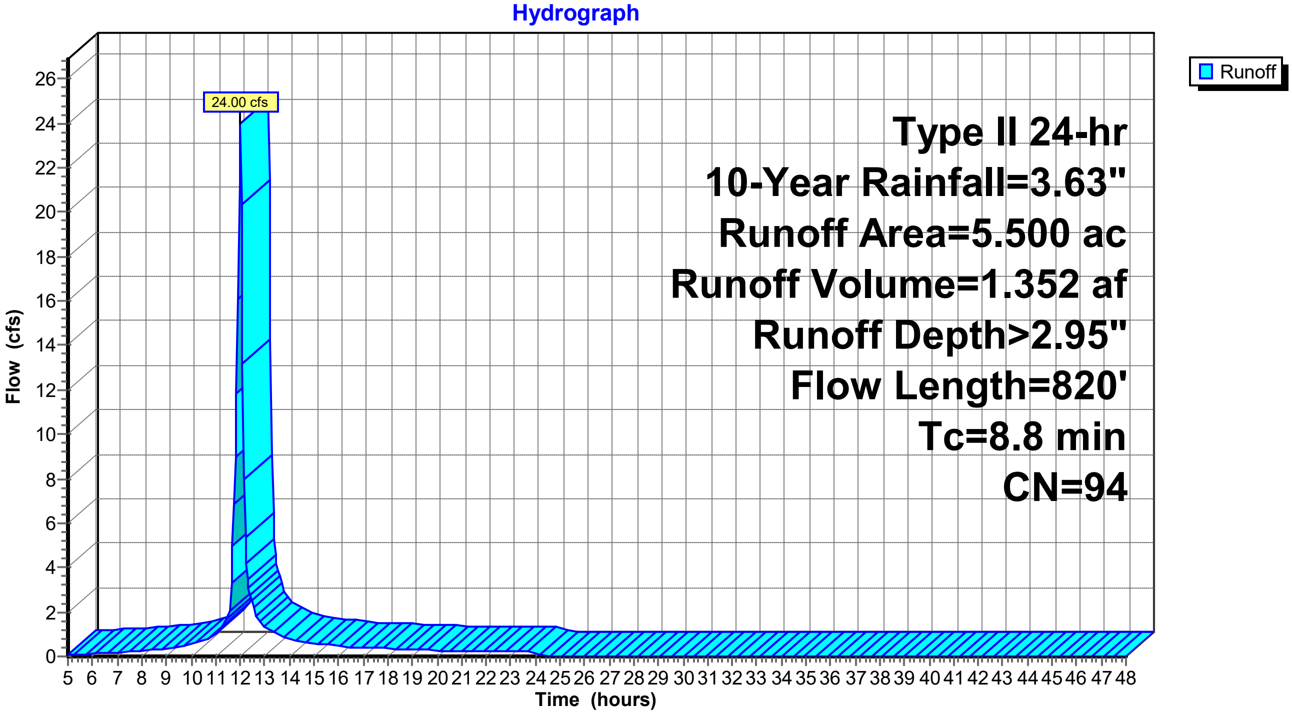
Runoff = 24.00 cfs @ 12.00 hrs, Volume= 1.352 af, Depth> 2.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	5.300	95	Dense Graded Aggregate
	0.200	80	>75% Grass cover, Good, HSG D
	5.500	94	Weighted Average
	5.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
4.9	470	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.6	250	0.0050	6.67	47.16	**Pipe Channel,** 36.0" Round Area= 7.1 sf Perim= 9.4' r= 0.75' n= 0.013
8.8	820	Total			

Subcatchment DR-2: Storage



Summary for Subcatchment DR-3: Rail & Storage

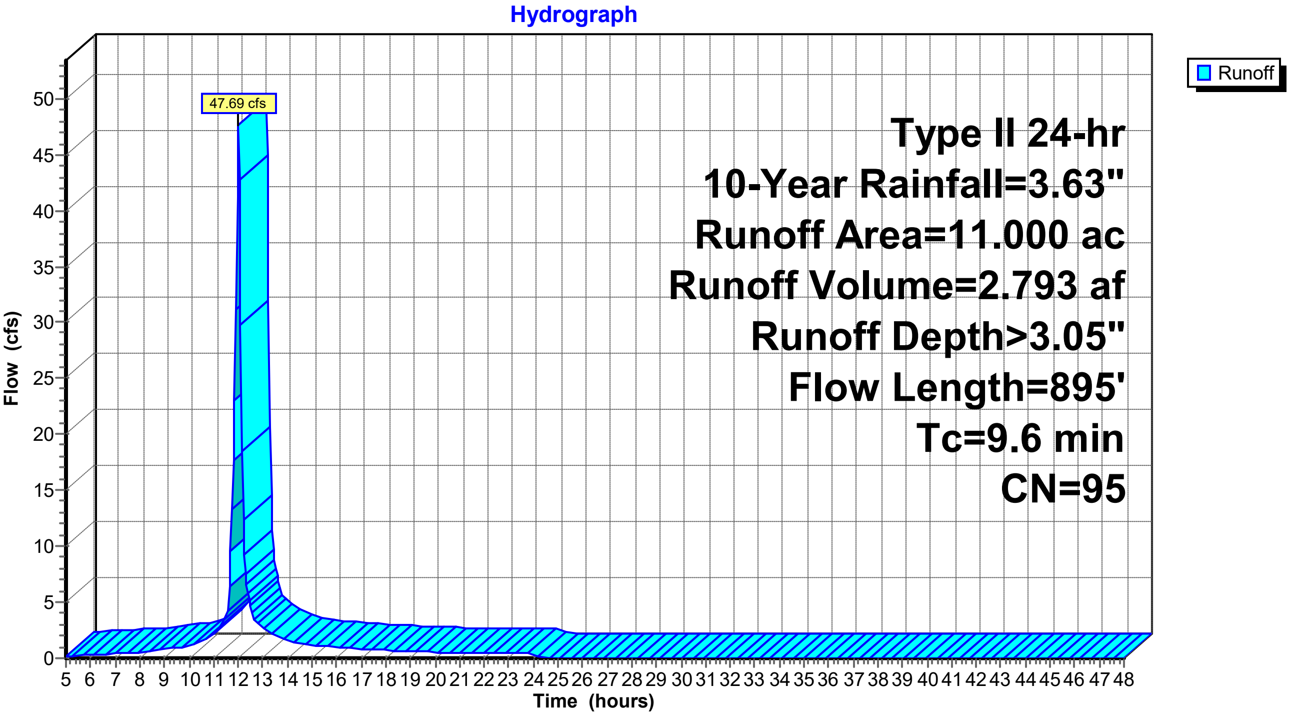
Runoff = 47.69 cfs @ 12.00 hrs, Volume= 2.793 af, Depth> 3.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	8.300	95	Compacted Gravel
	0.400	80	>75% Grass cover, Good, HSG D
*	2.300	98	Rail
	11.000	95	Weighted Average
	8.700		79.09% Pervious Area
	2.300		20.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
5.4	525	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.9	270	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
9.6	895	Total			

Subcatchment DR-3: Rail & Storage



Summary for Subcatchment DR-4: Storage

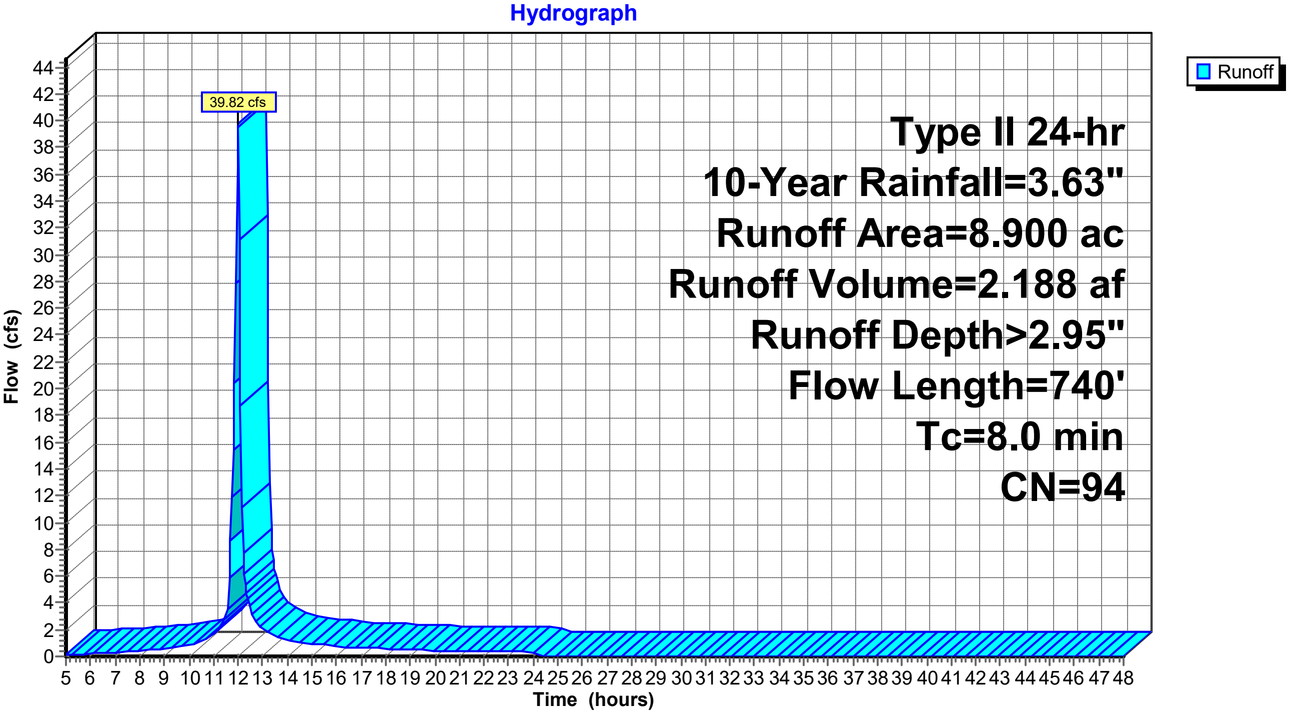
Runoff = 39.82 cfs @ 11.99 hrs, Volume= 2.188 af, Depth> 2.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	8.600	95	Compacted Gravel
	0.300	80	>75% Grass cover, Good, HSG D
	8.900	94	Weighted Average
	8.900		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
4.1	400	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.6	240	0.0050	6.67	47.16	**Pipe Channel,** 36.0" Round Area= 7.1 sf Perim= 9.4' r= 0.75' n= 0.013 Corrugated PE, smooth interior
8.0	740	Total			

Subcatchment DR-4: Storage



Summary for Subcatchment DR-5: Storage

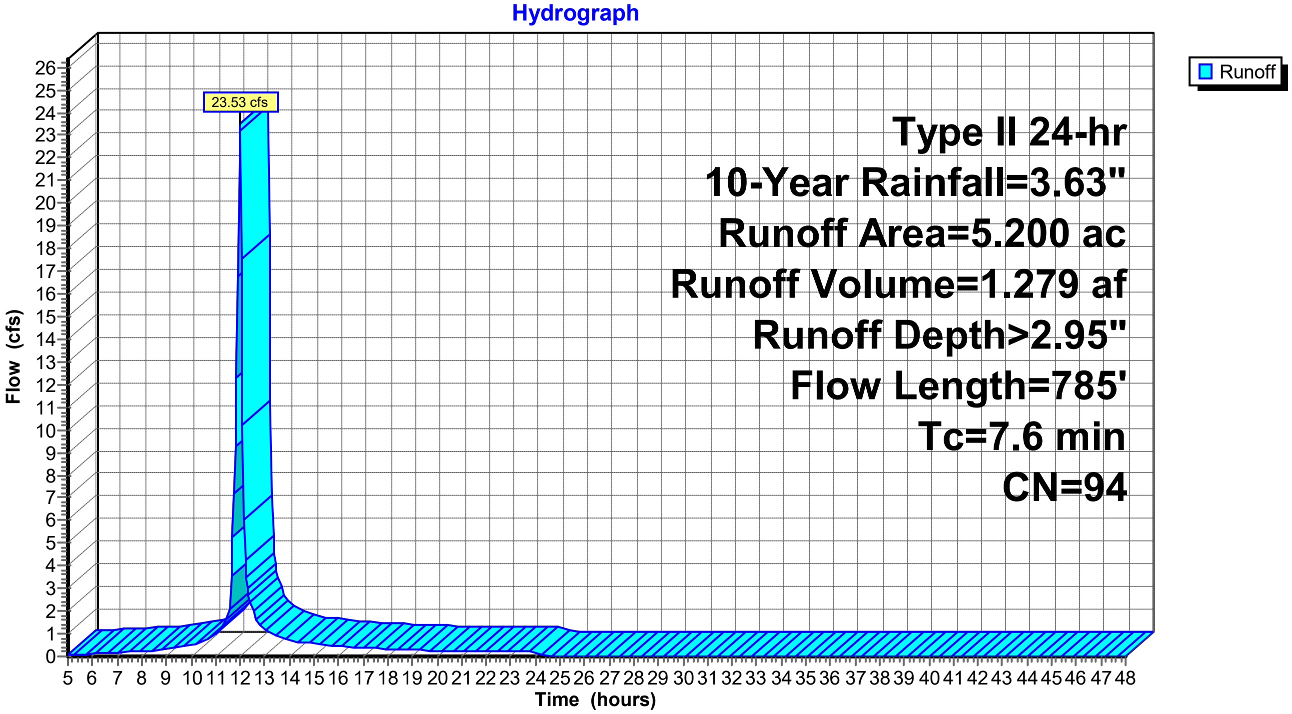
Runoff = 23.53 cfs @ 11.98 hrs, Volume= 1.279 af, Depth> 2.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	4.900	95	Dense Graded Aggregate
	0.300	80	>75% Grass cover, Good, HSG D
	5.200	94	Weighted Average
	5.200		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
3.0	285	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
1.3	400	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
7.6	785	Total			

Subcatchment DR-5: Storage



Summary for Subcatchment DR-6: Buldings B & D

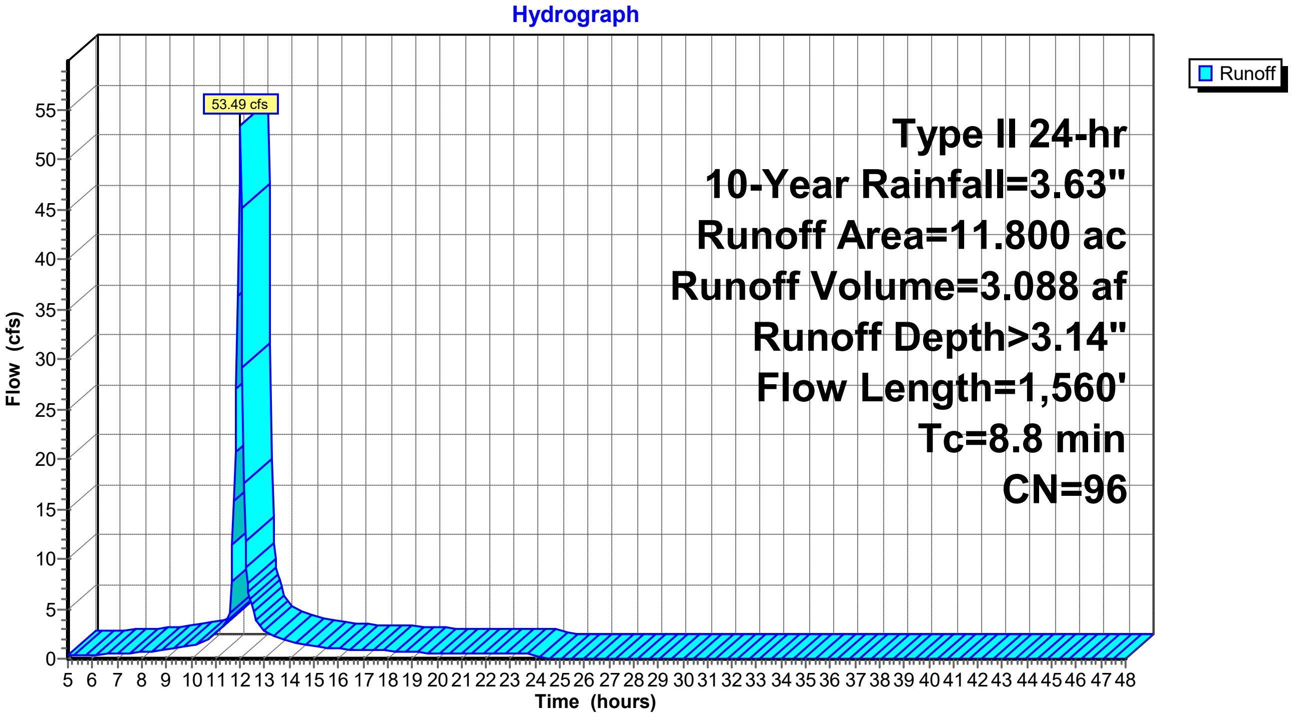
Runoff = 53.49 cfs @ 12.00 hrs, Volume= 3.088 af, Depth> 3.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	2.549	98	Building B
*	1.413	98	Building D
	0.200	80	>75% Grass cover, Good, HSG D
*	7.638	95	Dense Graded Aggregate
	11.800	96	Weighted Average
	7.838		66.42% Pervious Area
	3.962		33.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
1.0	100	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
4.5	1,360	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
8.8	1,560	Total			

Subcatchment DR-6: Buldings B & D



Summary for Subcatchment DR-7: Building C & Rail

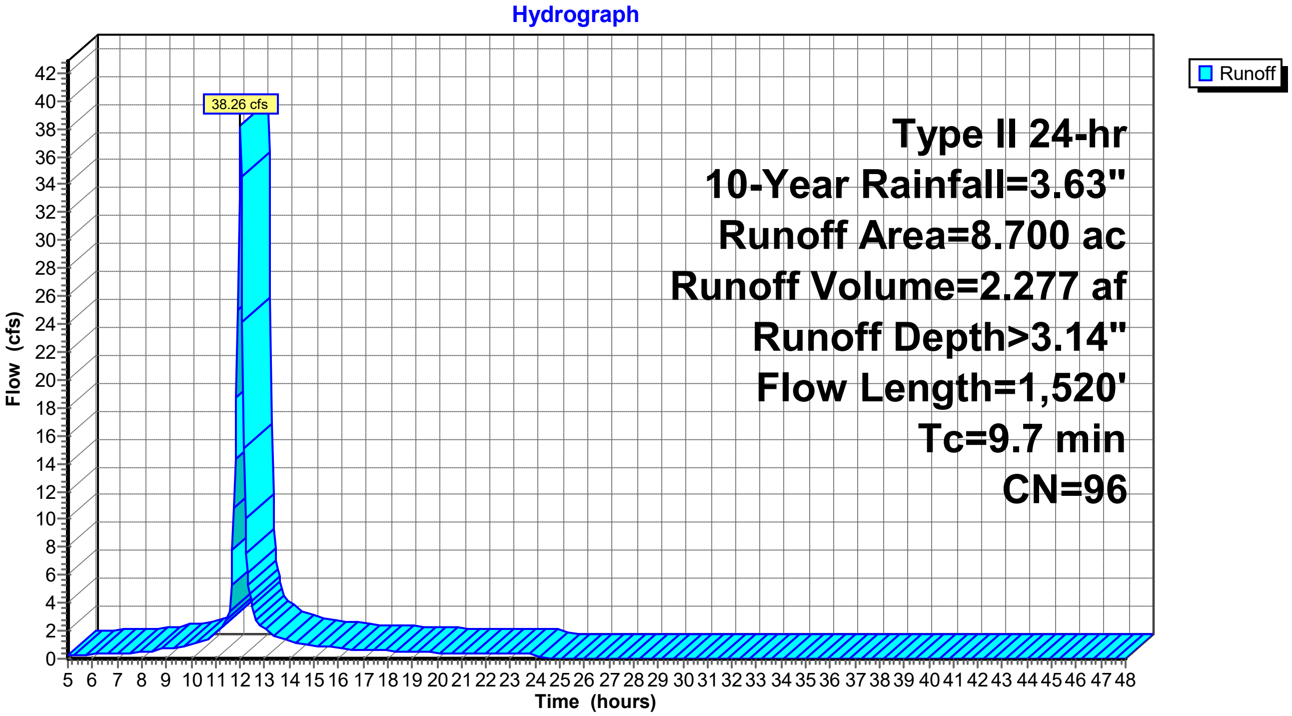
Runoff = 38.26 cfs @ 12.00 hrs, Volume= 2.277 af, Depth> 3.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	3.030	98	Building C
*	0.970	98	Rail
*	4.400	95	Dense Graded Aggregate
	0.300	80	>75% Grass cover, Good, HSG D
	8.700	96	Weighted Average
	4.700		54.02% Pervious Area
	4.000		45.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
2.6	250	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
3.8	1,170	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
9.7	1,520	Total			

Subcatchment DR-7: Building C & Rail



Summary for Subcatchment DR-8A: Parking

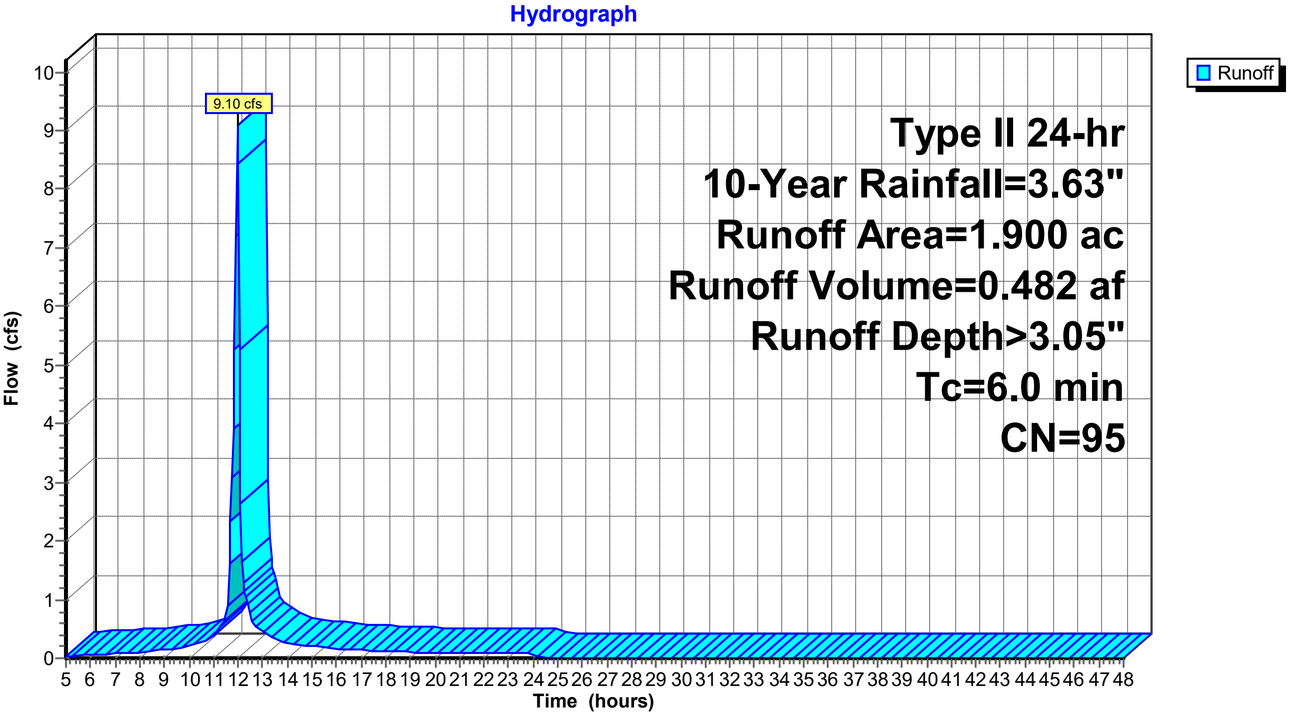
Runoff = 9.10 cfs @ 11.96 hrs, Volume= 0.482 af, Depth> 3.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	1.600	98	Parking
	0.300	80	>75% Grass cover, Good, HSG D
	1.900	95	Weighted Average
	0.300		15.79% Pervious Area
	1.600		84.21% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-8A: Parking



Summary for Subcatchment DR-8B: Roadway & Pond

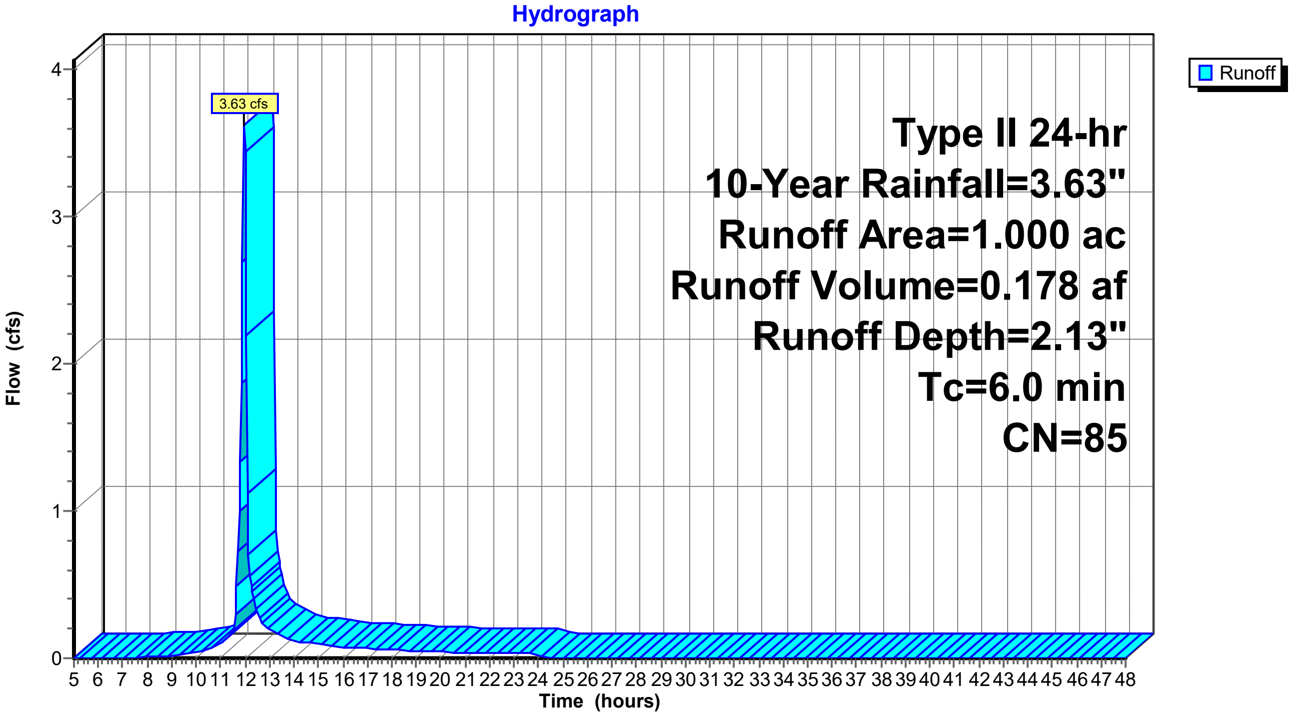
Runoff = 3.63 cfs @ 11.97 hrs, Volume= 0.178 af, Depth= 2.13"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	0.300	98	Roadway
	0.700	80	>75% Grass cover, Good, HSG D
	1.000	85	Weighted Average
	0.700		70.00% Pervious Area
	0.300		30.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-8B: Roadway & Pond



Summary for Subcatchment DR-9A: Parking & Substation

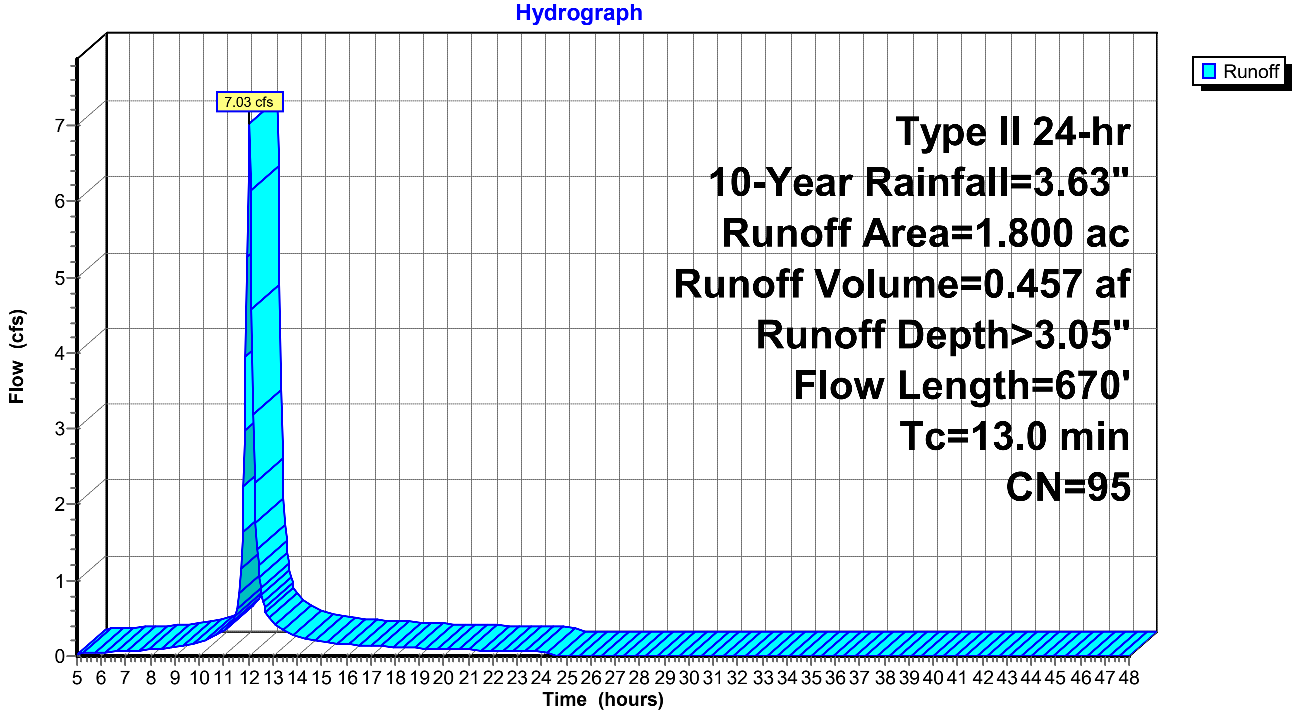
Runoff = 7.03 cfs @ 12.04 hrs, Volume= 0.457 af, Depth> 3.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
	0.230	80	>75% Grass cover, Good, HSG D
*	0.200	92	Compacted Gravel
*	1.200	98	Parking and Road
*	0.170	98	Substation
	1.800	95	Weighted Average
	0.430		23.89% Pervious Area
	1.370		76.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.4	100	0.0200	1.19		**Sheet Flow, Parking Lot Runoff** Smooth surfaces n= 0.011 P2= 2.40"
11.6	570	0.0030	0.82		**Shallow Concentrated Flow, Grass Lined Ditch to Pond** Grassed Waterway Kv= 15.0 fps
13.0	670	Total			

Subcatchment DR-9A: Parking & Substation



Summary for Subcatchment DR-9B: Roadway

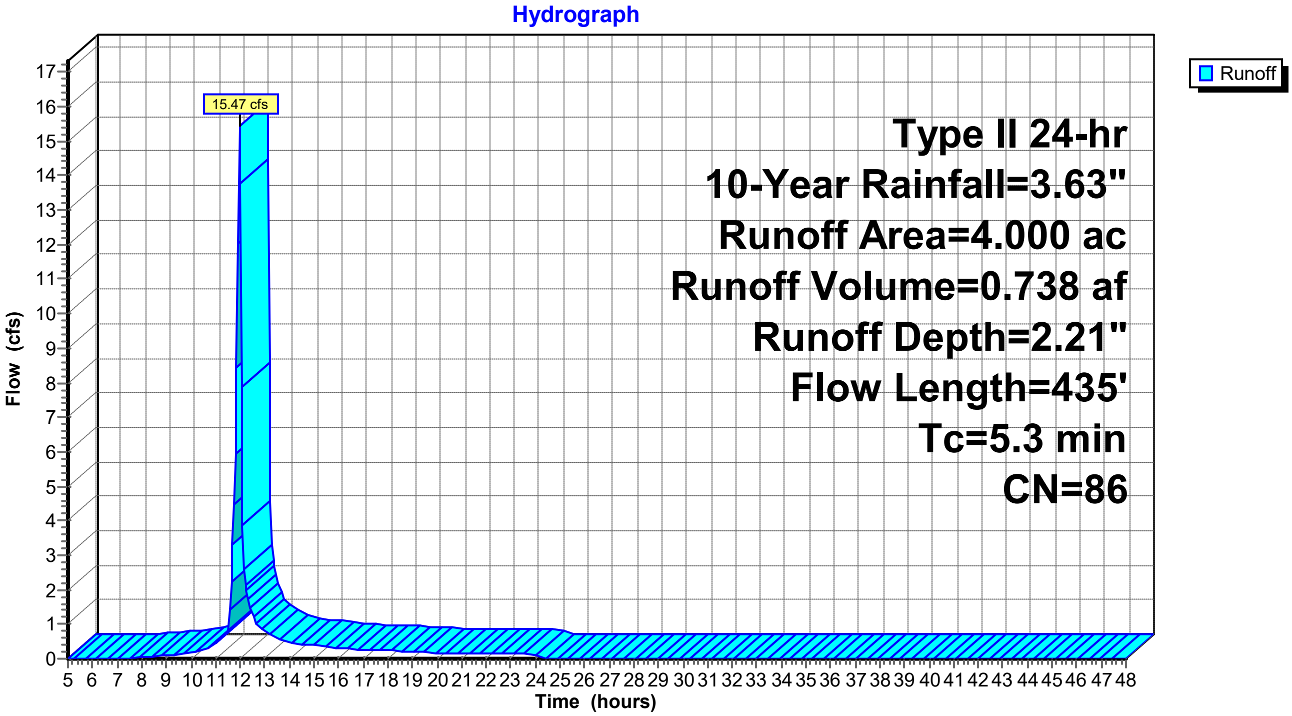
Runoff = 15.47 cfs @ 11.96 hrs, Volume= 0.738 af, Depth= 2.21"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	1.050	95	Dense Graded Aggregate
*	0.480	98	Roadway
	2.470	80	>75% Grass cover, Good, HSG D
	4.000	86	Weighted Average
	3.520		88.00% Pervious Area
	0.480		12.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.6	100	0.0250	0.46		**Sheet Flow, Dense Graded Aggregate Yard** n= 0.040 P2= 2.40"
1.3	230	0.0100	3.07	9.20	**Channel Flow, Grass lined ditch** Area= 3.0 sf Perim= 4.0' r= 0.75' n= 0.040 Earth, cobble bottom, clean sides
0.4	105	0.0050	4.20	7.43	**Pipe Channel, driveway culvert** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013 Corrugated PE, smooth interior
5.3	435	Total			

Subcatchment DR-9B: Roadway



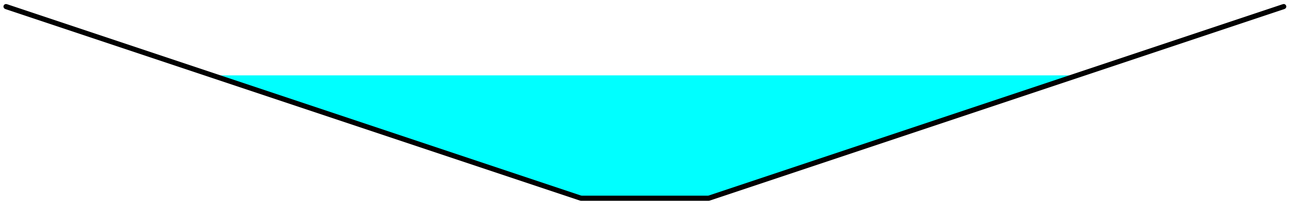
Summary for Reach 1R: Swale

Inflow Area = 1.900 ac, 84.21% Impervious, Inflow Depth > 3.05" for 10-Year event
Inflow = 9.10 cfs @ 11.96 hrs, Volume= 0.482 af
Outflow = 7.58 cfs @ 12.11 hrs, Volume= 0.482 af, Atten= 17%, Lag= 8.6 min

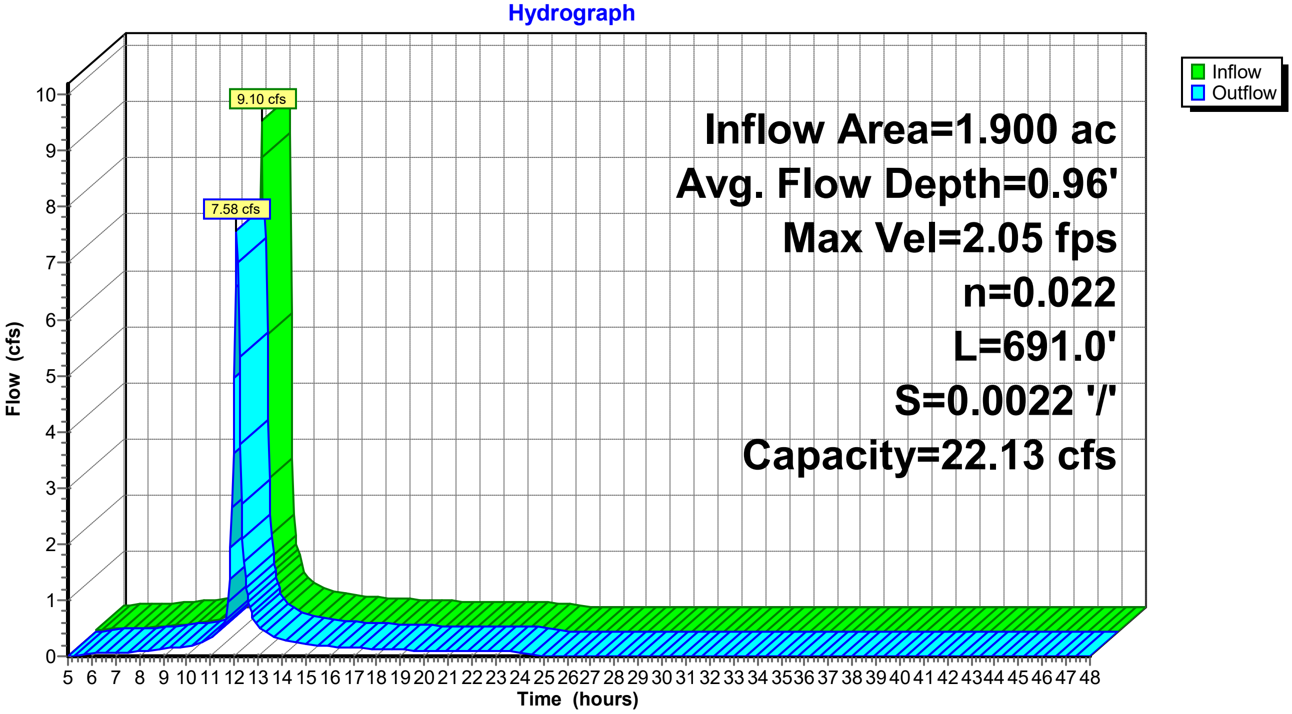
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.05 fps, Min. Travel Time= 5.6 min
Avg. Velocity = 0.60 fps, Avg. Travel Time= 19.1 min

Peak Storage= 2,580 cf @ 12.01 hrs
Average Depth at Peak Storage= 0.96' , Surface Width= 6.77'
Bank-Full Depth= 1.50' Flow Area= 8.3 sf, Capacity= 22.13 cfs

1.00' x 1.50' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 10.00'
Length= 691.0' Slope= 0.0022 '/'
Inlet Invert= 15.50', Outlet Invert= 14.00'



Reach 1R: Swale



Summary for Reach 1W: Wetland #1 / Analysis Point 1A

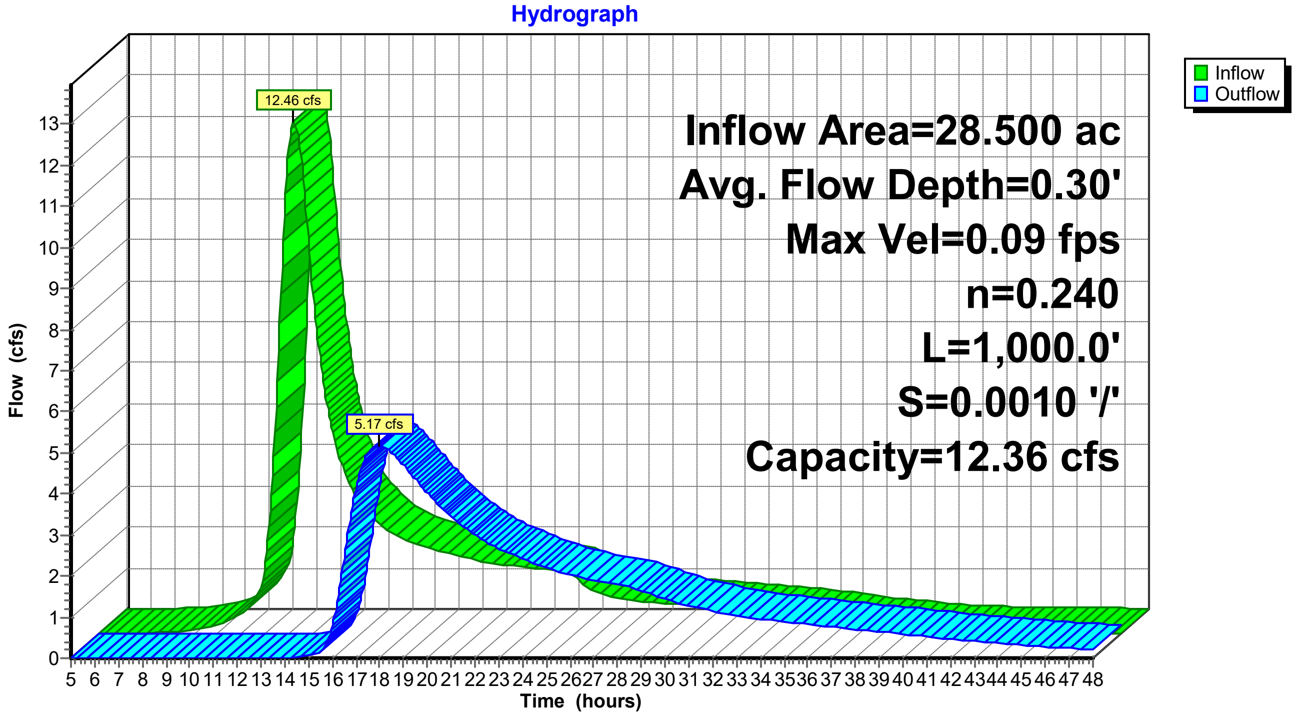
Inflow Area = 28.500 ac, 13.16% Impervious, Inflow Depth > 1.93" for 10-Year event
Inflow = 12.46 cfs @ 13.19 hrs, Volume= 4.590 af
Outflow = 5.17 cfs @ 17.95 hrs, Volume= 4.381 af, Atten= 58%, Lag= 285.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.09 fps, Min. Travel Time= 192.0 min
Avg. Velocity = 0.04 fps, Avg. Travel Time= 405.5 min

Peak Storage= 59,576 cf @ 14.75 hrs
Average Depth at Peak Storage= 0.30' , Surface Width= 201.78'
Bank-Full Depth= 0.50' Flow Area= 100.8 sf, Capacity= 12.36 cfs

200.00' x 0.50' deep channel, n= 0.240
Side Slope Z-value= 3.0 '/' Top Width= 203.00'
Length= 1,000.0' Slope= 0.0010 '/'
Inlet Invert= 6.00', Outlet Invert= 5.00'

Reach 1W: Wetland #1 / Analysis Point 1A



Summary for Reach 2R: Overflow

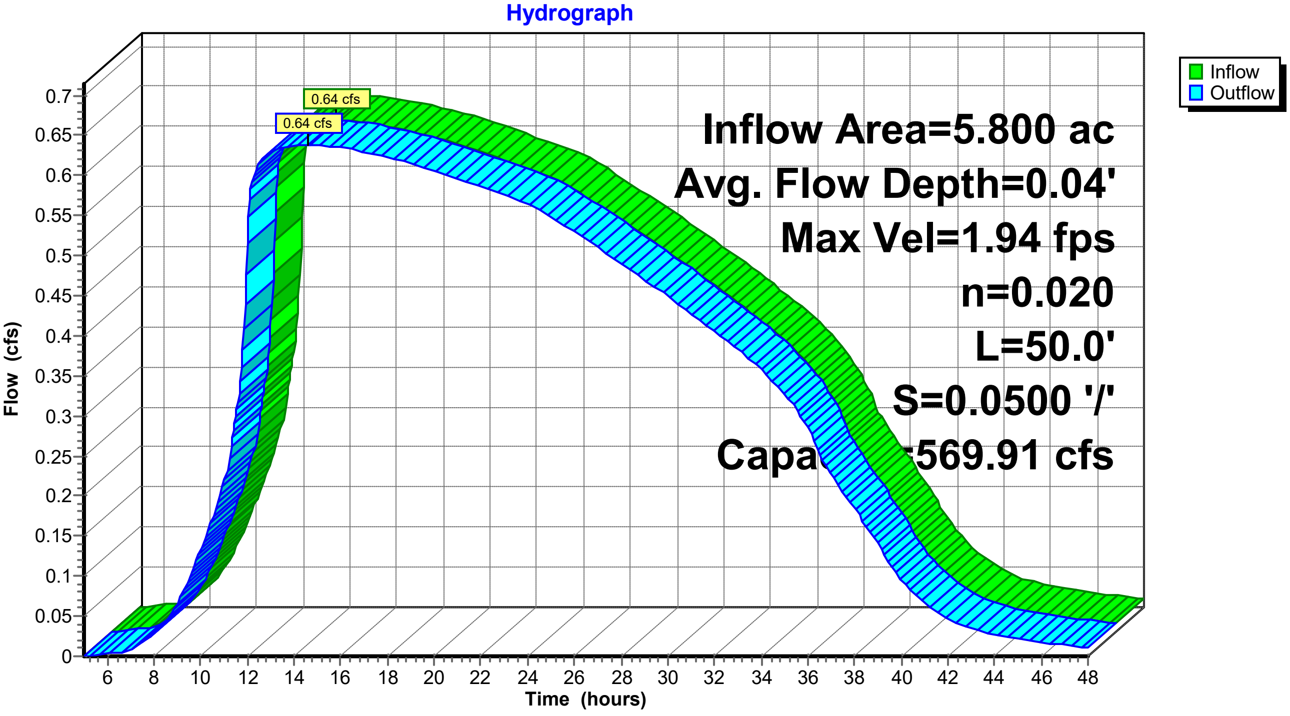
Inflow Area = 5.800 ac, 31.90% Impervious, Inflow Depth > 2.45" for 10-Year event
Inflow = 0.64 cfs @ 14.62 hrs, Volume= 1.187 af
Outflow = 0.64 cfs @ 14.63 hrs, Volume= 1.186 af, Atten= 0%, Lag= 0.7 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.94 fps, Min. Travel Time= 0.4 min
Avg. Velocity = 1.56 fps, Avg. Travel Time= 0.5 min

Peak Storage= 16 cf @ 14.62 hrs
Average Depth at Peak Storage= 0.04' , Surface Width= 8.24'
Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 569.91 cfs

8.00' x 2.00' deep channel, n= 0.020 Corrugated PE, corrugated interior
Side Slope Z-value= 3.0 '/' Top Width= 20.00'
Length= 50.0' Slope= 0.0500 '/'
Inlet Invert= 16.50', Outlet Invert= 14.00'

Reach 2R: Overflow



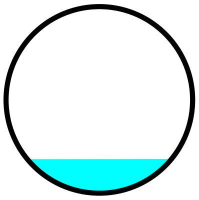
Summary for Reach 3R: Outlet Pipe

Inflow Area = 28.500 ac, 13.16% Impervious, Inflow Depth > 1.84" for 10-Year event
Inflow = 5.17 cfs @ 17.95 hrs, Volume= 4.381 af
Outflow = 5.17 cfs @ 17.95 hrs, Volume= 4.381 af, Atten= 0%, Lag= 0.4 min

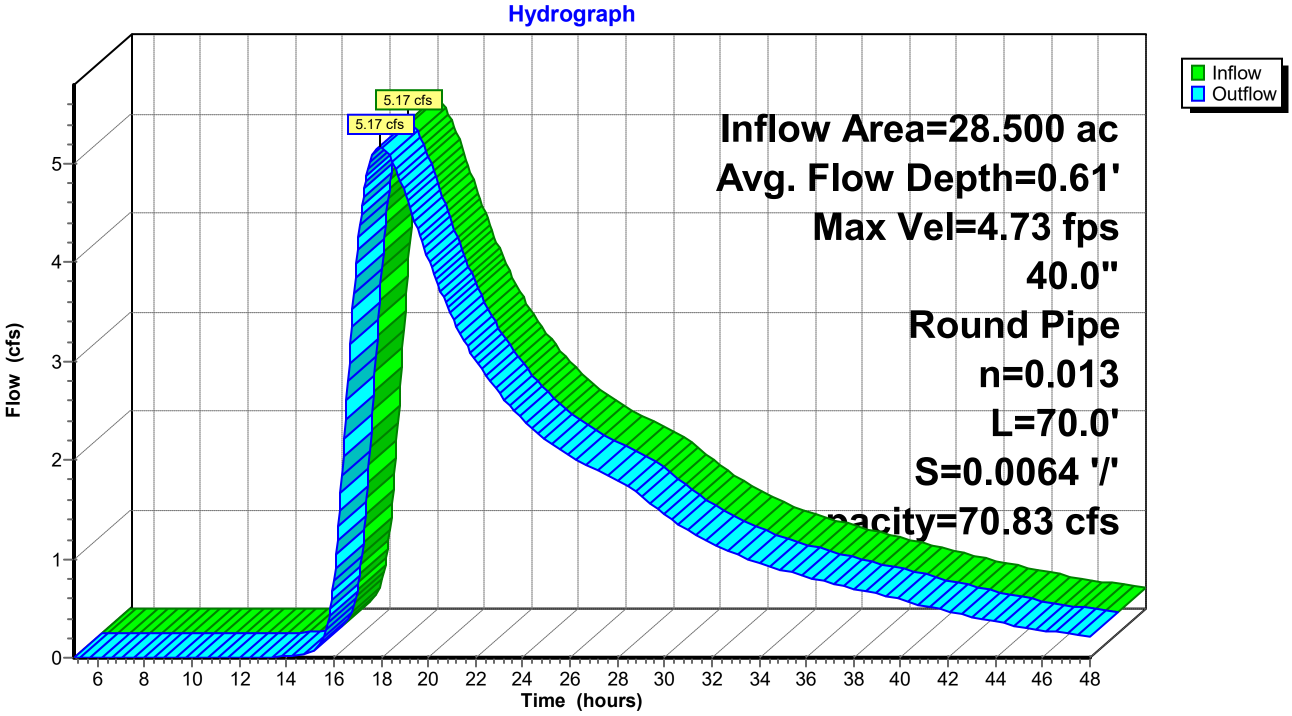
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.73 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 2.76 fps, Avg. Travel Time= 0.4 min

Peak Storage= 76 cf @ 17.95 hrs
Average Depth at Peak Storage= 0.61' , Surface Width= 2.58'
Bank-Full Depth= 3.33' Flow Area= 8.7 sf, Capacity= 70.83 cfs

40.0" Round Pipe
n= 0.013 Corrugated PE, smooth interior
Length= 70.0' Slope= 0.0064 '/'
Inlet Invert= 4.25', Outlet Invert= 3.80'



Reach 3R: Outlet Pipe



Summary for Reach 4R: Overflow

Inflow Area = 1.000 ac, 95.00% Impervious, Inflow Depth > 3.05" for 10-Year event
Inflow = 4.69 cfs @ 11.98 hrs, Volume= 0.254 af
Outflow = 4.56 cfs @ 12.00 hrs, Volume= 0.254 af, Atten= 3%, Lag= 0.8 min

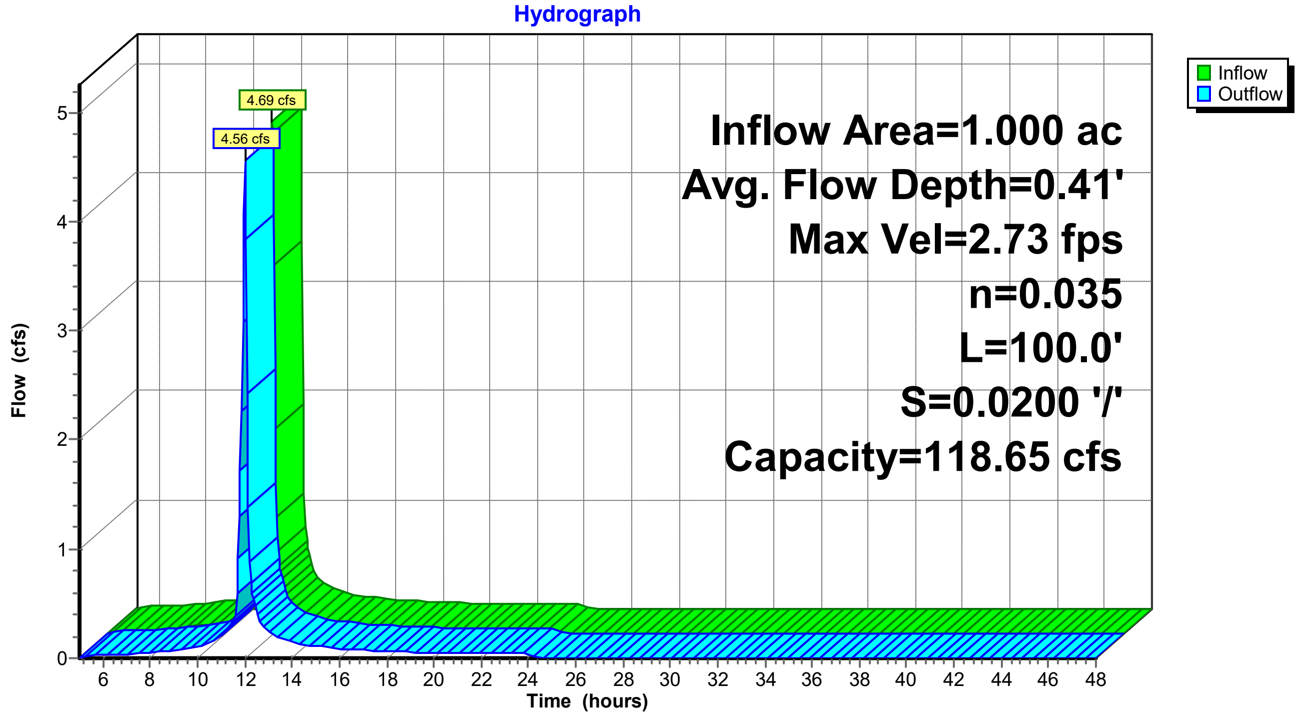
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.73 fps, Min. Travel Time= 0.6 min
Avg. Velocity = 0.70 fps, Avg. Travel Time= 2.4 min

Peak Storage= 171 cf @ 11.99 hrs
Average Depth at Peak Storage= 0.41' , Surface Width= 5.43'
Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 118.65 cfs

3.00' x 2.00' deep channel, n= 0.035 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 15.00'
Length= 100.0' Slope= 0.0200 '/'
Inlet Invert= 12.00', Outlet Invert= 10.00'

‡

Reach 4R: Overflow



Summary for Reach 5R: Overflow

Inflow Area =	1.200 ac,	37.50% Impervious,	Inflow Depth = 0.00"	for 10-Year event
Inflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af	
Outflow =	0.00 cfs @	5.00 hrs, Volume=	0.000 af,	Atten= 0%, Lag= 0.0 min

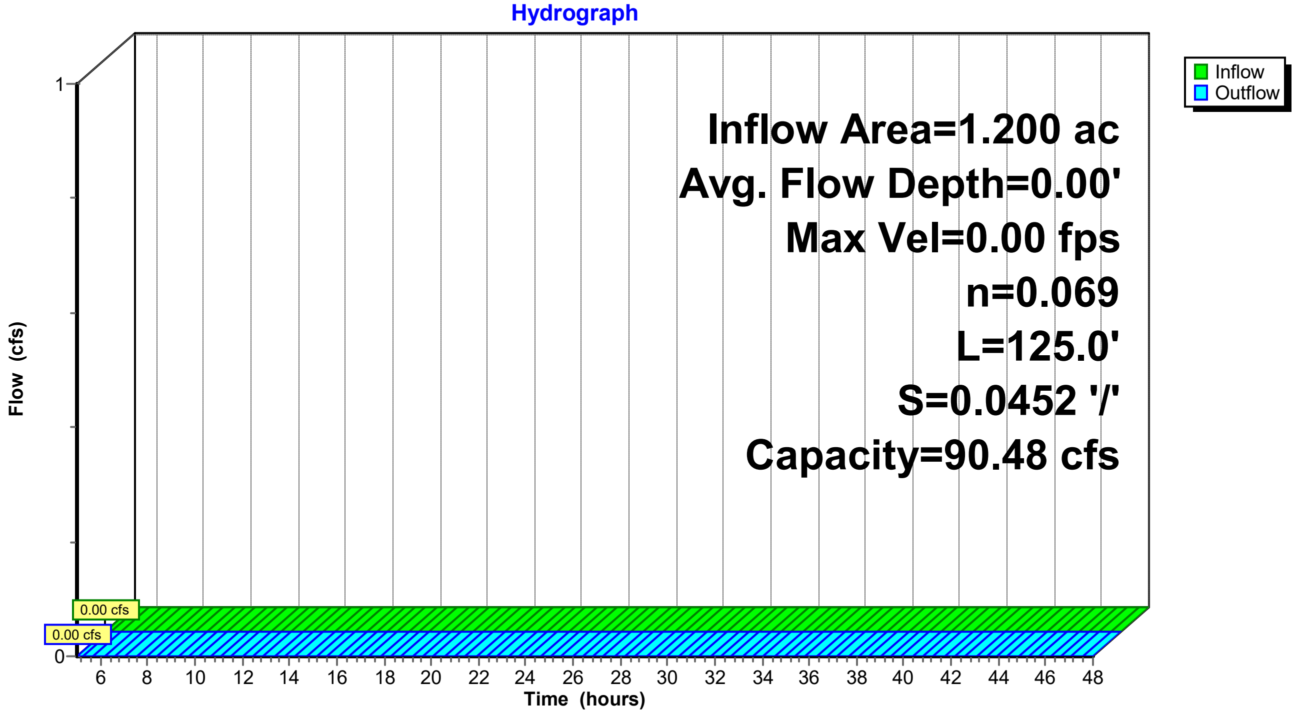
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 90.48 cfs

3.00' x 2.00' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 15.00'
Length= 125.0' Slope= 0.0452 '/'
Inlet Invert= 11.65', Outlet Invert= 6.00'

‡

Reach 5R: Overflow



Summary for Reach 6R: Overflow

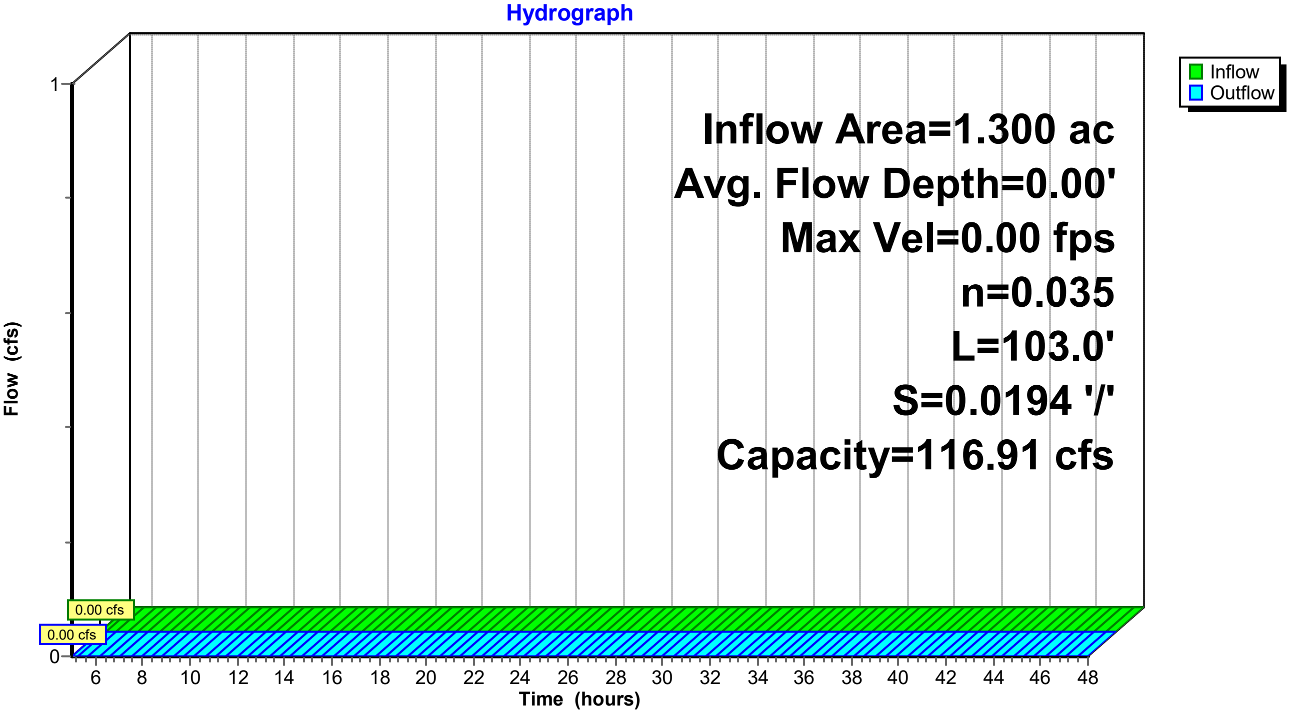
Inflow Area = 1.300 ac, 57.69% Impervious, Inflow Depth = 0.00" for 10-Year event
Inflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af
Outflow = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min
Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 5.00 hrs
Average Depth at Peak Storage= 0.00'
Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 116.91 cfs

3.00' x 2.00' deep channel, n= 0.035 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 15.00'
Length= 103.0' Slope= 0.0194 '/'
Inlet Invert= 8.50', Outlet Invert= 6.50'

Reach 6R: Overflow



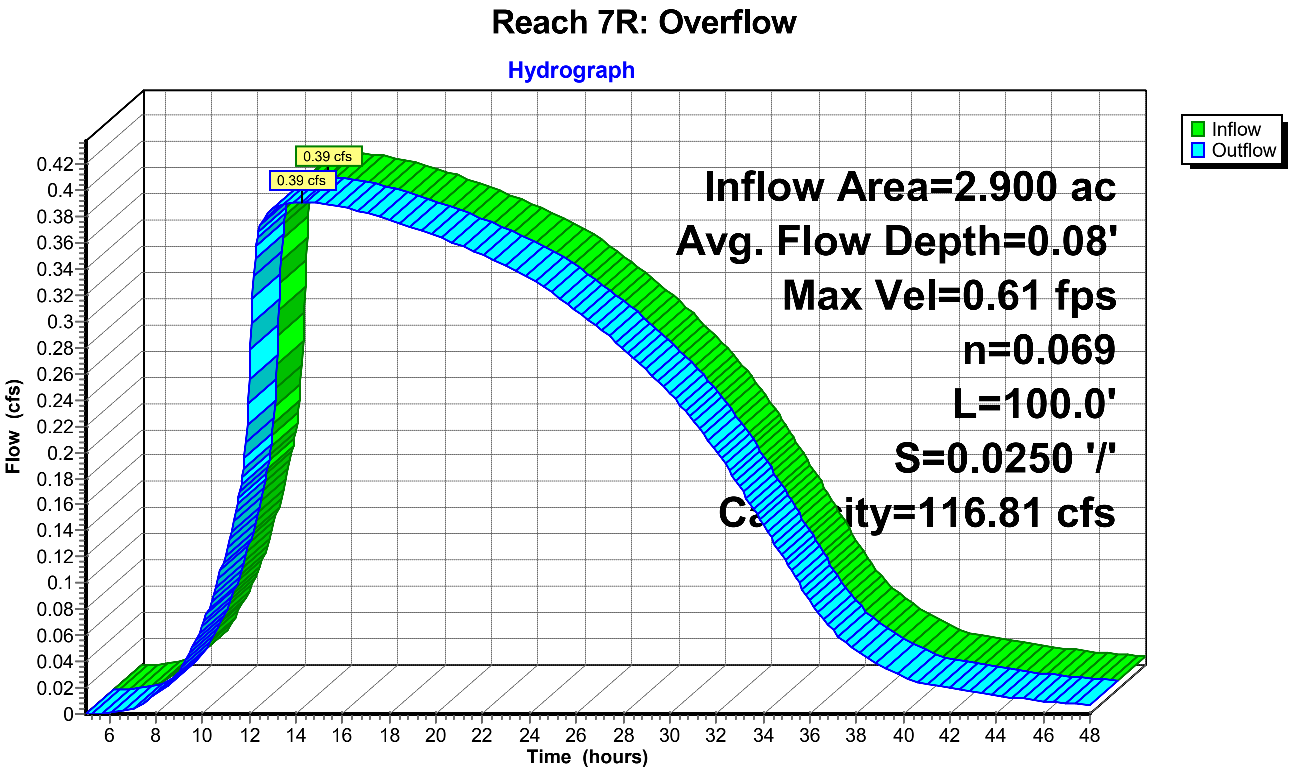
Summary for Reach 7R: Overflow

Inflow Area = 2.900 ac, 65.52% Impervious, Inflow Depth > 2.70" for 10-Year event
Inflow = 0.39 cfs @ 14.15 hrs, Volume= 0.652 af
Outflow = 0.39 cfs @ 14.22 hrs, Volume= 0.652 af, Atten= 0%, Lag= 4.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.61 fps, Min. Travel Time= 2.7 min
Avg. Velocity = 0.43 fps, Avg. Travel Time= 3.9 min

Peak Storage= 64 cf @ 14.18 hrs
Average Depth at Peak Storage= 0.08' , Surface Width= 8.47'
Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 116.81 cfs

8.00' x 2.00' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 20.00'
Length= 100.0' Slope= 0.0250 '/'
Inlet Invert= 14.50', Outlet Invert= 12.00'



Summary for Reach 8R: Dry Swale #1

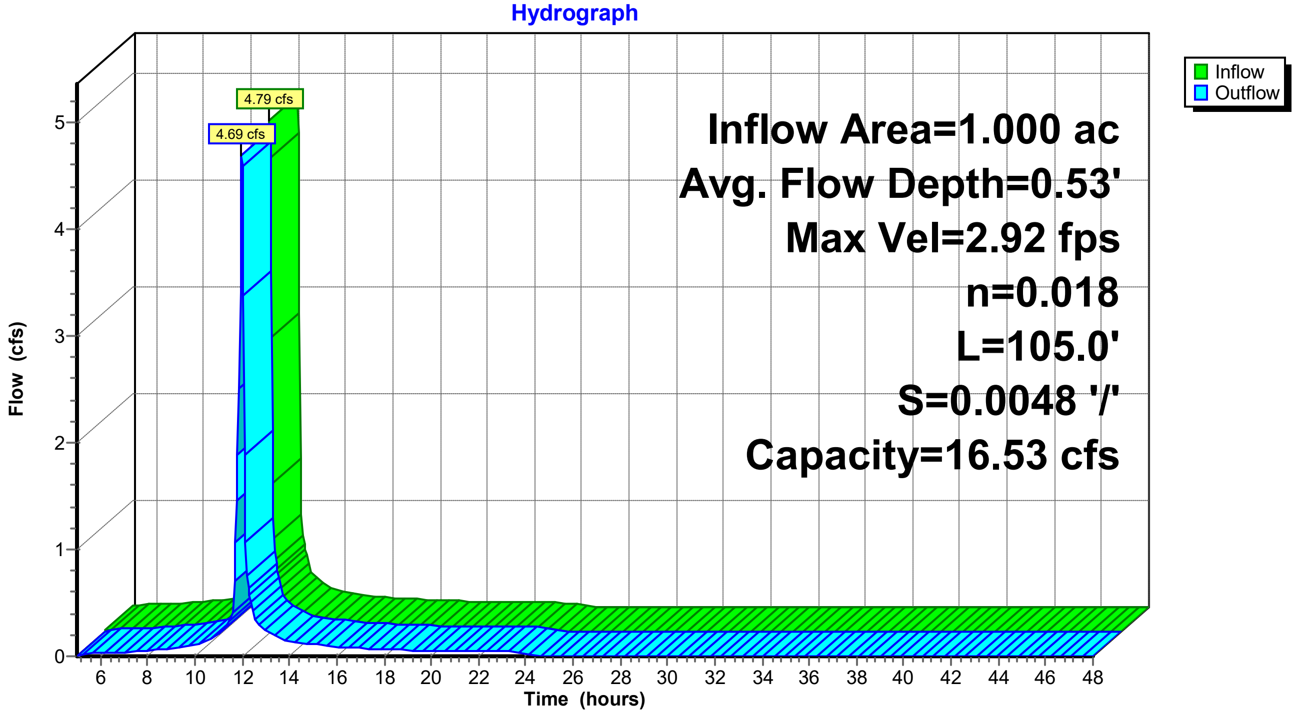
Inflow Area = 1.000 ac, 95.00% Impervious, Inflow Depth > 3.05" for 10-Year event
Inflow = 4.79 cfs @ 11.96 hrs, Volume= 0.254 af
Outflow = 4.69 cfs @ 11.98 hrs, Volume= 0.254 af, Atten= 2%, Lag= 1.1 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.92 fps, Min. Travel Time= 0.6 min
Avg. Velocity = 0.77 fps, Avg. Travel Time= 2.3 min

Peak Storage= 169 cf @ 11.97 hrs
Average Depth at Peak Storage= 0.53' , Surface Width= 4.11'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 16.53 cfs

2.00' x 1.00' deep channel, n= 0.018 Earth, clean & straight
Side Slope Z-value= 2.0 '/' Top Width= 6.00'
Length= 105.0' Slope= 0.0048 '/'
Inlet Invert= 10.00', Outlet Invert= 9.50'

Reach 8R: Dry Swale #1



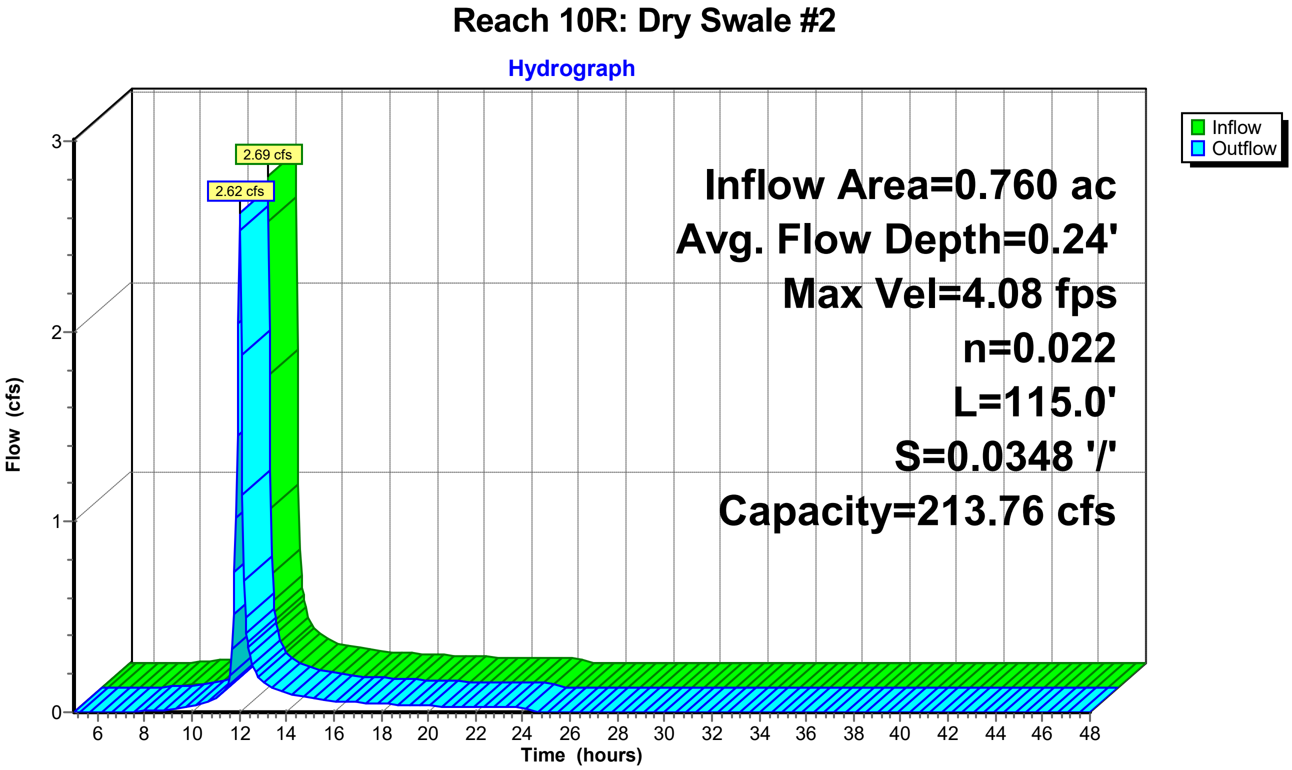
Summary for Reach 10R: Dry Swale #2

Inflow Area = 0.760 ac, 78.95% Impervious, Inflow Depth = 2.21" for 10-Year event
Inflow = 2.69 cfs @ 12.01 hrs, Volume= 0.140 af
Outflow = 2.62 cfs @ 12.02 hrs, Volume= 0.140 af, Atten= 3%, Lag= 0.8 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 4.08 fps, Min. Travel Time= 0.5 min
Avg. Velocity = 1.13 fps, Avg. Travel Time= 1.7 min

Peak Storage= 75 cf @ 12.01 hrs
Average Depth at Peak Storage= 0.24' , Surface Width= 3.44'
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 213.76 cfs

2.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 14.00'
Length= 115.0' Slope= 0.0348 '/'
Inlet Invert= 37.00', Outlet Invert= 33.00'



Summary for Reach 12R: Sediment Basin Overflow

Inflow Area = 1.200 ac, 37.50% Impervious, Inflow Depth = 0.46" for 10-Year event
Inflow = 0.26 cfs @ 12.31 hrs, Volume= 0.046 af
Outflow = 0.26 cfs @ 12.32 hrs, Volume= 0.046 af, Atten= 1%, Lag= 0.2 min

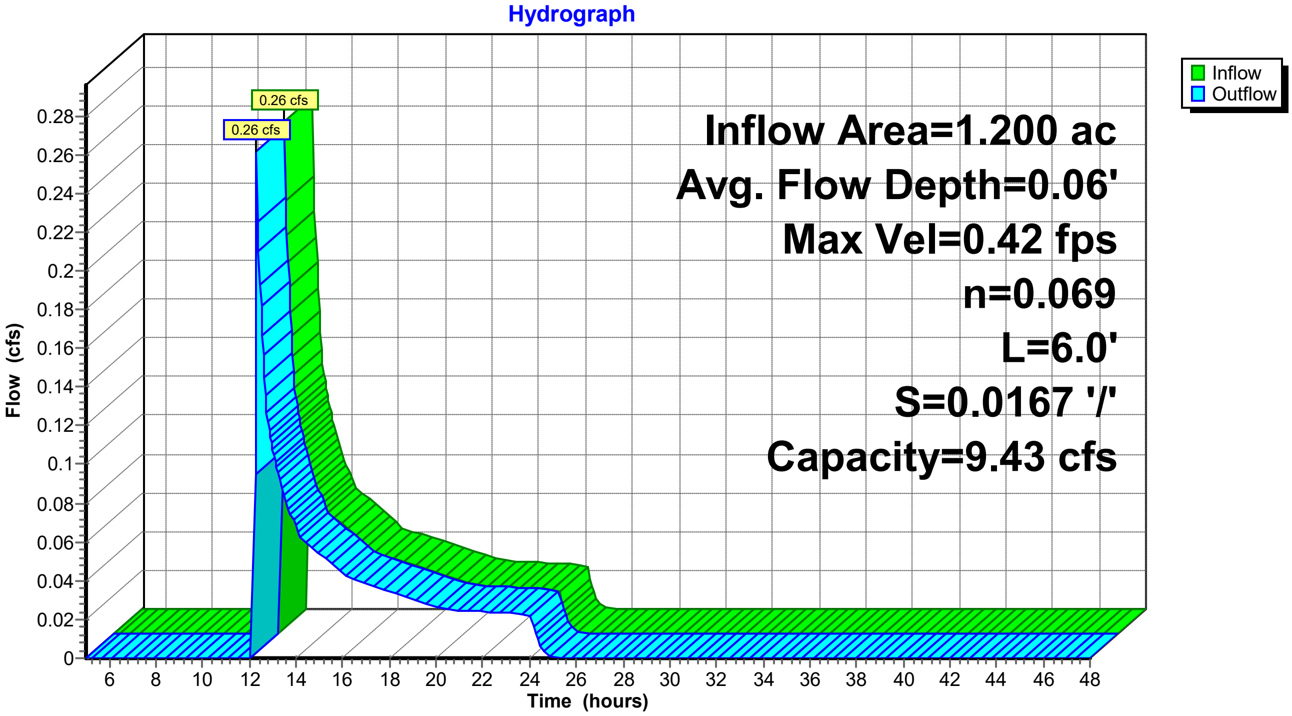
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.42 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 0.19 fps, Avg. Travel Time= 0.5 min

Peak Storage= 4 cf @ 12.32 hrs
Average Depth at Peak Storage= 0.06' , Surface Width= 10.50'
Bank-Full Depth= 0.50' Flow Area= 6.0 sf, Capacity= 9.43 cfs

10.00' x 0.50' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 4.0 '/' Top Width= 14.00'
Length= 6.0' Slope= 0.0167 '/'
Inlet Invert= 12.00', Outlet Invert= 11.90'

‡

Reach 12R: Sediment Basin Overflow



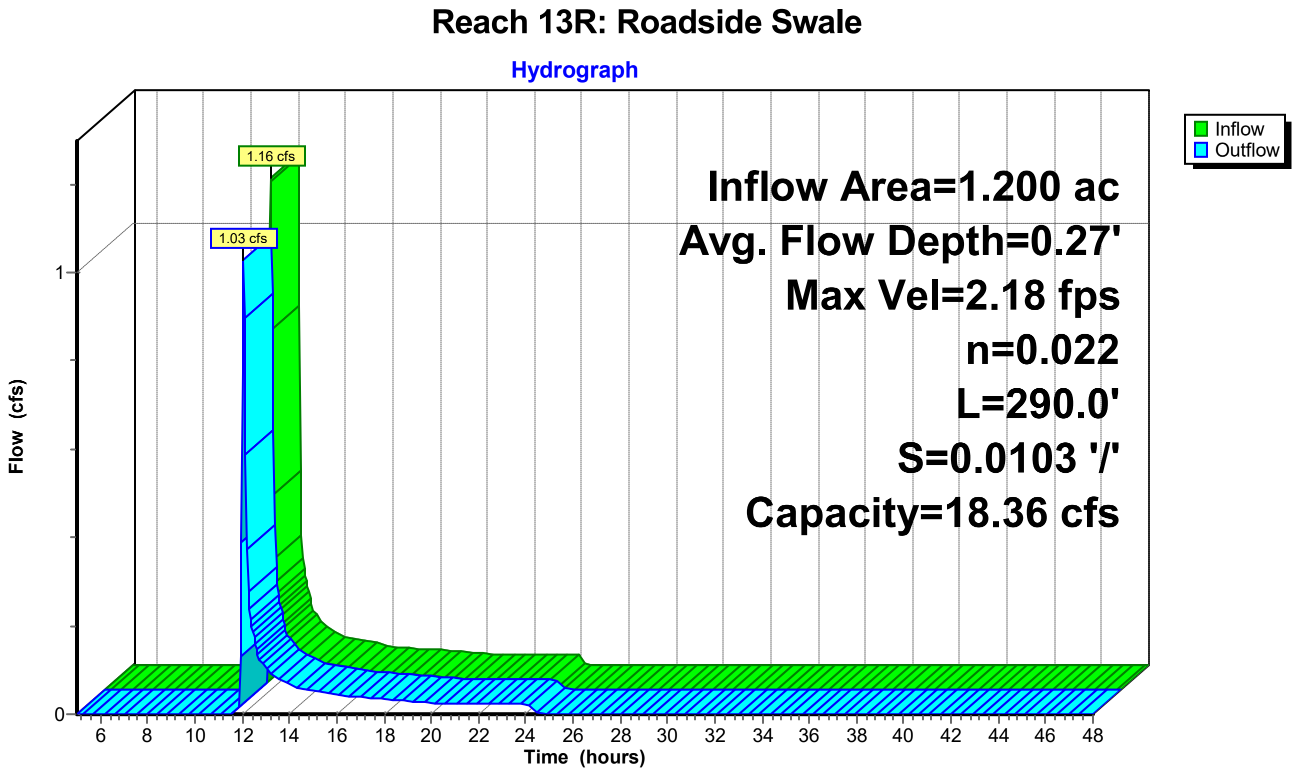
Summary for Reach 13R: Roadside Swale

Inflow Area = 1.200 ac, 37.50% Impervious, Inflow Depth = 0.63" for 10-Year event
Inflow = 1.16 cfs @ 11.99 hrs, Volume= 0.063 af
Outflow = 1.03 cfs @ 12.06 hrs, Volume= 0.063 af, Atten= 11%, Lag= 3.9 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.18 fps, Min. Travel Time= 2.2 min
Avg. Velocity = 0.77 fps, Avg. Travel Time= 6.3 min

Peak Storage= 141 cf @ 12.02 hrs
Average Depth at Peak Storage= 0.27' , Surface Width= 2.62'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 18.36 cfs

1.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 7.00'
Length= 290.0' Slope= 0.0103 '/'
Inlet Invert= 15.00', Outlet Invert= 12.00'



Summary for Reach 14R: Roadside Swale

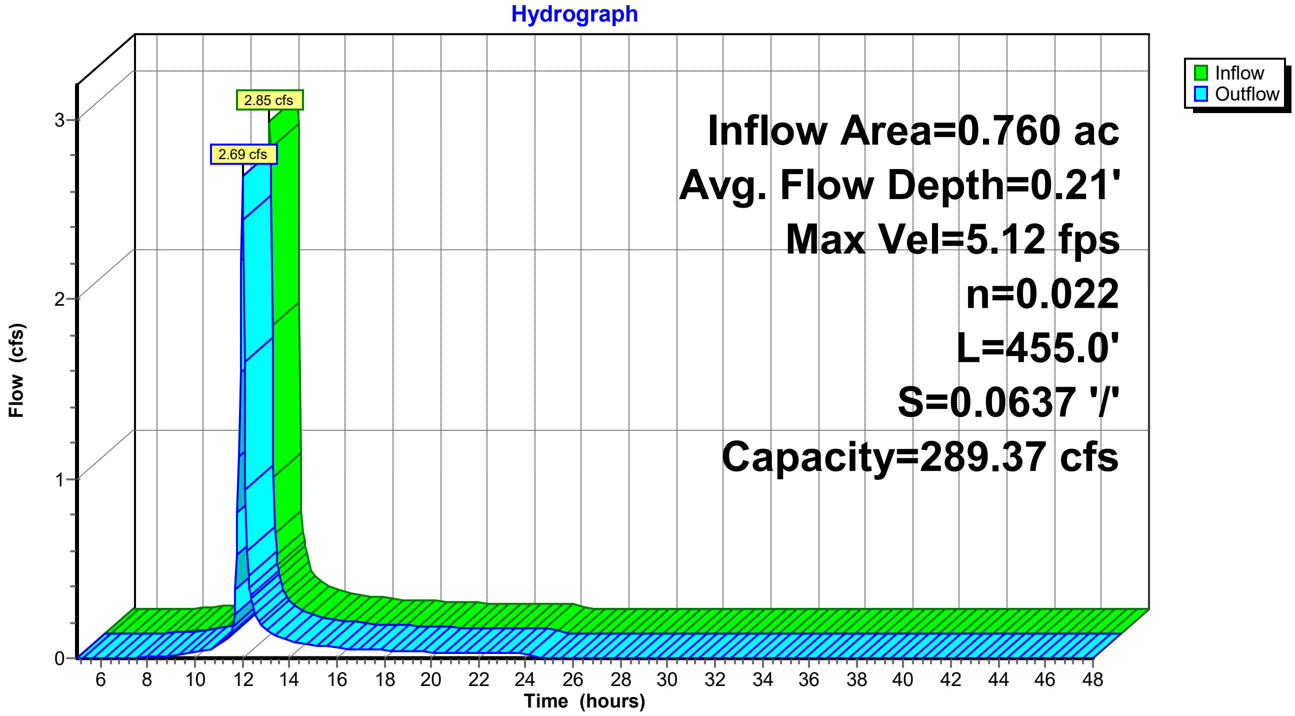
Inflow Area =	0.760 ac, 78.95% Impervious, Inflow Depth = 2.21"	for 10-Year event	
Inflow =	2.85 cfs @ 11.97 hrs, Volume=	0.140 af	
Outflow =	2.69 cfs @ 12.01 hrs, Volume=	0.140 af, Atten= 6%, Lag= 2.4 min	

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.12 fps, Min. Travel Time= 1.5 min
Avg. Velocity = 1.44 fps, Avg. Travel Time= 5.3 min

Peak Storage= 250 cf @ 11.99 hrs
Average Depth at Peak Storage= 0.21' , Surface Width= 3.25'
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 289.37 cfs

2.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 14.00'
Length= 455.0' Slope= 0.0637 '/'
Inlet Invert= 66.00', Outlet Invert= 37.00'

Reach 14R: Roadside Swale



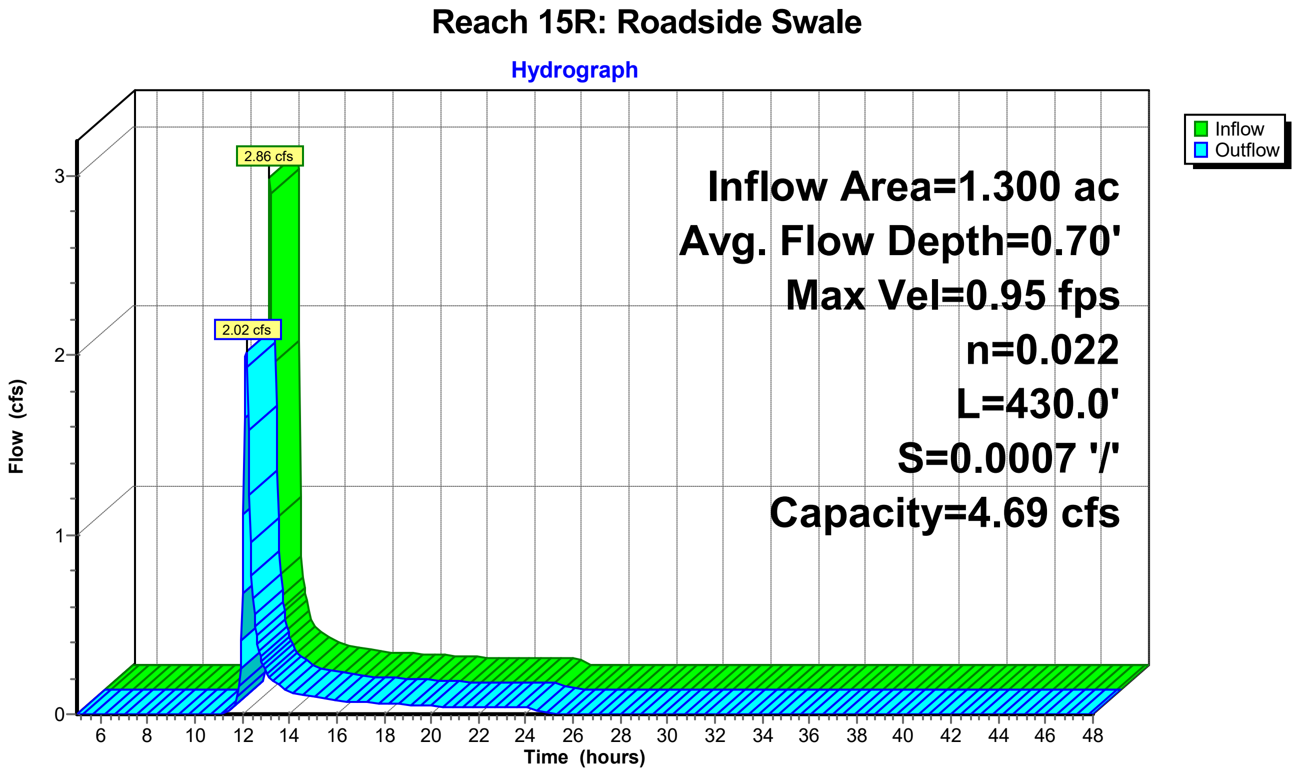
Summary for Reach 15R: Roadside Swale

Inflow Area = 1.300 ac, 57.69% Impervious, Inflow Depth = 1.27" for 10-Year event
Inflow = 2.86 cfs @ 11.98 hrs, Volume= 0.137 af
Outflow = 2.02 cfs @ 12.17 hrs, Volume= 0.137 af, Atten= 29%, Lag= 11.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.95 fps, Min. Travel Time= 7.5 min
Avg. Velocity = 0.27 fps, Avg. Travel Time= 26.8 min

Peak Storage= 939 cf @ 12.04 hrs
Average Depth at Peak Storage= 0.70' , Surface Width= 5.22'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 4.69 cfs

1.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 7.00'
Length= 430.0' Slope= 0.0007 '/'
Inlet Invert= 10.00', Outlet Invert= 9.71'



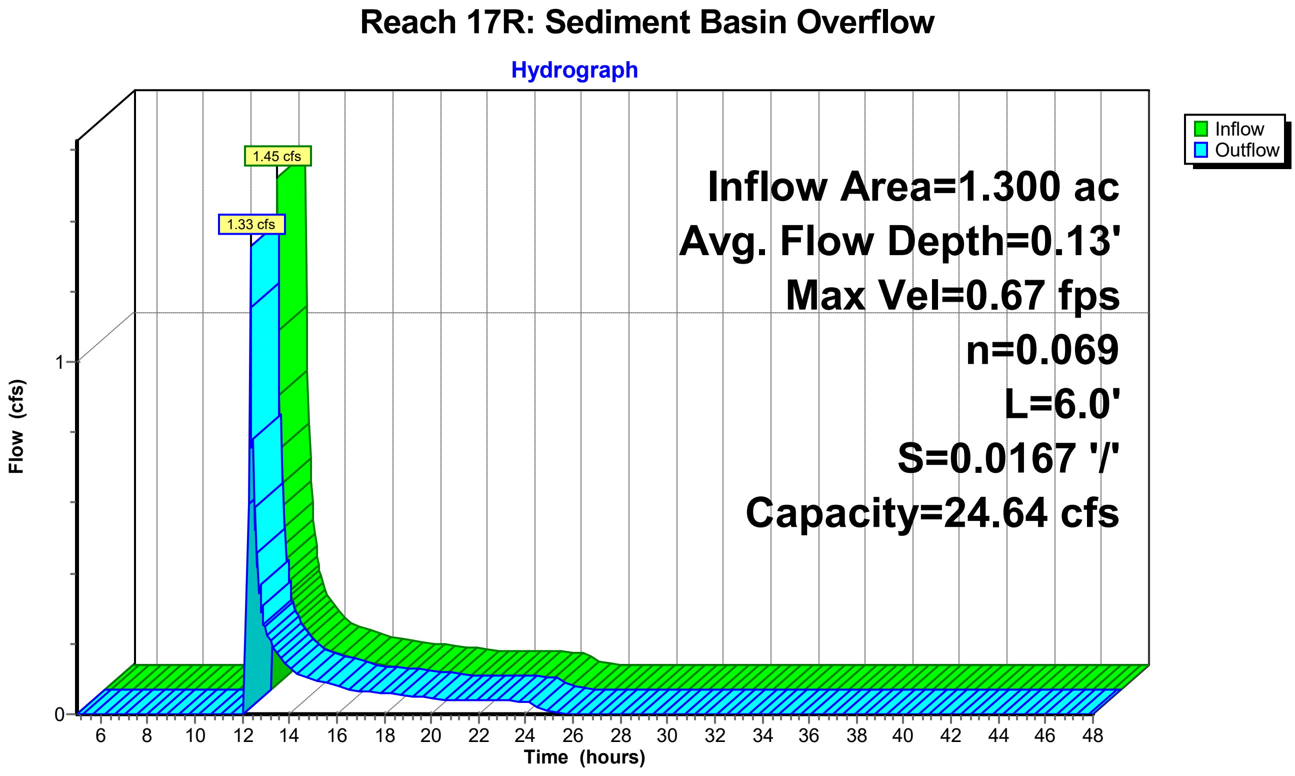
Summary for Reach 17R: Sediment Basin Overflow

Inflow Area =	1.300 ac, 57.69% Impervious,	Inflow Depth = 0.91"	for 10-Year event
Inflow =	1.45 cfs @ 12.31 hrs,	Volume=	0.098 af
Outflow =	1.33 cfs @ 12.32 hrs,	Volume=	0.098 af, Atten= 9%, Lag= 0.5 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.67 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 0.17 fps, Avg. Travel Time= 0.6 min

Peak Storage= 12 cf @ 12.32 hrs
Average Depth at Peak Storage= 0.13' , Surface Width= 16.07'
Bank-Full Depth= 0.70' Flow Area= 12.5 sf, Capacity= 24.64 cfs

15.00' x 0.70' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 4.0 '/' Top Width= 20.60'
Length= 6.0' Slope= 0.0167 '/'
Inlet Invert= 9.00', Outlet Invert= 8.90'



Summary for Pond 1P: WQv Pond #1

Inflow Area = 2.900 ac, 65.52% Impervious, Inflow Depth = 2.73" for 10-Year event
Inflow = 8.78 cfs @ 12.07 hrs, Volume= 0.660 af
Outflow = 0.39 cfs @ 14.15 hrs, Volume= 0.652 af, Atten= 96%, Lag= 125.0 min
Primary = 0.39 cfs @ 14.15 hrs, Volume= 0.652 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Starting Elev= 14.00' Surf.Area= 9,229 sf Storage= 19,003 cf
Peak Elev= 15.59' @ 14.15 hrs Surf.Area= 19,456 sf Storage= 36,781 cf (17,778 cf above start)

Plug-Flow detention time= 1,195.5 min calculated for 0.216 af (33% of inflow)
Center-of-Mass det. time= 536.4 min (1,336.8 - 800.3)

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	4,795 cf	**Forebay (Prismatic)** Listed below (Recalc)
#2	9.00'	57,882 cf	**Permanent Pool (Prismatic)** Listed below (Recalc)
		62,677 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	232	0	0
11.00	569	401	401
12.00	1,018	794	1,194
13.00	1,467	1,243	2,437
14.00	3,249	2,358	4,795

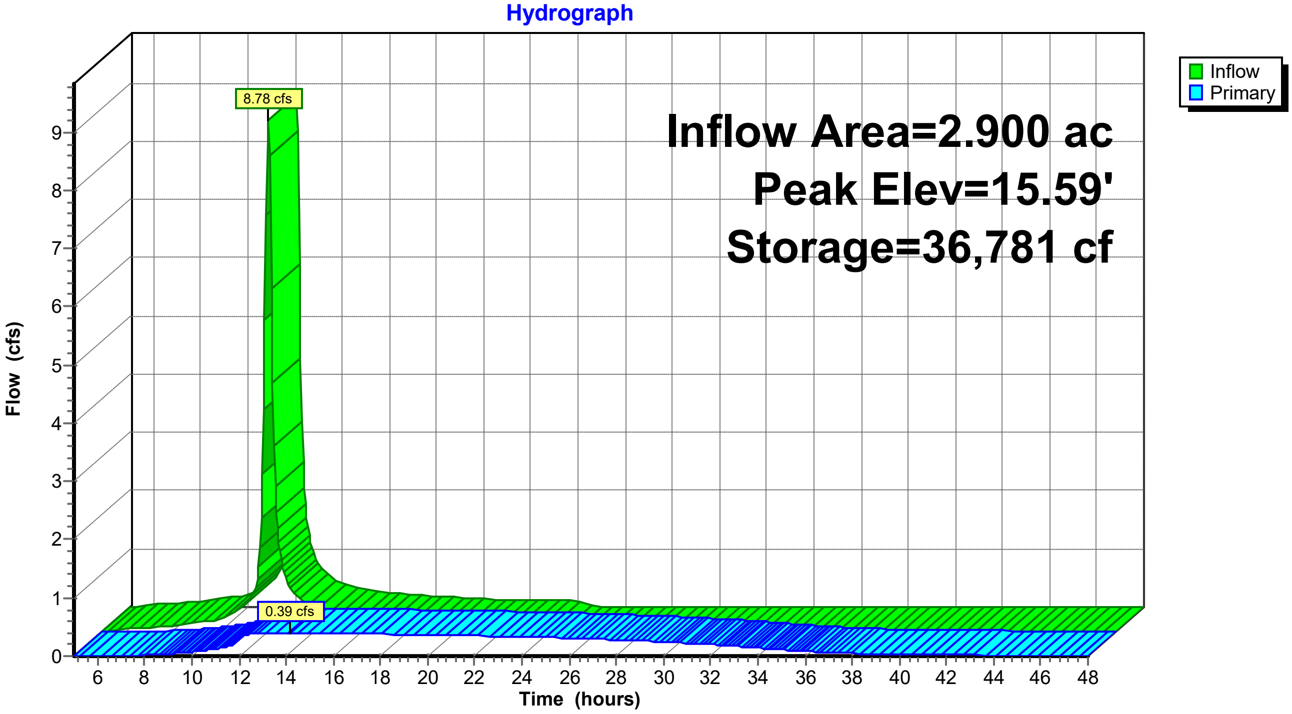
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.00	1,145	0	0
10.00	1,751	1,448	1,448
11.00	2,339	2,045	3,493
12.00	2,959	2,649	6,142
13.00	3,597	3,278	9,420
14.00	5,980	4,789	14,209
14.50	7,240	3,305	17,514
15.00	14,392	5,408	22,922
16.00	17,455	15,924	38,845
17.00	20,619	19,037	57,882

Device	Routing	Invert	Outlet Devices
#1	Primary	13.50'	**12.0" Round Culvert** L= 50.0' Box, headwall w/3 square edges, Ke= 0.500 Inlet / Outlet Invert= 13.50' / 13.23' S= 0.0054 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	16.00'	**24.0" x 24.0" Horiz. Outlet Structure Top Grate** C= 0.600 in 24.0" x 24.0" Grate (100% open area) Limited to weir flow at low heads
#3	Device 1	14.00'	**4.0" Round Reverse Slope Pipe** L= 40.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 9.00' / 14.00' S= -0.1250 '/' Cc= 0.900

			n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf
#4	Primary	16.25'	**6.0' long x 1.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Primary OutFlow Max=0.39 cfs @ 14.15 hrs HW=15.59' (Free Discharge)
↑—**1=Culvert** (Passes 0.39 cfs of 4.20 cfs potential flow)
  ↑—**2=Outlet Structure Top Grate** (Controls 0.00 cfs)
  └—**3=Reverse Slope Pipe** (Outlet Controls 0.39 cfs @ 4.48 fps)
└—**4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 1P: WQv Pond #1



Summary for Pond 2P: WQv Pond #2

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Inflow Area =       5.800 ac,  31.90% Impervious,  Inflow Depth >   2.47"    for  10-Year event
Inflow      =     21.14 cfs @   11.97 hrs,  Volume=          1.195 af
Outflow     =      0.64 cfs @   14.62 hrs,  Volume=          1.187 af,  Atten= 97%,  Lag= 158.7 min
Primary     =      0.64 cfs @   14.62 hrs,  Volume=          1.187 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Starting Elev= 14.00'   Surf.Area= 5,917 sf   Storage= 9,000 cf
Peak Elev= 16.47' @ 14.62 hrs   Surf.Area= 24,629 sf   Storage= 41,496 cf   (32,496 cf above start)

Plug-Flow detention time= 808.8 min calculated for 0.980 af (82% of inflow)
Center-of-Mass det. time= 597.8 min ( 1,399.8 - 802.0 )
```

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	4,020 cf	**Forebay #1 (Prismatic)** Listed below (Recalc)
#2	10.00'	2,575 cf	**Forebay #2 (Prismatic)** Listed below (Recalc)
#3	10.00'	58,093 cf	**Permanent Pool (Prismatic)** Listed below (Recalc)
		64,688 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	141	0	0
11.00	330	236	236
12.00	562	446	682
13.00	866	714	1,396
14.00	2,023	1,445	2,840
14.50	2,696	1,180	4,020

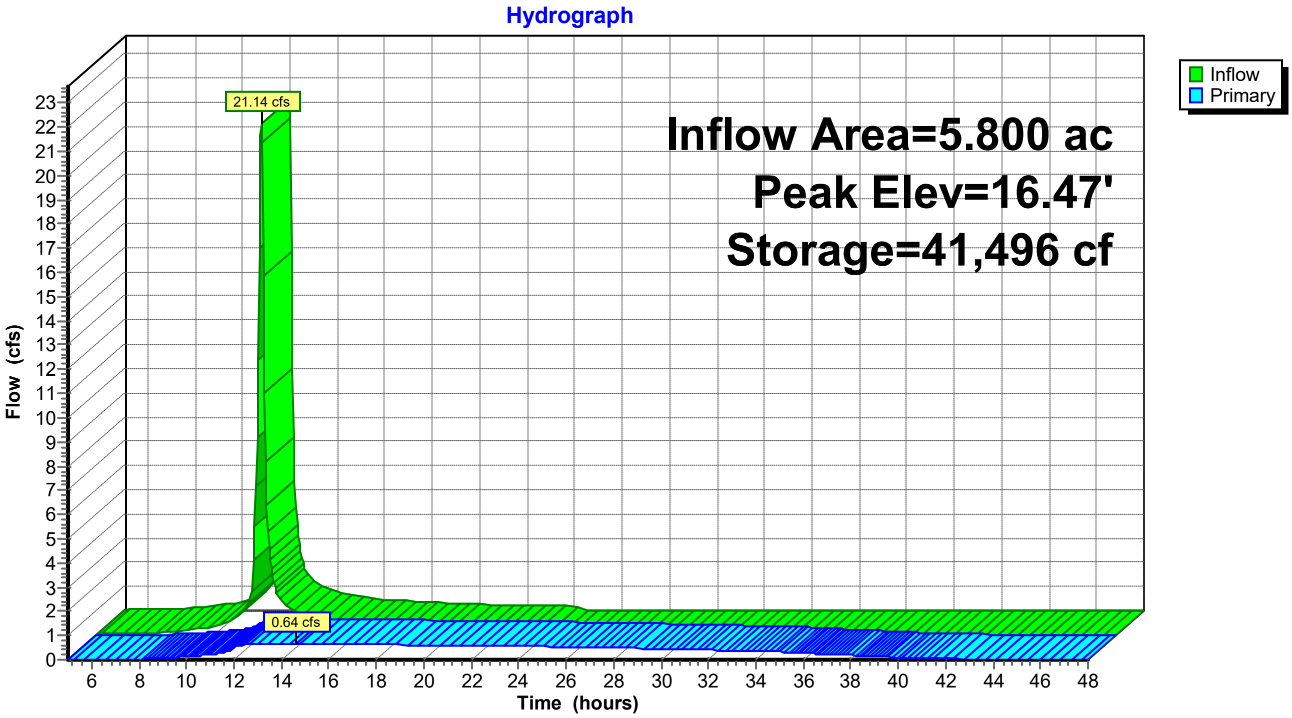
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	82	0	0
11.00	202	142	142
12.00	351	277	419
13.00	535	443	862
14.00	1,323	929	1,791
14.50	1,815	785	2,575

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	375	0	0
11.00	653	514	514
12.00	957	805	1,319
13.00	1,286	1,122	2,441
14.00	2,571	1,929	4,369
14.50	3,307	1,470	5,839
15.00	13,814	4,280	10,119
16.00	17,852	15,833	25,952
17.00	22,659	20,256	46,207
17.50	24,884	11,886	58,093

Device	Routing	Invert	Outlet Devices
#1	Device 3	16.50'	**24.0" x 24.0" Horiz. Orifice/Grate** C= 0.600 in 24.0" x 24.0" Grate (100% open area) Limited to weir flow at low heads
#2	Device 3	14.00'	**4.0" Vert. Reverse Slope Pipe** C= 0.600 Limited to weir flow at low heads
#3	Primary	14.00'	**12.0" Round Outlet Structure Discard Pipe** L= 46.0' Box, headwall w/3 square edges, Ke= 0.500 Inlet / Outlet Invert= 14.00' / 13.77' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#4	Primary	16.60'	**10.0' long x 0.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.64 cfs @ 14.62 hrs HW=16.47' (Free Discharge)
└**3=Outlet Structure Discard Pipe** (Passes 0.64 cfs of 4.79 cfs potential flow)
  └**1=Orifice/Grate** (Controls 0.00 cfs)
  └**2=Reverse Slope Pipe** (Orifice Controls 0.64 cfs @ 7.31 fps)
└**4=Broad-Crested Rectangular Weir** (Controls 0.00 cfs)

Pond 2P: WQv Pond #2



Summary for Pond 3P: Infiltration Basin #1

```
Inflow Area =         1.200 ac,  37.50% Impervious,  Inflow Depth =   0.46"    for  10-Year event
Inflow      =        0.26 cfs @   12.32 hrs,  Volume=            0.046 af
Outflow     =        0.02 cfs @   24.21 hrs,  Volume=            0.034 af,  Atten= 94%,  Lag= 713.8 min
Discarded  =         0.02 cfs @   24.21 hrs,  Volume=            0.034 af
Primary     =        0.00 cfs @    5.00 hrs,  Volume=            0.000 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 11.15' @ 24.21 hrs Surf.Area= 1,004 sf Storage= 1,468 cf

Plug-Flow detention time= 879.4 min calculated for 0.034 af (73% of inflow)
Center-of-Mass det. time= 771.6 min (1,748.3 - 976.8)

Volume	Invert	Avail.Storage	Storage Description
#1	8.10'	2,492 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

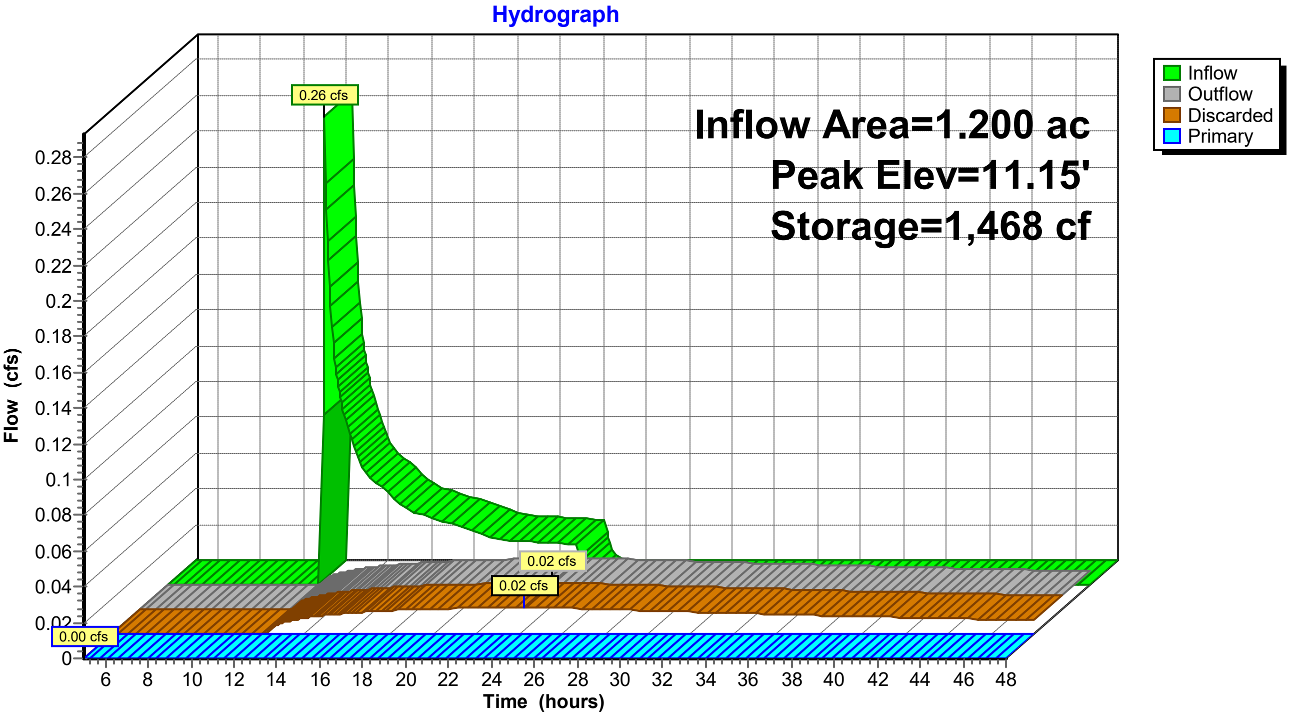
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
8.10	109	56.0	0	0	109
12.00	1,415	149.0	2,492	2,492	1,678

Device	Routing	Invert	Outlet Devices
#1	Primary	11.65'	**Channel/Reach** using Reach 5R: Overflow
#2	Discarded	8.10'	**0.500 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 4.00'

Discarded OutFlow Max=0.02 cfs @ 24.21 hrs HW=11.15' (Free Discharge)
↑—**2=Exfiltration** (Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=8.10' (Free Discharge)
↑—**1=Channel/Reach** (Controls 0.00 cfs)

Pond 3P: Infiltration Basin #1



Summary for Pond 4P: Infiltration Basin #2

```
Inflow Area =       1.300 ac, 57.69% Impervious, Inflow Depth =  0.91"   for 10-Year event
Inflow    =      1.33 cfs @  12.32 hrs, Volume=          0.098 af
Outflow   =      0.26 cfs @  12.92 hrs, Volume=          0.098 af, Atten= 80%, Lag= 35.5 min
Discarded =      0.26 cfs @  12.92 hrs, Volume=          0.098 af
Primary   =      0.00 cfs @   5.00 hrs, Volume=          0.000 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 8.10' @ 12.92 hrs Surf.Area= 701 sf Storage= 834 cf

Plug-Flow detention time= 35.0 min calculated for 0.098 af (100% of inflow)
Center-of-Mass det. time= 35.0 min (973.5 - 938.5)

Volume	Invert	Avail.Storage	Storage Description
#1	5.80'	2,495 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

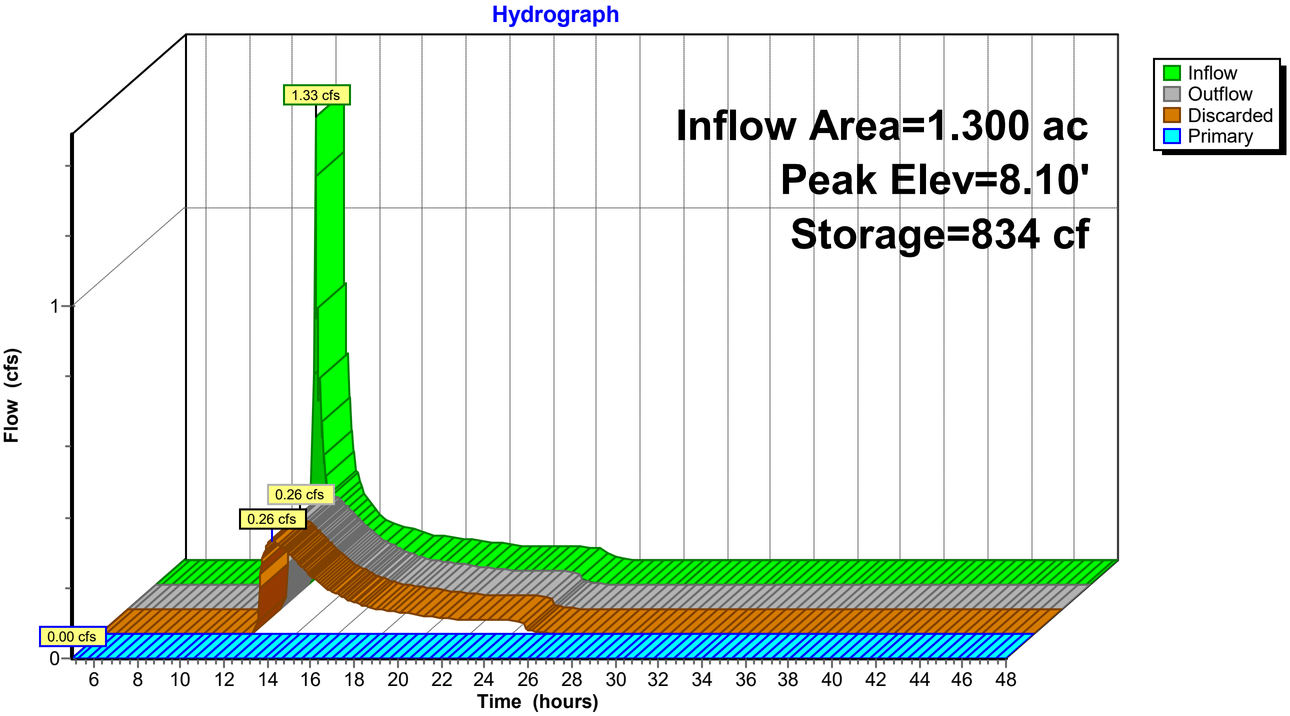
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
5.80	110	56.0	0	0	110
9.70	1,415	150.0	2,495	2,495	1,702

Device	Routing	Invert	Outlet Devices
#1	Primary	8.50'	**Channel/Reach** using Reach 6R: Overflow
#2	Discarded	5.80'	**12.000 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 3.00'

Discarded OutFlow Max=0.26 cfs @ 12.92 hrs HW=8.10' (Free Discharge)
↑—**2=Exfiltration** (Controls 0.26 cfs)

Primary OutFlow Max=0.00 cfs @ 5.00 hrs HW=5.80' (Free Discharge)
↑—**1=Channel/Reach** (Controls 0.00 cfs)

Pond 4P: Infiltration Basin #2



Summary for Pond 5P: Sedimentation Basin #1

```
Inflow Area =        1.200 ac, 37.50% Impervious, Inflow Depth =  0.63"   for 10-Year event
Inflow      =      1.03 cfs @  12.06 hrs, Volume=          0.063 af
Outflow     =      0.26 cfs @  12.31 hrs, Volume=          0.046 af, Atten= 74%, Lag= 15.4 min
Primary     =      0.26 cfs @  12.31 hrs, Volume=          0.046 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 12.06' @ 12.32 hrs   Surf.Area= 566 sf   Storage= 783 cf

Plug-Flow detention time= 185.4 min calculated for 0.046 af (73% of inflow)
Center-of-Mass det. time= 71.3 min ( 976.1 - 904.8 )
```

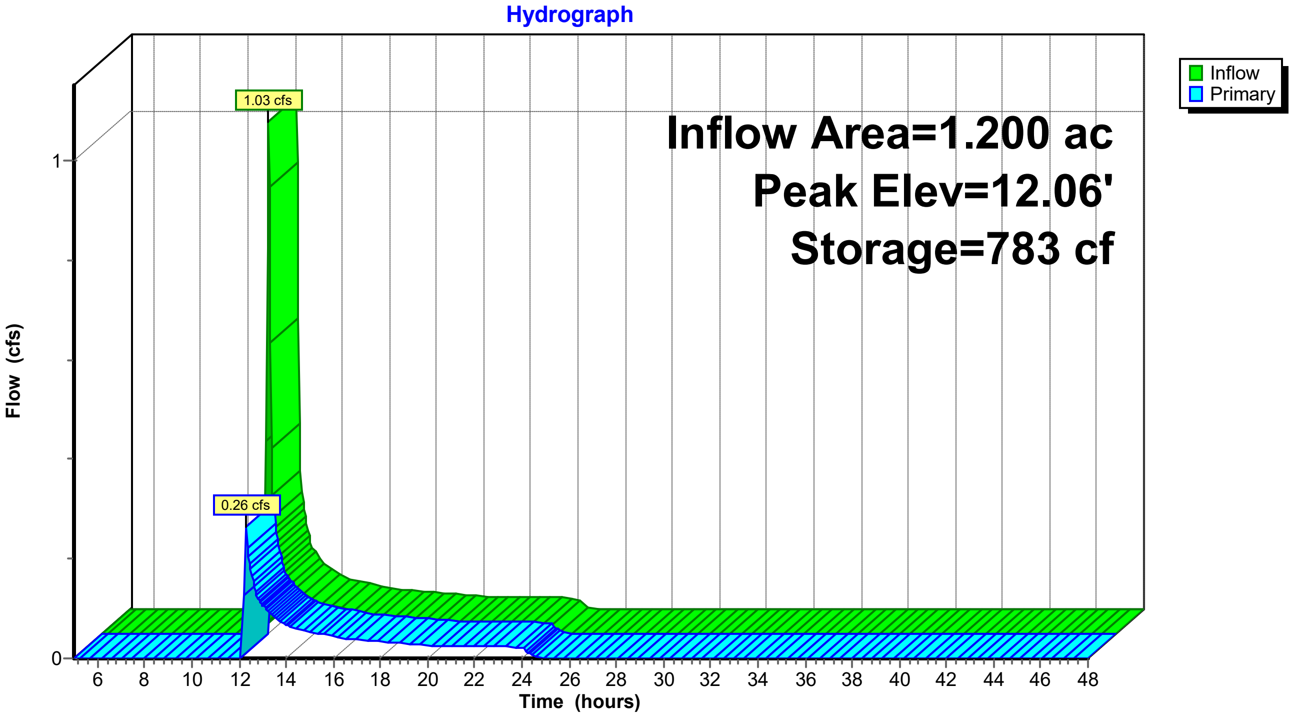
Volume	Invert	Avail.Storage	Storage Description
#1	9.00'	1,058 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
9.00	41	25.8	0	0	41
10.00	150	46.7	90	90	167
11.00	320	66.6	230	320	355
12.00	550	86.4	430	749	608
12.50	687	96.2	309	1,058	758

Device	Routing	Invert	Outlet Devices
#1	Primary	12.00'	**Channel/Reach** using Reach 12R: Sediment Basin Overflow

Primary OutFlow Max=0.24 cfs @ 12.31 hrs HW=12.06' (Free Discharge)
↑—**1=Channel/Reach** (Channel Controls 0.24 cfs @ 0.41 fps)

Pond 5P: Sedimentation Basin #1



Summary for Pond 16P: Sedimentation Basin #2

Inflow Area = 1.300 ac, 57.69% Impervious, Inflow Depth = 1.27" for 10-Year event
Inflow = 2.02 cfs @ 12.17 hrs, Volume= 0.137 af
Outflow = 1.45 cfs @ 12.31 hrs, Volume= 0.098 af, Atten= 28%, Lag= 8.9 min
Primary = 1.45 cfs @ 12.31 hrs, Volume= 0.098 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 9.14' @ 12.32 hrs Surf.Area= 928 sf Storage= 1,826 cf

Plug-Flow detention time= 174.4 min calculated for 0.098 af (72% of inflow)
Center-of-Mass det. time= 62.3 min (938.0 - 875.7)

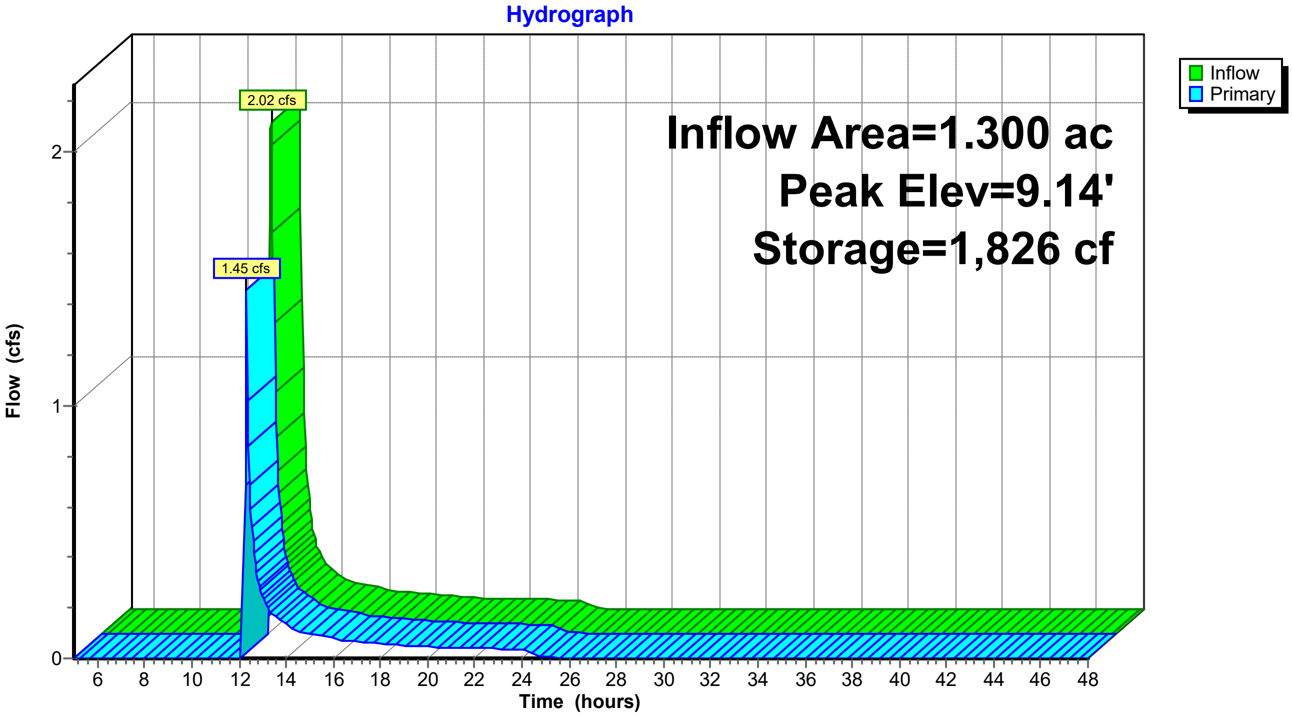
Volume	Invert	Avail.Storage	Storage Description
#1	5.80'	2,389 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
5.80	224	80.0	0	0	224
6.00	257	83.0	48	48	266
7.00	438	98.0	344	392	500
8.00	650	113.0	541	932	773
9.00	892	128.0	768	1,700	1,085
9.70	1,079	139.0	689	2,389	1,337

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	**Channel/Reach** using Reach 17R: Sediment Basin Overflow

Primary OutFlow Max=1.25 cfs @ 12.31 hrs HW=9.12' (Free Discharge)
↑—**1=Channel/Reach** (Channel Controls 1.25 cfs @ 0.67 fps)

Pond 16P: Sedimentation Basin #2

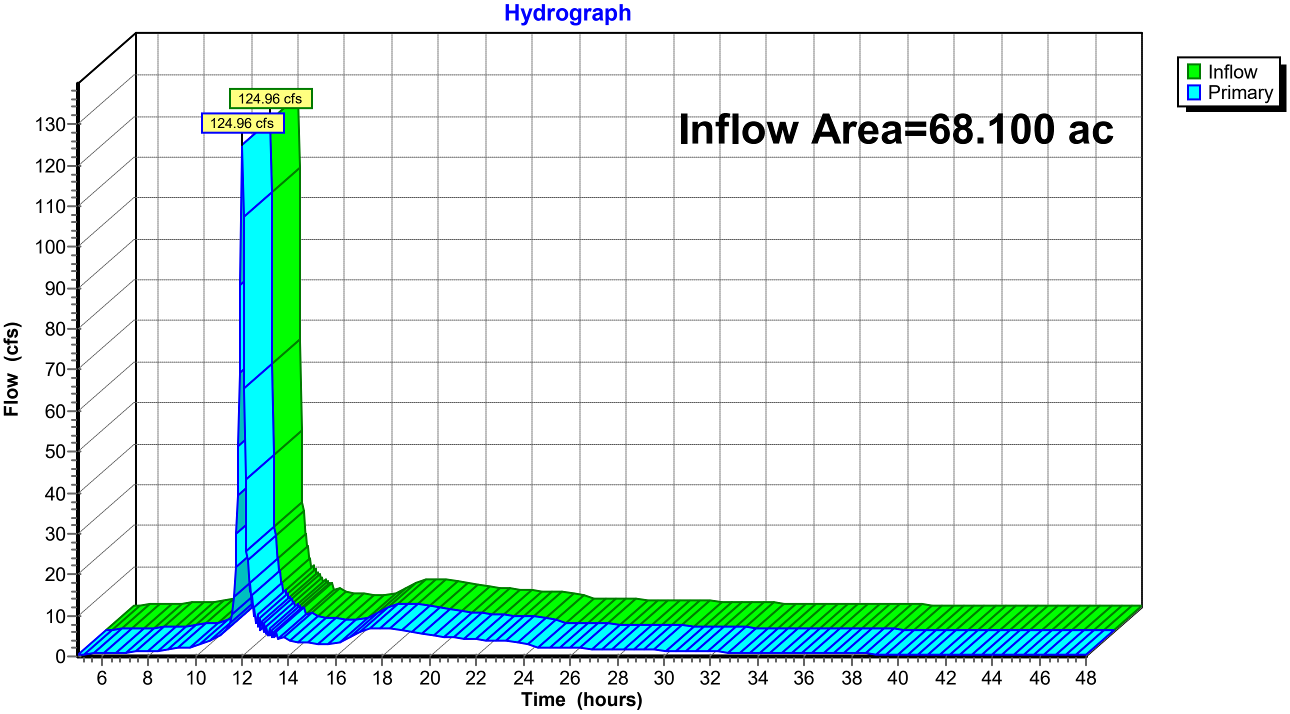


Summary for Pond AP-1: Analysis Point #1

Inflow Area = 68.100 ac, 20.36% Impervious, Inflow Depth > 1.94" for 10-Year event
Inflow = 124.96 cfs @ 12.01 hrs, Volume= 10.994 af
Primary = 124.96 cfs @ 12.01 hrs, Volume= 10.994 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

Pond AP-1: Analysis Point #1

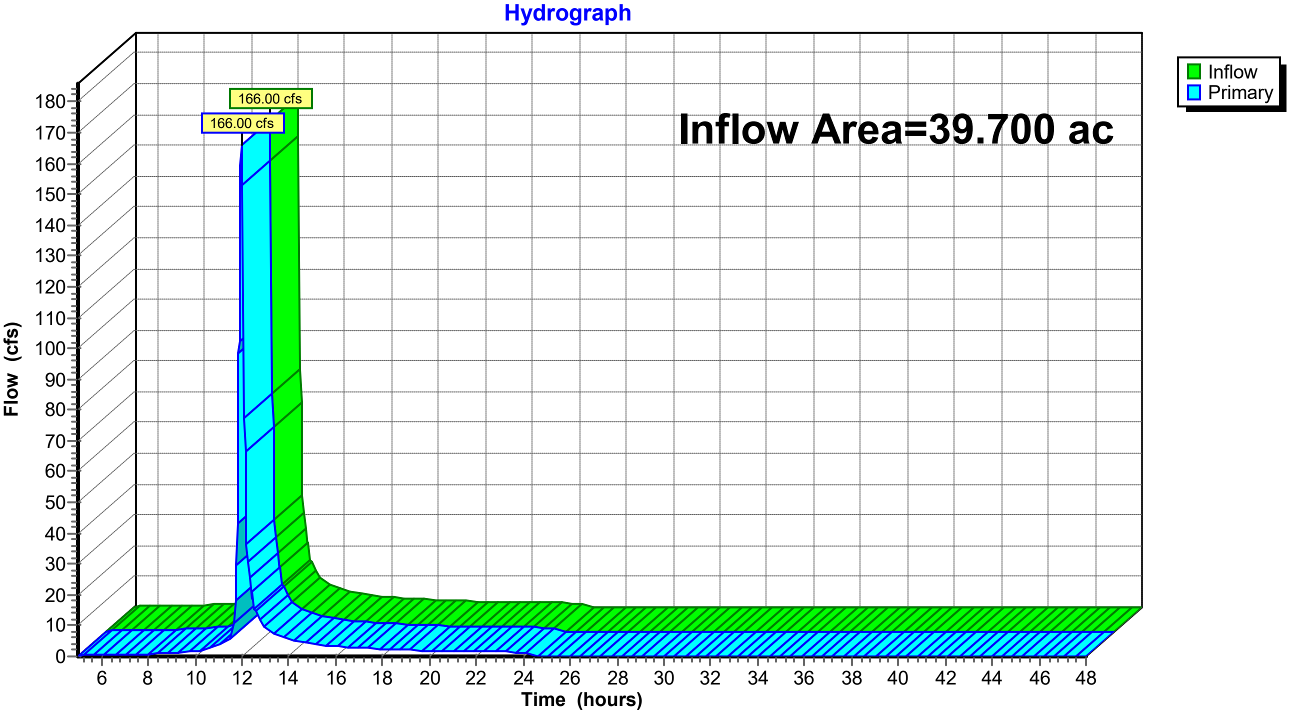


Summary for Pond AP-2: Analysis Point #2

Inflow Area =	39.700 ac,	23.10% Impervious,	Inflow Depth > 2.53"	for 10-Year event	
Inflow =	166.00 cfs @	11.97 hrs,	Volume=	8.362 af	
Primary =	166.00 cfs @	11.97 hrs,	Volume=	8.362 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

Pond AP-2: Analysis Point #2

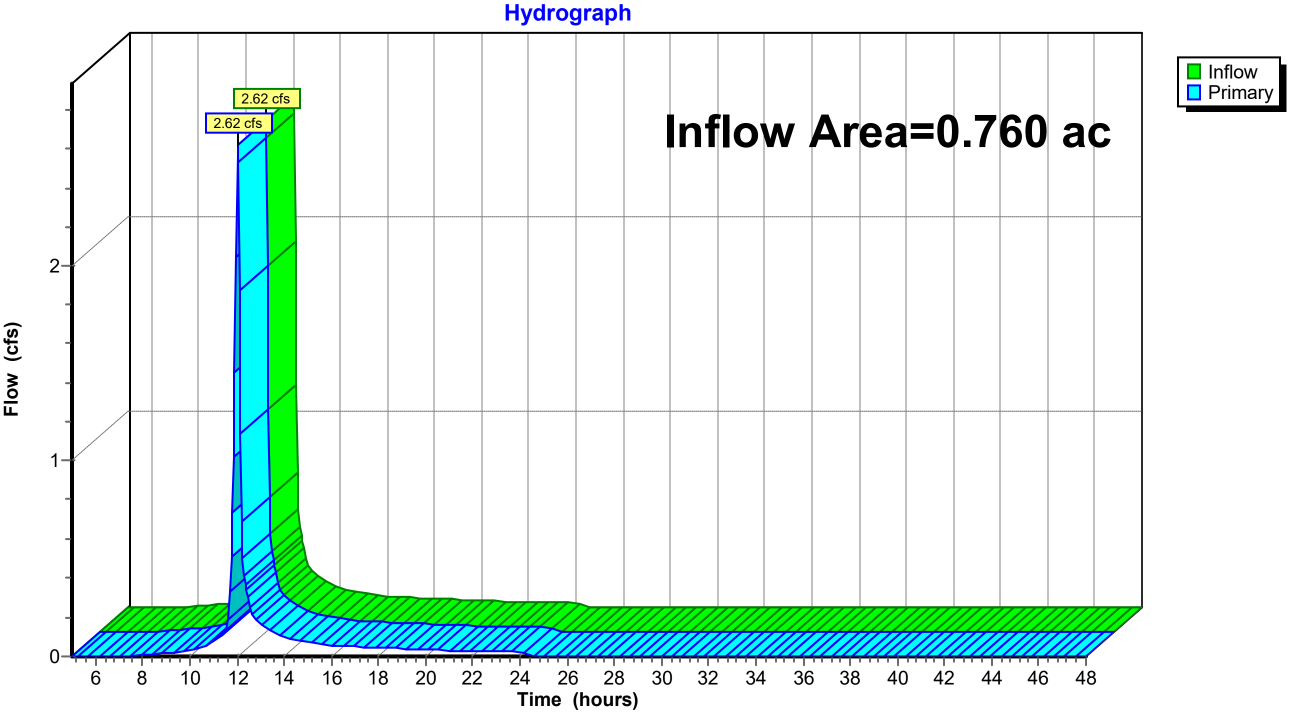


Summary for Pond AP-3: Analysis Point #3

Inflow Area = 0.760 ac, 78.95% Impervious, Inflow Depth = 2.21" for 10-Year event
Inflow = 2.62 cfs @ 12.02 hrs, Volume= 0.140 af
Primary = 2.62 cfs @ 12.02 hrs, Volume= 0.140 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

Pond AP-3: Analysis Point #3



Summary for Pond C-1: Chamber Series 1

```
Inflow Area =      10.300 ac, 66.70% Impervious, Inflow Depth >  3.23"   for 10-Year event
Inflow     =      49.84 cfs @  11.98 hrs, Volume=          2.772 af
Outflow    =      50.37 cfs @  11.97 hrs, Volume=          2.412 af, Atten= 0%, Lag= 0.0 min
Discarded  =       0.13 cfs @  11.97 hrs, Volume=          0.347 af
Primary    =      50.24 cfs @  11.97 hrs, Volume=          2.065 af
Secondary =        0.00 cfs @   5.00 hrs, Volume=          0.000 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 13.64' @ 11.97 hrs Surf.Area= 12,440 sf Storage= 29,816 cf

Plug-Flow detention time= 222.6 min calculated for 2.411 af (87% of inflow)
Center-of-Mass det. time= 160.8 min (929.8 - 769.0)

Volume	Invert	Avail.Storage	Storage Description
#1B	6.00'	11,722 cf	**37.08'W x 227.97'L x 5.50'H Field B** 46,496 cf Overall - 17,192 cf Embedded = 29,305 cf x 40.0% Voids
#2B	6.75'	17,192 cf	**ADS_StormTech MC-3500 d +Cap** x 155 Inside #1 Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap 155 Chambers in 5 Rows Cap Storage= +14.9 cf x 2 x 5 rows = 149.0 cf
#3	13.25'	6,100 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)
		35,013 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
13.25	600	0	0
13.75	4,900	1,375	1,375
14.00	8,200	1,638	3,013
14.25	16,500	3,088	6,100

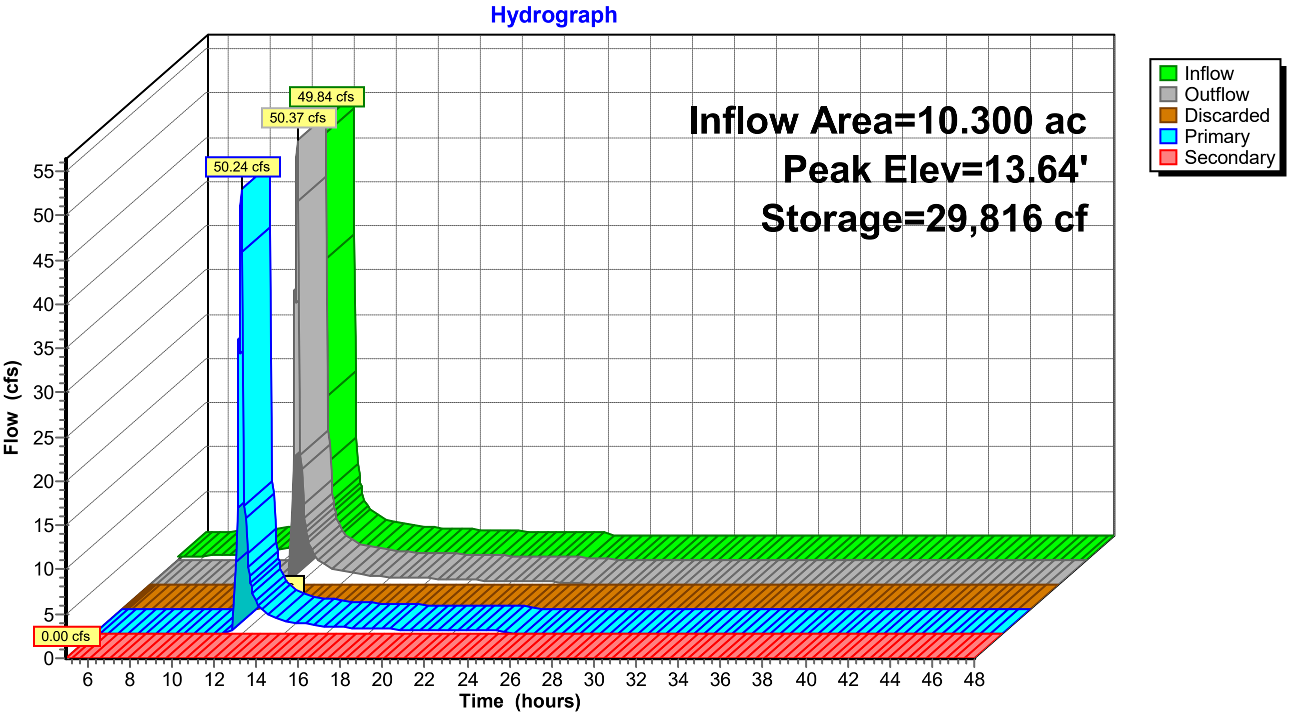
Device	Routing	Invert	Outlet Devices
#1	Discarded	6.00'	**0.500 in/hr Exfiltration over Surface area**
#2	Primary	10.00'	**36.0" Round Culvert** L= 55.9' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.00' / 7.59' S= 0.0431 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf
#3	Secondary	14.24'	**50.0' long x 0.7' breadth Concrete Curb** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 Coef. (English) 2.76 2.82 2.93 3.09 3.18 3.22 3.27 3.30 3.32 3.31 3.32

Discarded OutFlow Max=0.13 cfs @ 11.97 hrs HW=13.50' (Free Discharge)
↑—**1=Exfiltration** (Exfiltration Controls 0.13 cfs)

Primary OutFlow Max=48.14 cfs @ 11.97 hrs HW=13.50' (Free Discharge)
↑—**2=Culvert** (Inlet Controls 48.14 cfs @ 6.81 fps)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=6.01' (Free Discharge)
↑—**3=Concrete Curb** (Controls 0.00 cfs)

Pond C-1: Chamber Series 1



Summary for Pond C-4: Chamber Series 4

```
Inflow Area =        8.900 ac,   0.00% Impervious,  Inflow Depth >   2.95"    for  10-Year event
Inflow      =       39.82 cfs @  11.99 hrs,  Volume=           2.188 af
Outflow     =       44.40 cfs @  11.96 hrs,  Volume=           1.858 af,  Atten= 0%,  Lag= 0.0 min
Discarded  =         0.08 cfs @  11.95 hrs,  Volume=           0.261 af
Primary     =       44.32 cfs @  11.96 hrs,  Volume=           1.597 af
Secondary =          0.00 cfs @    5.00 hrs,  Volume=           0.000 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 14.18' @ 11.96 hrs Surf.Area= 7,104 sf Storage= 21,828 cf

Plug-Flow detention time= 221.0 min calculated for 1.858 af (85% of inflow)
Center-of-Mass det. time= 153.0 min (936.6 - 783.6)

Volume	Invert	Avail.Storage	Storage Description
#1B	6.00'	8,911 cf	**29.92'W x 213.63'L x 5.50'H Field B** 35,151 cf Overall - 12,874 cf Embedded = 22,277 cf x 40.0% Voids
#2B	6.75'	12,874 cf	**ADS_StormTech MC-3500 d +Cap** x 116 Inside #1 Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap 116 Chambers in 4 Rows Cap Storage= +14.9 cf x 2 x 4 rows = 119.2 cf
#3	14.10'	8,015 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)
		29,800 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
14.10	400	0	0
14.60	2,400	700	700
14.80	6,300	870	1,570
15.10	10,000	2,445	4,015
15.50	10,000	4,000	8,015

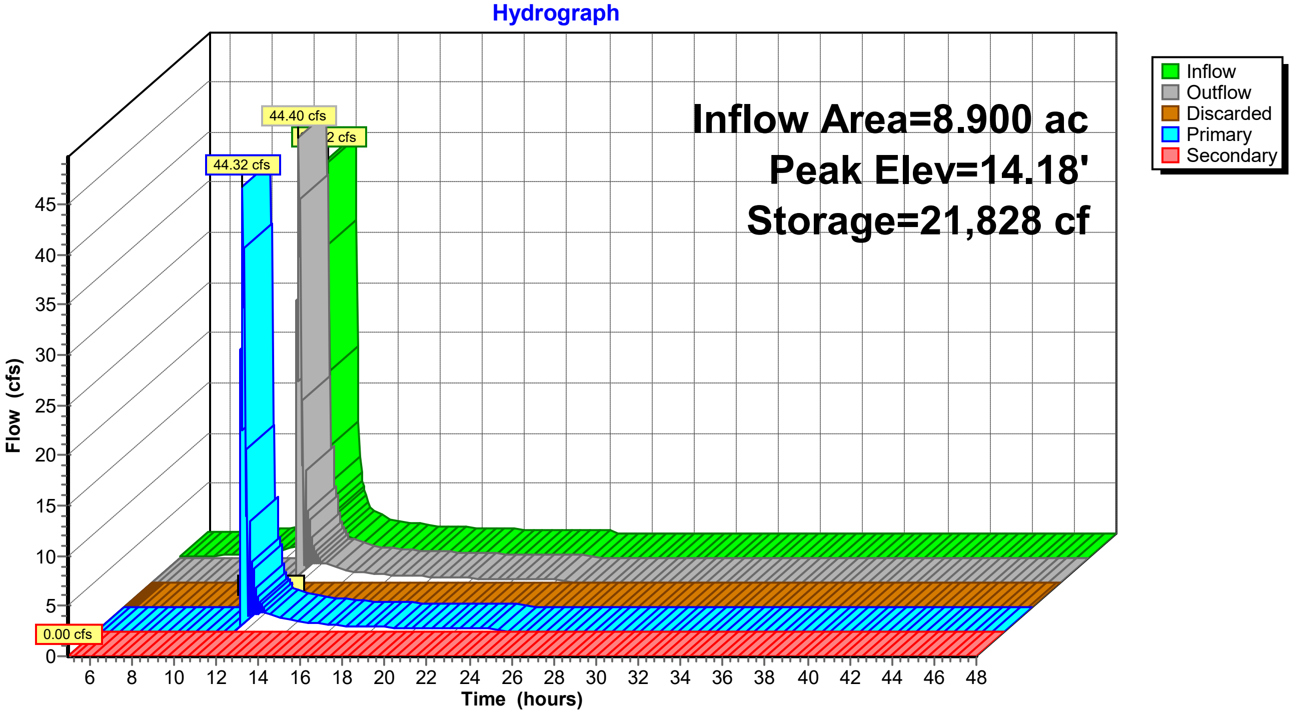
Device	Routing	Invert	Outlet Devices
#1	Discarded	6.00'	**0.500 in/hr Exfiltration over Surface area**
#2	Primary	11.00'	**36.0" Round Culvert** L= 24.3' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 11.00' / 9.42' S= 0.0650 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf
#3	Secondary	15.50'	**100.0' long x 0.5' breadth Wharf** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.08 cfs @ 11.95 hrs HW=14.15' (Free Discharge)
↑—**1=Exfiltration** (Exfiltration Controls 0.08 cfs)

Primary OutFlow Max=42.04 cfs @ 11.96 hrs HW=14.03' (Free Discharge)
↑—**2=Culvert** (Inlet Controls 42.04 cfs @ 5.95 fps)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=6.00' (Free Discharge)
↑—**3=Wharf** (Controls 0.00 cfs)

Pond C-4: Chamber Series 4



Summary for Pond C-5: Chamber Series 5

Inflow Area = 5.200 ac, 0.00% Impervious, Inflow Depth > 2.95" for 10-Year event
Inflow = 23.53 cfs @ 11.98 hrs, Volume= 1.279 af
Outflow = 24.69 cfs @ 12.02 hrs, Volume= 1.023 af, Atten= 0%, Lag= 2.2 min
Discarded = 0.19 cfs @ 12.00 hrs, Volume= 0.391 af
Primary = 24.51 cfs @ 12.02 hrs, Volume= 0.633 af
Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 15.19' @ 12.02 hrs Surf.Area= 16,669 sf Storage= 24,121 cf

Plug-Flow detention time= 425.0 min calculated for 1.022 af (80% of inflow)
Center-of-Mass det. time= 347.1 min (1,130.4 - 783.3)

Volume	Invert	Avail.Storage	Storage Description
#1B	6.00'	7,757 cf	**28.50'W x 168.47'L x 6.75'H Field B** 32,409 cf Overall - 13,016 cf Embedded = 19,393 cf x 40.0% Voids
#2B	6.75'	13,016 cf	**ADS_StormTech MC-4500 b +Cap** x 120 Inside #1 Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 120 Chambers in 3 Rows Cap Storage= +39.5 cf x 2 x 3 rows = 237.0 cf
#3	14.60'	7,420 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)
		28,193 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
14.60	200	0	0
14.80	1,000	120	120
15.00	10,000	1,100	1,220
15.50	14,800	6,200	7,420

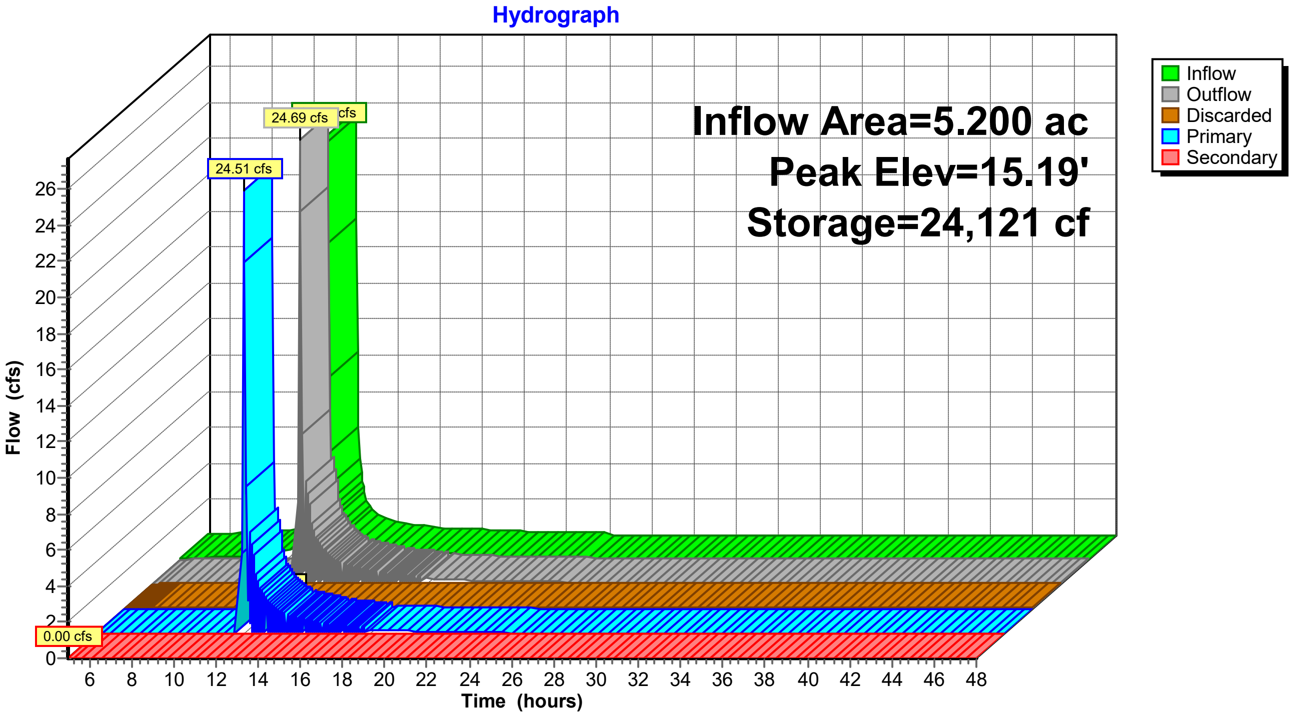
Device	Routing	Invert	Outlet Devices
#1	Discarded	6.00'	**1.000 in/hr Exfiltration over Surface area**
#2	Primary	12.60'	**36.0" Round Culvert** L= 62.8' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 12.60' / 12.29' S= 0.0049 '/' Cc= 0.900 n= 0.013 Cast iron, coated, Flow Area= 7.07 sf
#3	Secondary	15.50'	**100.0' long x 0.5' breadth Wharf** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.19 cfs @ 12.00 hrs HW=14.85' (Free Discharge)
↰**1=Exfiltration** (Exfiltration Controls 0.19 cfs)

Primary OutFlow Max=21.56 cfs @ 12.02 hrs HW=14.81' (Free Discharge)
↰**2=Culvert** (Barrel Controls 21.56 cfs @ 5.38 fps)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=6.00' (Free Discharge)
↰**3=Wharf** (Controls 0.00 cfs)

Pond C-5: Chamber Series 5



Summary for Subcatchment DR-1: Building A & Storage

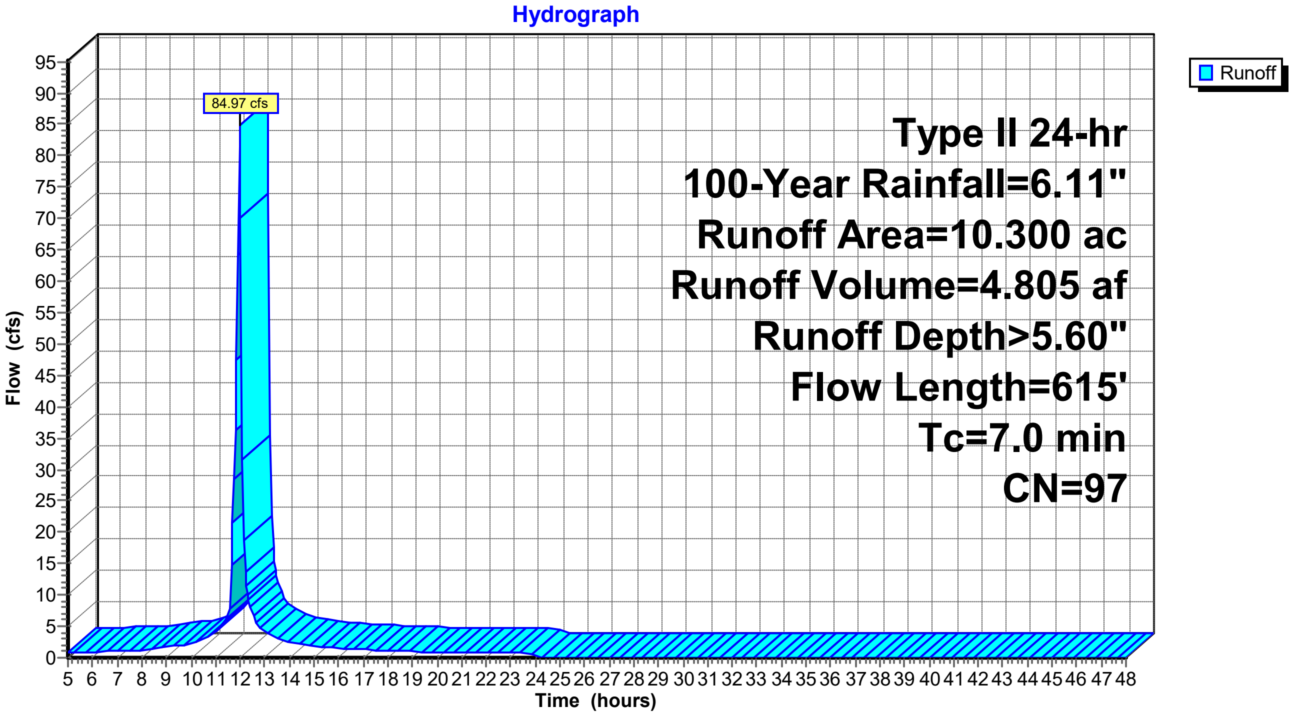
Runoff = 84.97 cfs @ 11.98 hrs, Volume= 4.805 af, Depth> 5.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	6.870	98	Building A
	0.100	80	>75% Grass cover, Good, HSG D
*	3.330	95	Dense Graded Aggregate
	10.300	97	Weighted Average
	3.430		33.30% Pervious Area
	6.870		66.70% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
3.1	300	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.6	215	0.0050	5.91	29.00	**Pipe Channel,** 30.0" Round Area= 4.9 sf Perim= 7.9' r= 0.63' n= 0.013
7.0	615	Total			

Subcatchment DR-1: Building A & Storage



Summary for Subcatchment DR-10: Undisturbed Area

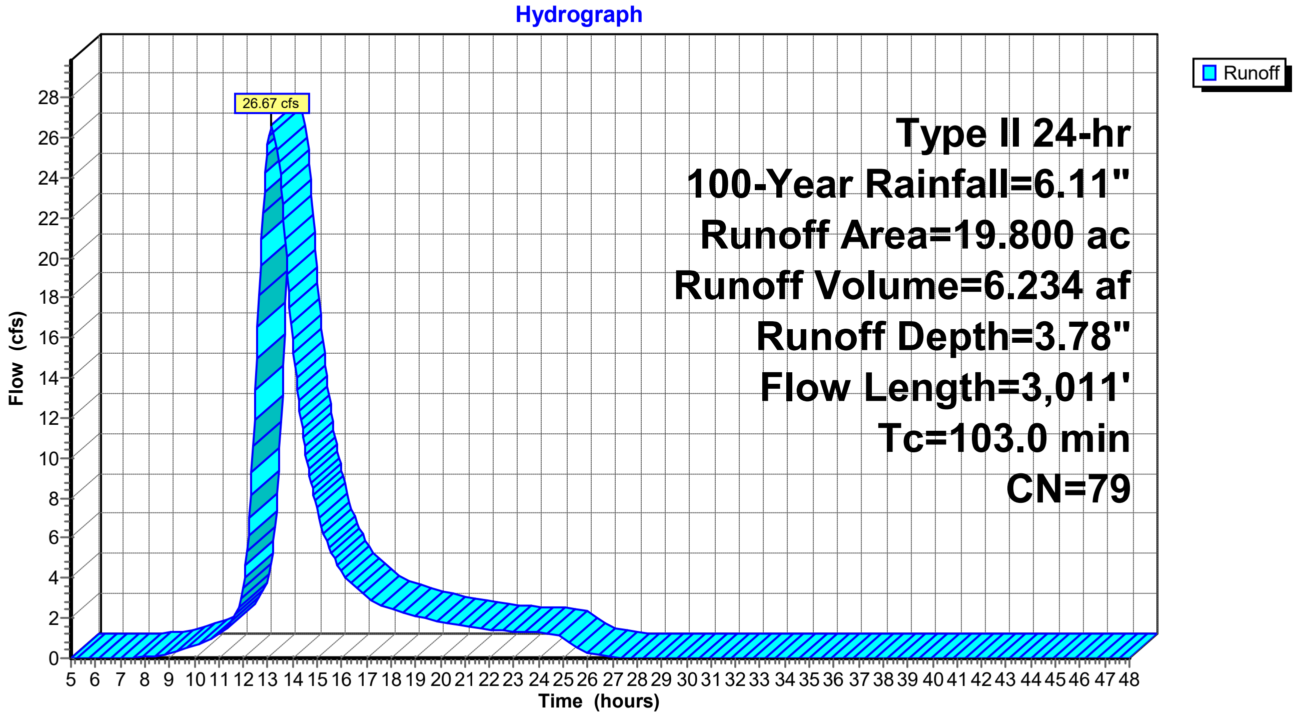
Runoff = 26.67 cfs @ 13.16 hrs, Volume= 6.234 af, Depth= 3.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

Area (ac)	CN	Description
19.800	79	Woods, Fair, HSG D
19.800		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
19.7	150	0.0800	0.13		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.40"
3.0	200	0.0500	1.12		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
1.6	250	0.2600	2.55		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
78.7	2,361	0.0100	0.50		**Shallow Concentrated Flow, Wetland Flow** Woodland Kv= 5.0 fps
0.0	50	0.0500	22.86	161.57	**Pipe Channel,** 36.0" Round Area= 7.1 sf Perim= 9.4' r= 0.75' n= 0.012 Corrugated PP, smooth interior
103.0	3,011	Total			

Subcatchment DR-10: Undisturbed Area



Summary for Subcatchment DR-11: Hudson River Bank

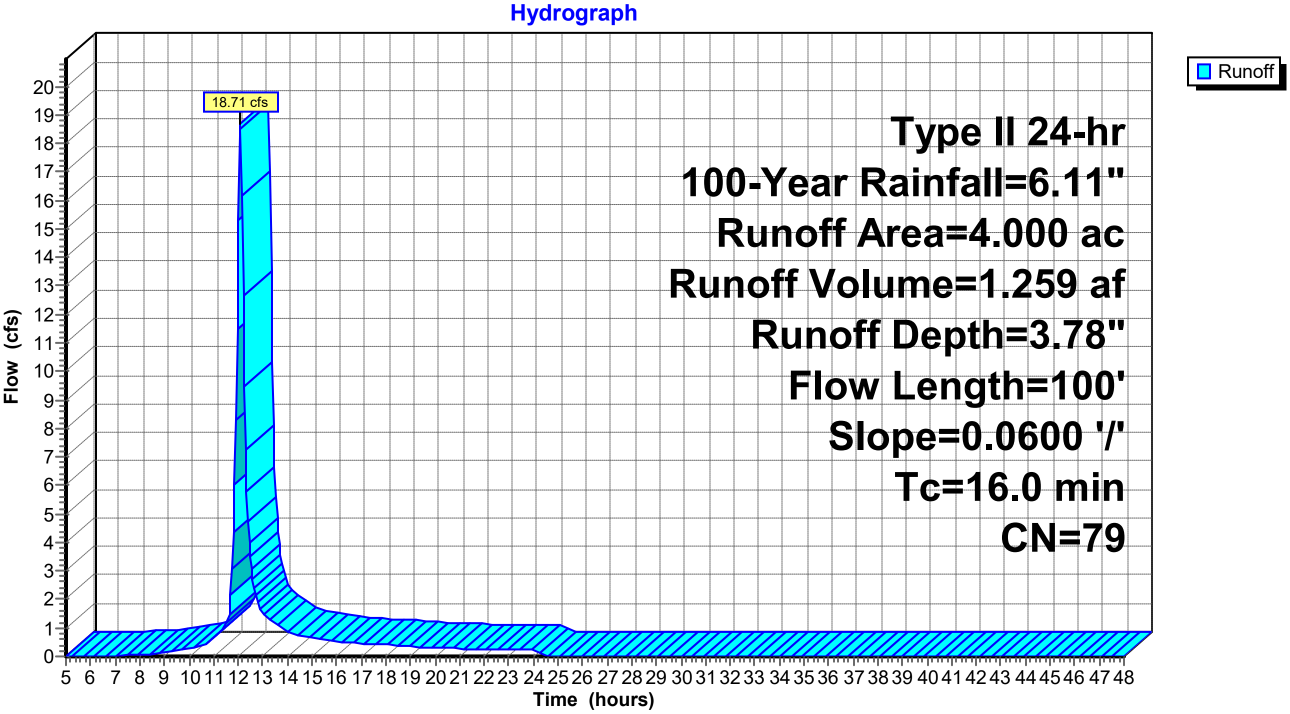
Runoff = 18.71 cfs @ 12.08 hrs, Volume= 1.259 af, Depth= 3.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

Area (ac)	CN	Description
4.000	79	Woods, Fair, HSG D
4.000		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.0	100	0.0600	0.10		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.40"

Subcatchment DR-11: Hudson River Bank



Summary for Subcatchment DR-12: Normans Kill Bank

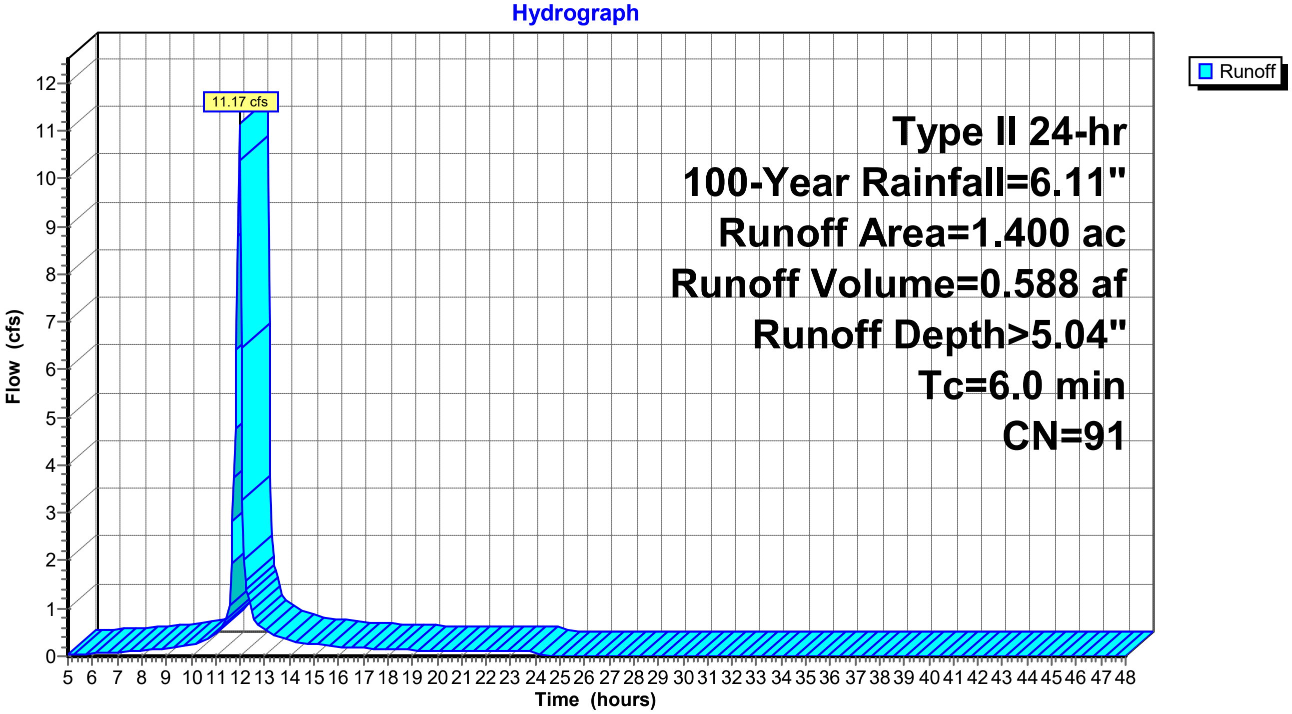
Runoff = 11.17 cfs @ 11.96 hrs, Volume= 0.588 af, Depth> 5.04"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

Area (ac)	CN	Description
0.430	79	Woods, Fair, HSG D
0.970	96	Gravel surface, HSG D
1.400	91	Weighted Average
1.400		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-12: Normans Kill Bank



Summary for Subcatchment DR-13: Roadway

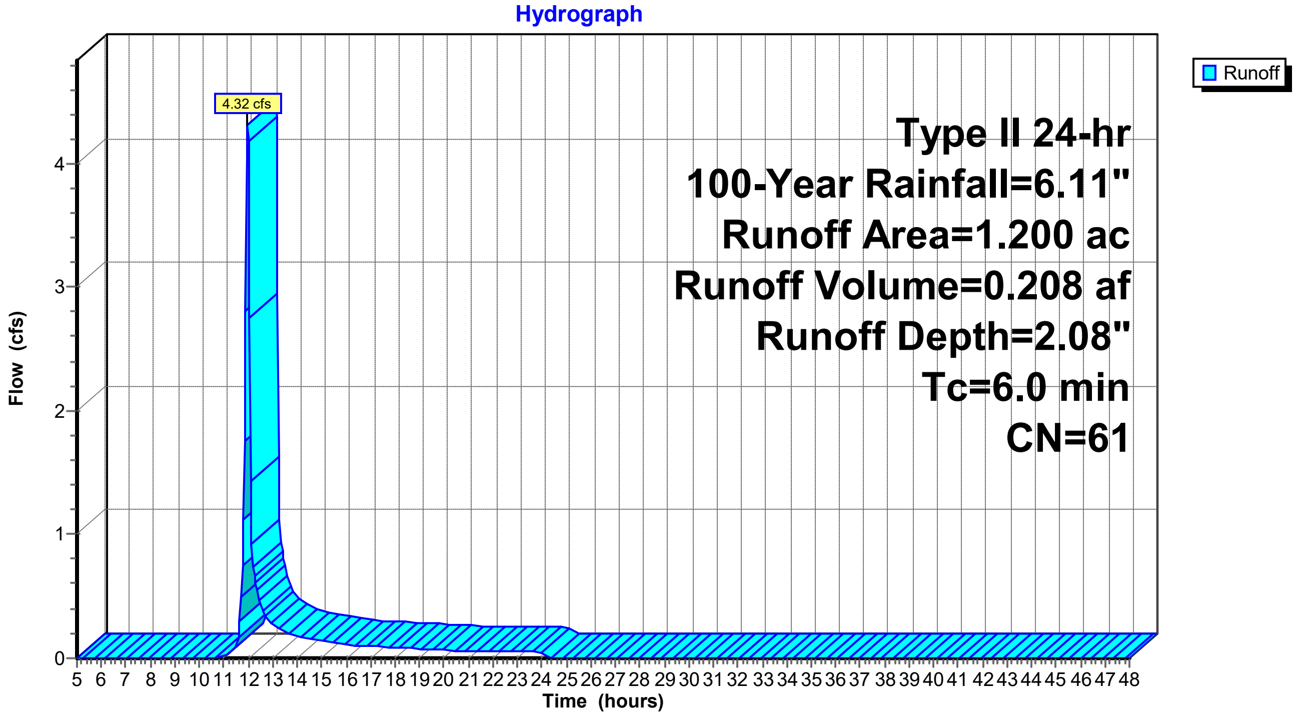
Runoff = 4.32 cfs @ 11.98 hrs, Volume= 0.208 af, Depth= 2.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	0.450	98	Pavement
	0.750	39	>75% Grass cover, Good, HSG A
	1.200	61	Weighted Average
	0.750		62.50% Pervious Area
	0.450		37.50% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-13: Roadway



Summary for Subcatchment DR-14: Roadway

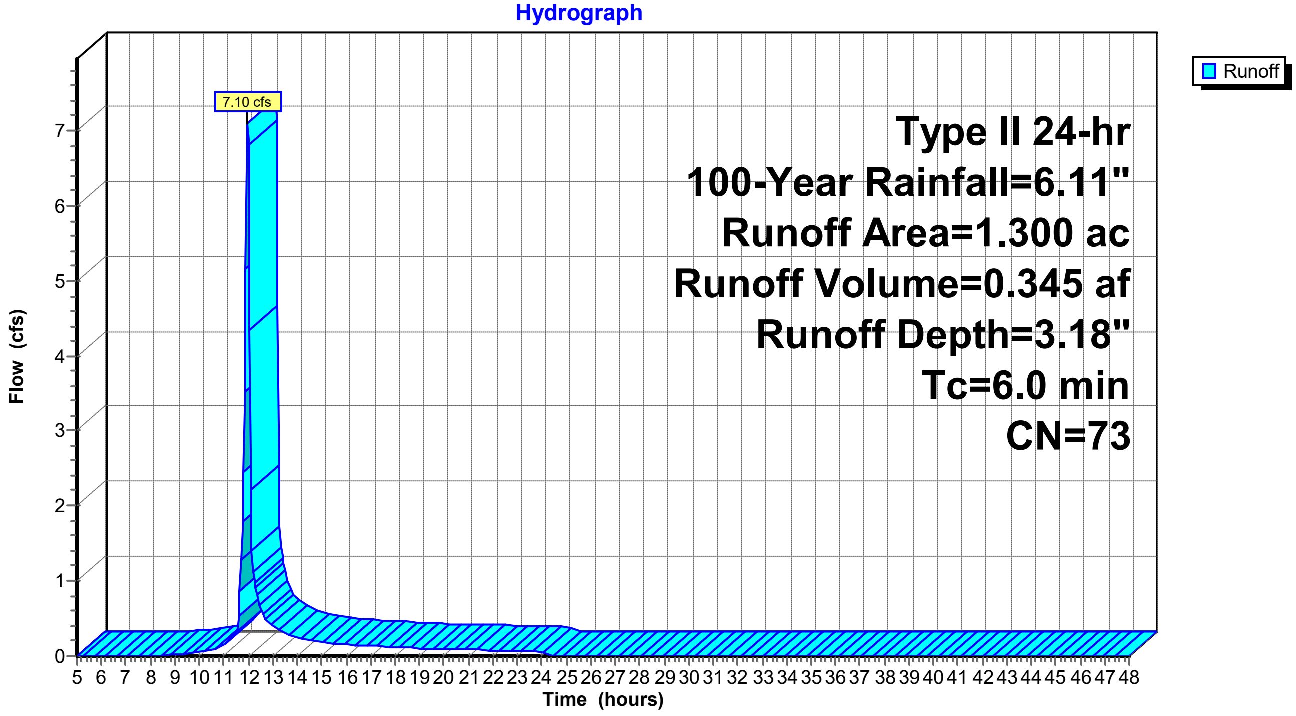
Runoff = 7.10 cfs @ 11.97 hrs, Volume= 0.345 af, Depth= 3.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	0.550	98	New Pavement
	0.550	39	>75% Grass cover, Good, HSG A
*	0.200	98	Mill & Fill of Old Pavement
	1.300	73	Weighted Average
	0.550		42.31% Pervious Area
	0.750		57.69% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-14: Roadway



Summary for Subcatchment DR-15: Roadway

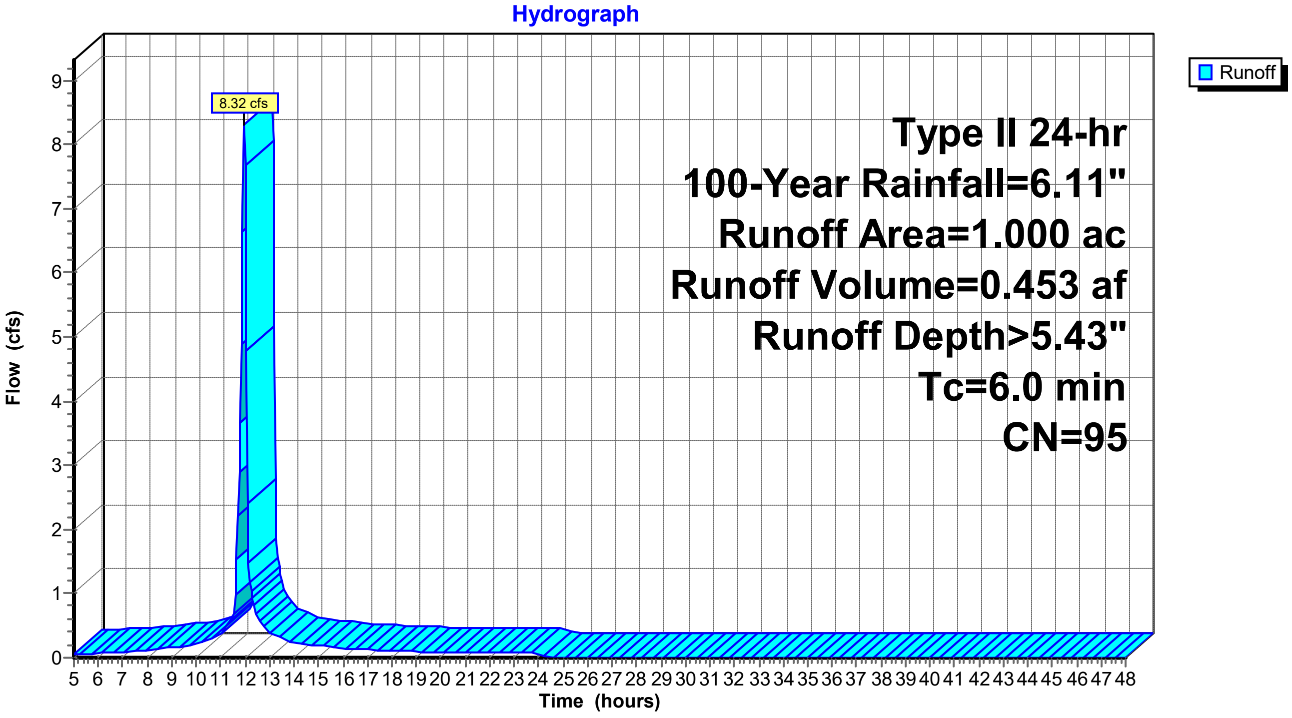
Runoff = 8.32 cfs @ 11.96 hrs, Volume= 0.453 af, Depth> 5.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	0.050	98	New Pavement
	0.050	39	>75% Grass cover, Good, HSG A
*	0.900	98	Mill & Fill of Old Pavement
	1.000	95	Weighted Average
	0.050		5.00% Pervious Area
	0.950		95.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Min**

Subcatchment DR-15: Roadway



Summary for Subcatchment DR-16: Undisturbed Area

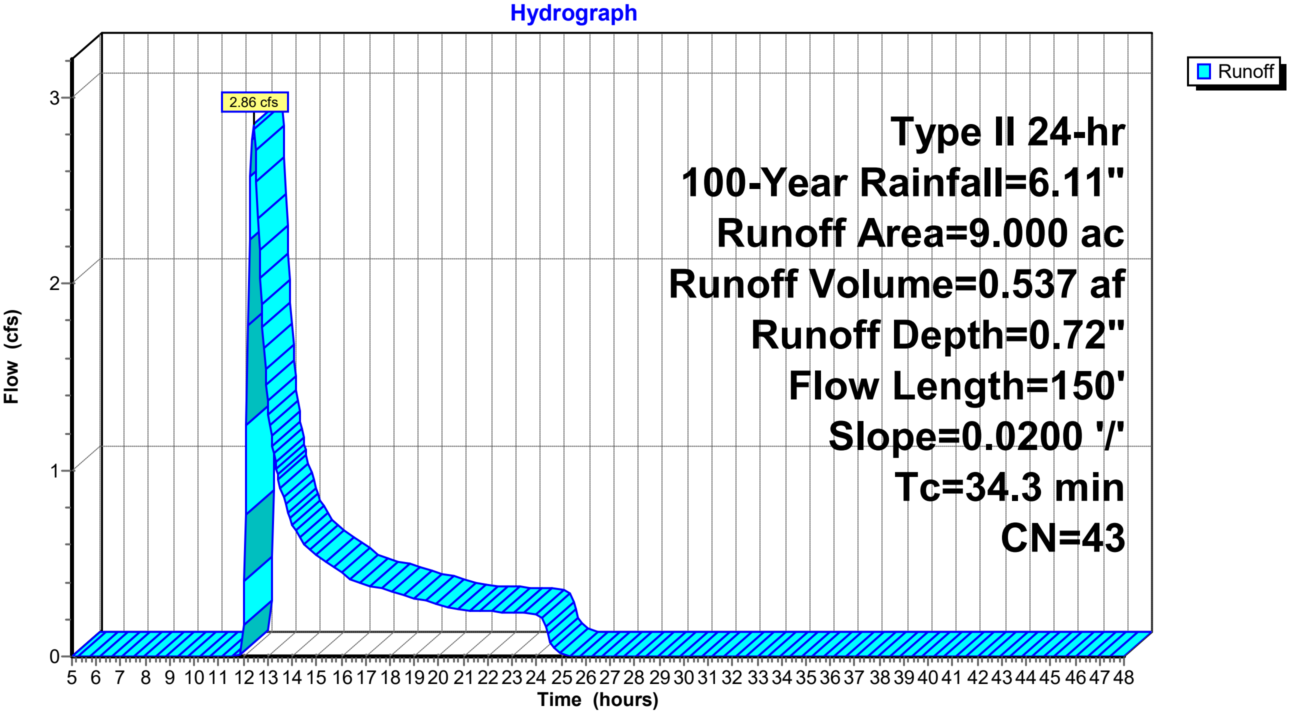
Runoff = 2.86 cfs @ 12.41 hrs, Volume= 0.537 af, Depth= 0.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

Area (ac)	CN	Description
9.000	43	Woods/grass comb., Fair, HSG A
9.000		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
34.3	150	0.0200	0.07		**Sheet Flow,** Woods: Light underbrush n= 0.400 P2= 2.40"

Subcatchment DR-16: Undisturbed Area



Summary for Subcatchment DR-17: Roadway

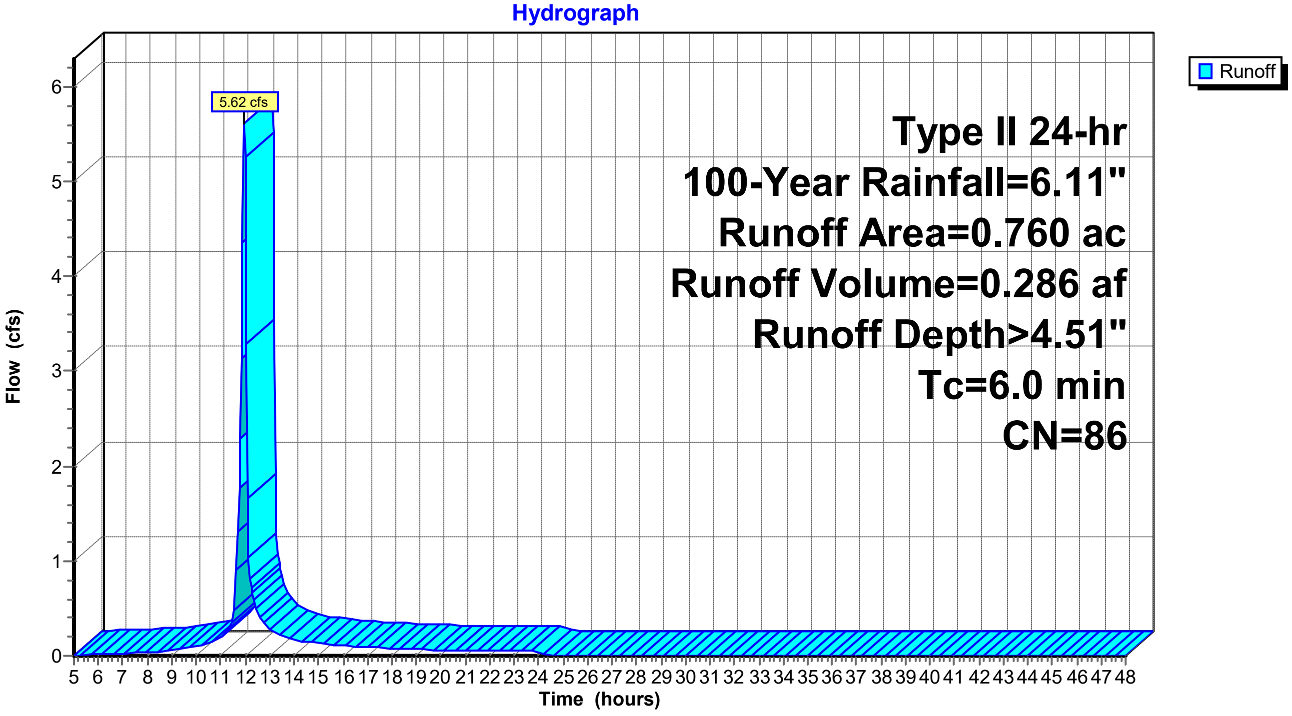
Runoff = 5.62 cfs @ 11.97 hrs, Volume= 0.286 af, Depth> 4.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	0.140	98	Road Widening
*	0.460	98	Roadway
	0.160	39	>75% Grass cover, Good, HSG A
	0.760	86	Weighted Average
	0.160		21.05% Pervious Area
	0.600		78.95% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-17: Roadway



Summary for Subcatchment DR-2: Storage

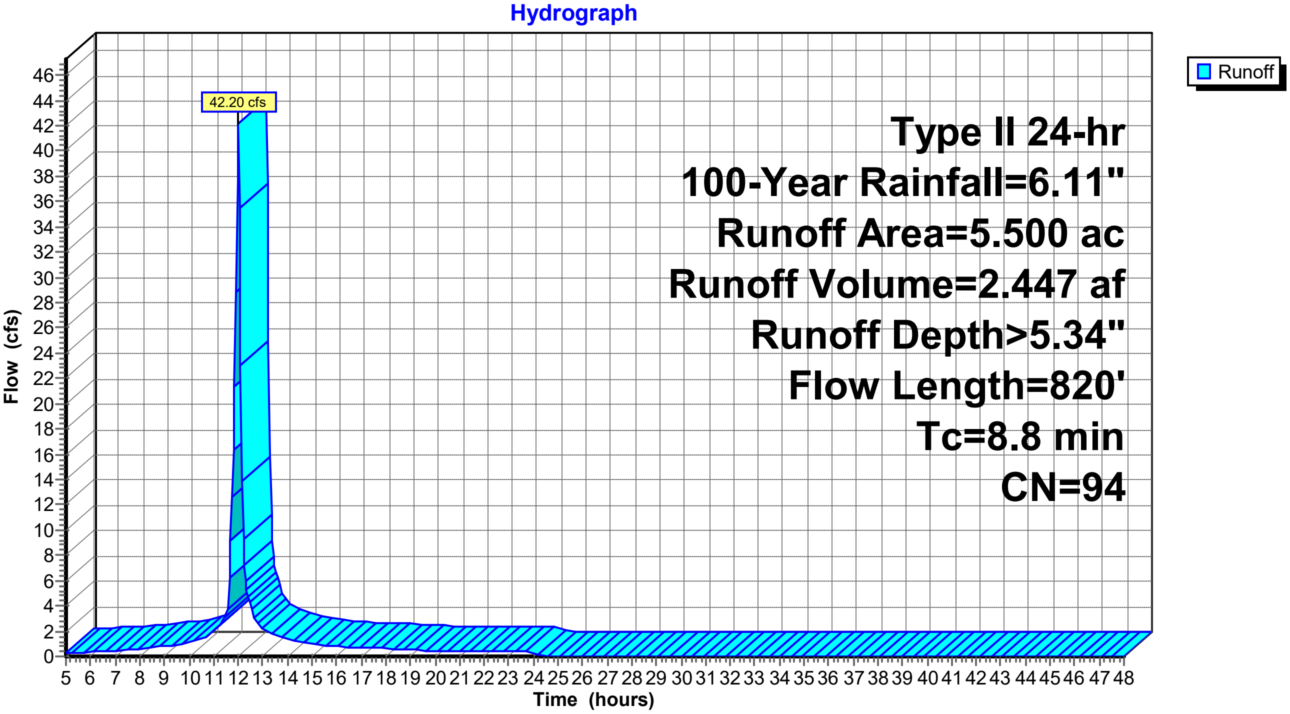
Runoff = 42.20 cfs @ 12.00 hrs, Volume= 2.447 af, Depth> 5.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	5.300	95	Dense Graded Aggregate
	0.200	80	>75% Grass cover, Good, HSG D
	5.500	94	Weighted Average
	5.500		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
4.9	470	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.6	250	0.0050	6.67	47.16	**Pipe Channel,** 36.0" Round Area= 7.1 sf Perim= 9.4' r= 0.75' n= 0.013
8.8	820	Total			

Subcatchment DR-2: Storage



Summary for Subcatchment DR-3: Rail & Storage

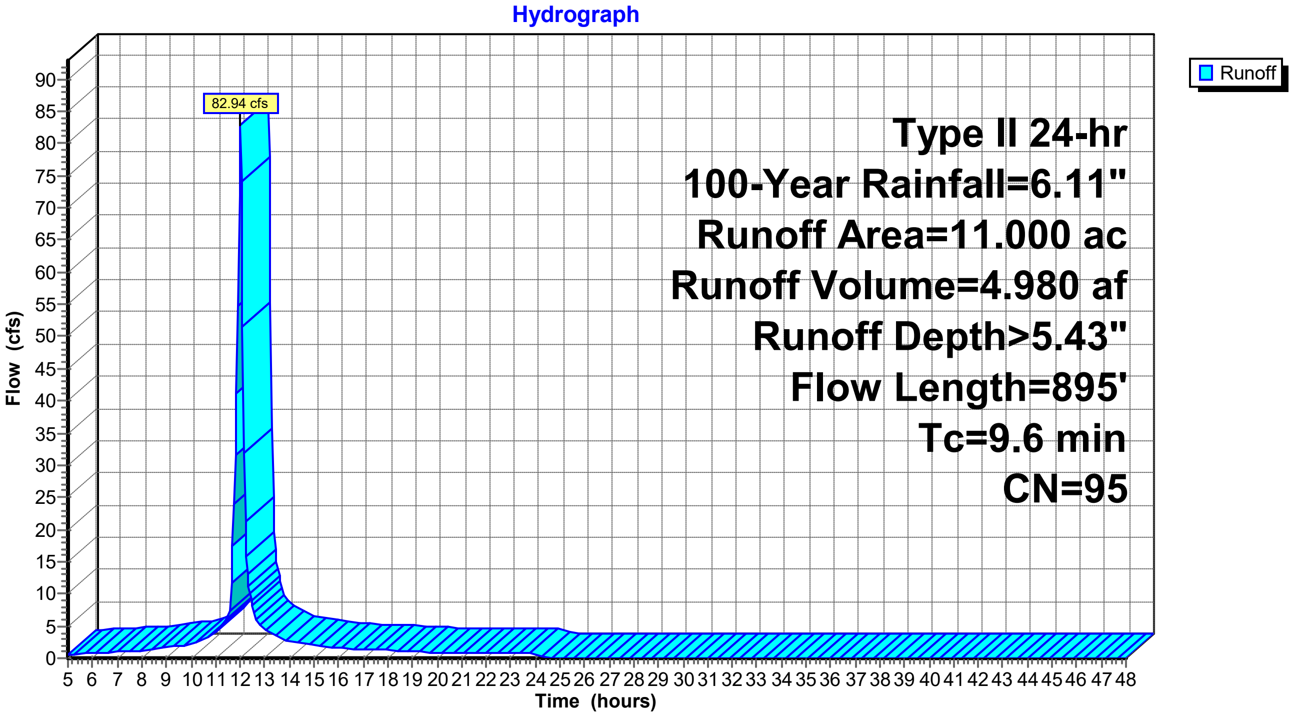
Runoff = 82.94 cfs @ 12.00 hrs, Volume= 4.980 af, Depth> 5.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	8.300	95	Compacted Gravel
	0.400	80	>75% Grass cover, Good, HSG D
*	2.300	98	Rail
	11.000	95	Weighted Average
	8.700		79.09% Pervious Area
	2.300		20.91% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
5.4	525	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.9	270	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
9.6	895	Total			

Subcatchment DR-3: Rail & Storage



Summary for Subcatchment DR-4: Storage

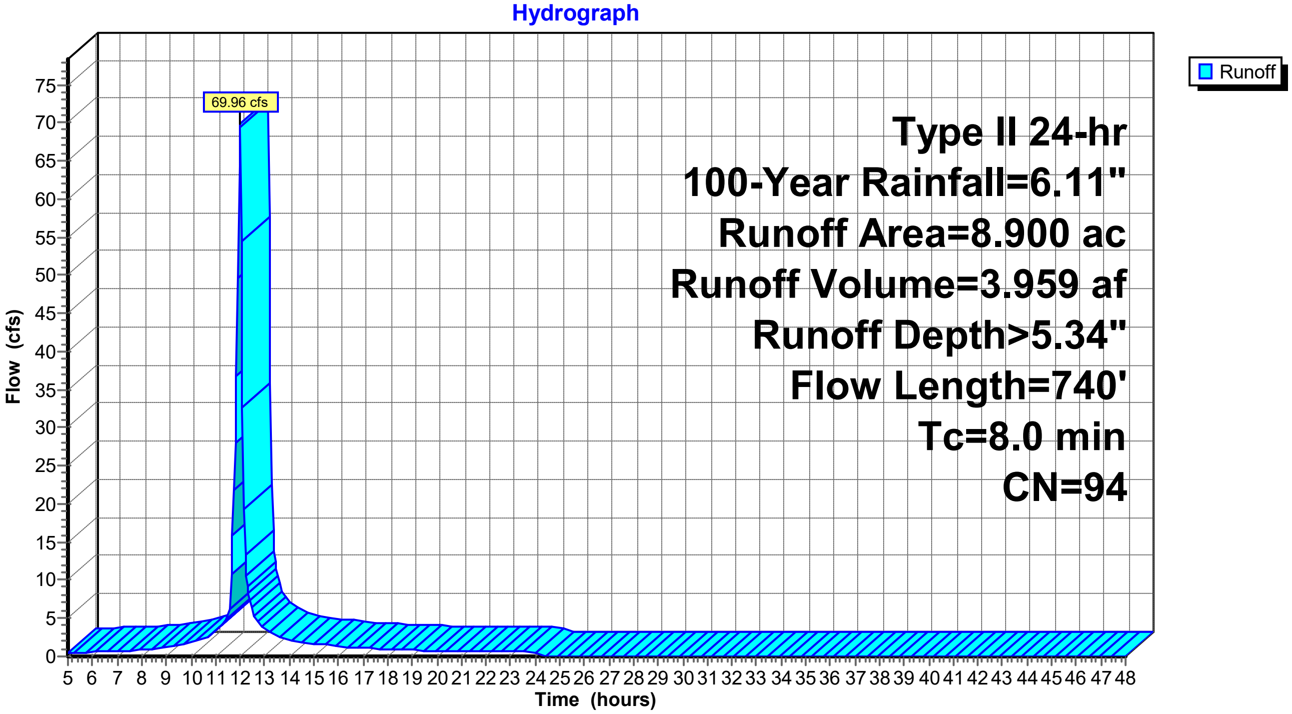
Runoff = 69.96 cfs @ 11.99 hrs, Volume= 3.959 af, Depth> 5.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	8.600	95	Compacted Gravel
	0.300	80	>75% Grass cover, Good, HSG D
	8.900	94	Weighted Average
	8.900		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
4.1	400	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
0.6	240	0.0050	6.67	47.16	**Pipe Channel,** 36.0" Round Area= 7.1 sf Perim= 9.4' r= 0.75' n= 0.013 Corrugated PE, smooth interior
8.0	740	Total			

Subcatchment DR-4: Storage



Summary for Subcatchment DR-5: Storage

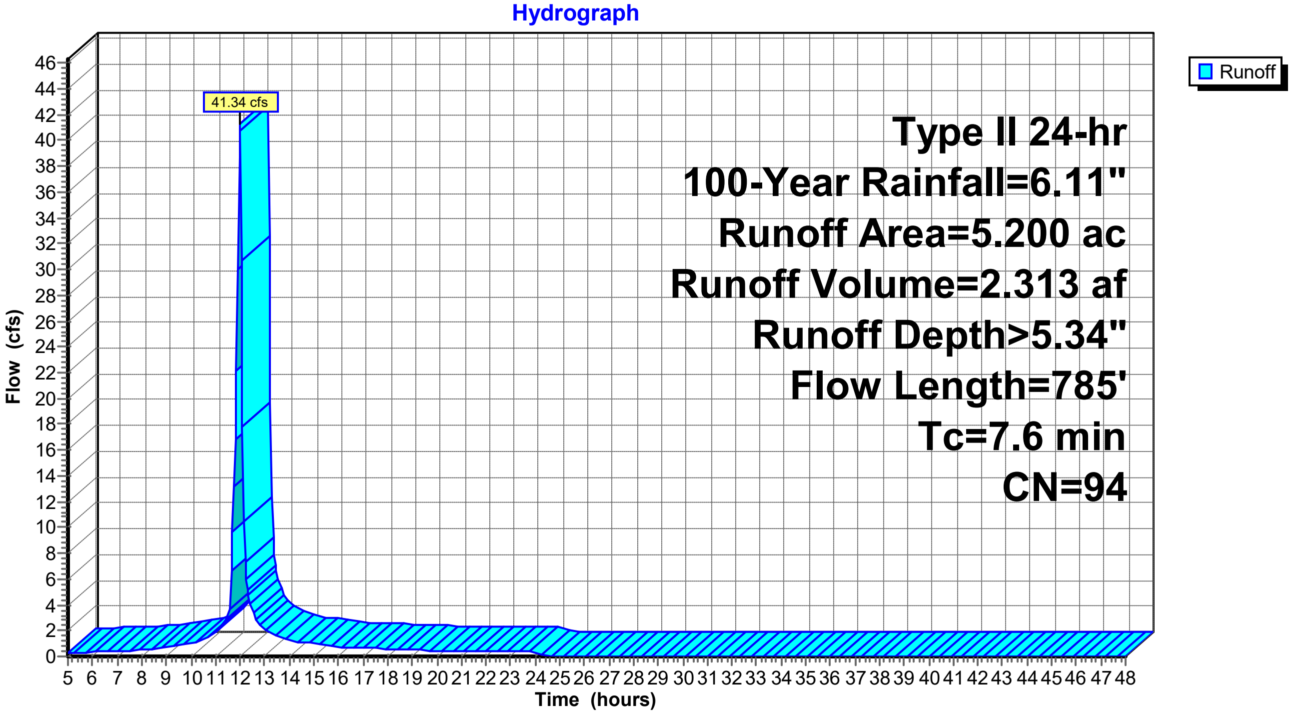
Runoff = 41.34 cfs @ 11.98 hrs, Volume= 2.313 af, Depth> 5.34"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	4.900	95	Dense Graded Aggregate
	0.300	80	>75% Grass cover, Good, HSG D
	5.200	94	Weighted Average
	5.200		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
3.0	285	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
1.3	400	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
7.6	785	Total			

Subcatchment DR-5: Storage



Summary for Subcatchment DR-6: Buldings B & D

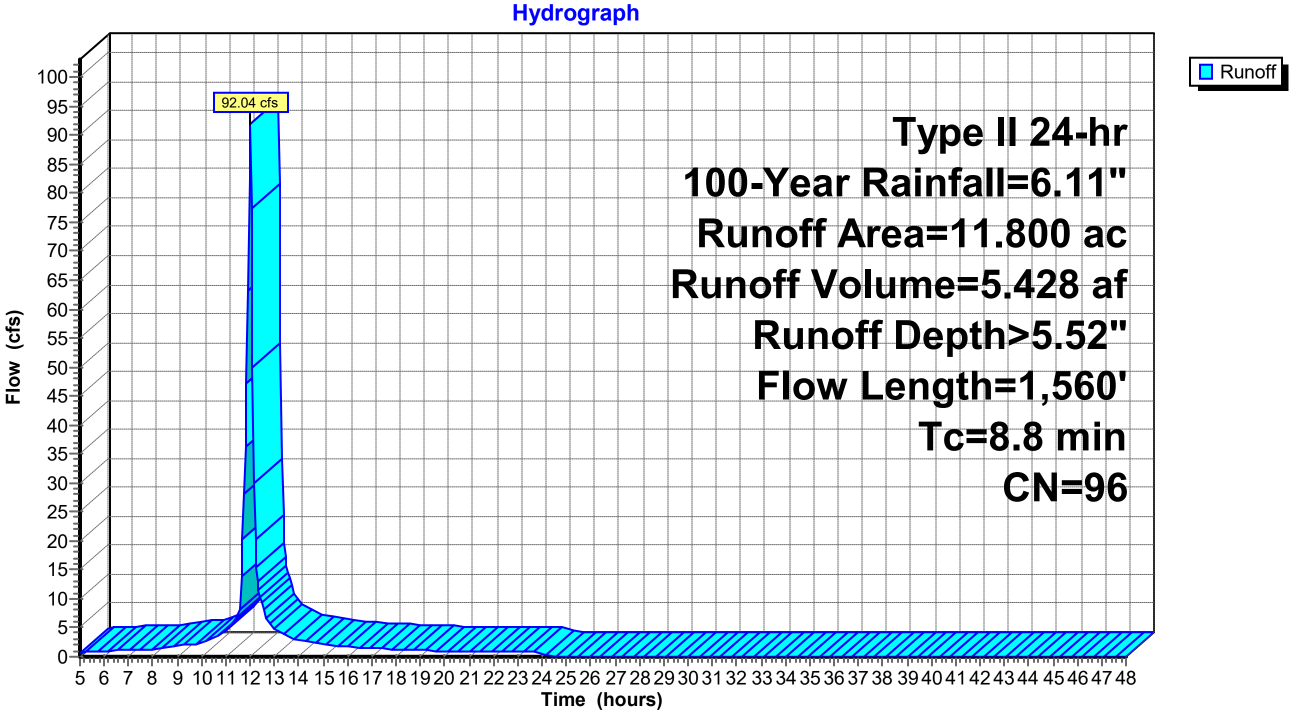
Runoff = 92.04 cfs @ 11.99 hrs, Volume= 5.428 af, Depth> 5.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	2.549	98	Building B
*	1.413	98	Building D
	0.200	80	>75% Grass cover, Good, HSG D
*	7.638	95	Dense Graded Aggregate
	11.800	96	Weighted Average
	7.838		66.42% Pervious Area
	3.962		33.58% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
1.0	100	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
4.5	1,360	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
8.8	1,560	Total			

Subcatchment DR-6: Buldings B & D



Summary for Subcatchment DR-7: Building C & Rail

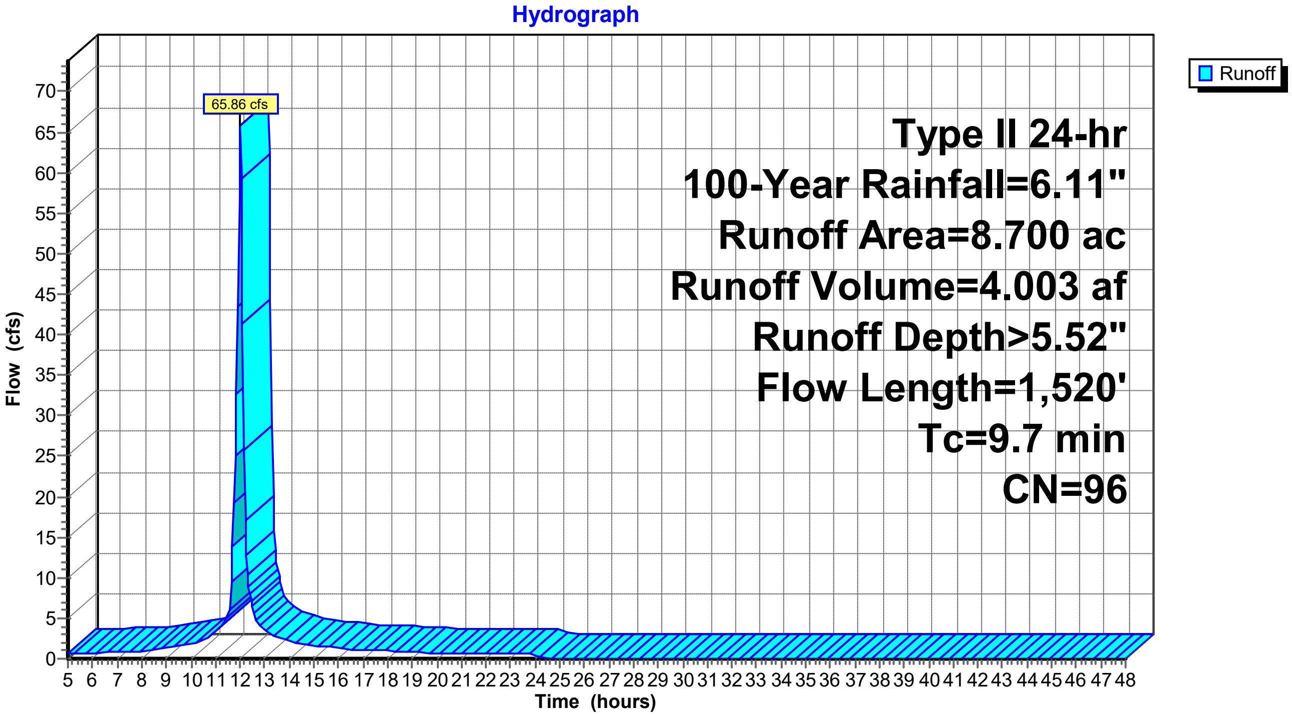
Runoff = 65.86 cfs @ 12.00 hrs, Volume= 4.003 af, Depth> 5.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	3.030	98	Building C
*	0.970	98	Rail
*	4.400	95	Dense Graded Aggregate
	0.300	80	>75% Grass cover, Good, HSG D
	8.700	96	Weighted Average
	4.700		54.02% Pervious Area
	4.000		45.98% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.3	100	0.0100	0.50		**Sheet Flow,** n= 0.023 P2= 2.40"
2.6	250	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
3.8	1,170	0.0050	5.09	16.00	**Pipe Channel,** 24.0" Round Area= 3.1 sf Perim= 6.3' r= 0.50' n= 0.013
9.7	1,520	Total			

Subcatchment DR-7: Building C & Rail



Summary for Subcatchment DR-8A: Parking

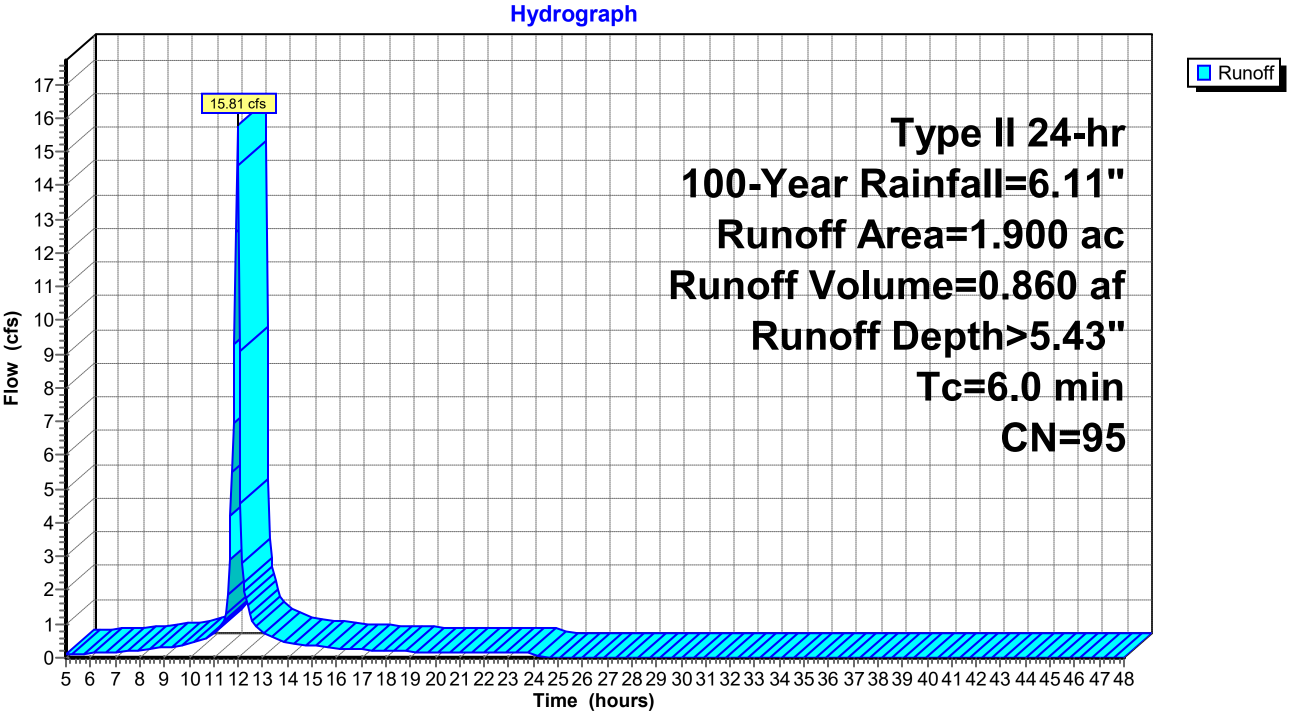
Runoff = 15.81 cfs @ 11.96 hrs, Volume= 0.860 af, Depth> 5.43"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	1.600	98	Parking
	0.300	80	>75% Grass cover, Good, HSG D
	1.900	95	Weighted Average
	0.300		15.79% Pervious Area
	1.600		84.21% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-8A: Parking



Summary for Subcatchment DR-8B: Roadway & Pond

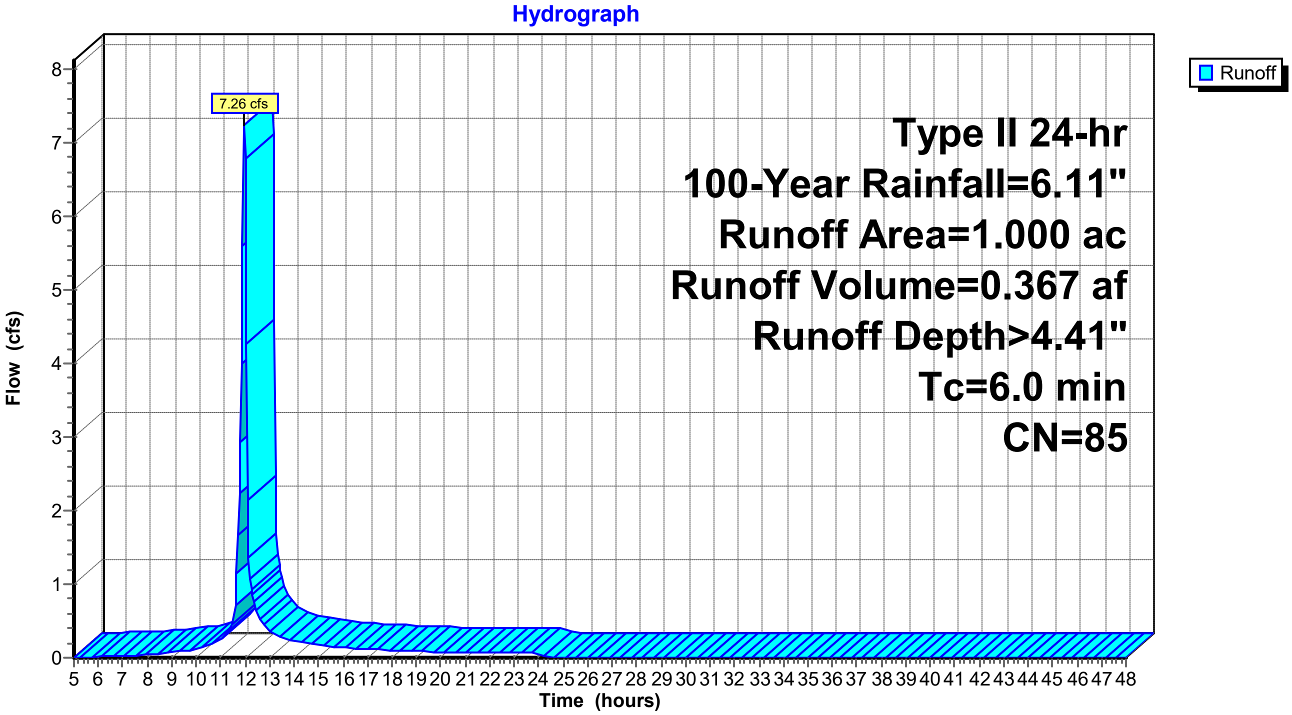
Runoff = 7.26 cfs @ 11.97 hrs, Volume= 0.367 af, Depth> 4.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	0.300	98	Roadway
	0.700	80	>75% Grass cover, Good, HSG D
	1.000	85	Weighted Average
	0.700		70.00% Pervious Area
	0.300		30.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry, Minimum**

Subcatchment DR-8B: Roadway & Pond



Summary for Subcatchment DR-9A: Parking & Substation

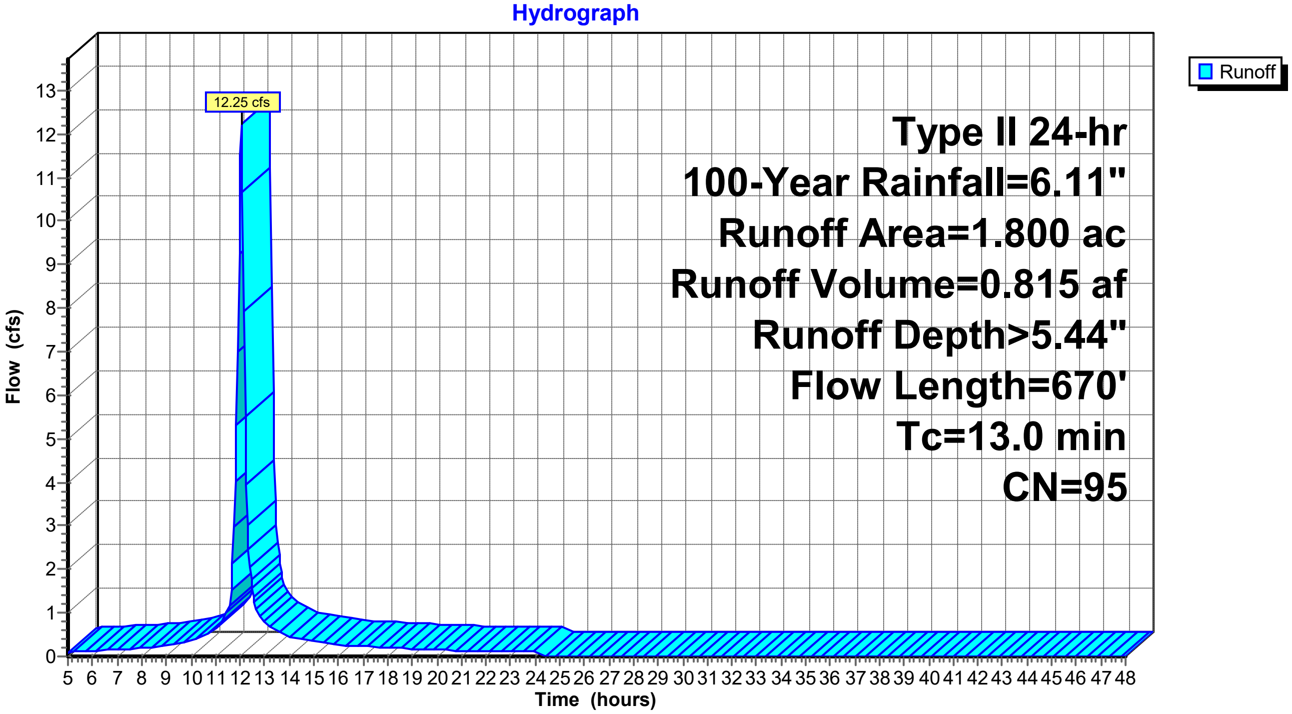
Runoff = 12.25 cfs @ 12.04 hrs, Volume= 0.815 af, Depth> 5.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
	0.230	80	>75% Grass cover, Good, HSG D
*	0.200	92	Compacted Gravel
*	1.200	98	Parking and Road
*	0.170	98	Substation
	1.800	95	Weighted Average
	0.430		23.89% Pervious Area
	1.370		76.11% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.4	100	0.0200	1.19		**Sheet Flow, Parking Lot Runoff** Smooth surfaces n= 0.011 P2= 2.40"
11.6	570	0.0030	0.82		**Shallow Concentrated Flow, Grass Lined Ditch to Pond** Grassed Waterway Kv= 15.0 fps
13.0	670	Total			

Subcatchment DR-9A: Parking & Substation



Summary for Subcatchment DR-9B: Roadway

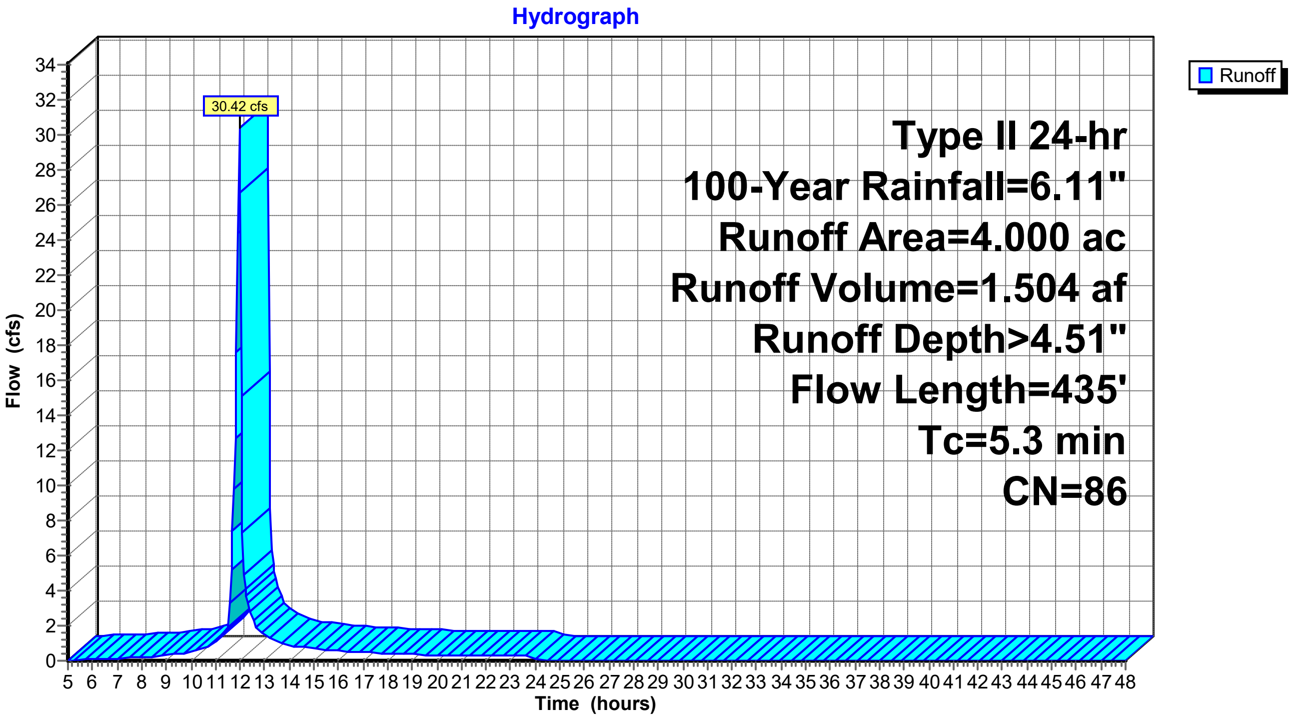
Runoff = 30.42 cfs @ 11.96 hrs, Volume= 1.504 af, Depth> 4.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Type II 24-hr 100-Year Rainfall=6.11"

	Area (ac)	CN	Description
*	1.050	95	Dense Graded Aggregate
*	0.480	98	Roadway
	2.470	80	>75% Grass cover, Good, HSG D
	4.000	86	Weighted Average
	3.520		88.00% Pervious Area
	0.480		12.00% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.6	100	0.0250	0.46		**Sheet Flow, Dense Graded Aggregate Yard** n= 0.040 P2= 2.40"
1.3	230	0.0100	3.07	9.20	**Channel Flow, Grass lined ditch** Area= 3.0 sf Perim= 4.0' r= 0.75' n= 0.040 Earth, cobble bottom, clean sides
0.4	105	0.0050	4.20	7.43	**Pipe Channel, driveway culvert** 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013 Corrugated PE, smooth interior
5.3	435	Total			

Subcatchment DR-9B: Roadway



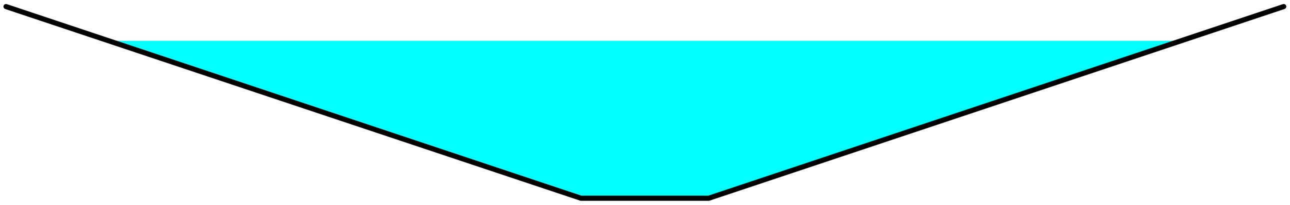
Summary for Reach 1R: Swale

Inflow Area = 1.900 ac, 84.21% Impervious, Inflow Depth > 5.43" for 100-Year event
Inflow = 15.81 cfs @ 11.96 hrs, Volume= 0.860 af
Outflow = 13.37 cfs @ 12.09 hrs, Volume= 0.860 af, Atten= 15%, Lag= 7.6 min

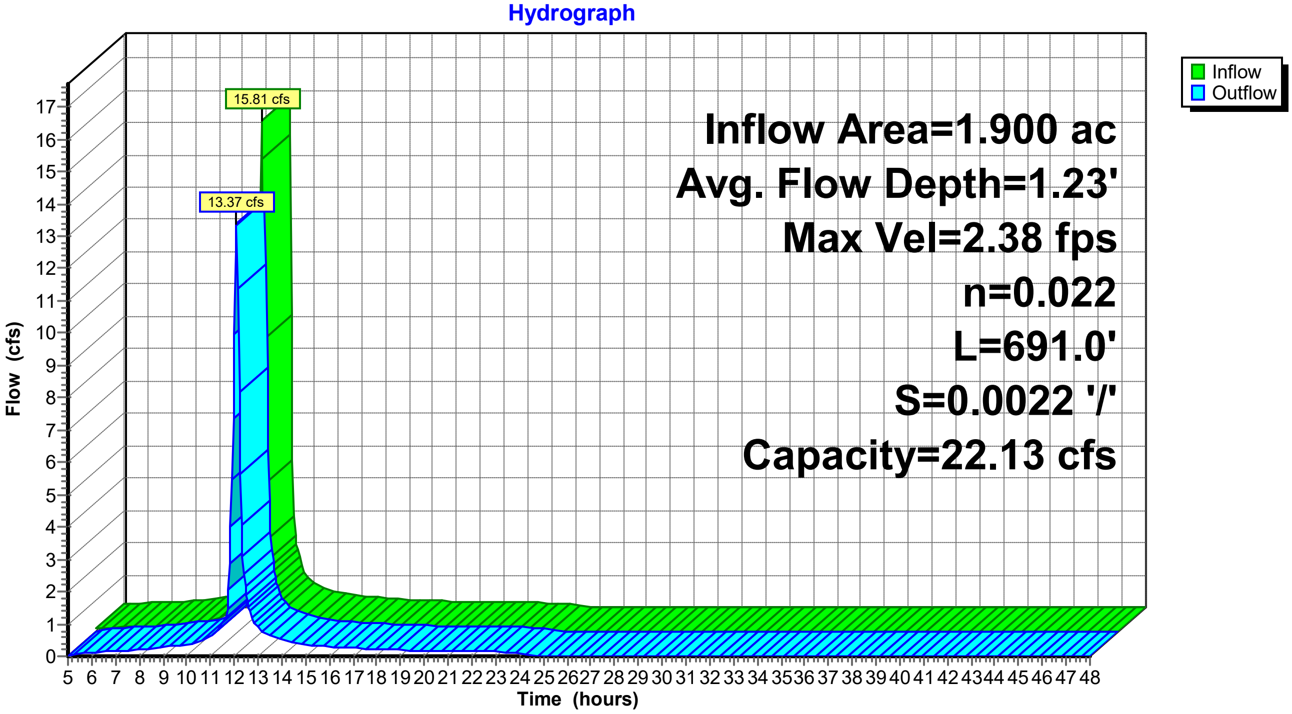
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.38 fps, Min. Travel Time= 4.8 min
Avg. Velocity = 0.71 fps, Avg. Travel Time= 16.3 min

Peak Storage= 3,995 cf @ 12.01 hrs
Average Depth at Peak Storage= 1.23' , Surface Width= 8.39'
Bank-Full Depth= 1.50' Flow Area= 8.3 sf, Capacity= 22.13 cfs

1.00' x 1.50' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 10.00'
Length= 691.0' Slope= 0.0022 '/'
Inlet Invert= 15.50', Outlet Invert= 14.00'



Reach 1R: Swale



Summary for Reach 1W: Wetland #1 / Analysis Point 1A

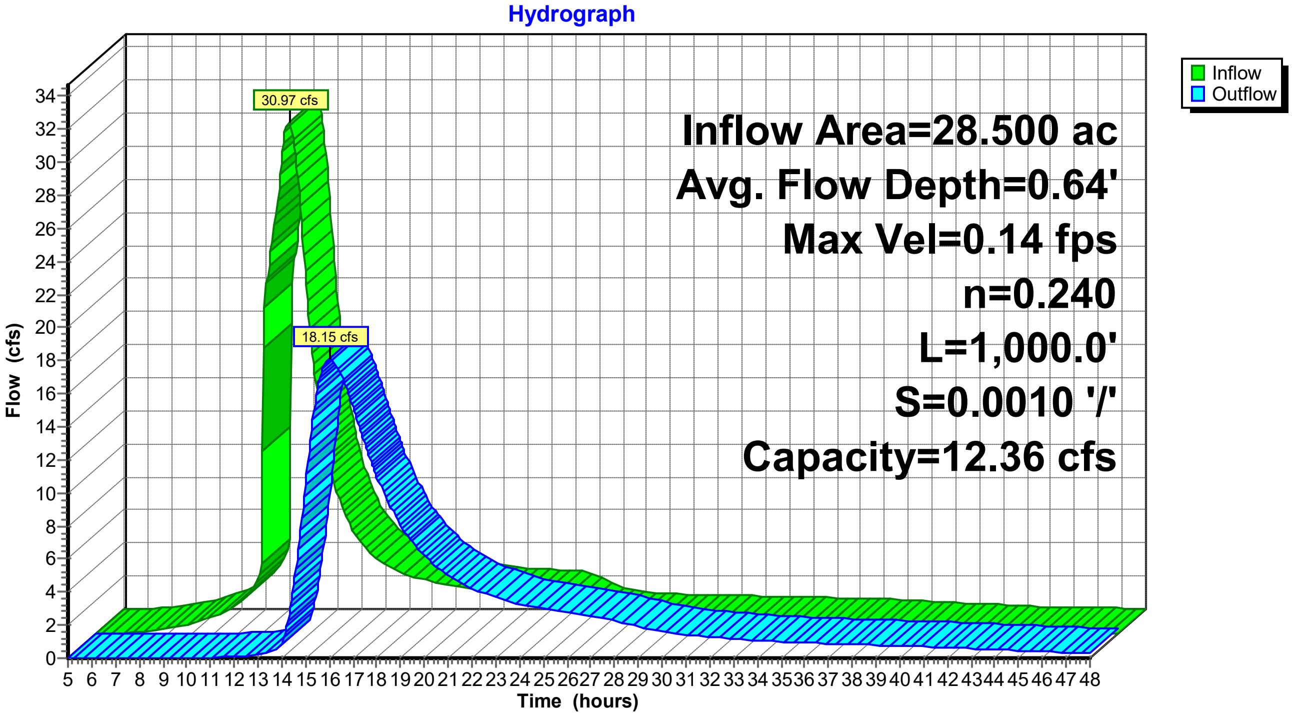
Inflow Area = 28.500 ac, 13.16% Impervious, Inflow Depth > 4.11" for 100-Year event
Inflow = 30.97 cfs @ 13.14 hrs, Volume= 9.753 af
Outflow = 18.15 cfs @ 16.00 hrs, Volume= 9.491 af, Atten= 41%, Lag= 172.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 0.14 fps, Min. Travel Time= 118.8 min
Avg. Velocity = 0.05 fps, Avg. Travel Time= 314.3 min

Peak Storage= 129,381 cf @ 14.02 hrs
Average Depth at Peak Storage= 0.64' , Surface Width= 203.85'
Bank-Full Depth= 0.50' Flow Area= 100.8 sf, Capacity= 12.36 cfs

200.00' x 0.50' deep channel, n= 0.240
Side Slope Z-value= 3.0 '/' Top Width= 203.00'
Length= 1,000.0' Slope= 0.0010 '/'
Inlet Invert= 6.00', Outlet Invert= 5.00'

Reach 1W: Wetland #1 / Analysis Point 1A



Summary for Reach 2R: Overflow

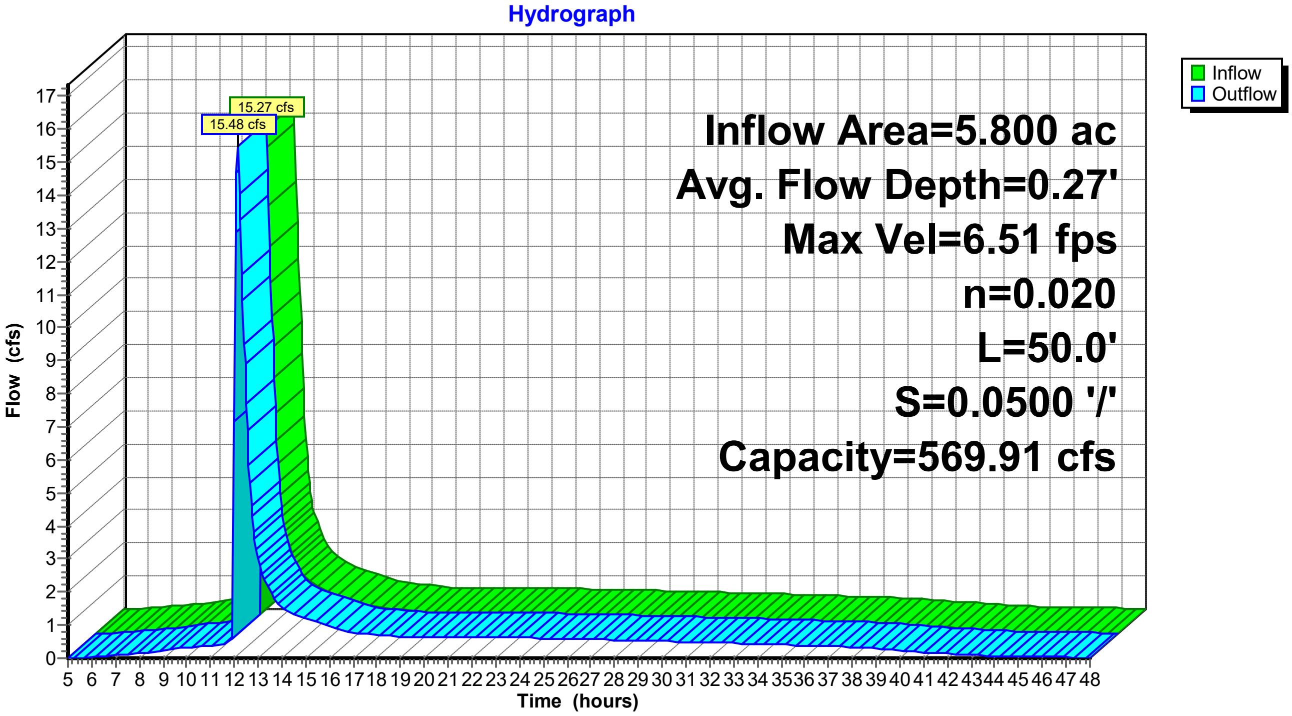
Inflow Area = 5.800 ac, 31.90% Impervious, Inflow Depth > 4.77" for 100-Year event
Inflow = 15.27 cfs @ 12.13 hrs, Volume= 2.307 af
Outflow = 15.48 cfs @ 12.14 hrs, Volume= 2.307 af, Atten= 0%, Lag= 0.9 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.51 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 1.77 fps, Avg. Travel Time= 0.5 min

Peak Storage= 119 cf @ 12.14 hrs
Average Depth at Peak Storage= 0.27' , Surface Width= 9.62'
Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 569.91 cfs

8.00' x 2.00' deep channel, n= 0.020 Corrugated PE, corrugated interior
Side Slope Z-value= 3.0 '/' Top Width= 20.00'
Length= 50.0' Slope= 0.0500 '/'
Inlet Invert= 16.50', Outlet Invert= 14.00'

Reach 2R: Overflow



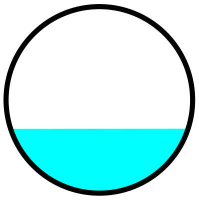
Summary for Reach 3R: Outlet Pipe

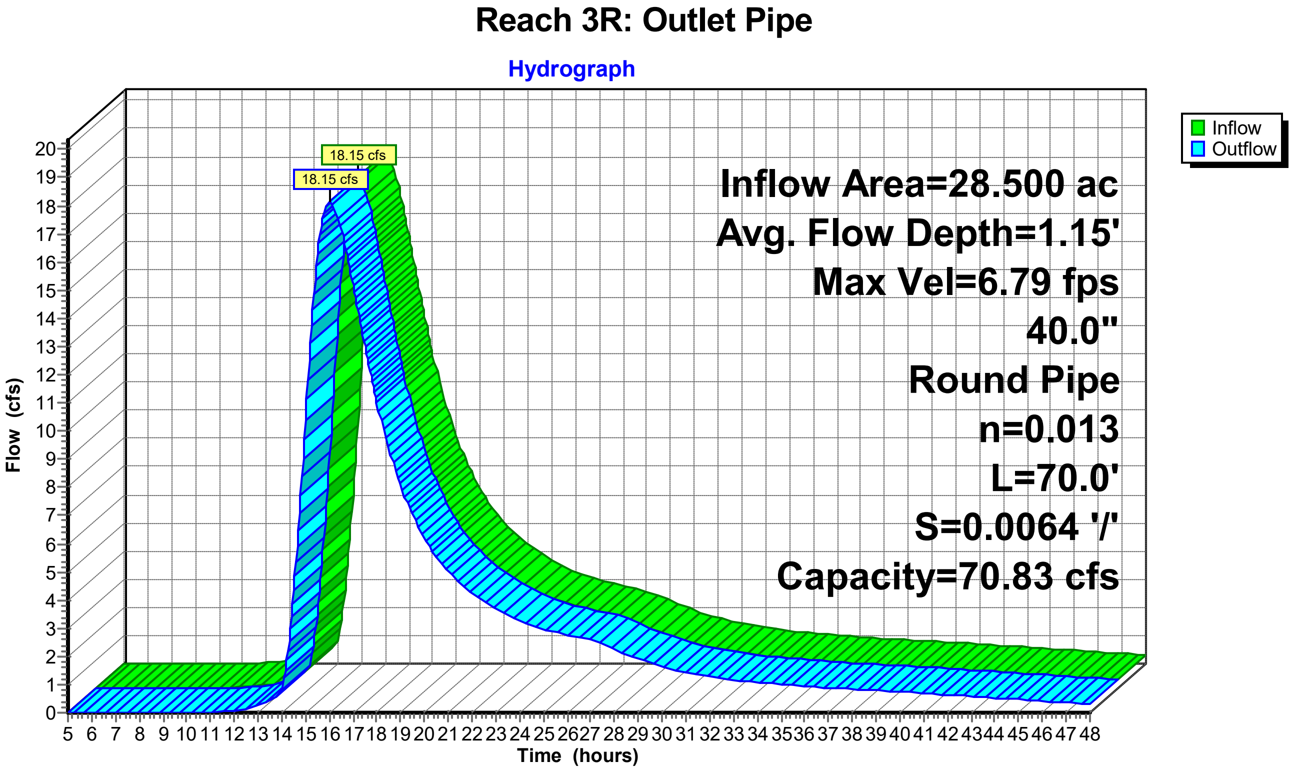
Inflow Area = 28.500 ac, 13.16% Impervious, Inflow Depth > 4.00" for 100-Year event
Inflow = 18.15 cfs @ 16.00 hrs, Volume= 9.491 af
Outflow = 18.15 cfs @ 16.01 hrs, Volume= 9.490 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.79 fps, Min. Travel Time= 0.2 min
Avg. Velocity = 3.26 fps, Avg. Travel Time= 0.4 min

Peak Storage= 187 cf @ 16.00 hrs
Average Depth at Peak Storage= 1.15' , Surface Width= 3.17'
Bank-Full Depth= 3.33' Flow Area= 8.7 sf, Capacity= 70.83 cfs

40.0" Round Pipe
n= 0.013 Corrugated PE, smooth interior
Length= 70.0' Slope= 0.0064 '/'
Inlet Invert= 4.25', Outlet Invert= 3.80'





Summary for Reach 4R: Overflow

Inflow Area = 1.000 ac, 95.00% Impervious, Inflow Depth > 5.43" for 100-Year event
Inflow = 8.17 cfs @ 11.98 hrs, Volume= 0.452 af
Outflow = 8.00 cfs @ 11.99 hrs, Volume= 0.452 af, Atten= 2%, Lag= 0.7 min

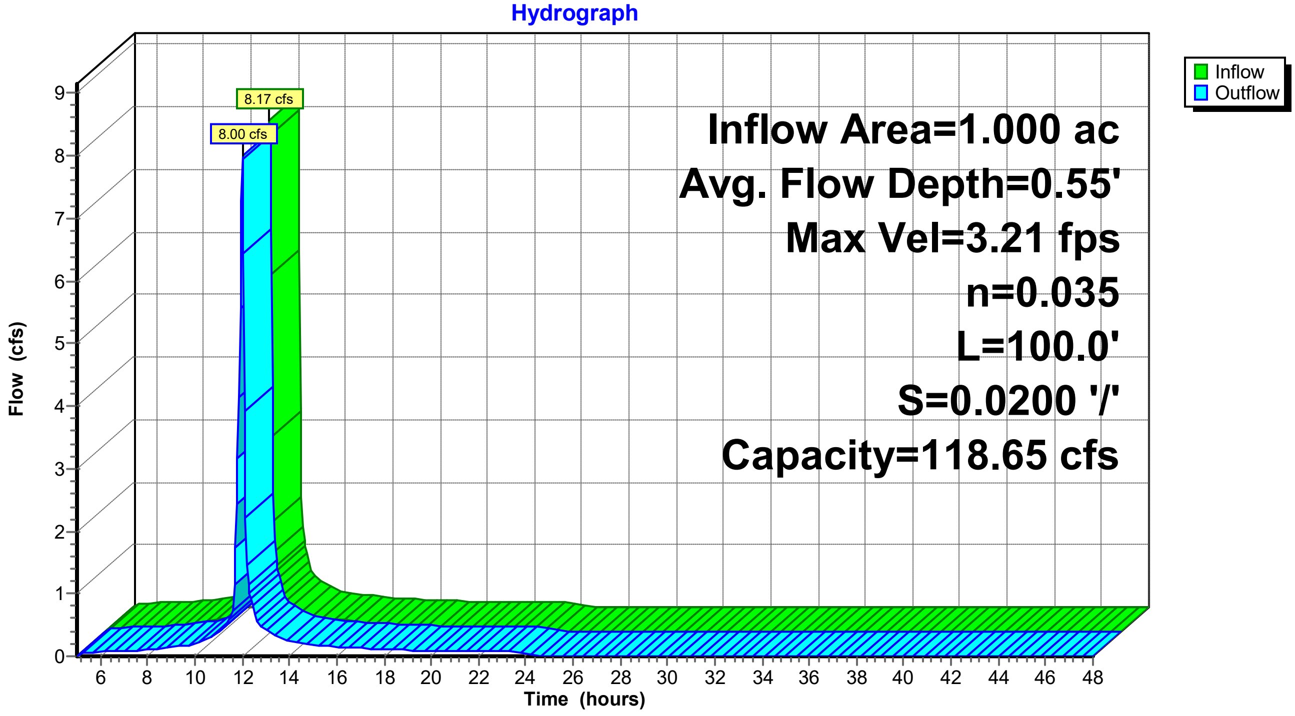
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.21 fps, Min. Travel Time= 0.5 min
Avg. Velocity = 0.87 fps, Avg. Travel Time= 1.9 min

Peak Storage= 253 cf @ 11.98 hrs
Average Depth at Peak Storage= 0.55' , Surface Width= 6.28'
Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 118.65 cfs

3.00' x 2.00' deep channel, n= 0.035 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 15.00'
Length= 100.0' Slope= 0.0200 '/'
Inlet Invert= 12.00', Outlet Invert= 10.00'

‡

Reach 4R: Overflow



Summary for Reach 5R: Overflow

Inflow Area = 1.200 ac, 37.50% Impervious, Inflow Depth = 1.24" for 100-Year event
Inflow = 1.86 cfs @ 12.17 hrs, Volume= 0.124 af
Outflow = 1.56 cfs @ 12.22 hrs, Volume= 0.124 af, Atten= 16%, Lag= 3.2 min

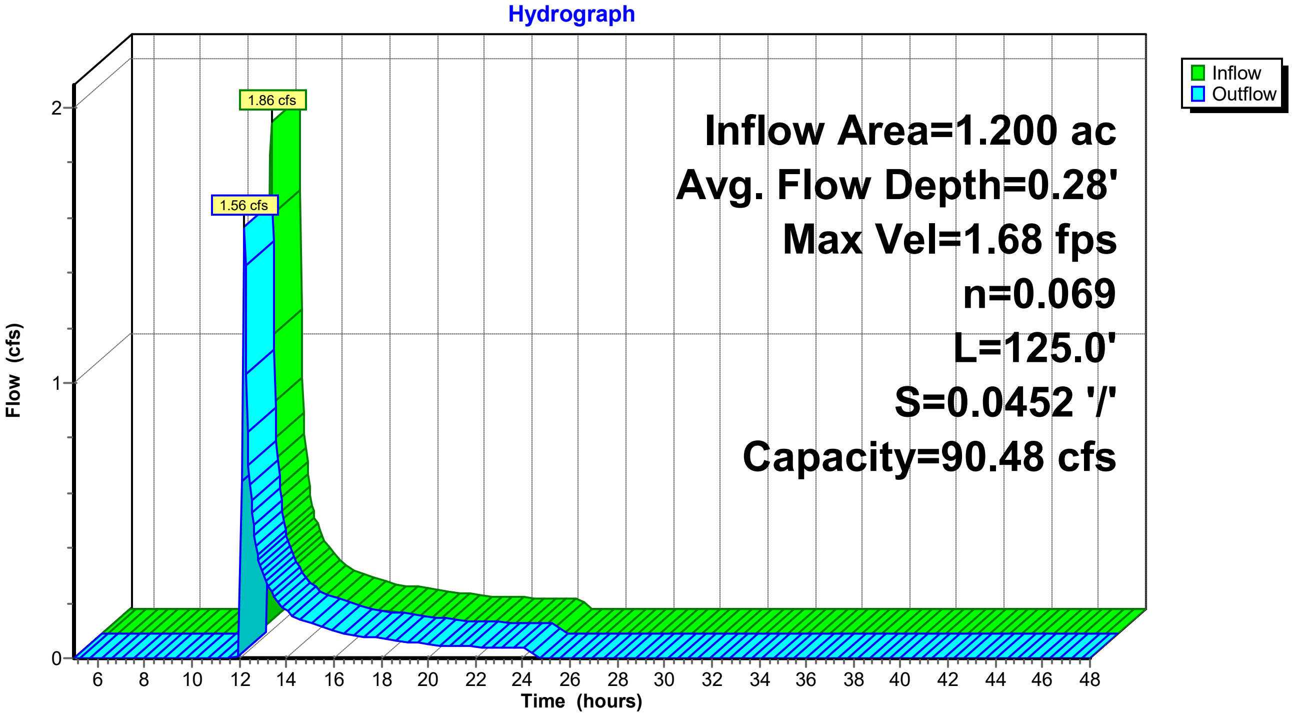
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.68 fps, Min. Travel Time= 1.2 min
Avg. Velocity = 0.58 fps, Avg. Travel Time= 3.6 min

Peak Storage= 132 cf @ 12.20 hrs
Average Depth at Peak Storage= 0.28' , Surface Width= 4.65'
Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 90.48 cfs

3.00' x 2.00' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 15.00'
Length= 125.0' Slope= 0.0452 '/'
Inlet Invert= 11.65', Outlet Invert= 6.00'

‡

Reach 5R: Overflow



Summary for Reach 6R: Overflow

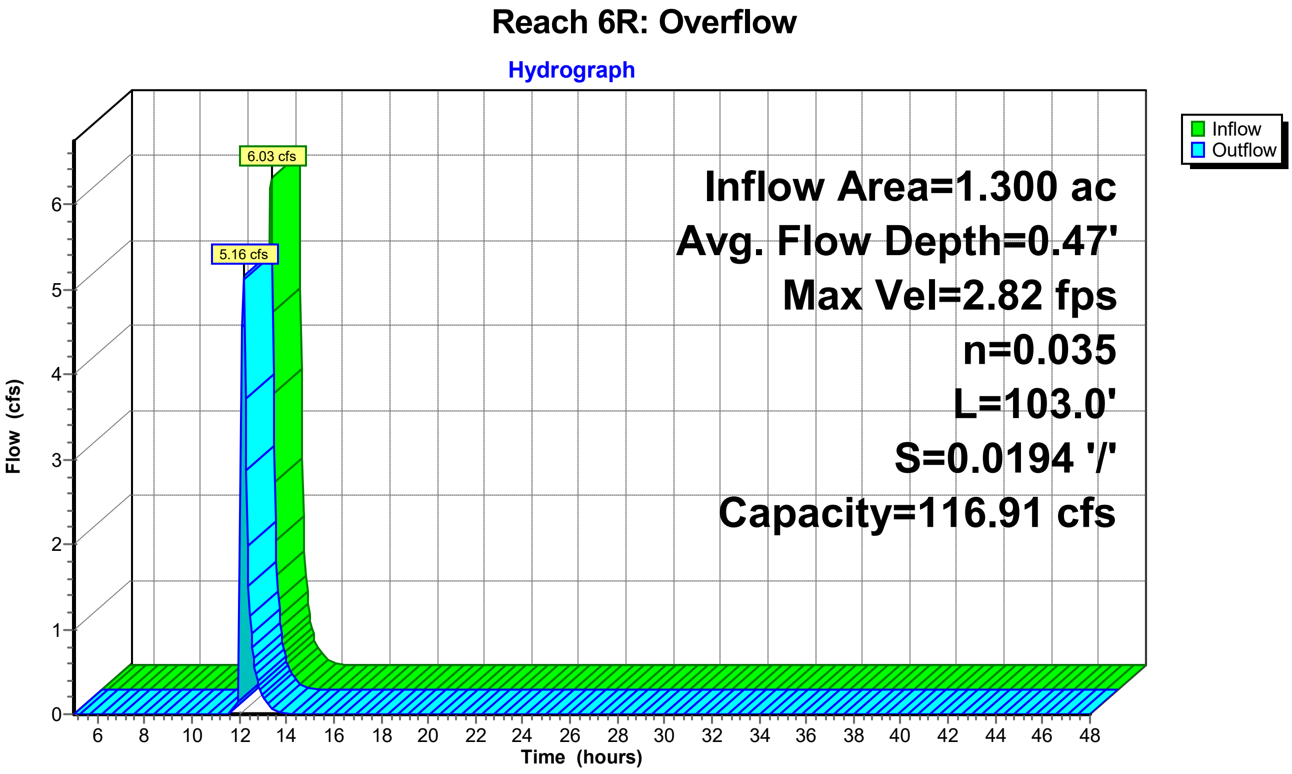
Inflow Area =	1.300 ac,	57.69% Impervious,	Inflow Depth =	1.08"	for 100-Year event
Inflow =	6.03 cfs @	12.16 hrs,	Volume=	0.117 af	
Outflow =	5.16 cfs @	12.19 hrs,	Volume=	0.117 af,	Atten= 14%, Lag= 1.7 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 2.82 fps, Min. Travel Time= 0.6 min
Avg. Velocity = 1.06 fps, Avg. Travel Time= 1.6 min

Peak Storage= 209 cf @ 12.17 hrs
Average Depth at Peak Storage= 0.47' , Surface Width= 5.80'
Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 116.91 cfs

3.00' x 2.00' deep channel, n= 0.035 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 15.00'
Length= 103.0' Slope= 0.0194 '/'
Inlet Invert= 8.50', Outlet Invert= 6.50'

‡



Summary for Reach 7R: Overflow

Inflow Area = 2.900 ac, 65.52% Impervious, Inflow Depth > 5.02" for 100-Year event
Inflow = 3.28 cfs @ 12.41 hrs, Volume= 1.212 af
Outflow = 3.26 cfs @ 12.44 hrs, Volume= 1.212 af, Atten= 0%, Lag= 2.3 min

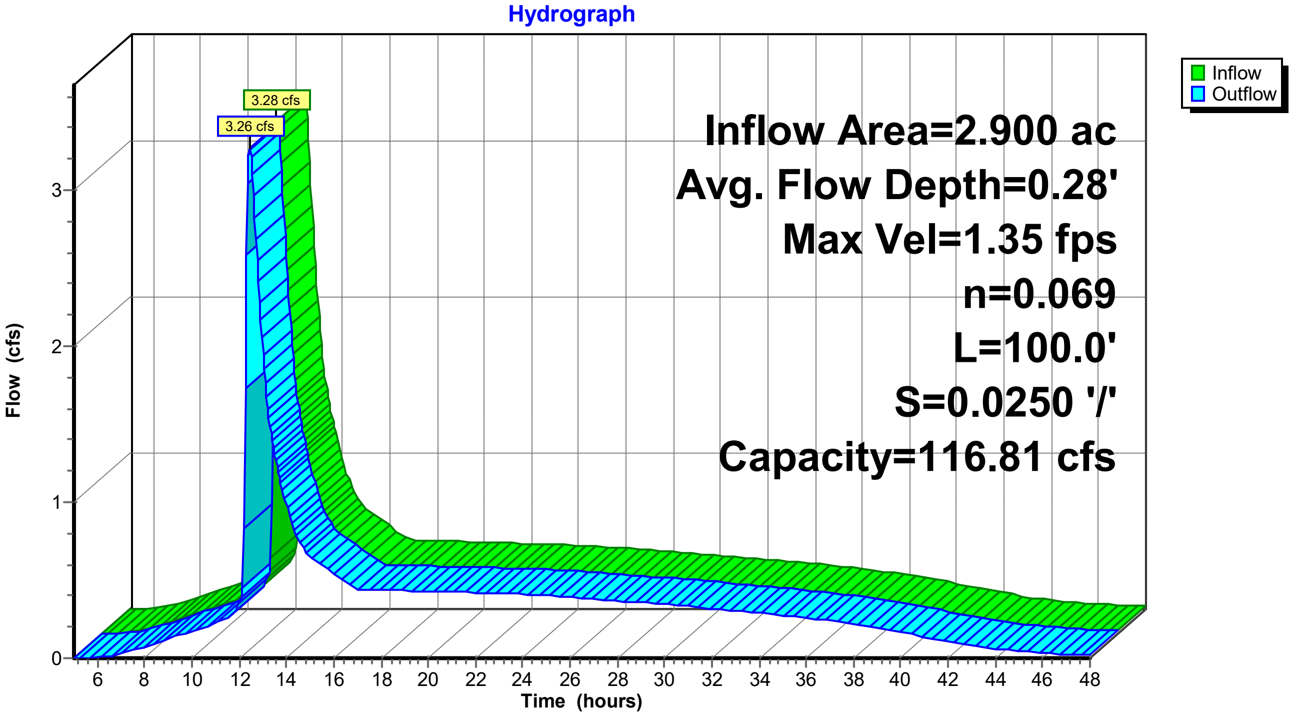
Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.35 fps, Min. Travel Time= 1.2 min
Avg. Velocity = 0.53 fps, Avg. Travel Time= 3.2 min

Peak Storage= 243 cf @ 12.42 hrs
Average Depth at Peak Storage= 0.28' , Surface Width= 9.65'
Bank-Full Depth= 2.00' Flow Area= 28.0 sf, Capacity= 116.81 cfs

8.00' x 2.00' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 3.0 '/' Top Width= 20.00'
Length= 100.0' Slope= 0.0250 '/'
Inlet Invert= 14.50', Outlet Invert= 12.00'

‡

Reach 7R: Overflow



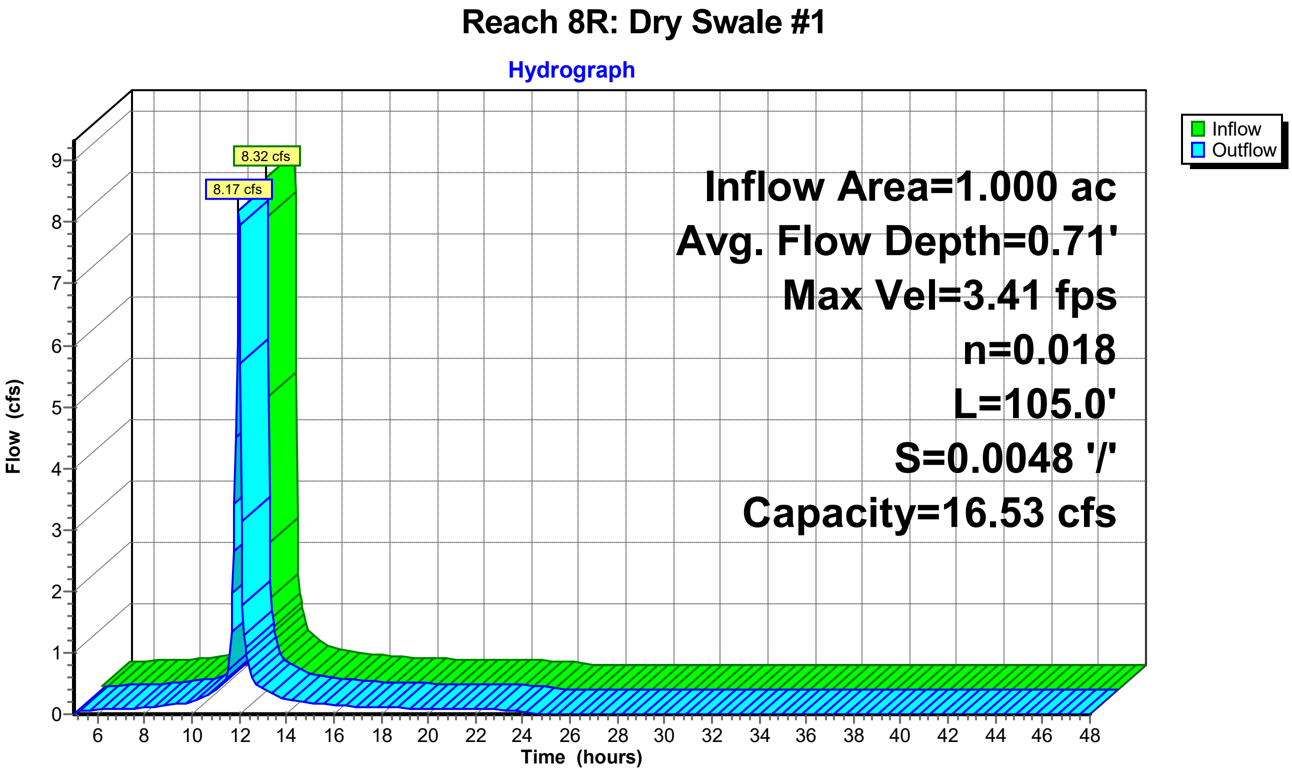
Summary for Reach 8R: Dry Swale #1

Inflow Area = 1.000 ac, 95.00% Impervious, Inflow Depth > 5.43" for 100-Year event
Inflow = 8.32 cfs @ 11.96 hrs, Volume= 0.453 af
Outflow = 8.17 cfs @ 11.98 hrs, Volume= 0.452 af, Atten= 2%, Lag= 1.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.41 fps, Min. Travel Time= 0.5 min
Avg. Velocity = 0.96 fps, Avg. Travel Time= 1.8 min

Peak Storage= 252 cf @ 11.97 hrs
Average Depth at Peak Storage= 0.71' , Surface Width= 4.82'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 16.53 cfs

2.00' x 1.00' deep channel, n= 0.018 Earth, clean & straight
Side Slope Z-value= 2.0 '/' Top Width= 6.00'
Length= 105.0' Slope= 0.0048 '/'
Inlet Invert= 10.00', Outlet Invert= 9.50'



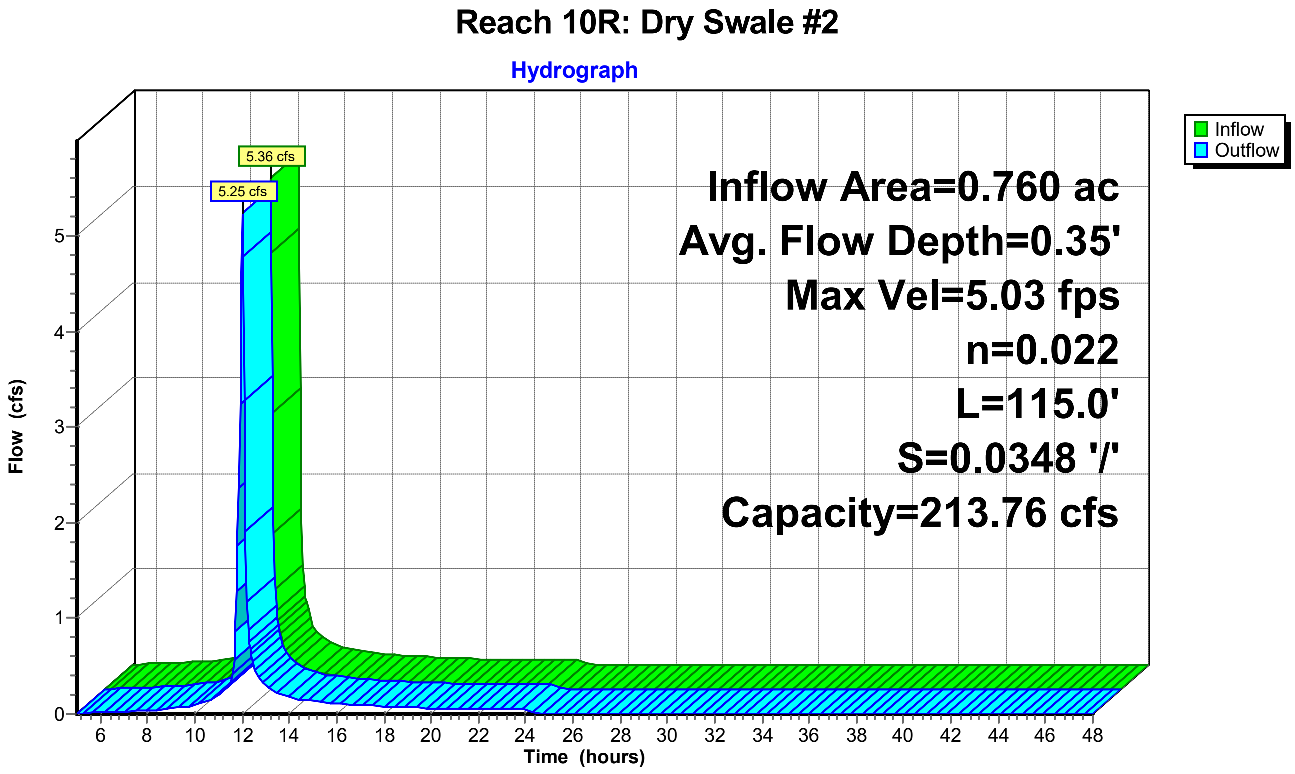
Summary for Reach 10R: Dry Swale #2

Inflow Area = 0.760 ac, 78.95% Impervious, Inflow Depth > 4.51" for 100-Year event
Inflow = 5.36 cfs @ 12.00 hrs, Volume= 0.286 af
Outflow = 5.25 cfs @ 12.01 hrs, Volume= 0.286 af, Atten= 2%, Lag= 0.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 5.03 fps, Min. Travel Time= 0.4 min
Avg. Velocity = 1.34 fps, Avg. Travel Time= 1.4 min

Peak Storage= 122 cf @ 12.00 hrs
Average Depth at Peak Storage= 0.35' , Surface Width= 4.09'
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 213.76 cfs

2.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 14.00'
Length= 115.0' Slope= 0.0348 '/'
Inlet Invert= 37.00', Outlet Invert= 33.00'



Summary for Reach 12R: Sediment Basin Overflow

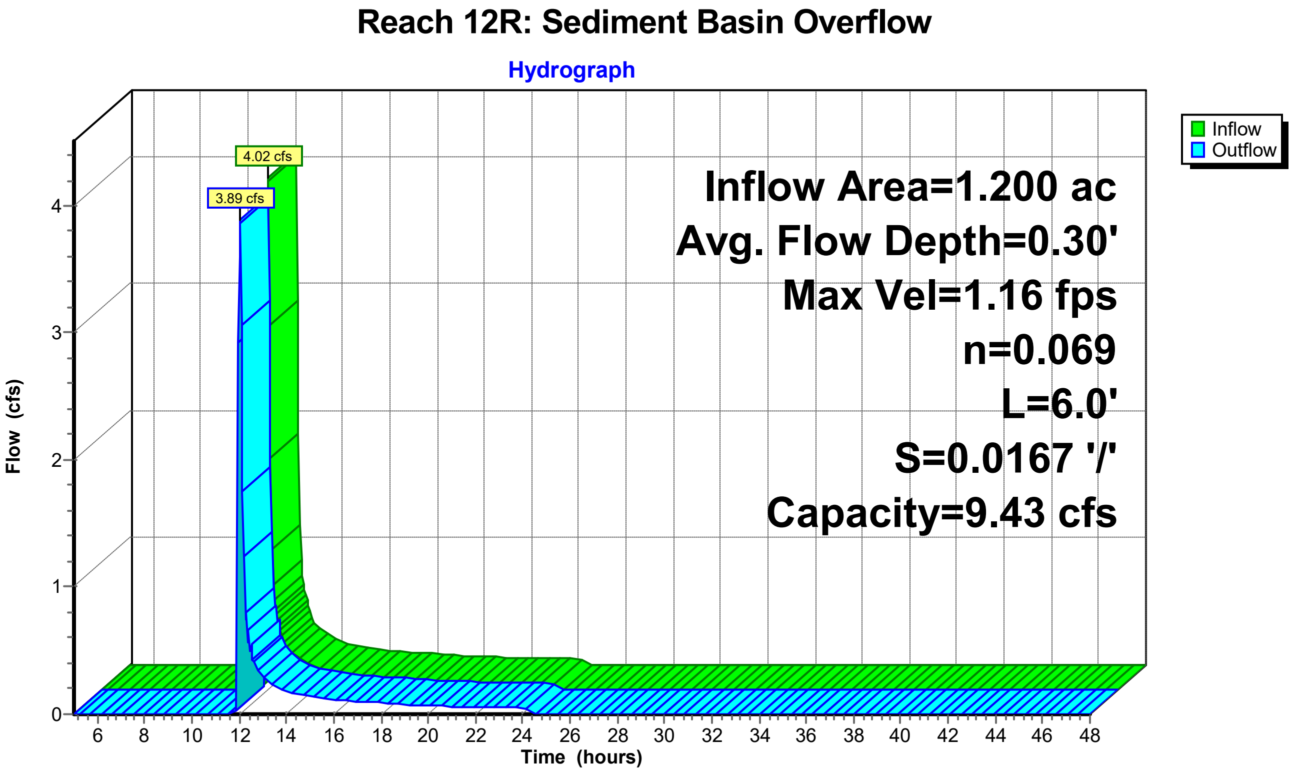
Inflow Area = 1.200 ac, 37.50% Impervious, Inflow Depth = 1.91" for 100-Year event
Inflow = 4.02 cfs @ 12.04 hrs, Volume= 0.191 af
Outflow = 3.89 cfs @ 12.04 hrs, Volume= 0.191 af, Atten= 3%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.16 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 0.28 fps, Avg. Travel Time= 0.4 min

Peak Storage= 20 cf @ 12.04 hrs
Average Depth at Peak Storage= 0.30' , Surface Width= 12.40'
Bank-Full Depth= 0.50' Flow Area= 6.0 sf, Capacity= 9.43 cfs

10.00' x 0.50' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 4.0 '/' Top Width= 14.00'
Length= 6.0' Slope= 0.0167 '/'
Inlet Invert= 12.00', Outlet Invert= 11.90'

‡



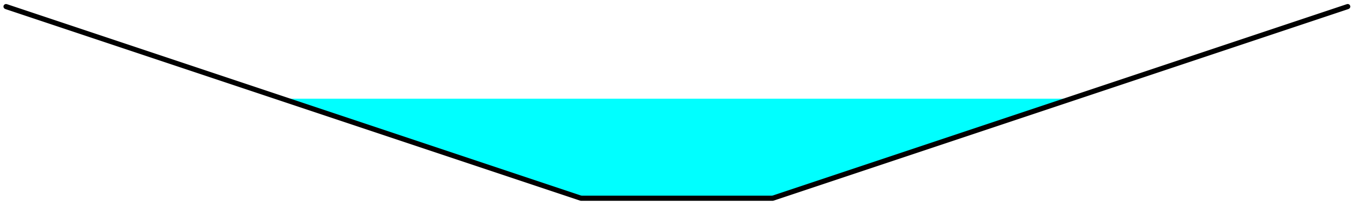
Summary for Reach 13R: Roadside Swale

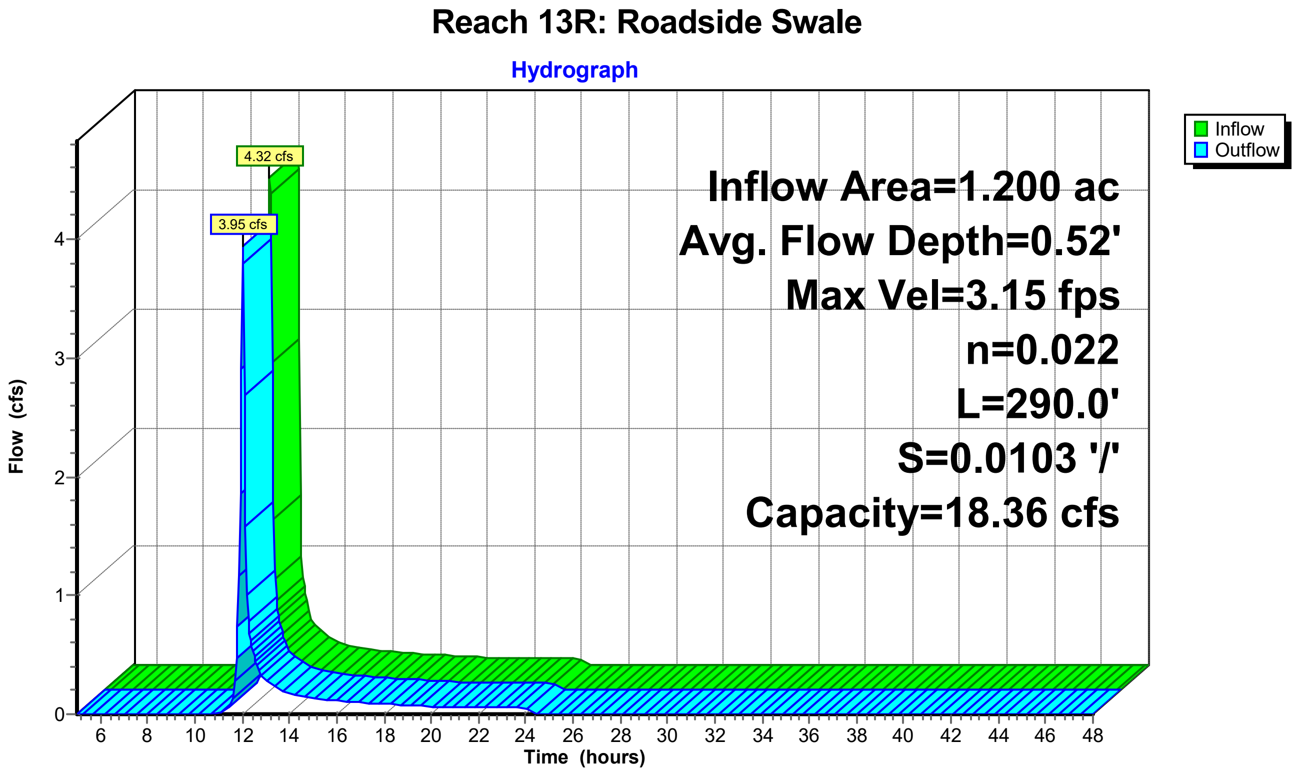
Inflow Area = 1.200 ac, 37.50% Impervious, Inflow Depth = 2.08" for 100-Year event
Inflow = 4.32 cfs @ 11.98 hrs, Volume= 0.208 af
Outflow = 3.95 cfs @ 12.02 hrs, Volume= 0.208 af, Atten= 9%, Lag= 2.6 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 3.15 fps, Min. Travel Time= 1.5 min
Avg. Velocity = 1.04 fps, Avg. Travel Time= 4.6 min

Peak Storage= 386 cf @ 12.00 hrs
Average Depth at Peak Storage= 0.52' , Surface Width= 4.12'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 18.36 cfs

1.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 7.00'
Length= 290.0' Slope= 0.0103 '/'
Inlet Invert= 15.00', Outlet Invert= 12.00'





Summary for Reach 14R: Roadside Swale

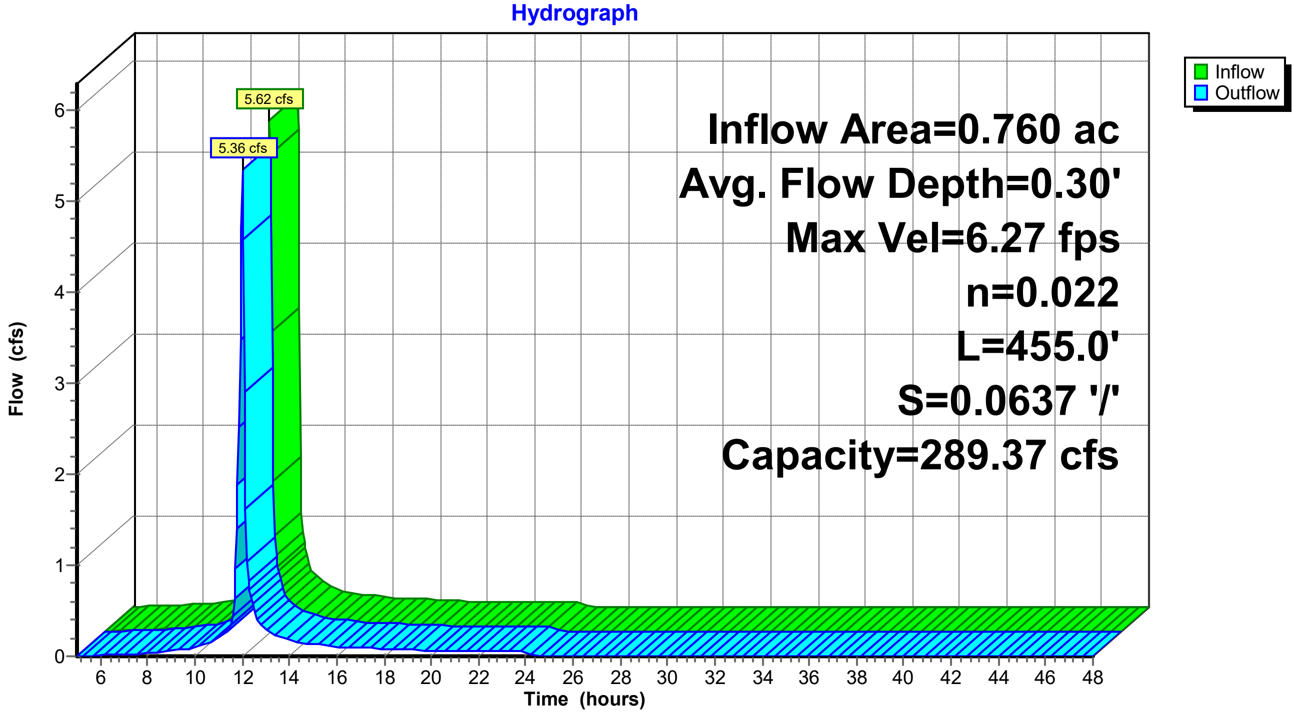
Inflow Area =	0.760 ac,	78.95% Impervious, Inflow Depth >	4.51"	for 100-Year event
Inflow =	5.62 cfs @	11.97 hrs, Volume=	0.286 af	
Outflow =	5.36 cfs @	12.00 hrs, Volume=	0.286 af, Atten= 5%, Lag= 2.0 min	

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.27 fps, Min. Travel Time= 1.2 min
Avg. Velocity = 1.65 fps, Avg. Travel Time= 4.6 min

Peak Storage= 402 cf @ 11.98 hrs
Average Depth at Peak Storage= 0.30' , Surface Width= 3.82'
Bank-Full Depth= 2.00' Flow Area= 16.0 sf, Capacity= 289.37 cfs

2.00' x 2.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 14.00'
Length= 455.0' Slope= 0.0637 '/'
Inlet Invert= 66.00', Outlet Invert= 37.00'

Reach 14R: Roadside Swale



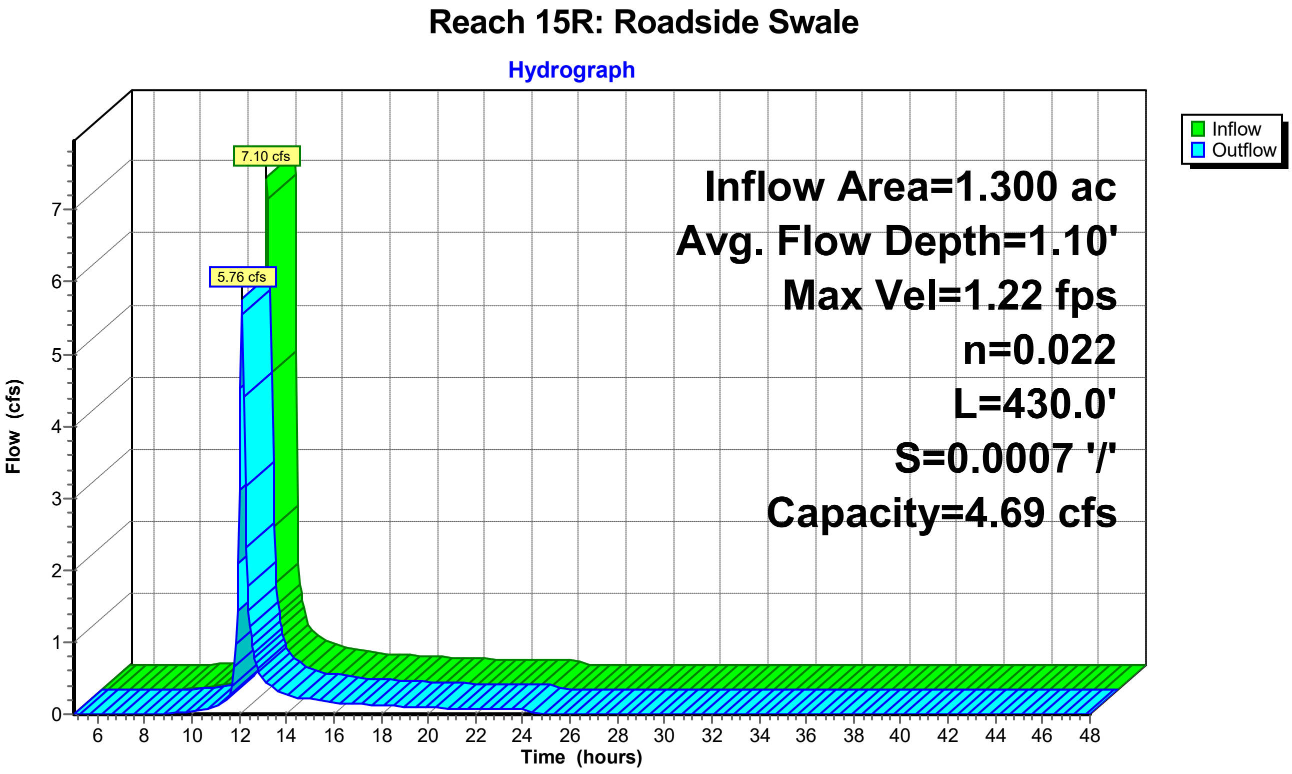
Summary for Reach 15R: Roadside Swale

Inflow Area = 1.300 ac, 57.69% Impervious, Inflow Depth = 3.18" for 100-Year event
Inflow = 7.10 cfs @ 11.97 hrs, Volume= 0.345 af
Outflow = 5.76 cfs @ 12.12 hrs, Volume= 0.345 af, Atten= 19%, Lag= 9.0 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.22 fps, Min. Travel Time= 5.9 min
Avg. Velocity = 0.33 fps, Avg. Travel Time= 22.0 min

Peak Storage= 2,022 cf @ 12.02 hrs
Average Depth at Peak Storage= 1.10' , Surface Width= 7.60'
Bank-Full Depth= 1.00' Flow Area= 4.0 sf, Capacity= 4.69 cfs

1.00' x 1.00' deep channel, n= 0.022 Earth, clean & straight
Side Slope Z-value= 3.0 '/' Top Width= 7.00'
Length= 430.0' Slope= 0.0007 '/'
Inlet Invert= 10.00', Outlet Invert= 9.71'



Summary for Reach 17R: Sediment Basin Overflow

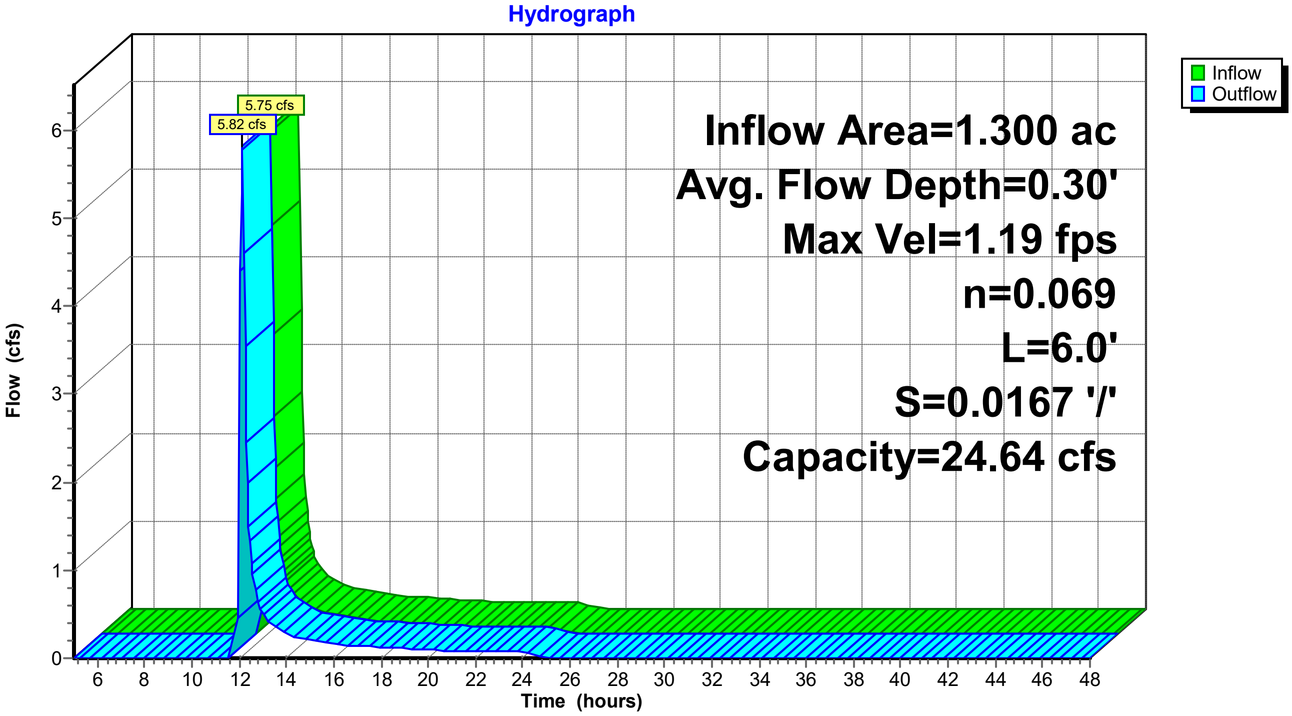
Inflow Area = 1.300 ac, 57.69% Impervious, Inflow Depth = 2.82" for 100-Year event
Inflow = 5.75 cfs @ 12.13 hrs, Volume= 0.305 af
Outflow = 5.82 cfs @ 12.14 hrs, Volume= 0.305 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Max. Velocity= 1.19 fps, Min. Travel Time= 0.1 min
Avg. Velocity = 0.23 fps, Avg. Travel Time= 0.4 min

Peak Storage= 29 cf @ 12.14 hrs
Average Depth at Peak Storage= 0.30' , Surface Width= 17.42'
Bank-Full Depth= 0.70' Flow Area= 12.5 sf, Capacity= 24.64 cfs

15.00' x 0.70' deep channel, n= 0.069 Riprap, 6-inch
Side Slope Z-value= 4.0 '/' Top Width= 20.60'
Length= 6.0' Slope= 0.0167 '/'
Inlet Invert= 9.00', Outlet Invert= 8.90'

Reach 17R: Sediment Basin Overflow



Summary for Pond 1P: WQv Pond #1

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Inflow Area =        2.900 ac,  65.52% Impervious,  Inflow Depth >   5.08"    for  100-Year event
Inflow      =       16.96 cfs @   12.03 hrs,  Volume=          1.227 af
Outflow     =        3.28 cfs @   12.41 hrs,  Volume=          1.212 af,  Atten= 81%,  Lag= 22.8 min
Primary     =        3.28 cfs @   12.41 hrs,  Volume=          1.212 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Starting Elev= 14.00' Surf.Area= 9,229 sf Storage= 19,003 cf
Peak Elev= 16.23' @ 12.41 hrs Surf.Area= 21,418 sf Storage= 47,659 cf (28,656 cf above start)

Plug-Flow detention time= 930.0 min calculated for 0.775 af (63% of inflow)
Center-of-Mass det. time= 522.7 min (1,309.6 - 786.9)

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	4,795 cf	**Forebay (Prismatic)** Listed below (Recalc)
#2	9.00'	57,882 cf	**Permanent Pool (Prismatic)** Listed below (Recalc)
		62,677 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	232	0	0
11.00	569	401	401
12.00	1,018	794	1,194
13.00	1,467	1,243	2,437
14.00	3,249	2,358	4,795

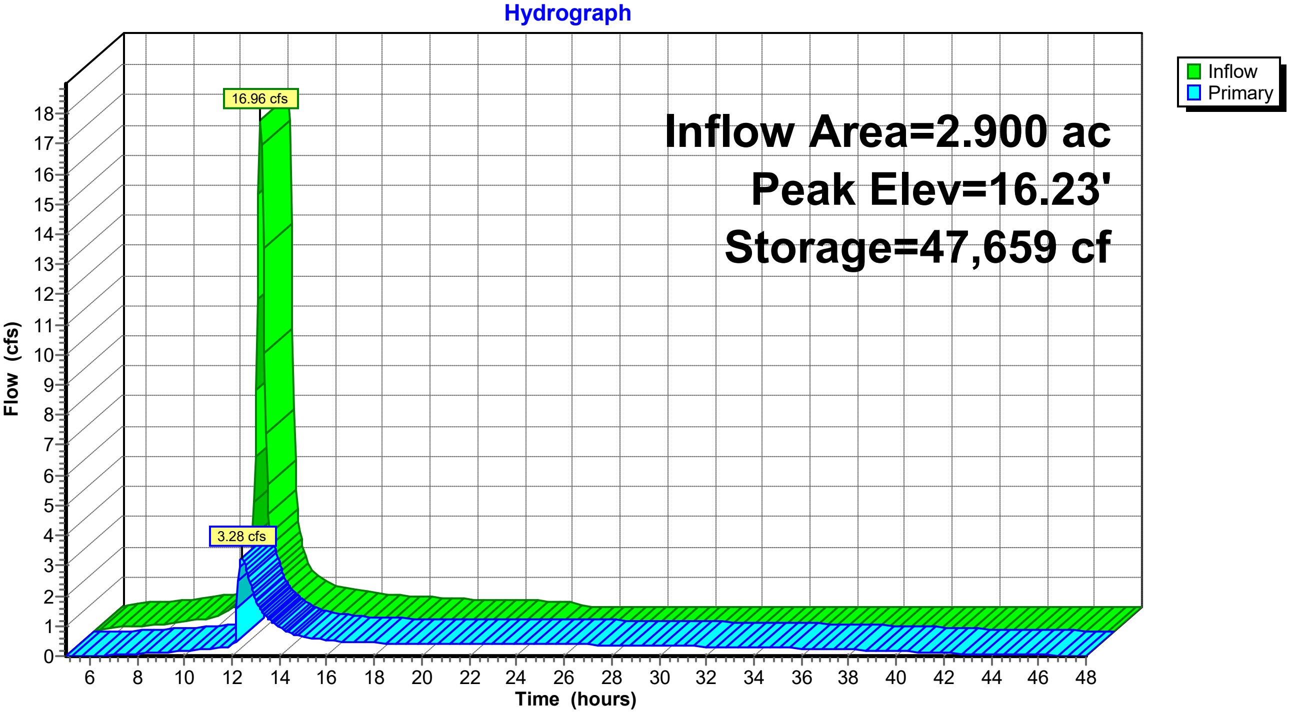
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
9.00	1,145	0	0
10.00	1,751	1,448	1,448
11.00	2,339	2,045	3,493
12.00	2,959	2,649	6,142
13.00	3,597	3,278	9,420
14.00	5,980	4,789	14,209
14.50	7,240	3,305	17,514
15.00	14,392	5,408	22,922
16.00	17,455	15,924	38,845
17.00	20,619	19,037	57,882

Device	Routing	Invert	Outlet Devices
#1	Primary	13.50'	**12.0" Round Culvert** L= 50.0' Box, headwall w/3 square edges, Ke= 0.500 Inlet / Outlet Invert= 13.50' / 13.23' S= 0.0054 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Device 1	16.00'	**24.0" x 24.0" Horiz. Outlet Structure Top Grate** C= 0.600 in 24.0" x 24.0" Grate (100% open area) Limited to weir flow at low heads
#3	Device 1	14.00'	**4.0" Round Reverse Slope Pipe** L= 40.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 9.00' / 14.00' S= -0.1250 '/' Cc= 0.900

n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf
#4 Primary 16.25' **6.0' long x 1.0' breadth Broad-Crested Rectangular Weir**
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00
Coef. (English) 2.69 2.72 2.75 2.85 2.98 3.08 3.20 3.28 3.31 3.30 3.31 3.32

Primary OutFlow Max=3.26 cfs @ 12.41 hrs HW=16.23' (Free Discharge)
1=Culvert (Passes 3.26 cfs of 5.08 cfs potential flow)
  2=Outlet Structure Top Grate (Weir Controls 2.80 cfs @ 1.55 fps)
  3=Reverse Slope Pipe (Outlet Controls 0.46 cfs @ 5.29 fps)
4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 1P: WQv Pond #1



Summary for Pond 2P: WQv Pond #2

```
Inflow Area =        5.800 ac, 31.90% Impervious, Inflow Depth >  4.80"   for 100-Year event
Inflow      =      40.22 cfs @  11.97 hrs, Volume=          2.320 af
Outflow     =      15.27 cfs @  12.13 hrs, Volume=          2.307 af, Atten= 62%, Lag= 9.6 min
Primary     =      15.27 cfs @  12.13 hrs, Volume=          2.307 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Starting Elev= 14.00' Surf.Area= 5,917 sf Storage= 9,000 cf
Peak Elev= 17.07' @ 12.13 hrs Surf.Area= 27,493 sf Storage= 54,456 cf (45,456 cf above start)

Plug-Flow detention time= 493.6 min calculated for 2.098 af (90% of inflow)
Center-of-Mass det. time= 393.8 min (1,180.5 - 786.7)

Volume	Invert	Avail.Storage	Storage Description
#1	10.00'	4,020 cf	**Forebay #1 (Prismatic)** Listed below (Recalc)
#2	10.00'	2,575 cf	**Forebay #2 (Prismatic)** Listed below (Recalc)
#3	10.00'	58,093 cf	**Permanent Pool (Prismatic)** Listed below (Recalc)
		64,688 cf	Total Available Storage

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	141	0	0
11.00	330	236	236
12.00	562	446	682
13.00	866	714	1,396
14.00	2,023	1,445	2,840
14.50	2,696	1,180	4,020

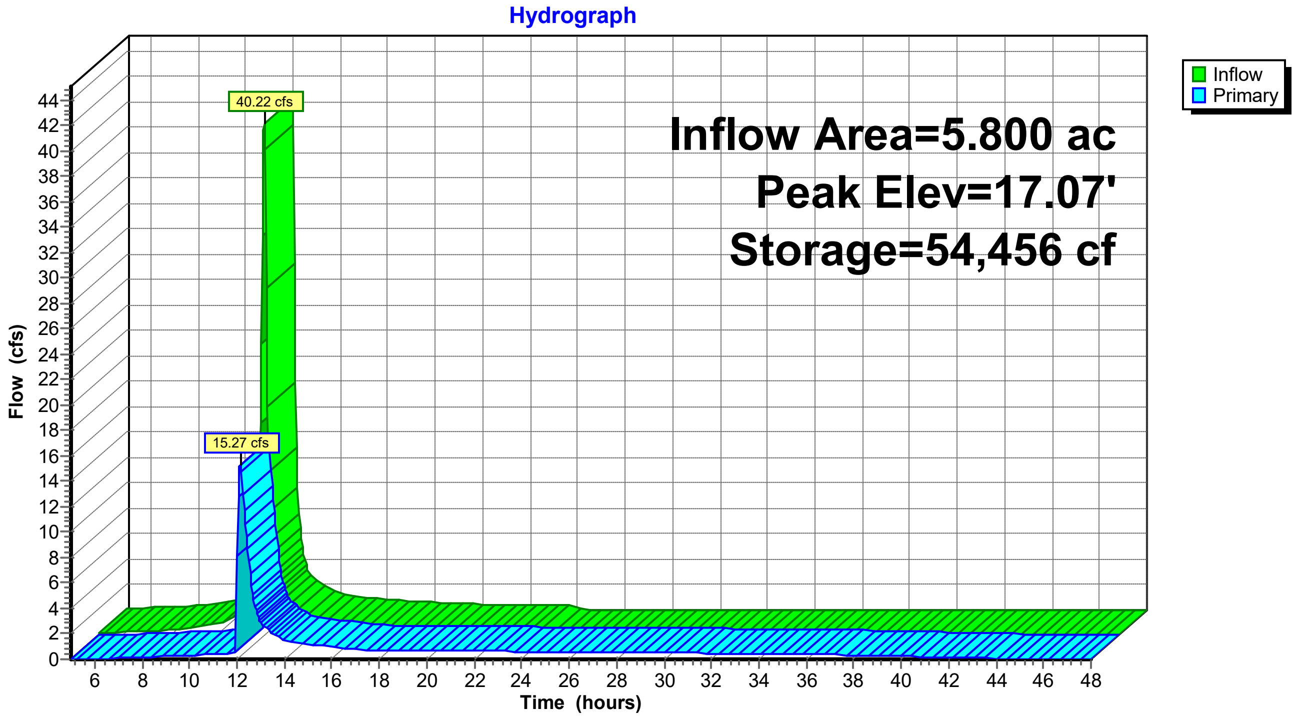
Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	82	0	0
11.00	202	142	142
12.00	351	277	419
13.00	535	443	862
14.00	1,323	929	1,791
14.50	1,815	785	2,575

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
10.00	375	0	0
11.00	653	514	514
12.00	957	805	1,319
13.00	1,286	1,122	2,441
14.00	2,571	1,929	4,369
14.50	3,307	1,470	5,839
15.00	13,814	4,280	10,119
16.00	17,852	15,833	25,952
17.00	22,659	20,256	46,207
17.50	24,884	11,886	58,093

Device	Routing	Invert	Outlet Devices
#1	Device 3	16.50'	**24.0" x 24.0" Horiz. Orifice/Grate** C= 0.600 in 24.0" x 24.0" Grate (100% open area) Limited to weir flow at low heads
#2	Device 3	14.00'	**4.0" Vert. Reverse Slope Pipe** C= 0.600 Limited to weir flow at low heads
#3	Primary	14.00'	**12.0" Round Outlet Structure Discard Pipe** L= 46.0' Box, headwall w/3 square edges, Ke= 0.500 Inlet / Outlet Invert= 14.00' / 13.77' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#4	Primary	16.60'	**10.0' long x 0.5' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=15.12 cfs @ 12.13 hrs HW=17.07' (Free Discharge)
3=Outlet Structure Discard Pipe (Barrel Controls 5.57 cfs @ 7.09 fps)
  1=Orifice/Grate (Passes < 11.22 cfs potential flow)
  2=Reverse Slope Pipe (Passes < 0.72 cfs potential flow)
4=Broad-Crested Rectangular Weir (Weir Controls 9.55 cfs @ 2.04 fps)

Pond 2P: WQv Pond #2



Summary for Pond 3P: Infiltration Basin #1

Inflow Area = 1.200 ac, 37.50% Impervious, Inflow Depth = 1.91" for 100-Year event
Inflow = 3.89 cfs @ 12.04 hrs, Volume= 0.191 af
Outflow = 1.88 cfs @ 12.17 hrs, Volume= 0.172 af, Atten= 52%, Lag= 7.9 min
Discarded = 0.02 cfs @ 12.17 hrs, Volume= 0.048 af
Primary = 1.86 cfs @ 12.17 hrs, Volume= 0.124 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 11.94' @ 12.17 hrs Surf.Area= 1,383 sf Storage= 2,406 cf

Plug-Flow detention time= 296.0 min calculated for 0.172 af (90% of inflow)
Center-of-Mass det. time= 245.1 min (1,121.7 - 876.7)

Volume	Invert	Avail.Storage	Storage Description
#1	8.10'	2,492 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

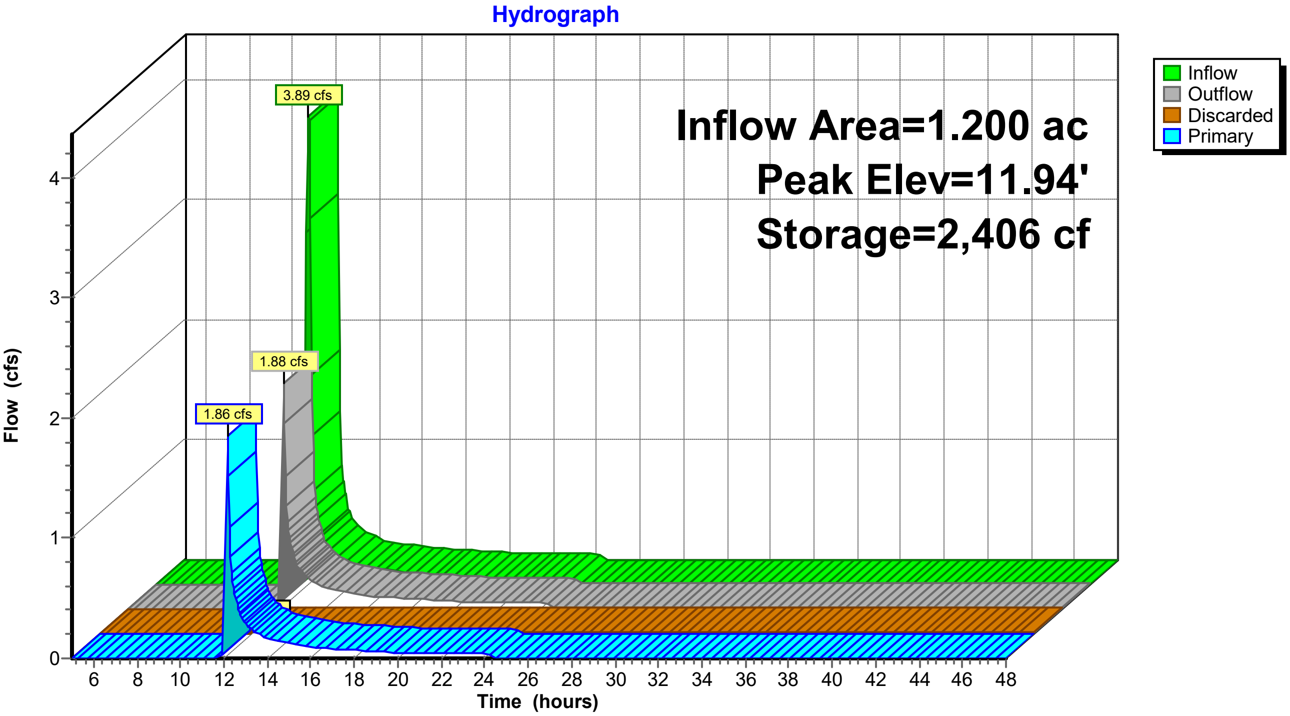
Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
8.10	109	56.0	0	0	109
12.00	1,415	149.0	2,492	2,492	1,678

Device	Routing	Invert	Outlet Devices
#1	Primary	11.65'	**Channel/Reach** using Reach 5R: Overflow
#2	Discarded	8.10'	**0.500 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 4.00'

Discarded OutFlow Max=0.02 cfs @ 12.17 hrs HW=11.91' (Free Discharge)
↑—**2=Exfiltration** (Controls 0.02 cfs)

Primary OutFlow Max=1.65 cfs @ 12.17 hrs HW=11.91' (Free Discharge)
↑—**1=Channel/Reach** (Channel Controls 1.65 cfs @ 1.64 fps)

Pond 3P: Infiltration Basin #1



Summary for Pond 4P: Infiltration Basin #2

```
Inflow Area =        1.300 ac, 57.69% Impervious, Inflow Depth =  2.82"   for 100-Year event
Inflow     =       5.82 cfs @  12.14 hrs, Volume=          0.305 af
Outflow    =       6.44 cfs @  12.16 hrs, Volume=          0.305 af, Atten= 0%, Lag= 1.3 min
Discarded  =       0.42 cfs @  12.16 hrs, Volume=          0.189 af
Primary    =       6.03 cfs @  12.16 hrs, Volume=          0.117 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 8.97' @ 12.16 hrs Surf.Area= 1,059 sf Storage= 1,597 cf

Plug-Flow detention time= 29.1 min calculated for 0.305 af (100% of inflow)
Center-of-Mass det. time= 29.1 min (895.2 - 866.1)

Volume	Invert	Avail.Storage	Storage Description
#1	5.80'	2,495 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
5.80	110	56.0	0	0	110
9.70	1,415	150.0	2,495	2,495	1,702

Device	Routing	Invert	Outlet Devices
#1	Primary	8.50'	**Channel/Reach** using Reach 6R: Overflow
#2	Discarded	5.80'	**12.000 in/hr Exfiltration over Surface area** Conductivity to Groundwater Elevation = 3.00'

Discarded OutFlow Max=0.41 cfs @ 12.16 hrs HW=8.95' (Free Discharge)
↑—**2=Exfiltration** (Controls 0.41 cfs)

Primary OutFlow Max=5.55 cfs @ 12.16 hrs HW=8.95' (Free Discharge)
↑—**1=Channel/Reach** (Channel Controls 5.55 cfs @ 2.85 fps)

APPENDIX B-3C

RFP # 2025-12

BEACON ISLAND PHASE 3
Packaged Wastewater Treatment Plant
and
Fire Pump House and Marine Inlet

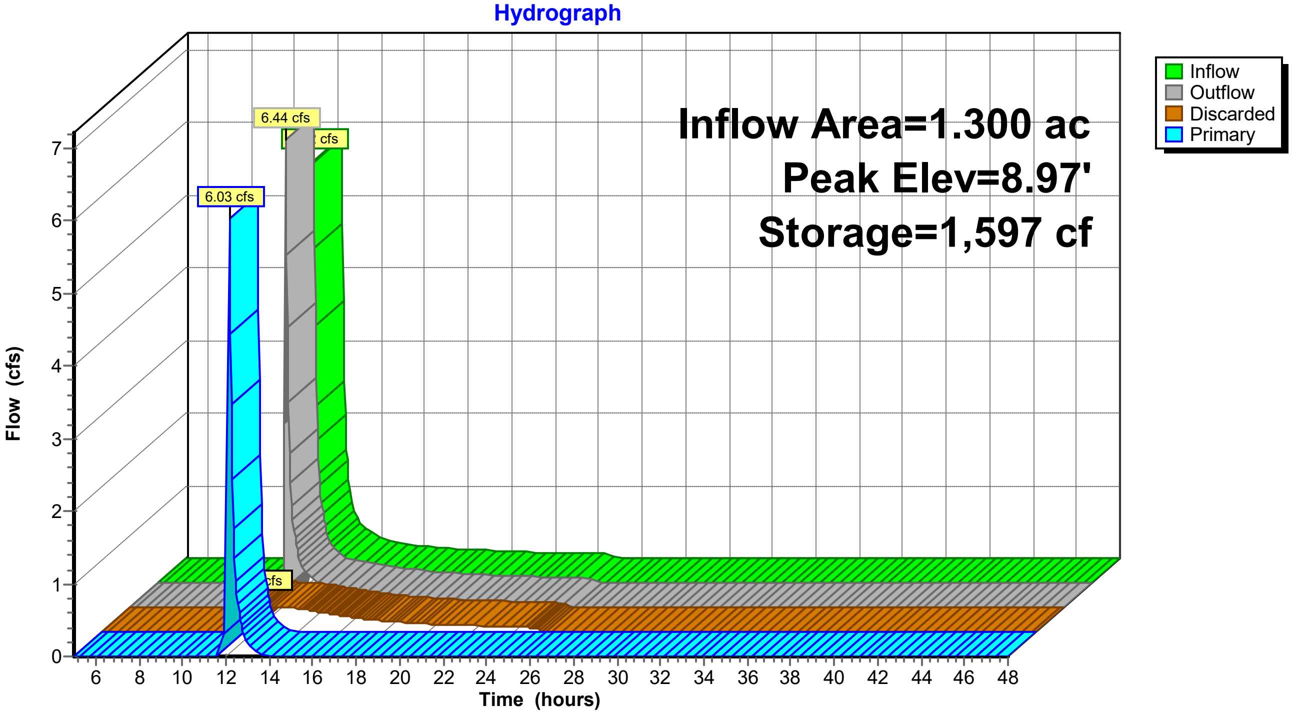
REFERENCE DOCUMENTS (3 of 5)

Reference Documents

Beacon Island Phase 3 Program

- NYSDEC Article 11 and 15 Permits. Dated 11/10/22 - Page 1
- State Pollutant Discharge Elimination System (SPDES) Permit. Dated 10/1/23 - Page 21
- Stormwater Pollution Protection Plan (SWPPP). Dated 6/20/22 - Page 59
- Soil Management Plan. Dated 10/23/22 - Page 564
- Landfill Closure Certification Report. Dated 10/21/24 - Page 634
- Geotechnical Engineering Report. Dated 2/2/2023 - Page 659
- Army Corps of Engineers Permit. Date 4/10/23 - Page 779
- Community Air Monitoring Plan (CAMP). Dated 10/23/22 – Page 806

Pond 4P: Infiltration Basin #2



Summary for Pond 5P: Sedimentation Basin #1

```
Inflow Area =      1.200 ac, 37.50% Impervious, Inflow Depth =  2.08"   for 100-Year event
Inflow      =     3.95 cfs @  12.02 hrs, Volume=          0.208 af
Outflow     =     4.02 cfs @  12.04 hrs, Volume=          0.191 af, Atten= 0%, Lag= 1.1 min
Primary     =     4.02 cfs @  12.04 hrs, Volume=          0.191 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 12.31' @ 12.04 hrs Surf.Area= 632 sf Storage= 930 cf

Plug-Flow detention time= 59.3 min calculated for 0.191 af (92% of inflow)
Center-of-Mass det. time= 15.9 min (876.3 - 860.4)

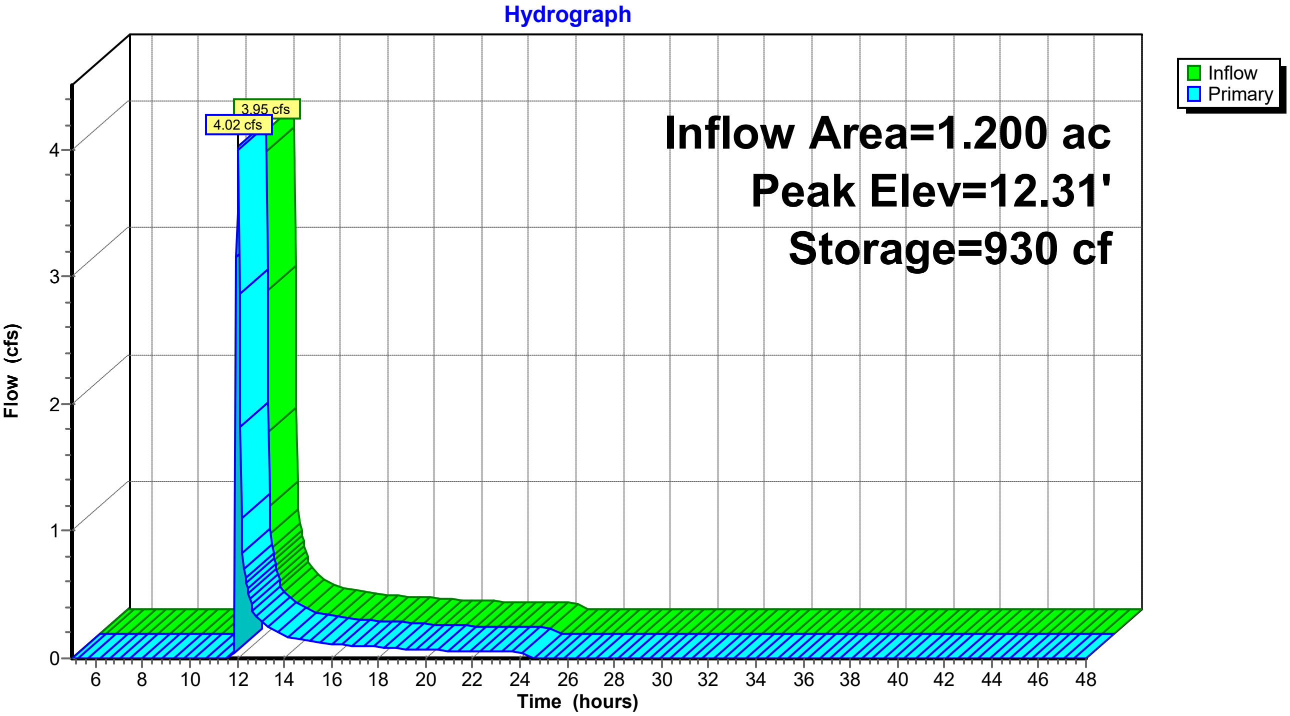
Volume	Invert	Avail.Storage	Storage Description
#1	9.00'	1,058 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
9.00	41	25.8	0	0	41
10.00	150	46.7	90	90	167
11.00	320	66.6	230	320	355
12.00	550	86.4	430	749	608
12.50	687	96.2	309	1,058	758

Device	Routing	Invert	Outlet Devices
#1	Primary	12.00'	**Channel/Reach** using Reach 12R: Sediment Basin Overflow

Primary OutFlow Max=3.89 cfs @ 12.04 hrs HW=12.30' (Free Discharge)
└─**1=Channel/Reach** (Channel Controls 3.89 cfs @ 1.16 fps)

Pond 5P: Sedimentation Basin #1



Summary for Pond 16P: Sedimentation Basin #2

Inflow Area = 1.300 ac, 57.69% Impervious, Inflow Depth = 3.18" for 100-Year event
Inflow = 5.76 cfs @ 12.12 hrs, Volume= 0.345 af
Outflow = 5.75 cfs @ 12.13 hrs, Volume= 0.305 af, Atten= 0%, Lag= 0.8 min
Primary = 5.75 cfs @ 12.13 hrs, Volume= 0.305 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 9.30' @ 12.13 hrs Surf.Area= 970 sf Storage= 1,979 cf

Plug-Flow detention time= 79.9 min calculated for 0.305 af (89% of inflow)
Center-of-Mass det. time= 21.8 min (865.8 - 843.9)

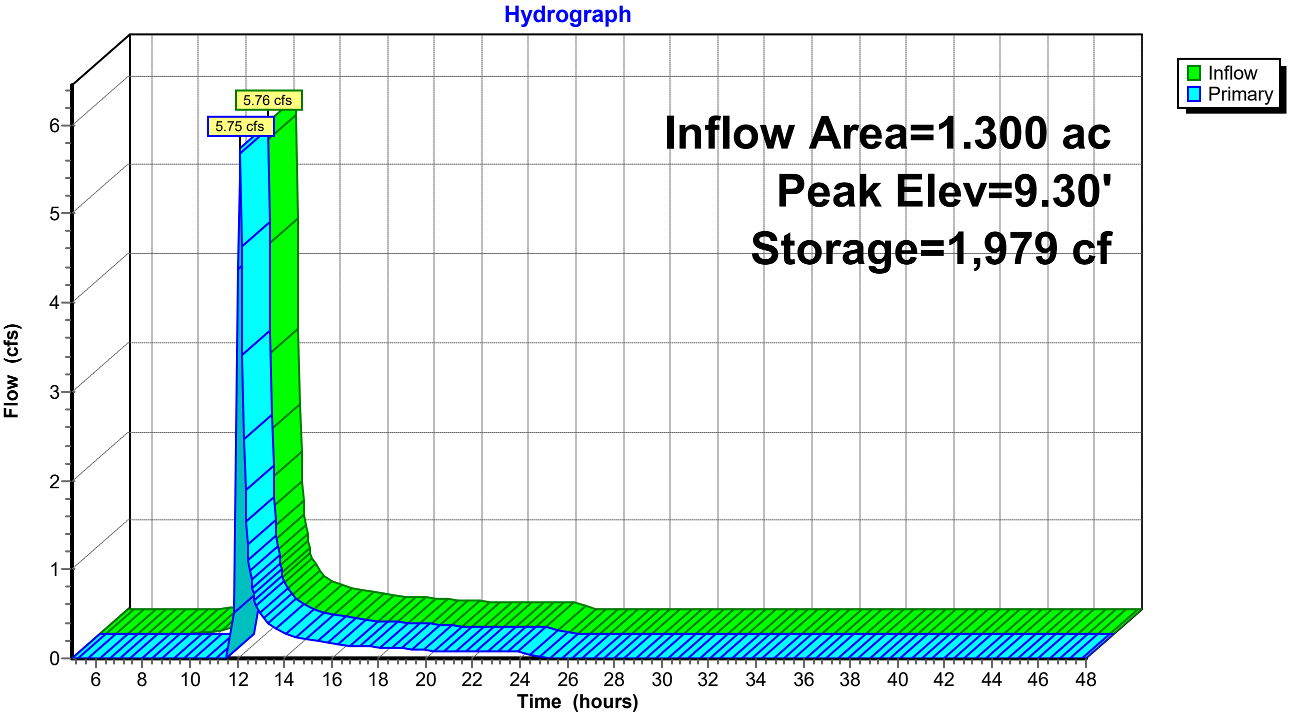
Volume	Invert	Avail.Storage	Storage Description
#1	5.80'	2,389 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
5.80	224	80.0	0	0	224
6.00	257	83.0	48	48	266
7.00	438	98.0	344	392	500
8.00	650	113.0	541	932	773
9.00	892	128.0	768	1,700	1,085
9.70	1,079	139.0	689	2,389	1,337

Device	Routing	Invert	Outlet Devices
#1	Primary	9.00'	**Channel/Reach** using Reach 17R: Sediment Basin Overflow

Primary OutFlow Max=5.60 cfs @ 12.13 hrs HW=9.30' (Free Discharge)
↑—**1=Channel/Reach** (Channel Controls 5.60 cfs @ 1.17 fps)

Pond 16P: Sedimentation Basin #2

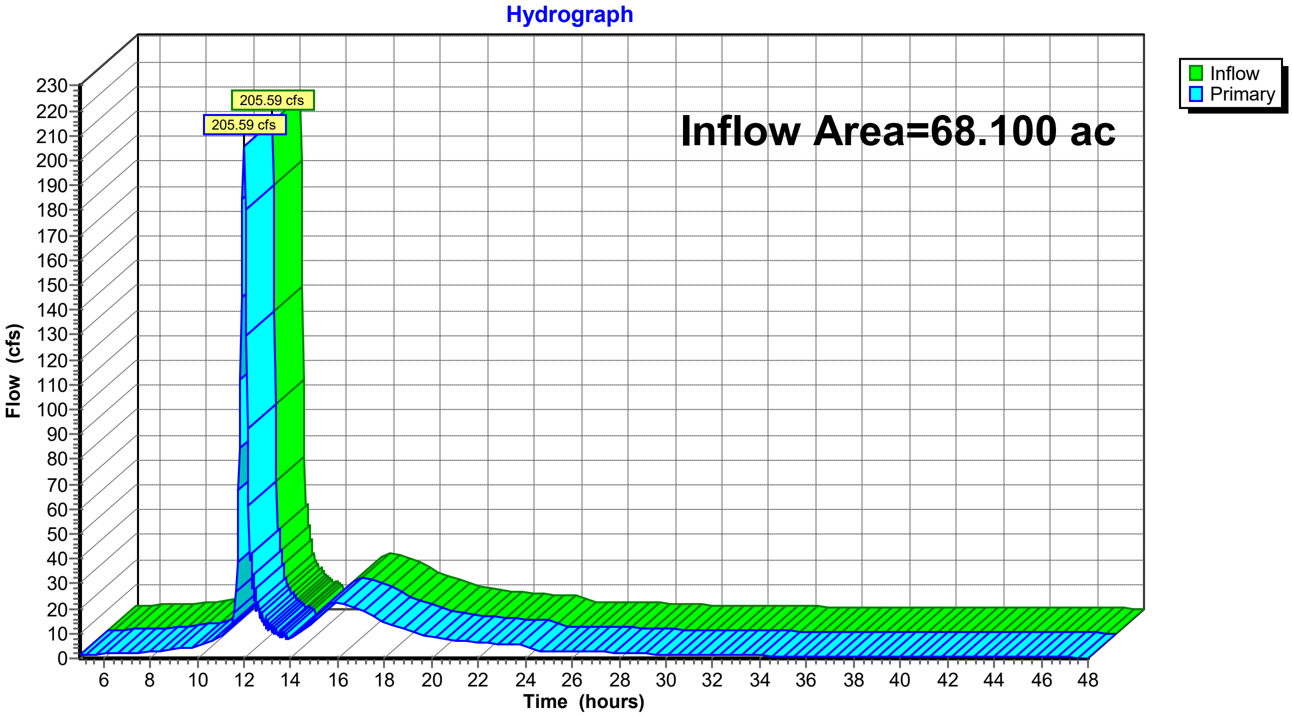


Summary for Pond AP-1: Analysis Point #1

Inflow Area = 68.100 ac, 20.36% Impervious, Inflow Depth > 3.95" for 100-Year event
Inflow = 205.59 cfs @ 12.00 hrs, Volume= 22.396 af
Primary = 205.59 cfs @ 12.00 hrs, Volume= 22.396 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

Pond AP-1: Analysis Point #1

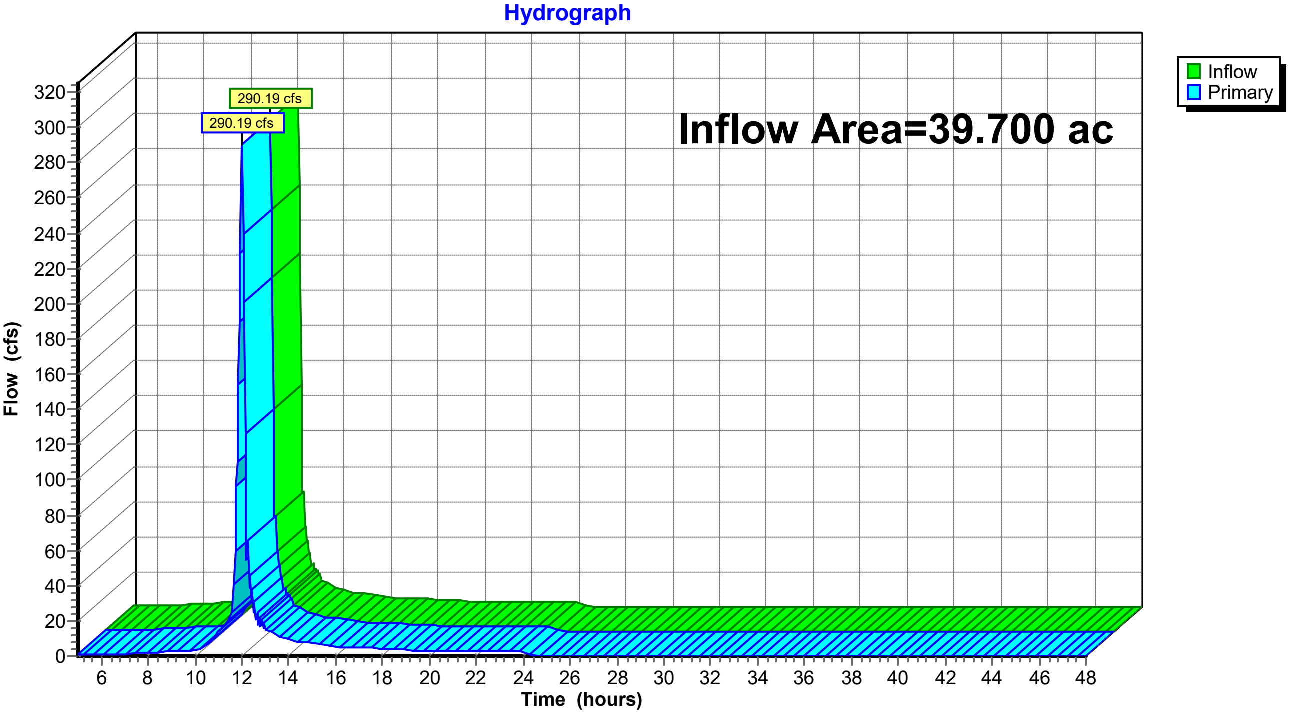


Summary for Pond AP-2: Analysis Point #2

Inflow Area = 39.700 ac, 23.10% Impervious, Inflow Depth > 4.88" for 100-Year event
Inflow = 290.19 cfs @ 12.00 hrs, Volume= 16.144 af
Primary = 290.19 cfs @ 12.00 hrs, Volume= 16.144 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

Pond AP-2: Analysis Point #2

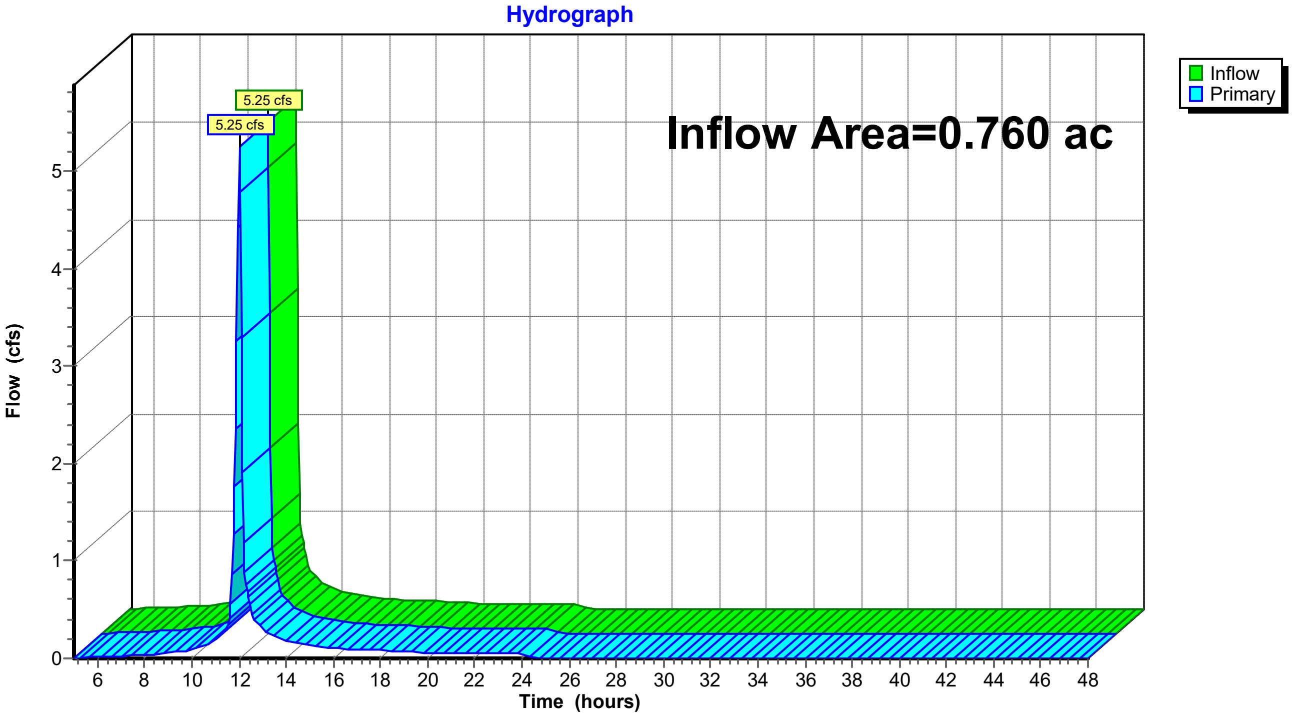


Summary for Pond AP-3: Analysis Point #3

Inflow Area = 0.760 ac, 78.95% Impervious, Inflow Depth > 4.51" for 100-Year event
Inflow = 5.25 cfs @ 12.01 hrs, Volume= 0.286 af
Primary = 5.25 cfs @ 12.01 hrs, Volume= 0.286 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs

Pond AP-3: Analysis Point #3



Summary for Pond C-1: Chamber Series 1

Inflow Area = 10.300 ac, 66.70% Impervious, Inflow Depth > 5.60" for 100-Year event
Inflow = 84.97 cfs @ 11.98 hrs, Volume= 4.805 af
Outflow = 95.05 cfs @ 12.00 hrs, Volume= 4.444 af, Atten= 0%, Lag= 1.4 min
Discarded = 0.29 cfs @ 12.00 hrs, Volume= 0.351 af
Primary = 60.22 cfs @ 12.00 hrs, Volume= 3.951 af
Secondary = 34.55 cfs @ 12.00 hrs, Volume= 0.143 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 14.63' @ 12.00 hrs Surf.Area= 24,954 sf Storage= 35,013 cf

Plug-Flow detention time= 141.1 min calculated for 4.438 af (92% of inflow)
Center-of-Mass det. time= 101.1 min (864.7 - 763.5)

Volume	Invert	Avail.Storage	Storage Description
#1B	6.00'	11,722 cf	**37.08'W x 227.97'L x 5.50'H Field B** 46,496 cf Overall - 17,192 cf Embedded = 29,305 cf x 40.0% Voids
#2B	6.75'	17,192 cf	**ADS_StormTech MC-3500 d +Cap** x 155 Inside #1 Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap 155 Chambers in 5 Rows Cap Storage= +14.9 cf x 2 x 5 rows = 149.0 cf
#3	13.25'	6,100 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)
		35,013 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
13.25	600	0	0
13.75	4,900	1,375	1,375
14.00	8,200	1,638	3,013
14.25	16,500	3,088	6,100

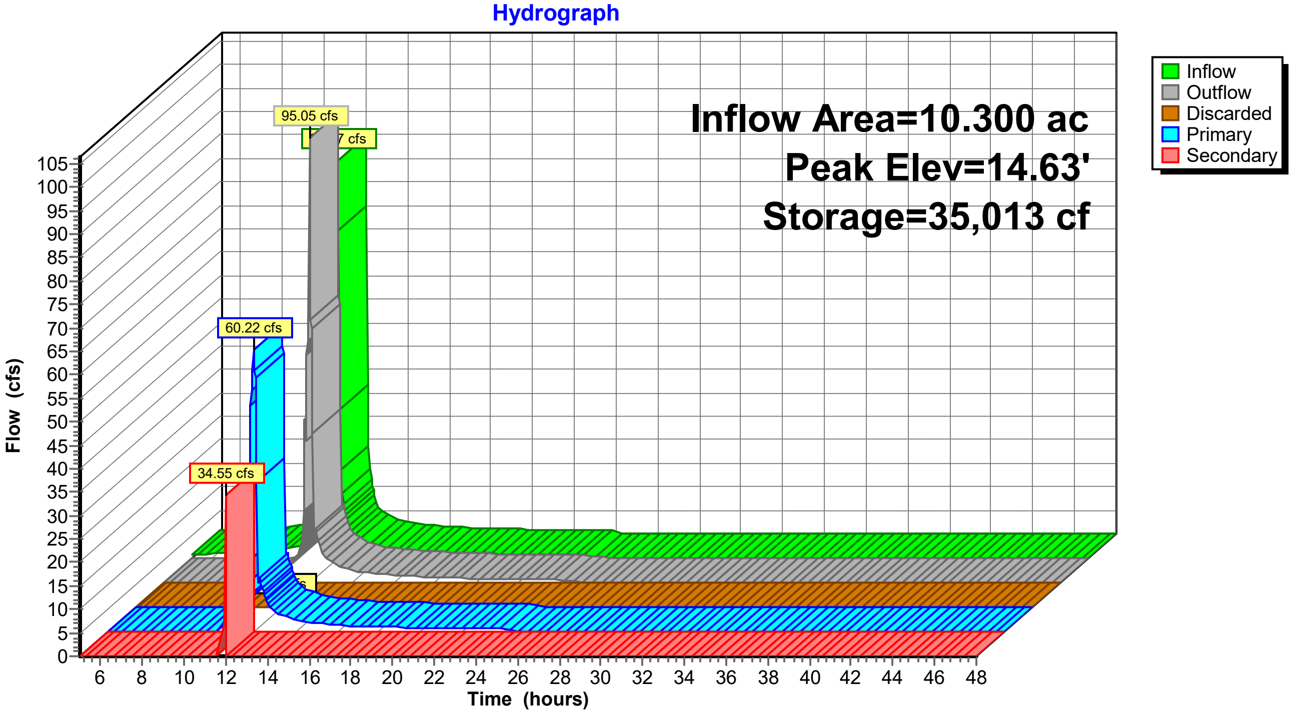
Device	Routing	Invert	Outlet Devices
#1	Discarded	6.00'	**0.500 in/hr Exfiltration over Surface area**
#2	Primary	10.00'	**36.0" Round Culvert** L= 55.9' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 10.00' / 7.59' S= 0.0431 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf
#3	Secondary	14.24'	**50.0' long x 0.7' breadth Concrete Curb** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 Coef. (English) 2.76 2.82 2.93 3.09 3.18 3.22 3.27 3.30 3.32 3.31 3.32

Discarded OutFlow Max=0.29 cfs @ 12.00 hrs HW=14.61' (Free Discharge)
↑—**1=Exfiltration** (Exfiltration Controls 0.29 cfs)

Primary OutFlow Max=60.19 cfs @ 12.00 hrs HW=14.63' (Free Discharge)
↑—**2=Culvert** (Inlet Controls 60.19 cfs @ 8.52 fps)

Secondary OutFlow Max=34.34 cfs @ 12.00 hrs HW=14.63' (Free Discharge)
↑—**3=Concrete Curb** (Weir Controls 34.34 cfs @ 1.76 fps)

Pond C-1: Chamber Series 1



Summary for Pond C-4: Chamber Series 4

```
Inflow Area =        8.900 ac,    0.00% Impervious,  Inflow Depth >   5.34"    for  100-Year event
Inflow      =      69.96 cfs @   11.99 hrs,  Volume=          3.959 af
Outflow     =      56.32 cfs @   12.04 hrs,  Volume=          3.628 af,  Atten= 20%,  Lag= 3.3 min
Discarded  =        0.19 cfs @   12.03 hrs,  Volume=          0.264 af
Primary     =      56.13 cfs @   12.04 hrs,  Volume=          3.364 af
Secondary =        0.00 cfs @     5.00 hrs,  Volume=          0.000 af
```

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 15.22' @ 12.04 hrs Surf.Area= 16,391 sf Storage= 26,993 cf

Plug-Flow detention time= 137.4 min calculated for 3.627 af (92% of inflow)
Center-of-Mass det. time= 92.3 min (865.1 - 772.9)

Volume	Invert	Avail.Storage	Storage Description
#1B	6.00'	8,911 cf	**29.92'W x 213.63'L x 5.50'H Field B** 35,151 cf Overall - 12,874 cf Embedded = 22,277 cf x 40.0% Voids
#2B	6.75'	12,874 cf	**ADS_StormTech MC-3500 d +Cap** x 116 Inside #1 Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap 116 Chambers in 4 Rows Cap Storage= +14.9 cf x 2 x 4 rows = 119.2 cf
#3	14.10'	8,015 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)
		29,800 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
14.10	400	0	0
14.60	2,400	700	700
14.80	6,300	870	1,570
15.10	10,000	2,445	4,015
15.50	10,000	4,000	8,015

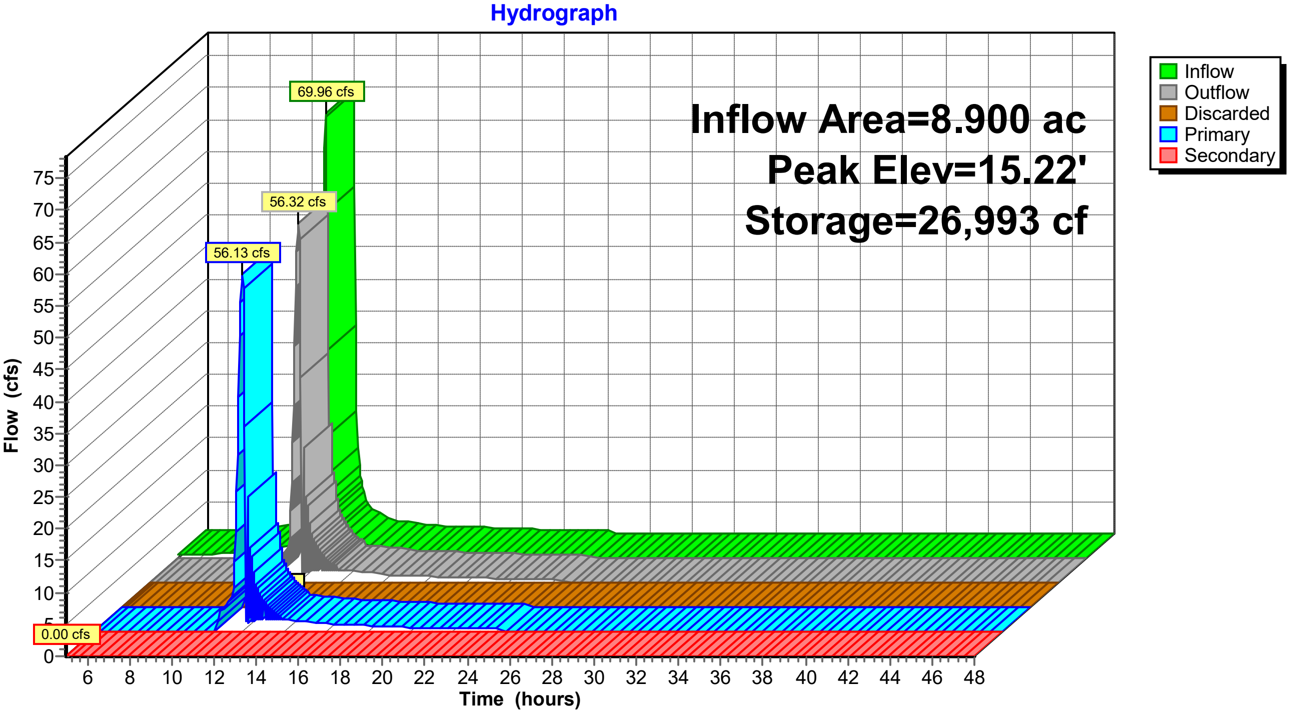
Device	Routing	Invert	Outlet Devices
#1	Discarded	6.00'	**0.500 in/hr Exfiltration over Surface area**
#2	Primary	11.00'	**36.0" Round Culvert** L= 24.3' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 11.00' / 9.42' S= 0.0650 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 7.07 sf
#3	Secondary	15.50'	**100.0' long x 0.5' breadth Wharf** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.19 cfs @ 12.03 hrs HW=15.16' (Free Discharge)
↑—**1=Exfiltration** (Exfiltration Controls 0.19 cfs)

Primary OutFlow Max=55.90 cfs @ 12.04 hrs HW=15.20' (Free Discharge)
↑—**2=Culvert** (Inlet Controls 55.90 cfs @ 7.91 fps)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=6.01' (Free Discharge)
↑—**3=Wharf** (Controls 0.00 cfs)

Pond C-4: Chamber Series 4



Summary for Pond C-5: Chamber Series 5

Inflow Area = 5.200 ac, 0.00% Impervious, Inflow Depth > 5.34" for 100-Year event
Inflow = 41.34 cfs @ 11.98 hrs, Volume= 2.313 af
Outflow = 30.89 cfs @ 12.05 hrs, Volume= 2.057 af, Atten= 25%, Lag= 3.9 min
Discarded = 0.42 cfs @ 12.05 hrs, Volume= 0.401 af
Primary = 30.47 cfs @ 12.05 hrs, Volume= 1.656 af
Secondary = 0.00 cfs @ 5.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 5.00-48.00 hrs, dt= 0.05 hrs
Peak Elev= 15.35' @ 12.05 hrs Surf.Area= 18,202 sf Storage= 26,138 cf

Plug-Flow detention time= 242.1 min calculated for 2.054 af (89% of inflow)
Center-of-Mass det. time= 188.6 min (961.1 - 772.5)

Volume	Invert	Avail.Storage	Storage Description
#1B	6.00'	7,757 cf	**28.50'W x 168.47'L x 6.75'H Field B** 32,409 cf Overall - 13,016 cf Embedded = 19,393 cf x 40.0% Voids
#2B	6.75'	13,016 cf	**ADS_StormTech MC-4500 b +Cap** x 120 Inside #1 Effective Size= 90.4"W x 60.0"H => 26.46 sf x 4.03'L = 106.5 cf Overall Size= 100.0"W x 60.0"H x 4.33'L with 0.31' Overlap 120 Chambers in 3 Rows Cap Storage= +39.5 cf x 2 x 3 rows = 237.0 cf
#3	14.60'	7,420 cf	**Custom Stage Data (Prismatic)** Listed below (Recalc)
		28,193 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
14.60	200	0	0
14.80	1,000	120	120
15.00	10,000	1,100	1,220
15.50	14,800	6,200	7,420

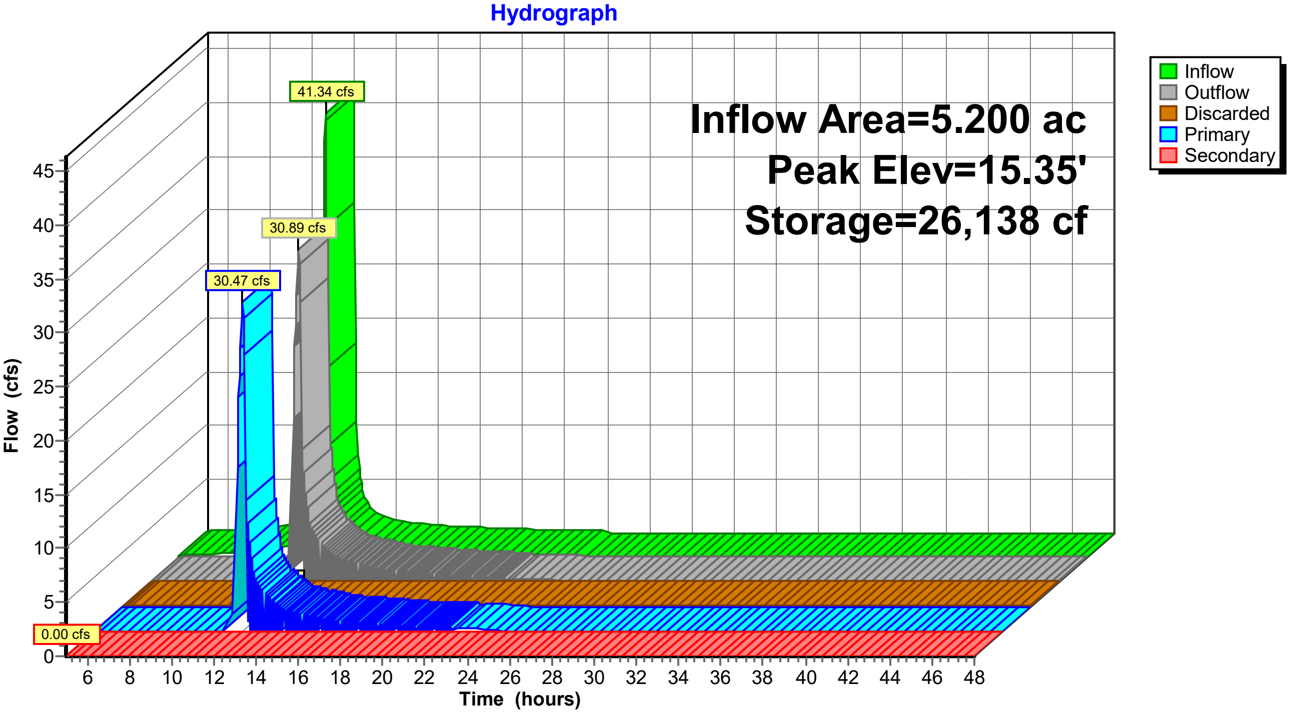
Device	Routing	Invert	Outlet Devices
#1	Discarded	6.00'	**1.000 in/hr Exfiltration over Surface area**
#2	Primary	12.60'	**36.0" Round Culvert** L= 62.8' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 12.60' / 12.29' S= 0.0049 '/' Cc= 0.900 n= 0.013 Cast iron, coated, Flow Area= 7.07 sf
#3	Secondary	15.50'	**100.0' long x 0.5' breadth Wharf** Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Discarded OutFlow Max=0.42 cfs @ 12.05 hrs HW=15.35' (Free Discharge)
↑—**1=Exfiltration** (Exfiltration Controls 0.42 cfs)

Primary OutFlow Max=30.43 cfs @ 12.05 hrs HW=15.35' (Free Discharge)
↑—**2=Culvert** (Barrel Controls 30.43 cfs @ 5.87 fps)

Secondary OutFlow Max=0.00 cfs @ 5.00 hrs HW=6.01' (Free Discharge)
↑—**3=Wharf** (Controls 0.00 cfs)

Pond C-5: Chamber Series 5



Appendix C

Water Quality and Runoff Reduction Volume Calculations

Version 1.8

Last Updated: 11/09/2015

Total Water Quality Volume Calculation

WQv(acre-feet) = [(P)(Rv)(A)] /12

Is this project subject to Chapter 10 of the NYS Design Manual (i.e. WQv is equal to post-development 1 year runoff volume)?... No

Design Point:		
P=	1.20	*inch*

Manually enter P, Total Area and Impervious Cover.

Breakdown of Subcatchments

Catchment Number	Total Area *(Acres)*	Impervious Area *(Acres)*	Percent Impervious *%*	Rv	WQv *(ft^3)*	Description
1	10.30	10.20	99%	0.94	42,055	
2	5.50	5.30	96%	0.92	21,884	
3	11.00	10.60	96%	0.92	43,769	
4	8.90	8.60	97%	0.92	35,505	
5	5.20	4.90	94%	0.90	20,258	
6	11.80	11.60	98%	0.93	47,846	
7	8.70	8.40	97%	0.92	34,681	
8	2.90	2.00	69%	0.67	8,437	
9	5.80	3.10	53%	0.53	13,361	
10	0.00	0.00				
Subtotal (1-30)	76.70	65.89	86%	0.82	**273,874**	**Subtotal 1**
Total	76.70	65.89	86%	0.82	**273,874**	**Initial WQv**

Identify Runoff Reduction Techniques By Area

Technique	Total Contributing Area *(Acre)*	Contributing Impervious Area *(Acre)*	Notes
Conservation of Natural Areas	4.00	0.00	*minimum 10,000 sf*
Riparian Buffers	0.00	0.00	*maximum contributing length 75 feet to 150 feet*
Filter Strips	0.00	0.00	
Tree Planting	0.00	0.00	*Up to 100 sf directly connected impervious area may be subtracted per tree*
Total	**4.00**	**0.00**	

Recalculate WQv after application of Area Reduction Techniques

	Total Area *(Acres)*	Impervious Area *(Acres)*	Percent Impervious *%*	Runoff Coefficient Rv	WQv *(ft^3)*
"<<Initial WQv"	76.70	65.89	86%	0.82	273,874
Subtract Area	-4.00	0.00			
WQv adjusted after Area Reductions	**72.70**	**65.89**	91%	0.87	273,007
Disconnection of Rooftops		0.00			
Adjusted WQv after Area Reduction and Rooftop Disconnect	72.70	65.89	91%	0.87	**273,007**
WQv reduced by Area Reduction techniques					868

Total Water Quality Volume Calculation

WQv(acre-feet) = [(P)(Rv)(A)] /12

Additional Subcatchments						
Catchment Number	**Total Area** *(Acres)*	**Impervious Area** *(Acres)*	**Percent Impervious** *%*	**Rv**	**WQv** *(ft^3)*	**Description**
11	4.00	0.00	0%	0.05	868	Conservation of Natural Areas
12	0.00	0.00				
13	1.10	0.45	41%	0.42	1,995	Infiltration Basin
14	1.10	0.55	50%	0.50	2,386	Infiltration Basin
15	0.10	0.05	50%	0.50	217	Dry Swale
16	0.00	0.00				
17	0.30	0.14	47%	0.47	612	Dry Swale
18						
19						
20						
21						
22						
23						
24						
25						
26						
27						
28						
29						
30						
Subtotal	6.60	1.19	18%	0.21	**6,077**	**Subtotal**

Total Water Quality Volume Calculation
WQv(acre-feet) = [(P)(Rv)(A)] /12

All Subcatchments						
Catchment	**Total Area** *(Acres)*	**Impervious Cover** *(Acres)*	**Percent Impervious** *%*	**Runoff Coefficient** *Rv*	**WQv** *(ft^3)*	**Description**
1	10.30	10.20	0.99	0.94	42055.46	
2	5.50	5.30	0.96	0.92	21,884	
3	11.00	10.60	0.96	0.92	43768.91	
4	8.90	8.60	0.97	0.92	35505.30	
5	5.20	4.90	0.94	0.90	20257.76	
6	11.80	11.60	0.98	0.93	47846.49	
7	8.70	8.40	0.97	0.92	34681.11	
8	2.90	2.00	0.69	0.67	8437.12	
9	5.80	3.10	0.53	0.53	13360.58	
10	0.00	0.00				
11	4.00	0.00	0.00	0.05	867.57	Conservation of Natural Areas
12	0.00	0.00				
13	1.10	0.45	0.41	0.42	1995.41	Infiltration Basin
14	1.10	0.55	0.50	0.50	2385.82	Infiltration Basin
15	0.10	0.05	0.50	0.50	216.89	Dry Swale
16	0.00	0.00				
17	0.30	0.14	0.47	0.47	611.64	Dry Swale
18						
19						
20						
21						
22						
23						
24						
25						
26						
27						
28						
29						
30						

Runoff Reduction Volume and Treated volumes

	Runoff Reduction Techiques/Standard SMPs		Total Contributing Area	Total Contributing Impervious Area	WQv Reduced (RRv)	WQv Treated
			(acres)	(acres)	cf	cf
Area/Volume Reduction	Conservation of Natural Areas	RR-1	4.00	0.00		
	Sheetflow to Riparian Buffers/Filter Strips	RR-2	0.00	0.00		
	Tree Planting/Tree Pit	RR-3	0.00	0.00		
	Disconnection of Rooftop Runoff	RR-4		0.00		
	Vegetated Swale	RR-5	0.00	0.00	0	
	Rain Garden	RR-6	0.00	0.00	0	
	Stormwater Planter	RR-7	0.00	0.00	0	
	Rain Barrel/Cistern	RR-8	0.00	0.00	0	
	Porous Pavement	RR-9	0.00	0.00	0	
	Green Roof (Intensive & Extensive)	RR-10	0.00	0.00	0	
Standard SMPs w/RRv Capacity	Infiltration Trench	I-1	0.00	0.00	0	0
	Infiltration Basin	I-2	26.60	24.70	62378	7042
	Dry Well	I-3	0.00	0.00	0	0
	Underground Infiltration System	I-4				
	Bioretention & Infiltration Bioretention	F-5	0.00	0.00	0	0
	Dry swale	O-1	0.40	0.19	87	766
Standard SMPs	Micropool Extended Detention (P-1)	P-1				21798.000
	Wet Pond (P-2)	P-2				
	Wet Extended Detention (P-3)	P-3				
	Multiple Pond system (P-4)	P-4				
	Pocket Pond (p-5)	P-5				
	Surface Sand filter (F-1)	F-1				
	Underground Sand filter (F-2)	F-2				
	Perimeter Sand Filter (F-3)	F-3				
	Organic Filter (F-4)	F-4				181790.000
	Shallow Wetland (W-1)	W-1				
	Extended Detention Wetland (W-2	W-2				
	Pond/Wetland System (W-3)	W-3				
	Pocket Wetland (W-4)	W-4				
	Wet Swale (O-2)	O-2				
Totals by Area Reduction		→	4.00	0.00	868	
Totals by Volume Reduction		→	0.00	0.00	0	
Totals by Standard SMP w/RRV		→	27.00	24.89	62466	7808
Totals by Standard SMP		→	0.00	0.00		203588
Totals (Area + Volume + all SMPs)		→	31.00	24.89	63,333	211,396
	Impervious Cover √	error				

Minimum RRv

Enter the Soils Data for the site		
Soil Group	Acres	S
A	2.30	55%
B		40%
C		30%
D	**70.40**	20%
Total Area	72.7	
Calculate the Minimum RRv		
S =	**0.21**	
Impervious =	65.89	*acre*
Precipitation	1.195	*in*
Rv	0.95	
Minimum RRv	**57,313**	***ft3***
	1.32	af

Planning

Practice	Description	Application
Preservation of Undisturbed Areas	Delineate and place into permanent conservation undisturbed forests, native vegetated areas, riparian corridors, wetlands, and natural terrain.	Considered & Applied
Preservation of Buffers	Define, delineate and preserve naturally vegetated buffers along perennial streams, rivers, shorelines and wetlands.	Considered & Not Applied
Reduction of Clearing and Grading	Limit clearing and grading to the minimum amount needed for roads, driveways, foundations, utilities and stormwater management facilities.	Considered & Applied
Locating Development in Less Sensitive Areas	Avoid sensitive resource areas such as floodplains, steep slopes, erodible soils, wetlands, mature forests and critical habitats by locating development to fit the terrain in areas that will create the least impact.	Considered & Applied
Open Space Design	Use clustering, conservation design or open space design to reduce impervious cover, preserve more open space and protect water resources.	N/A
Soil Restoration	Restore the original properties and porosity of the soil by deep till and amendment with compost to reduce the generation of runoff and enhance the runoff reduction performance of post construction practices.	N/A
Roadway Reduction	Minimize roadway widths and lengths to reduce site impervious area	Considered & Applied
Sidewalk Reduction	Minimize sidewalk lengths and widths to reduce site impervious area	Considered & Applied
Driveway Reduction	Minimize driveway lengths and widths to reduce site impervious area	N/A
Cul-de-sac Reduction	Minimize the number of cul-de-sacs and incorporate landscaped areas to reduce their impervious cover.	N/A
Building Footprint Reduction	Reduce the impervious footprint of residences and commercial buildings by using alternate or taller buildings while maintaining the same floor to area ratio.	Considered & Applied
Parking Reduction	Reduce imperviousness on parking lots by eliminating unneeded spaces, providing compact car spaces and efficient parking lanes, minimizing stall dimensions, using porous pavement surfaces in overflow parking areas, and using multi-storied parking decks where appropriate.	Considered & Applied

NOI QUESTIONS

#	**NOI Question**	**Reported Value**	
		cf	af
28	Total Water Quality Volume (WQv) Required	273874	*6.287*
30	Total RRV Provided	63333	*1.454*
31	Is RRv Provided ≥WQv Required?	No	
32	Minimum RRv	57313	*1.316*
32a	Is RRv Provided ≥ Minimum RRv Required?	Yes	
33a	Total WQv Treated	211396	*4.853*
34	Sum of Volume Reduced & Treated	274729	*6.307*
34	Sum of Volume Reduced and Treated	274729	*6.307*
35	Is Sum RRv Provided and WQv Provided ≥WQv Required?	Yes	

Apply Peak Flow Attenuation			
36	Channel Protection	*Cpv*	
37	Overbank	*Qp*	
37	Extreme Flood Control	*Qf*	
	Are Quantity Control requirements met?	*Yes*	Plan Completed

Infiltration Basin Worksheet

Design Point:

Enter Site Data For Drainage Area to be Treated by Practice

Catchment Number	**Total Area** *(Acres)*	**Impervious Area** *(Acres)*	**Percent Impervious** *%*	**Rv**	**WQv** *(ft³)*	**Precipitation** *(in)*	**Description**
1	10.30	10.20	0.99	0.94	42055.46	1.20	
Enter Impervious Area Reduced by Disconnection of Rooftops		0.00	99%	0.94	42,055	<<WQv after adjusting for Disconnected Rooftops	
Enter the portion of the WQv that is not reduced for all practices routed to this practice.					0	*ft³*	

Pretreatment Techniques to Prevent Clogging

Infiltration Rate	1.50	*in/hour*	*Okay*
Pretreatment Sizing	25	*% WQv*	*25% minimum;* *50% if >2 in/hr* *100% if >5in/hour*
Pretreatment Required Volume	10,514	*ft³*	
Pretreatment Provided	11,000	*ft³*	
Pretreatment Techniques utilized	*Other*		

Size An Infiltration Basin

Design Volume	42,055	*ft³*	WQv
Basal Area Required	42,055	*ft²*	*Infiltration practices shall be designed to exfiltrate the entire WQv through the floor of each practice.*
Basal Area Provided	23,609	*ft²*	
Design Depth	1.00	*ft*	
Volume Provided	23,609	*ft³*	*Storage Volume provided in infiltration basin area (not including pretreatment.*

Determine Runoff Reduction

RRv	**21,248**	***ft³***	***90% of the storage provided in the basin or WQv whichever is smaller***
Volume Treated	20,807	*ft³*	*This is the portion of the WQv that is not reduced/infiltrated*
Sizing √			*The infiltration basin must provide storage equal to or greater than the WQv of the contributing area.*

Infiltration Basin Worksheet

Design Point:	

Enter Site Data For Drainage Area to be Treated by Practice							
Catchment Number	**Total Area** *(Acres)*	**Impervious Area** *(Acres)*	**Percent Impervious** *%*	**Rv**	**WQv** *(ft^3)*	**Precipitation** *(in)*	**Description**
4	8.90	8.60	0.97	0.92	35505.30	1.20	
Enter Impervious Area Reduced by Disconnection of Rooftops		0.00	97%	0.92	35,505	<<WQv after adjusting for Disconnected Rooftops	
Enter the portion of the WQv that is not reduced for all practices routed to this practice.					0	*ft^3*	

Pretreatment Techniques to Prevent Clogging			
Infiltration Rate	0.80	*in/hour*	*Okay*
Pretreatment Sizing	25	*% WQv*	*25% minimum;* *50% if >2 in/hr* *100% if >5in/hour*
Pretreatment Required Volume	8,876	*ft^3*	
Pretreatment Provided	10,000	*ft^3*	
Pretreatment Techniques utilized	*Other*		

Size An Infiltration Basin			
Design Volume	35,505	*ft^3*	WQv
Basal Area Required	35,505	*ft^2*	*Infiltration practices shall be designed to exfiltrate the entire WQv through the floor of each practice.*
Basal Area Provided	20,516	*ft^2*	
Design Depth	1.00	*ft*	
Volume Provided	20,516	*ft^3*	*Storage Volume provided in infiltration basin area (not including pretreatment.*

Determine Runoff Reduction			
RRv	**18,464**	***ft^3***	***90% of the storage provided in the basin or WQv whichever is smaller***
Volume Treated	17,041	*ft^3*	*This is the portion of the WQv that is not reduced/infiltrated*
Sizing √			*The infiltration basin must provide storage equal to or greater than the WQv of the contributing area.*

Infiltration Basin Worksheet

Design Point:	

Enter Site Data For Drainage Area to be Treated by Practice

Catchment Number	**Total Area** *(Acres)*	**Impervious Area** *(Acres)*	**Percent Impervious** *%*	**Rv**	**WQv** *(ft^3)*	**Precipitation** *(in)*	**Description**
5	5.20	4.90	0.94	0.90	20257.76	1.20	
Enter Impervious Area Reduced by Disconnection of Rooftops			94%	0.90	20,258	<<WQv after adjusting for Disconnected Rooftops	
Enter the portion of the WQv that is not reduced for all practices routed to this practice.						*ft^3*	

Pretreatment Techniques to Prevent Clogging

Infiltration Rate	0.50	*in/hour*	*Okay*
Pretreatment Sizing	25	*% WQv*	*25% minimum;* *50% if >2 in/hr* *100% if >5in/hour*
Pretreatment Required Volume	5,064	*ft^3*	
Pretreatment Provided	5,100	*ft^3*	
Pretreatment Techniques utilized	*Other*		

Size An Infiltration Basin

Design Volume	20,258	*ft^3*	WQv
Basal Area Required	20,258	*ft^2*	*Infiltration practices shall be designed to exfiltrate the entire WQv through the floor of each practice.*
Basal Area Provided	20,473	*ft^2*	
Design Depth	1.00	*ft*	
Volume Provided	20,473	*ft^3*	*Storage Volume provided in infiltration basin area (not including pretreatment.*

Determine Runoff Reduction

RRv	**18,426**	***ft^3***	***90% of the storage provided in the basin or WQv whichever is smaller***
Volume Treated	1,832	*ft^3*	*This is the portion of the WQv that is not reduced/infiltrated*
Sizing √	OK		*The infiltration basin must provide storage equal to or greater than the WQv of the contributing area.*

Infiltration Basin Worksheet

Design Point:	

Enter Site Data For Drainage Area to be Treated by Practice

Catchment Number	**Total Area** *(Acres)*	**Impervious Area** *(Acres)*	**Percent Impervious** *%*	**Rv**	**WQv** *(ft^3)*	**Precipitation** *(in)*	**Description**
13	1.10	0.45	0.41	0.42	1995.41	1.20	Infiltration Basin
Enter Impervious Area Reduced by Disconnection of Rooftops			41%	0.42	1,995	<<WQv after adjusting for Disconnected Rooftops	
Enter the portion of the WQv that is not reduced for all practices routed to this practice.						*ft^3*	

Pretreatment Techniques to Prevent Clogging

Infiltration Rate	4.50	*in/hour*	*Okay*
Pretreatment Sizing	50	*% WQv*	*25% minimum;* *50% if >2 in/hr* *100% if >5in/hour*
Pretreatment Required Volume	998	*ft^3*	
Pretreatment Provided	1,058	*ft^3*	
Pretreatment Techniques utilized	*Sedimentation Basin*		

Size An Infiltration Basin

Design Volume	1,995	*ft^3*	WQv
Basal Area Required	512	*ft^2*	*Infiltration practices shall be designed to exfiltrate the entire WQv through the floor of each practice.*
Basal Area Provided	639	*ft^2*	
Design Depth	3.90	*ft*	
Volume Provided	2,492	*ft^3*	*Storage Volume provided in infiltration basin area (not including pretreatment.*

Determine Runoff Reduction

RRv	**1,995**	***ft^3***	***90% of the storage provided in the basin or WQv whichever is smaller***
Volume Treated	0	*ft^3*	*This is the portion of the WQv that is not reduced/infiltrated*
Sizing √	OK		*The infiltration basin must provide storage equal to or greater than the WQv of the contributing area.*

Infiltration Basin Worksheet

Design Point:							
Enter Site Data For Drainage Area to be Treated by Practice							
Catchment Number	**Total Area** *(Acres)*	**Impervious Area** *(Acres)*	**Percent Impervious** *%*	**Rv**	**WQv** *(ft^3)*	**Precipitation** *(in)*	**Description**
14	1.10	0.55	0.50	0.50	2385.82	1.20	Infiltration Basin
Enter Impervious Area Reduced by Disconnection of Rooftops		0.00	50%	0.50	2,386	<<WQv after adjusting for Disconnected Rooftops	
Enter the portion of the WQv that is not reduced for all practices routed to this practice.					0	*ft^3*	

Pretreatment Techniques to Prevent Clogging			
Infiltration Rate	12.00	*in/hour*	*Okay*
Pretreatment Sizing	100	*% WQv*	*25% minimum;* *50% if >2 in/hr* *100% if >5in/hour*
Pretreatment Required Volume	2,386	*ft^3*	
Pretreatment Provided	2,389	*ft^3*	
Pretreatment Techniques utilized	*Sedimentation Basin*		

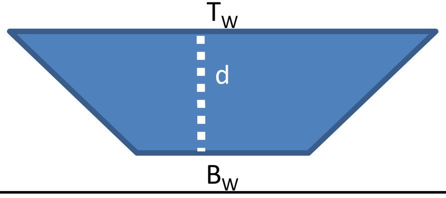
Size An Infiltration Basin			
Design Volume	2,386	*ft^3*	WQv
Basal Area Required	612	*ft^2*	*Infiltration practices shall be designed to exfiltrate the entire WQv through the floor of each practice.*
Basal Area Provided	640	*ft^2*	
Design Depth	3.90	*ft*	
Volume Provided	2,494	*ft^3*	*Storage Volume provided in infiltration basin area (not including pretreatment.*
Determine Runoff Reduction			
RRv	**2,245**	***ft^3***	***90% of the storage provided in the basin or WQv whichever is smaller***
Volume Treated	141	*ft^3*	*This is the portion of the WQv that is not reduced/infiltrated*
Sizing √	OK		*The infiltration basin must provide storage equal to or greater than the WQv of the contributing area.*

Dry Swale Worksheet

Design Point:

Enter Site Data For Drainage Area to be Treated by Practice							
Catchment Number	**Total Area** *(Acres)*	**Impervious Area** *(Acres)*	**Percent Impervious** *%*	**Rv**	**WQv** *(ft^3)*	**Precipitation** *(in)*	**Description**
15	0.10	0.05	0.50	0.50	216.89	1.20	Dry Swale
Enter Impervious Area Reduced by Disconnection of Rooftops		0.00	50%	0.50	217	<<WQv after adjusting for Disconnected Rooftops	

Pretreatment Provided			**Pretreatment Technique**
Pretreatment (10% of WQv)	22	*ft^3*	

Calculate Available Storage Capacity			
Bottom Width	1	*ft*	Design with a bottom width no greater than eight feet to avoid potential gullying and channel braiding, but no less than two feet
Side Slope (X:1)	2	*Okay*	Channels shall be designed with moderate side slopes (flatter than 3:1) for most conditions. 2:1 is the absolute maximum side slope
Longitudinal Slope	0%	*Okay*	*Maximum longitudinal slope shall be 4%*
Flow Depth	0.75	*ft*	*Maximum ponding depth of one foot at the mid-point of the channel, and a maximum depth of 18" at the end point of the channel (for storage of the WQv)*
Top Width	4	*ft*	
Area	1.88	*sf*	
Minimum Length	104	*ft*	
Actual Length	105	*ft*	
End Point Depth check	1.00	*Okay*	*A maximum depth of 18" at the end point of the channel (for storage of the WQv)*
Storage Capacity	219	*ft^3*	
Soil Group (HSG)		A	

Runoff Reduction			
Is the Dry Swale contributing flow to another practice?	No	Select Practice	
RRv	**87**	***ft^3***	**Runnoff Reduction equals 40% in HSG A and B and 20% in HSG C and D up to the WQv**
Volume Treated	129	*ft^3*	This is the difference between the WQv calculated and the runoff reduction achieved in the swale
Volume Directed	0	*ft^3*	This volume is directed another practice
Volume √	*Okay*		Check to be sure that channel is long enough to store WQv

Dry Swale Worksheet

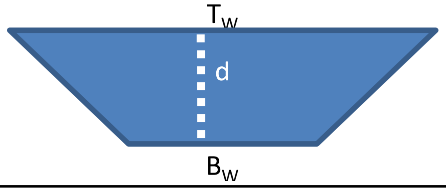
Design Point:	

Enter Site Data For Drainage Area to be Treated by Practice

Catchment Number	**Total Area** *(Acres)*	**Impervious Area** *(Acres)*	**Percent Impervious** *%*	**Rv**	**WQv** *(ft^3)*	**Precipitation** *(in)*	**Description**
17	0.30	0.14	0.47	0.47	611.64	1.20	Dry Swale
Enter Impervious Area Reduced by Disconnection of Rooftops		0.00	47%	0.47	612	<<WQv after adjusting for Disconnected Rooftops	

Pretreatment Provided			**Pretreatment Technique**
Pretreatment (10% of WQv)	61	*ft^3*	

Calculate Available Storage Capacity

Bottom Width	2	*ft*	Design with a bottom width no greater than eight feet to avoid potential gullying and channel braiding, but no less than two feet
Side Slope (X:1)	3	*Okay*	Channels shall be designed with moderate side slopes (flatter than 3:1) for most conditions. 2:1 is the absolute maximum side slope
Longitudinal Slope	3%	*Okay*	*Maximum longitudinal slope shall be 4%*
Flow Depth	1	*ft*	*Maximum ponding depth of one foot at the mid-point of the channel, and a maximum depth of 18" at the end point of the channel (for storage of the WQv)*
Top Width	8	*ft*	
Area	5.00	*sf*	
Minimum Length	110	*ft*	
Actual Length	115	*ft*	
End Point Depth check	1.50	*Okay*	*A maximum depth of 18" at the end point of the channel (for storage of the WQv)*
Storage Capacity	636	*ft^3*	
Soil Group (HSG)		D	

Runoff Reduction

Is the Dry Swale contributing flow to another practice?	No	Select Practice	
RRv	**127**	***ft^3***	**Runnoff Reduction equals 40% in HSG A and B and 20% in HSG C and D up to the WQv**
Volume Treated	484	*ft^3*	This is the difference between the WQv calculated and the runoff reduction achieved in the swale
Volume Directed	0	*ft^3*	This volume is directed another practice
Volume √	*Okay*		Check to be sure that channel is long enough to store WQv

Conservation of Natural Areas

Design Point:

Enter Site Data For Drainage Area to be Treated by Practice

Catchment Number	Total Area *(Acres)*	Impervious Area *(Acres)*	Percent Impervious *%*	Rv	WQv *(ft^3)*	Precipitation *(in)*	Description
11	4.00	0.00	0.00	0.05	867.57	1.20	Conservation of Natural Areas

Design Elements

Is Contiguous Area ≥ 10,000 ft2?	*Yes*
Will limits of disturbance be clearly shown on all construction drawings and marked in field/project development site with structural barriers?	*Yes*
Is the Conservation area located in an acceptable conservation easement instrument that ensures perpetual protection of proposed area?	*Yes*
Does the easement specify how the natural area vegetation will be managed and boundaries will be marked?	*Yes*
Does the conservation area receive runoff from other contributing areas?	*No*
Does Conservation Area drain to a Design Point?	*Yes*
Is Sheet Flow to Riparian Buffer or another area based practice already being Used for this area?	*No*

Are All Criteria in Section 5.3.1 Met? *Yes*

Area Reduction Adjustments

Subtract	***4.00***	***Acres from Total Area***
Subtract	***0.00***	***Acres from Total Impervious Area***

Date:	**1/18/2022**
Project:	**Port of Albany**
Location:	**Albany, NY**
Prepared For:	**Natalie Olivieri**

Purpose: To calculate the water quality flow rate (Qwq) over a given site area. In this situation the WQv to be analyzed is the runoff produced by the first 1.2 inch(es) of rainfall, per Fig 4.1 of the New York State Stormwater Management Design Manual

Reference: United States Department of Agriculture Natural Resources Conservation Service TR-55 Manual, New York State Stormwater Management Design Manual - 2015

Formulas:

$$\mathrm{WQv} = \frac{(P)(R_v)(A)}{12}$$

$$R_v = (0.05+0.009(I))$$

$$CN = 1000/[10+5P+10Qa-10(Qa^2+1.25QaP)^{1/2}]$$

$$Qwq = (q_u)*(A)*(Qa)$$

Structure:	WQU 1		**Structure:**	WQU 2		**Structure:**	WQU 3	
P	**1.20**	in.	P	**1.20**	in.	P	**1.20**	in.
A	10.300	ac	A	5.500	ac	A	11.000	ac
I	99.03	%	I	96.36	%	I	96.36	%
t_c	10.0	min.	t_c	10.0	min.	t_c	10.4	min.
t_c	0.167	hr.	t_c	0.167	hr.	t_c	0.173	hr.
R_v	0.941		R_v	0.917		R_v	0.917	
43% WQv	0.413	ac-ft	90% WQv	0.504	ac-ft	90% WQv	1.009	ac-ft
43% WQv	18003.65	ft^3	90% WQv	21971.66	ft^3	90% WQv	43938.97	ft^3
Qa	1.129	in.	Qa	1.101	in.	Qa	1.100	in.
CN	97.00		CN	94.00		CN	95.00	
I_a	0.062		I_a	0.128		I_a	0.105	
I_a/P	0.052		I_a/P	0.107		I_a/P	0.088	
qu	**1000**	(csm/in)	qu	**1000**	(csm/in)	qu	**1000**	(csm/in)
A	0.01609	$miles^2$	A	0.00859	$miles^2$	A	0.01719	$miles^2$
Qwq	7.81	cfs	Qwq	9.46	cfs	Qwq	18.91	cfs

Date: **1/18/2022**
Project: **Port of Albany**
Location: **Albany, NY**
Prepared For: **Natalie Olivieri**

Purpose: To calculate the water quality flow rate (Qwq) over a given site area. In this situation the WQv to be analyzed is the runoff produced by the first 1.2 inch(es) of rainfall, per Fig 4.1 of the New York State Stormwater Management Design Manual

Reference: United States Department of Agriculture Natural Resources Conservation Service TR-55 Manual, New York State Stormwater Management Design Manual - 2015

Formulas:

$$WQv = \frac{(P)(R_v)(A)}{12}$$

$$R_v = (0.05+0.009(I))$$

$$CN = 1000/[10+5P+10Qa-10(Qa^2+1.25QaP)^{1/2}]$$

$$Qwq = (q_u)*(A)*(Qa)$$

Structure: WQU 4

P	**1.20**	in.
A	8.900	ac
I	96.63	%
t_c	10.0	min.
t_c	0.167	hr.
R_v	0.92	
42% WQv	0.342	ac-ft
42% WQv	14912.10	ft^3
Qa	1.104	in.
CN	94.00	
I_a	0.128	
I_a/P	0.107	
qu	**1000**	(csm/in)
A	0.01391	$miles^2$
Qwq	6.44	cfs

Structure: WQU 6

P	**1.20**	in.
A	11.800	ac
I	98.31	%
t_c	10.0	min.
t_c	0.167	hr.
R_v	0.935	
90% WQv	1.103	ac-ft
90% WQv	48059.75	ft^3
Qa	1.122	in.
CN	96.00	
I_a	0.083	
I_a/P	0.069	
qu	**1000**	(csm/in)
A	0.01844	$miles^2$
Qwq	20.69	cfs

Date: **1/18/2022**
Project: **Port of Albany**
Location: **Albany, NY**
Prepared For: **Natalie Olivieri**

Purpose: To calculate the water quality flow rate (Qwq) over a given site area. In this situation the WQv to be analyzed is the runoff produced by the first 1.2 inch(es) of rainfall, per Fig 4.1 of the New York State Stormwater Management Design Manual

Reference: United States Department of Agriculture Natural Resources Conservation Service TR-55 Manual, New York State Stormwater Management Design Manual - 2015

Formulas:

$$WQv = \frac{(P)(R_v)(A)}{12}$$

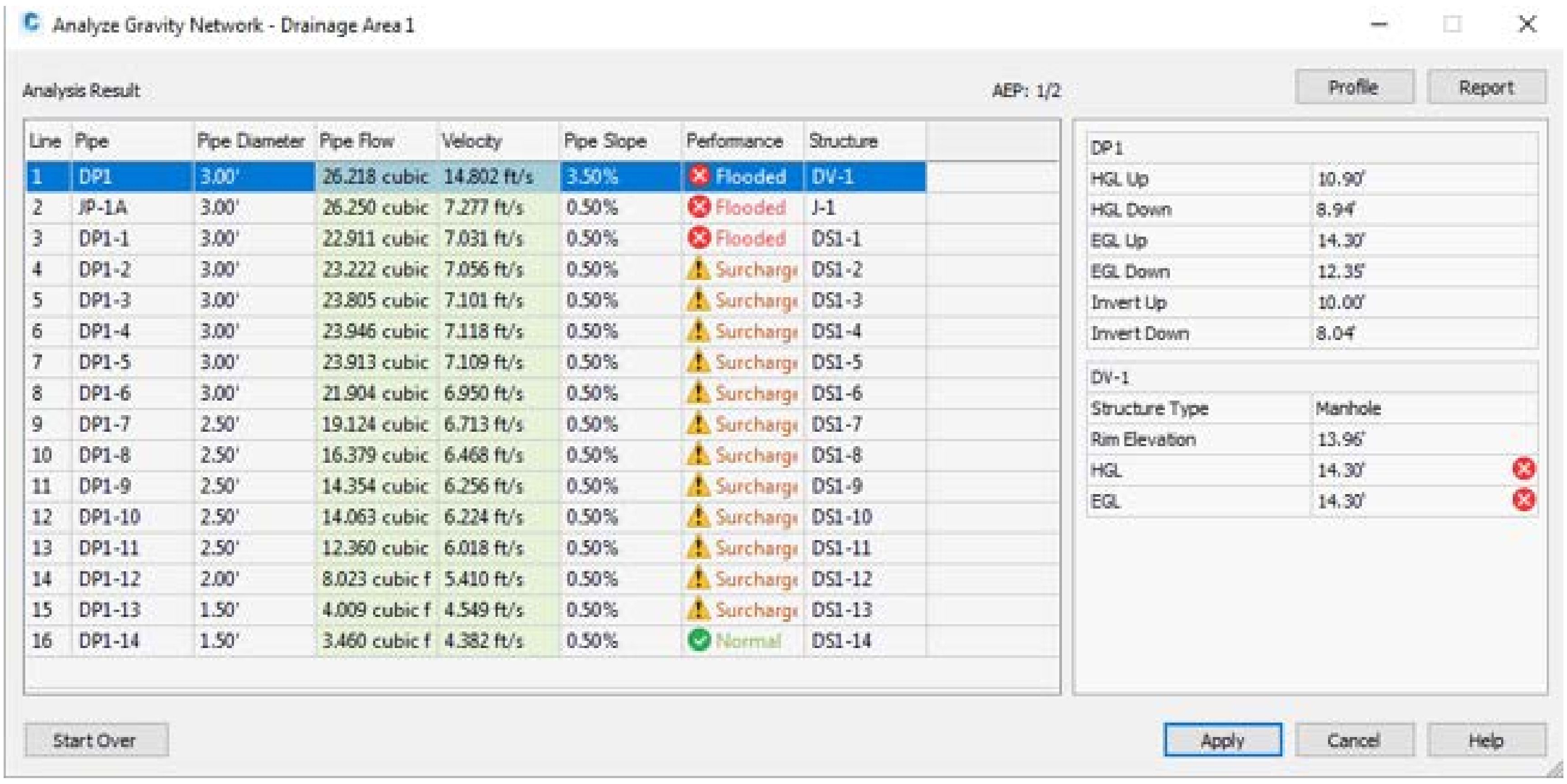
$$R_v = (0.05+0.009(I))$$

$$CN = 1000/[10+5P+10Qa-10(Qa^2+1.25QaP)^{1/2}]$$

$$Qwq = (q_u)*(A)*(Qa)$$

Structure: WQU 7

P	1.20	in.
A	8.700	ac
I	96.55	%
t_c	10.5	min.
t_c	0.175	hr.
R_v	0.919	
90% WQv	0.800	ac-ft
90% WQv	34826.22	ft^3
Qa	1.103	in.
CN	96.00	
I_a	0.083	
I_a/P	0.069	
qu	1000	(csm/in)
A	0.01359	$miles^2$
Qwq	14.99	cfs



Analyze Gravity Network - Drainage Area 1

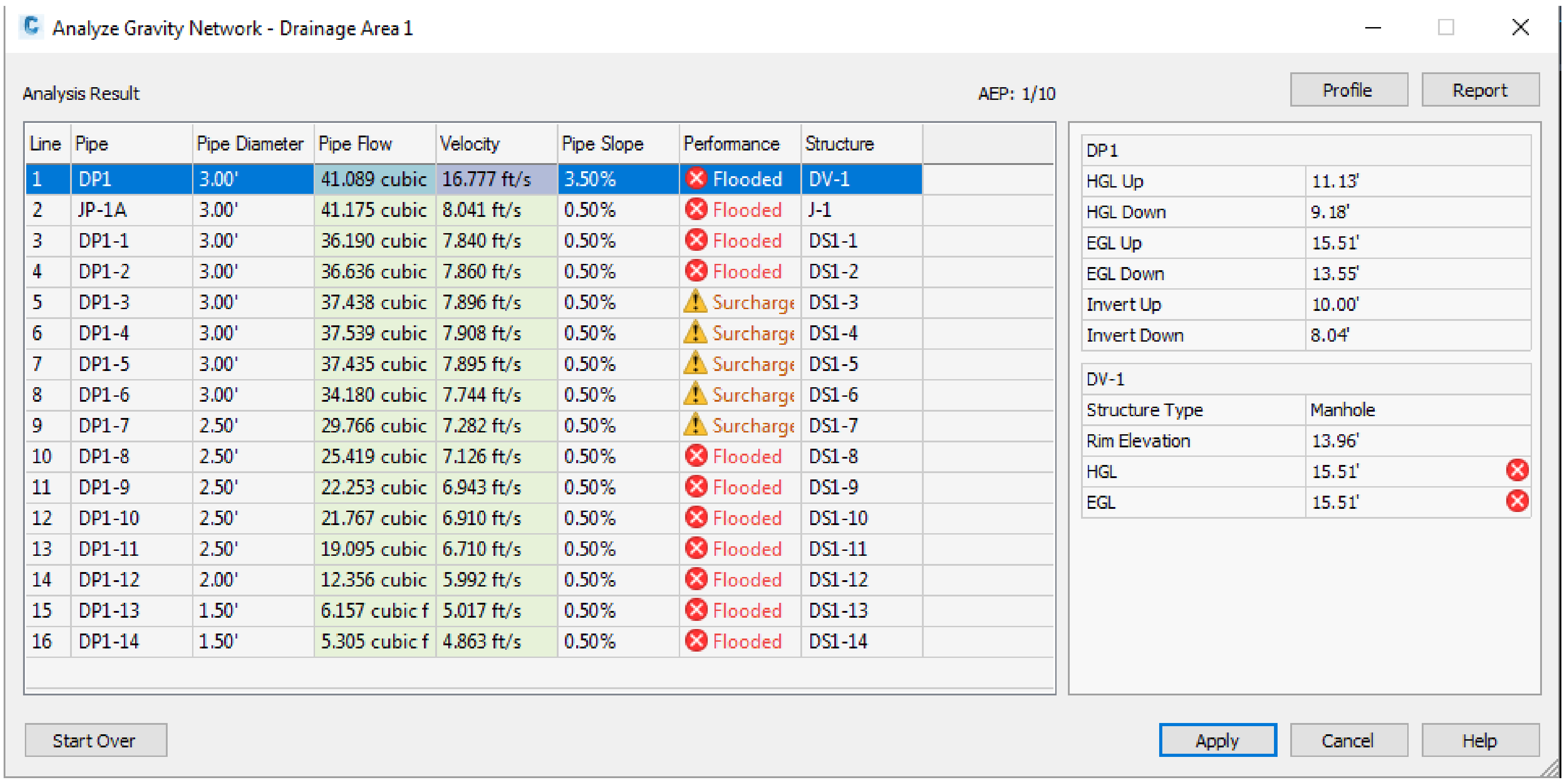
Analysis Result

AEP: 1/2

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DP1	3.00'	26.218 cubic	14.802 ft/s	3.50%	✖ Flooded	DV-1
2	JP-1A	3.00'	26.250 cubic	7.277 ft/s	0.50%	✖ Flooded	J-1
3	DP1-1	3.00'	22.911 cubic	7.031 ft/s	0.50%	✖ Flooded	DS1-1
4	DP1-2	3.00'	23.222 cubic	7.056 ft/s	0.50%	⚠ Surcharg	DS1-2
5	DP1-3	3.00'	23.805 cubic	7.101 ft/s	0.50%	⚠ Surcharg	DS1-3
6	DP1-4	3.00'	23.946 cubic	7.118 ft/s	0.50%	⚠ Surcharg	DS1-4
7	DP1-5	3.00'	23.913 cubic	7.109 ft/s	0.50%	⚠ Surcharg	DS1-5
8	DP1-6	3.00'	21.904 cubic	6.950 ft/s	0.50%	⚠ Surcharg	DS1-6
9	DP1-7	2.50'	19.124 cubic	6.713 ft/s	0.50%	⚠ Surcharg	DS1-7
10	DP1-8	2.50'	16.379 cubic	6.468 ft/s	0.50%	⚠ Surcharg	DS1-8
11	DP1-9	2.50'	14.354 cubic	6.256 ft/s	0.50%	⚠ Surcharg	DS1-9
12	DP1-10	2.50'	14.063 cubic	6.224 ft/s	0.50%	⚠ Surcharg	DS1-10
13	DP1-11	2.50'	12.360 cubic	6.018 ft/s	0.50%	⚠ Surcharg	DS1-11
14	DP1-12	2.00'	8.023 cubic f	5.410 ft/s	0.50%	⚠ Surcharg	DS1-12
15	DP1-13	1.50'	4.009 cubic f	4.549 ft/s	0.50%	⚠ Surcharg	DS1-13
16	DP1-14	1.50'	3.460 cubic f	4.382 ft/s	0.50%	✓ Normal	DS1-14

DP1		
HGL Up	10.90'	
HGL Down	8.94'	
EGL Up	14.30'	
EGL Down	12.35'	
Invert Up	10.00'	
Invert Down	8.04'	

DV-1		
Structure Type	Manhole	
Rim Elevation	13.96'	
HGL	14.30'	✖
EGL	14.30'	✖



Analyze Gravity Network - Drainage Area 1

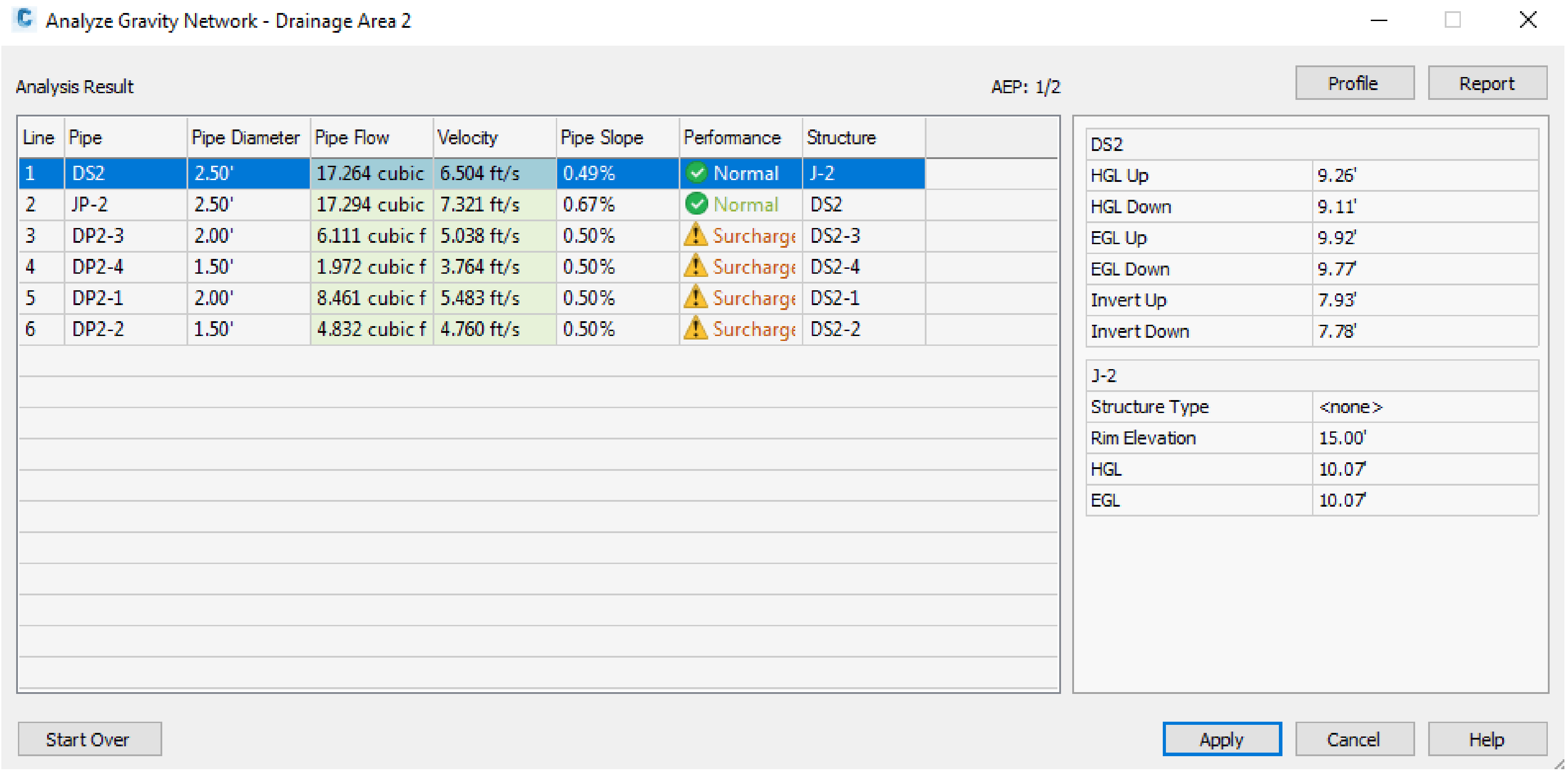
Analysis Result

AEP: 1/10

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DP1	3.00'	41.089 cubic	16.777 ft/s	3.50%	✖ Flooded	DV-1
2	JP-1A	3.00'	41.175 cubic	8.041 ft/s	0.50%	✖ Flooded	J-1
3	DP1-1	3.00'	36.190 cubic	7.840 ft/s	0.50%	✖ Flooded	DS1-1
4	DP1-2	3.00'	36.636 cubic	7.860 ft/s	0.50%	✖ Flooded	DS1-2
5	DP1-3	3.00'	37.438 cubic	7.896 ft/s	0.50%	⚠ Surcharg	DS1-3
6	DP1-4	3.00'	37.539 cubic	7.908 ft/s	0.50%	⚠ Surcharg	DS1-4
7	DP1-5	3.00'	37.435 cubic	7.895 ft/s	0.50%	⚠ Surcharg	DS1-5
8	DP1-6	3.00'	34.180 cubic	7.744 ft/s	0.50%	⚠ Surcharg	DS1-6
9	DP1-7	2.50'	29.766 cubic	7.282 ft/s	0.50%	⚠ Surcharg	DS1-7
10	DP1-8	2.50'	25.419 cubic	7.126 ft/s	0.50%	✖ Flooded	DS1-8
11	DP1-9	2.50'	22.253 cubic	6.943 ft/s	0.50%	✖ Flooded	DS1-9
12	DP1-10	2.50'	21.767 cubic	6.910 ft/s	0.50%	✖ Flooded	DS1-10
13	DP1-11	2.50'	19.095 cubic	6.710 ft/s	0.50%	✖ Flooded	DS1-11
14	DP1-12	2.00'	12.356 cubic	5.992 ft/s	0.50%	✖ Flooded	DS1-12
15	DP1-13	1.50'	6.157 cubic f	5.017 ft/s	0.50%	✖ Flooded	DS1-13
16	DP1-14	1.50'	5.305 cubic f	4.863 ft/s	0.50%	✖ Flooded	DS1-14

DP1		
HGL Up	11.13'	
HGL Down	9.18'	
EGL Up	15.51'	
EGL Down	13.55'	
Invert Up	10.00'	
Invert Down	8.04'	

DV-1		
Structure Type	Manhole	
Rim Elevation	13.96'	
HGL	15.51'	✖
EGL	15.51'	✖



Analyze Gravity Network - Drainage Area 2

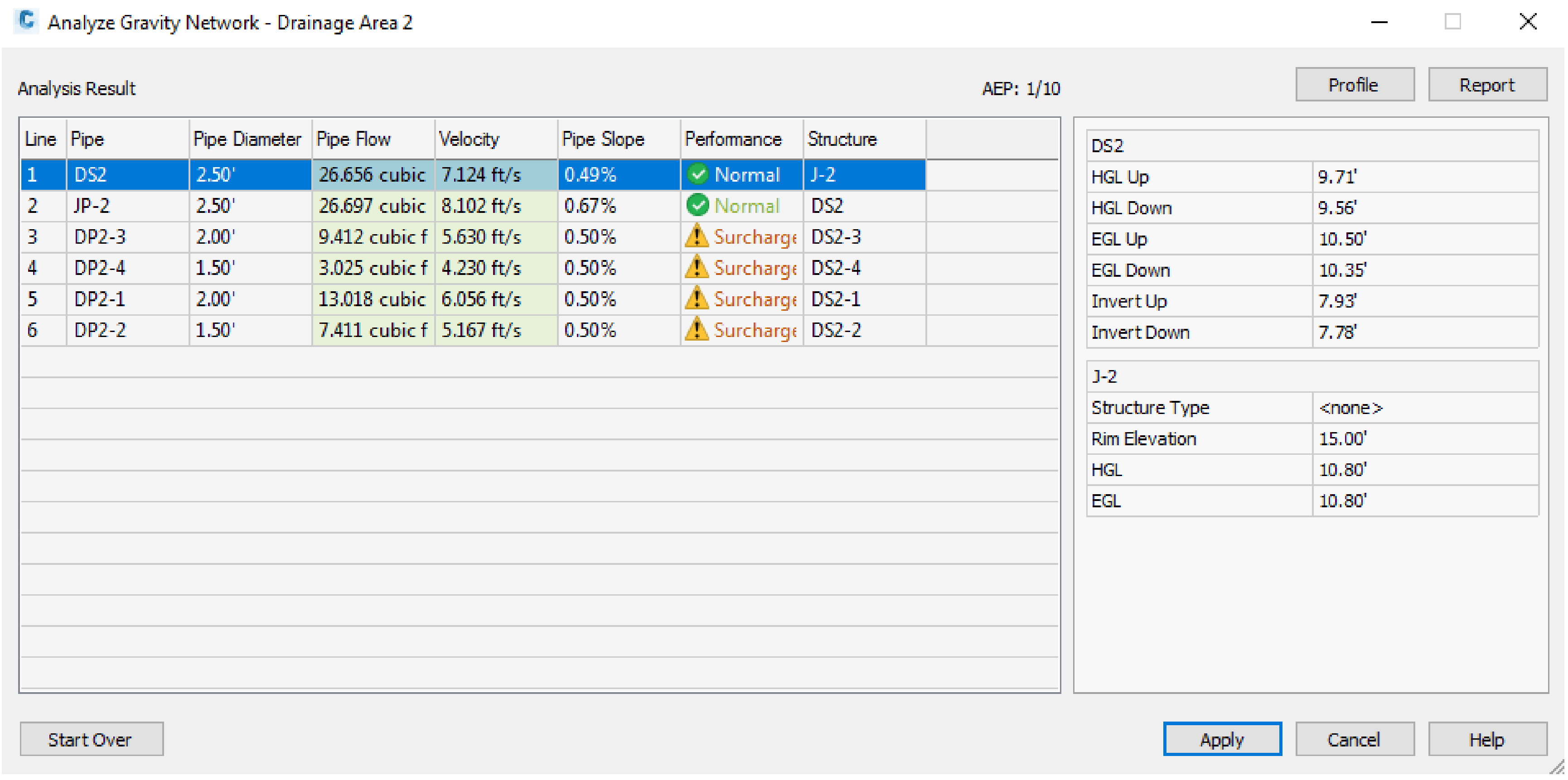
Analysis Result

AEP: 1/2

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DS2	2.50'	17.264 cubic	6.504 ft/s	0.49%	✓ Normal	J-2
2	JP-2	2.50'	17.294 cubic	7.321 ft/s	0.67%	✓ Normal	DS2
3	DP2-3	2.00'	6.111 cubic f	5.038 ft/s	0.50%	⚠ Surcharg	DS2-3
4	DP2-4	1.50'	1.972 cubic f	3.764 ft/s	0.50%	⚠ Surcharg	DS2-4
5	DP2-1	2.00'	8.461 cubic f	5.483 ft/s	0.50%	⚠ Surcharg	DS2-1
6	DP2-2	1.50'	4.832 cubic f	4.760 ft/s	0.50%	⚠ Surcharg	DS2-2

DS2	
HGL Up	9.26'
HGL Down	9.11'
EGL Up	9.92'
EGL Down	9.77'
Invert Up	7.93'
Invert Down	7.78'

J-2	
Structure Type	<none>
Rim Elevation	15.00'
HGL	10.07'
EGL	10.07'



Analyze Gravity Network - Drainage Area 2

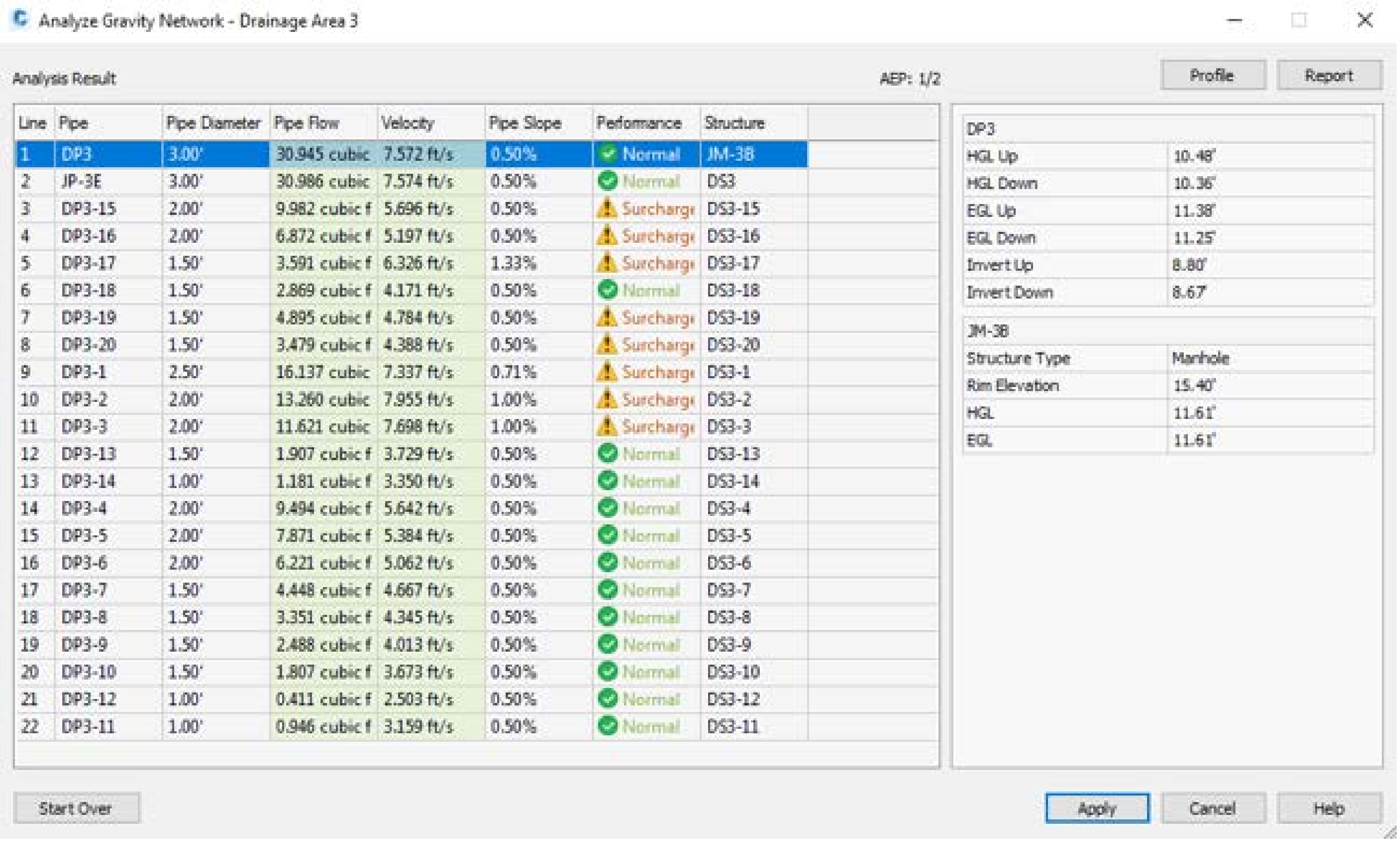
Analysis Result

AEP: 1/10

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DS2	2.50'	26.656 cubic	7.124 ft/s	0.49%	✓ Normal	J-2
2	JP-2	2.50'	26.697 cubic	8.102 ft/s	0.67%	✓ Normal	DS2
3	DP2-3	2.00'	9.412 cubic f	5.630 ft/s	0.50%	⚠ Surcharg	DS2-3
4	DP2-4	1.50'	3.025 cubic f	4.230 ft/s	0.50%	⚠ Surcharg	DS2-4
5	DP2-1	2.00'	13.018 cubic	6.056 ft/s	0.50%	⚠ Surcharg	DS2-1
6	DP2-2	1.50'	7.411 cubic f	5.167 ft/s	0.50%	⚠ Surcharg	DS2-2

DS2	
HGL Up	9.71'
HGL Down	9.56'
EGL Up	10.50'
EGL Down	10.35'
Invert Up	7.93'
Invert Down	7.78'

J-2	
Structure Type	<none>
Rim Elevation	15.00'
HGL	10.80'
EGL	10.80'



Analyze Gravity Network - Drainage Area 3

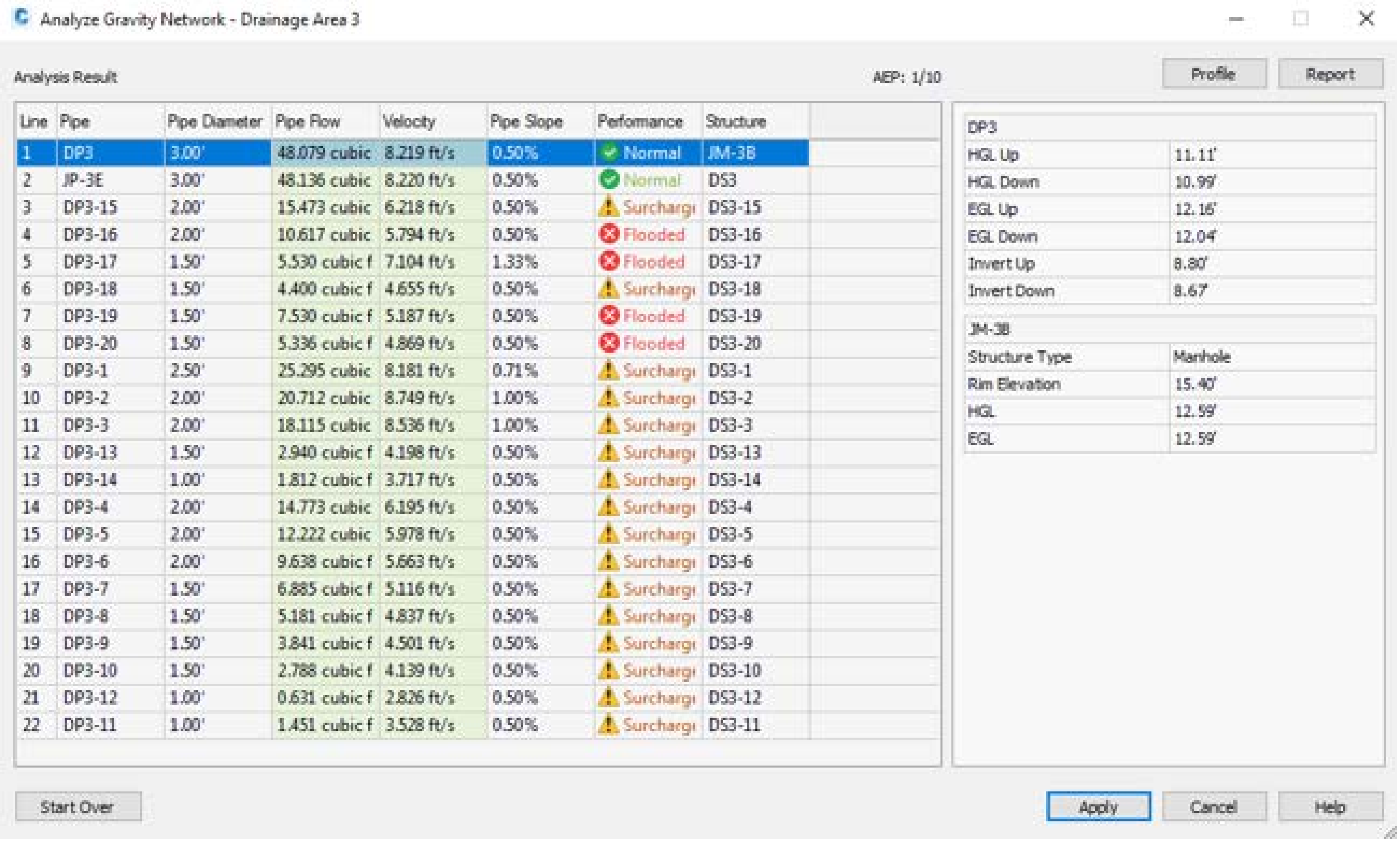
Analysis Result

AEP: 1/2

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DP3	3.00'	30.945 cubic	7.572 ft/s	0.50%	✓ Normal	JM-3B
2	JP-3E	3.00'	30.986 cubic	7.574 ft/s	0.50%	✓ Normal	DS3
3	DP3-15	2.00'	9.982 cubic f	5.696 ft/s	0.50%	⚠ Surcharg	DS3-15
4	DP3-16	2.00'	6.872 cubic f	5.197 ft/s	0.50%	⚠ Surcharg	DS3-16
5	DP3-17	1.50'	3.591 cubic f	6.326 ft/s	1.33%	⚠ Surcharg	DS3-17
6	DP3-18	1.50'	2.869 cubic f	4.171 ft/s	0.50%	✓ Normal	DS3-18
7	DP3-19	1.50'	4.895 cubic f	4.784 ft/s	0.50%	⚠ Surcharg	DS3-19
8	DP3-20	1.50'	3.479 cubic f	4.388 ft/s	0.50%	⚠ Surcharg	DS3-20
9	DP3-1	2.50'	16.137 cubic	7.337 ft/s	0.71%	⚠ Surcharg	DS3-1
10	DP3-2	2.00'	13.260 cubic	7.955 ft/s	1.00%	⚠ Surcharg	DS3-2
11	DP3-3	2.00'	11.621 cubic	7.698 ft/s	1.00%	⚠ Surcharg	DS3-3
12	DP3-13	1.50'	1.907 cubic f	3.729 ft/s	0.50%	✓ Normal	DS3-13
13	DP3-14	1.00'	1.181 cubic f	3.350 ft/s	0.50%	✓ Normal	DS3-14
14	DP3-4	2.00'	9.494 cubic f	5.642 ft/s	0.50%	✓ Normal	DS3-4
15	DP3-5	2.00'	7.871 cubic f	5.384 ft/s	0.50%	✓ Normal	DS3-5
16	DP3-6	2.00'	6.221 cubic f	5.062 ft/s	0.50%	✓ Normal	DS3-6
17	DP3-7	1.50'	4.448 cubic f	4.667 ft/s	0.50%	✓ Normal	DS3-7
18	DP3-8	1.50'	3.351 cubic f	4.345 ft/s	0.50%	✓ Normal	DS3-8
19	DP3-9	1.50'	2.488 cubic f	4.013 ft/s	0.50%	✓ Normal	DS3-9
20	DP3-10	1.50'	1.807 cubic f	3.673 ft/s	0.50%	✓ Normal	DS3-10
21	DP3-12	1.00'	0.411 cubic f	2.503 ft/s	0.50%	✓ Normal	DS3-12
22	DP3-11	1.00'	0.946 cubic f	3.159 ft/s	0.50%	✓ Normal	DS3-11

DP3	
HGL Up	10.48'
HGL Down	10.36'
EGL Up	11.38'
EGL Down	11.25'
Invert Up	8.80'
Invert Down	8.67'

JM-3B	
Structure Type	Manhole
Rim Elevation	15.40'
HGL	11.61'
EGL	11.61'



Analyze Gravity Network - Drainage Area 3

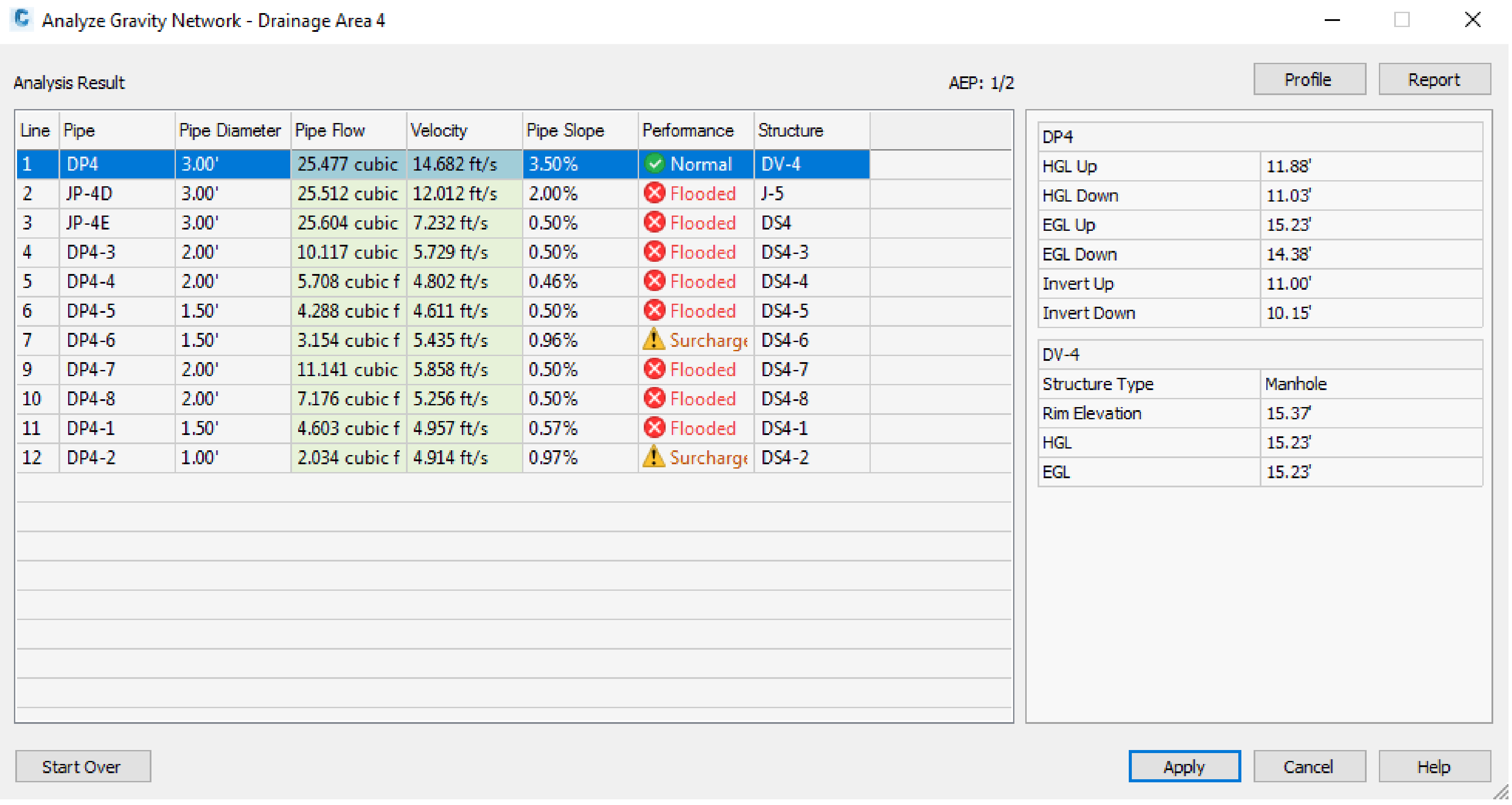
Analysis Result

AEP: 1/10

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DP3	3.00'	48.079 cubic	8.219 ft/s	0.50%	✓ Normal	JM-3B
2	JP-3E	3.00'	48.136 cubic	8.220 ft/s	0.50%	✓ Normal	DS3
3	DP3-15	2.00'	15.473 cubic	6.218 ft/s	0.50%	⚠ Surcharg	DS3-15
4	DP3-16	2.00'	10.617 cubic	5.794 ft/s	0.50%	⊗ Flooded	DS3-16
5	DP3-17	1.50'	5.530 cubic f	7.104 ft/s	1.33%	⊗ Flooded	DS3-17
6	DP3-18	1.50'	4.400 cubic f	4.655 ft/s	0.50%	⚠ Surcharg	DS3-18
7	DP3-19	1.50'	7.530 cubic f	5.187 ft/s	0.50%	⊗ Flooded	DS3-19
8	DP3-20	1.50'	5.336 cubic f	4.869 ft/s	0.50%	⊗ Flooded	DS3-20
9	DP3-1	2.50'	25.295 cubic	8.181 ft/s	0.71%	⚠ Surcharg	DS3-1
10	DP3-2	2.00'	20.712 cubic	8.749 ft/s	1.00%	⚠ Surcharg	DS3-2
11	DP3-3	2.00'	18.115 cubic	8.536 ft/s	1.00%	⚠ Surcharg	DS3-3
12	DP3-13	1.50'	2.940 cubic f	4.198 ft/s	0.50%	⚠ Surcharg	DS3-13
13	DP3-14	1.00'	1.812 cubic f	3.717 ft/s	0.50%	⚠ Surcharg	DS3-14
14	DP3-4	2.00'	14.773 cubic	6.195 ft/s	0.50%	⚠ Surcharg	DS3-4
15	DP3-5	2.00'	12.222 cubic	5.978 ft/s	0.50%	⚠ Surcharg	DS3-5
16	DP3-6	2.00'	9.638 cubic f	5.663 ft/s	0.50%	⚠ Surcharg	DS3-6
17	DP3-7	1.50'	6.885 cubic f	5.116 ft/s	0.50%	⚠ Surcharg	DS3-7
18	DP3-8	1.50'	5.181 cubic f	4.837 ft/s	0.50%	⚠ Surcharg	DS3-8
19	DP3-9	1.50'	3.841 cubic f	4.501 ft/s	0.50%	⚠ Surcharg	DS3-9
20	DP3-10	1.50'	2.788 cubic f	4.139 ft/s	0.50%	⚠ Surcharg	DS3-10
21	DP3-12	1.00'	0.631 cubic f	2.826 ft/s	0.50%	⚠ Surcharg	DS3-12
22	DP3-11	1.00'	1.451 cubic f	3.528 ft/s	0.50%	⚠ Surcharg	DS3-11

DP3	
HGL Up	11.11'
HGL Down	10.99'
EGL Up	12.16'
EGL Down	12.04'
Invert Up	8.80'
Invert Down	8.67'

JM-3B	
Structure Type	Manhole
Rim Elevation	15.40'
HGL	12.59'
EGL	12.59'



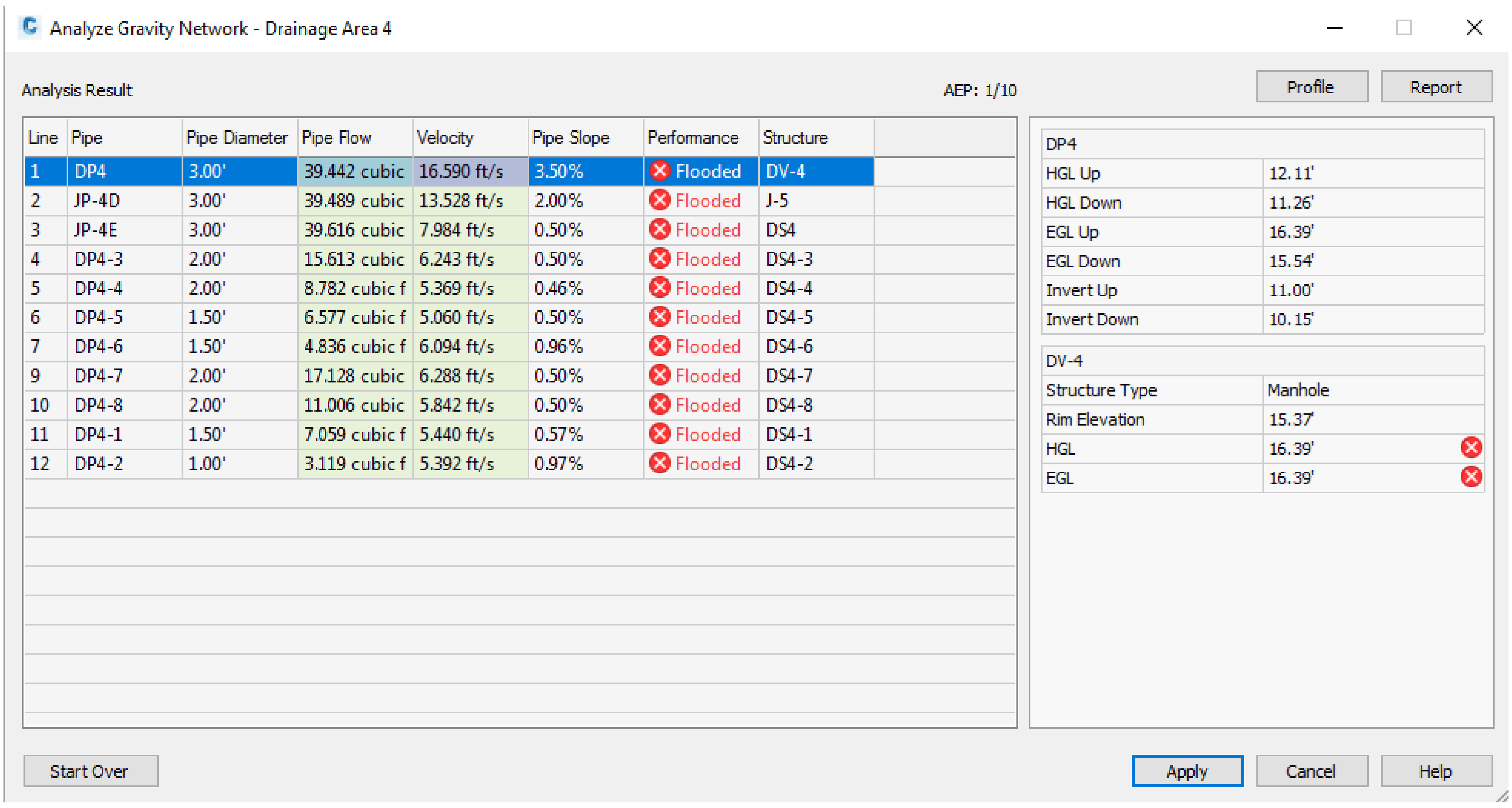
Analysis Result

AEP: 1/2

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DP4	3.00'	25.477 cubic	14.682 ft/s	3.50%	✓ Normal	DV-4
2	JP-4D	3.00'	25.512 cubic	12.012 ft/s	2.00%	☒ Flooded	J-5
3	JP-4E	3.00'	25.604 cubic	7.232 ft/s	0.50%	☒ Flooded	DS4
4	DP4-3	2.00'	10.117 cubic	5.729 ft/s	0.50%	☒ Flooded	DS4-3
5	DP4-4	2.00'	5.708 cubic f	4.802 ft/s	0.46%	☒ Flooded	DS4-4
6	DP4-5	1.50'	4.288 cubic f	4.611 ft/s	0.50%	☒ Flooded	DS4-5
7	DP4-6	1.50'	3.154 cubic f	5.435 ft/s	0.96%	⚠ Surcharge	DS4-6
9	DP4-7	2.00'	11.141 cubic	5.858 ft/s	0.50%	☒ Flooded	DS4-7
10	DP4-8	2.00'	7.176 cubic f	5.256 ft/s	0.50%	☒ Flooded	DS4-8
11	DP4-1	1.50'	4.603 cubic f	4.957 ft/s	0.57%	☒ Flooded	DS4-1
12	DP4-2	1.00'	2.034 cubic f	4.914 ft/s	0.97%	⚠ Surcharge	DS4-2

DP4	
HGL Up	11.88'
HGL Down	11.03'
EGL Up	15.23'
EGL Down	14.38'
Invert Up	11.00'
Invert Down	10.15'

DV-4	
Structure Type	Manhole
Rim Elevation	15.37'
HGL	15.23'
EGL	15.23'



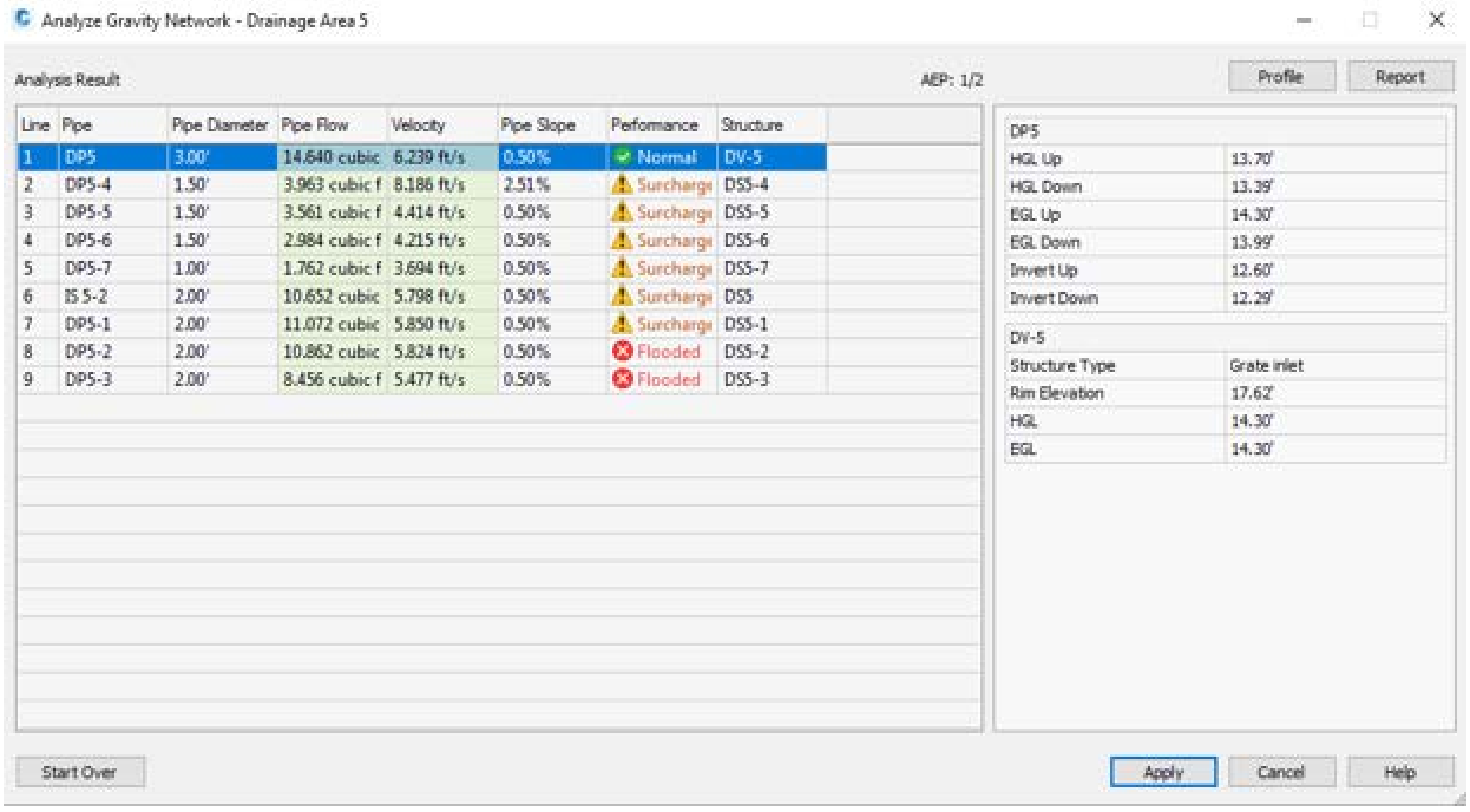
Analysis Result

AEP: 1/10

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DP4	3.00'	39.442 cubic	16.590 ft/s	3.50%	☒ Flooded	DV-4
2	JP-4D	3.00'	39.489 cubic	13.528 ft/s	2.00%	☒ Flooded	J-5
3	JP-4E	3.00'	39.616 cubic	7.984 ft/s	0.50%	☒ Flooded	DS4
4	DP4-3	2.00'	15.613 cubic	6.243 ft/s	0.50%	☒ Flooded	DS4-3
5	DP4-4	2.00'	8.782 cubic f	5.369 ft/s	0.46%	☒ Flooded	DS4-4
6	DP4-5	1.50'	6.577 cubic f	5.060 ft/s	0.50%	☒ Flooded	DS4-5
7	DP4-6	1.50'	4.836 cubic f	6.094 ft/s	0.96%	☒ Flooded	DS4-6
9	DP4-7	2.00'	17.128 cubic	6.288 ft/s	0.50%	☒ Flooded	DS4-7
10	DP4-8	2.00'	11.006 cubic	5.842 ft/s	0.50%	☒ Flooded	DS4-8
11	DP4-1	1.50'	7.059 cubic f	5.440 ft/s	0.57%	☒ Flooded	DS4-1
12	DP4-2	1.00'	3.119 cubic f	5.392 ft/s	0.97%	☒ Flooded	DS4-2

DP4	
HGL Up	12.11'
HGL Down	11.26'
EGL Up	16.39'
EGL Down	15.54'
Invert Up	11.00'
Invert Down	10.15'

DV-4		
Structure Type	Manhole	
Rim Elevation	15.37'	
HGL	16.39'	☒
EGL	16.39'	☒



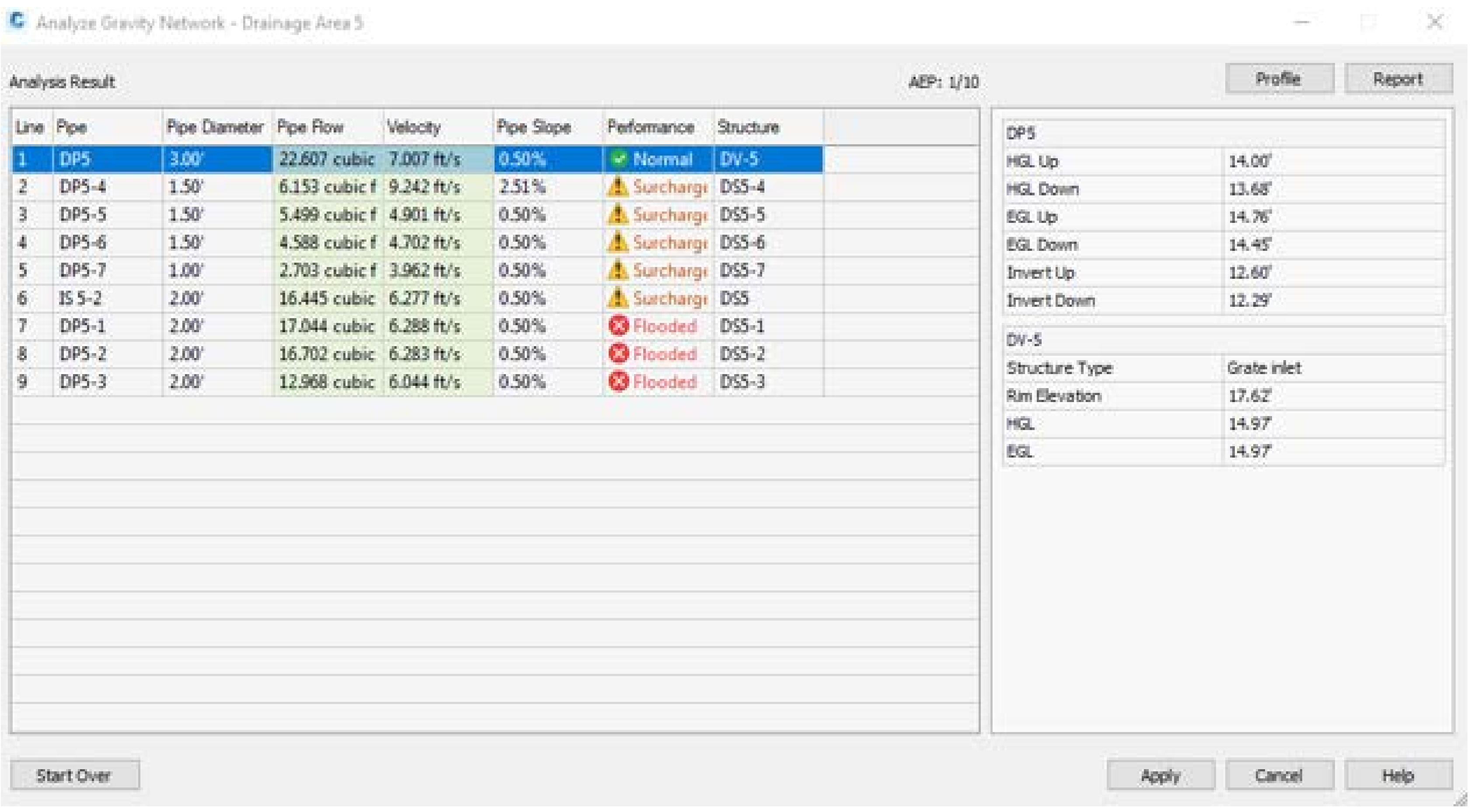
Analysis Result

AEP: 1/2

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DP5	3.00'	14.640 cubic	6.239 ft/s	0.50%	✓ Normal	DV-5
2	DP5-4	1.50'	3.963 cubic f	8.186 ft/s	2.51%	⚠ Surcharg	DS5-4
3	DP5-5	1.50'	3.561 cubic f	4.414 ft/s	0.50%	⚠ Surcharg	DS5-5
4	DP5-6	1.50'	2.984 cubic f	4.215 ft/s	0.50%	⚠ Surcharg	DS5-6
5	DP5-7	1.00'	1.762 cubic f	3.694 ft/s	0.50%	⚠ Surcharg	DS5-7
6	IS 5-2	2.00'	10.652 cubic	5.798 ft/s	0.50%	⚠ Surcharg	DS5
7	DP5-1	2.00'	11.072 cubic	5.850 ft/s	0.50%	⚠ Surcharg	DS5-1
8	DP5-2	2.00'	10.862 cubic	5.824 ft/s	0.50%	☒ Flooded	DS5-2
9	DP5-3	2.00'	8.456 cubic f	5.477 ft/s	0.50%	☒ Flooded	DS5-3

DP5	
HGL Up	13.70'
HGL Down	13.39'
EGL Up	14.30'
EGL Down	13.99'
Invert Up	12.60'
Invert Down	12.29'

DV-5	
Structure Type	Grate inlet
Rim Elevation	17.62'
HGL	14.30'
EGL	14.30'



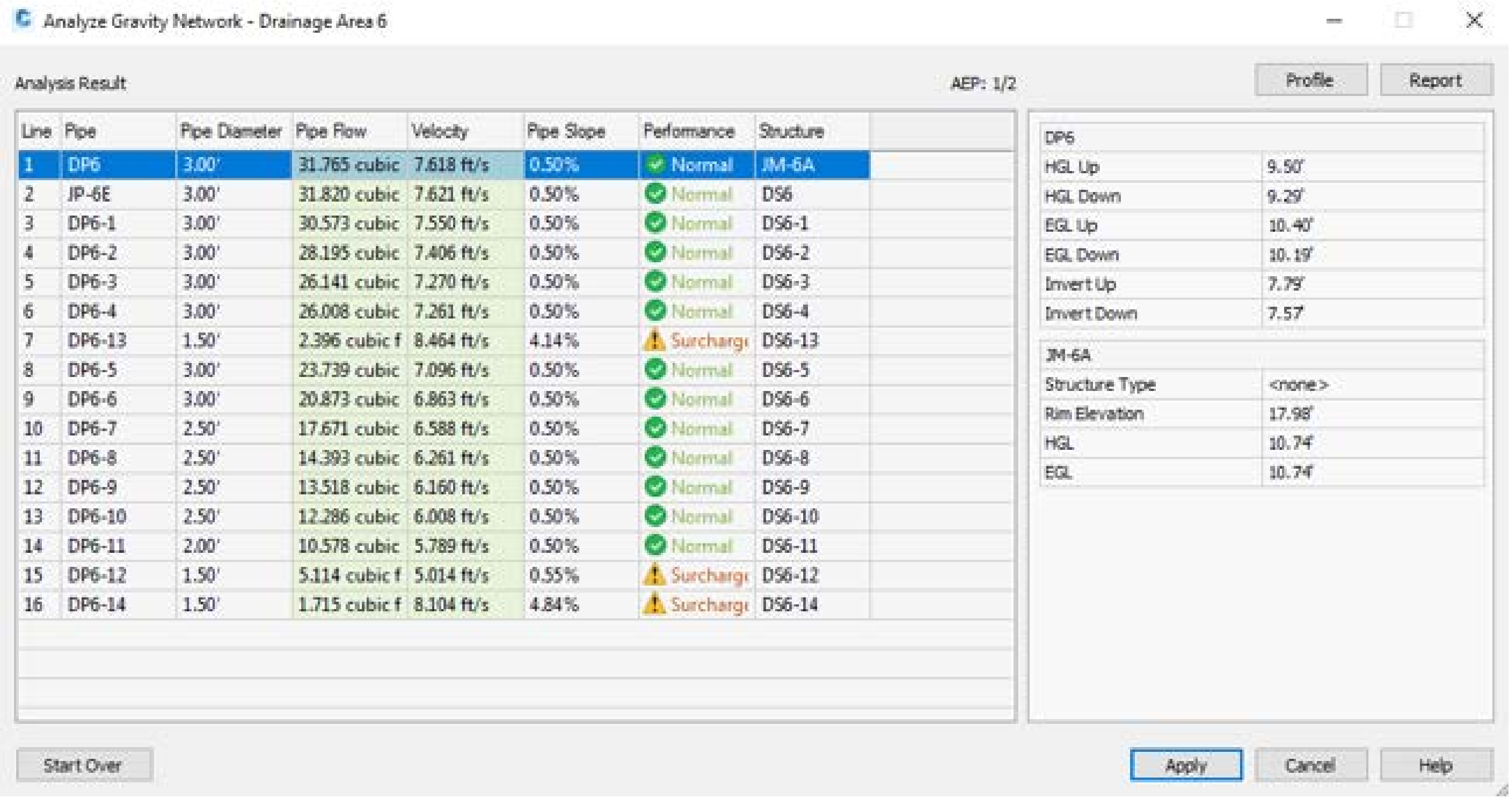
Analysis Result

AEP: 1/10

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Perfomance	Structure
1	DP5	3.00'	22.607 cubic	7.007 ft/s	0.50%	✓ Normal	DV-5
2	DP5-4	1.50'	6.153 cubic f	9.242 ft/s	2.51%	⚠ Surcharg	DS5-4
3	DP5-5	1.50'	5.499 cubic f	4.901 ft/s	0.50%	⚠ Surcharg	DS5-5
4	DP5-6	1.50'	4.588 cubic f	4.702 ft/s	0.50%	⚠ Surcharg	DS5-6
5	DP5-7	1.00'	2.703 cubic f	3.962 ft/s	0.50%	⚠ Surcharg	DS5-7
6	IS 5-2	2.00'	16.445 cubic	6.277 ft/s	0.50%	⚠ Surcharg	DS5
7	DP5-1	2.00'	17.044 cubic	6.288 ft/s	0.50%	⊗ Flooded	DS5-1
8	DP5-2	2.00'	16.702 cubic	6.283 ft/s	0.50%	⊗ Flooded	DS5-2
9	DP5-3	2.00'	12.968 cubic	6.044 ft/s	0.50%	⊗ Flooded	DS5-3

DP5	
HGL Up	14.00'
HGL Down	13.68'
EGL Up	14.76'
EGL Down	14.45'
Invert Up	12.60'
Invert Down	12.29'

DV-5	
Structure Type	Grate inlet
Rim Elevation	17.62'
HGL	14.97'
EGL	14.97'



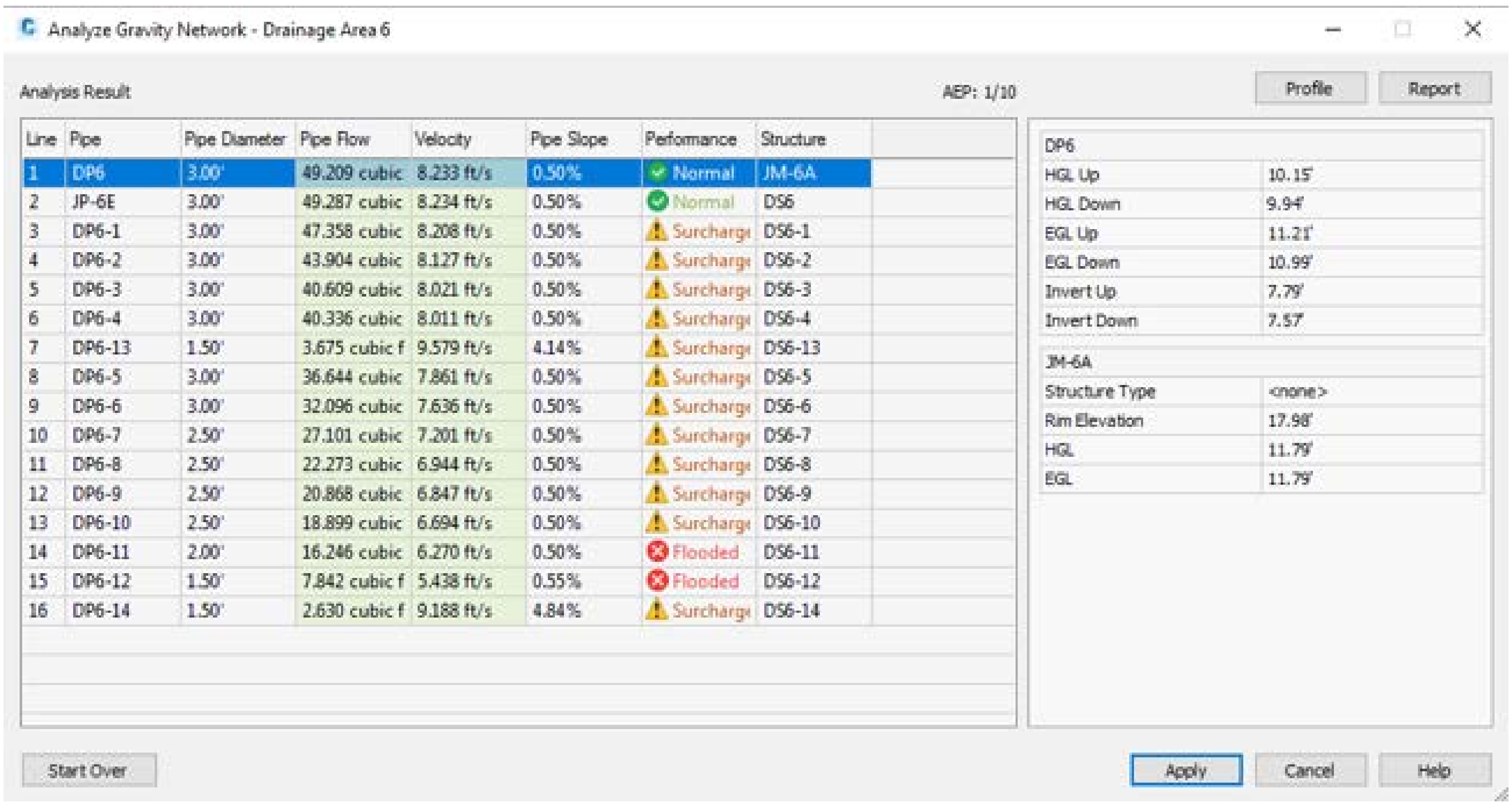
Analysis Result

AEP: 1/2

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Perfomance	Structure
1	DP6	3.00'	31.765 cubic	7.618 ft/s	0.50%	✓ Normal	JM-6A
2	JP-6E	3.00'	31.820 cubic	7.621 ft/s	0.50%	✓ Normal	DS6
3	DP6-1	3.00'	30.573 cubic	7.550 ft/s	0.50%	✓ Normal	DS6-1
4	DP6-2	3.00'	28.195 cubic	7.406 ft/s	0.50%	✓ Normal	DS6-2
5	DP6-3	3.00'	26.141 cubic	7.270 ft/s	0.50%	✓ Normal	DS6-3
6	DP6-4	3.00'	26.008 cubic	7.261 ft/s	0.50%	✓ Normal	DS6-4
7	DP6-13	1.50'	2.396 cubic f	8.464 ft/s	4.14%	⚠ Surcharg	DS6-13
8	DP6-5	3.00'	23.739 cubic	7.096 ft/s	0.50%	✓ Normal	DS6-5
9	DP6-6	3.00'	20.873 cubic	6.863 ft/s	0.50%	✓ Normal	DS6-6
10	DP6-7	2.50'	17.671 cubic	6.588 ft/s	0.50%	✓ Normal	DS6-7
11	DP6-8	2.50'	14.393 cubic	6.261 ft/s	0.50%	✓ Normal	DS6-8
12	DP6-9	2.50'	13.518 cubic	6.160 ft/s	0.50%	✓ Normal	DS6-9
13	DP6-10	2.50'	12.286 cubic	6.008 ft/s	0.50%	✓ Normal	DS6-10
14	DP6-11	2.00'	10.578 cubic	5.789 ft/s	0.50%	✓ Normal	DS6-11
15	DP6-12	1.50'	5.114 cubic f	5.014 ft/s	0.55%	⚠ Surcharg	DS6-12
16	DP6-14	1.50'	1.715 cubic f	8.104 ft/s	4.84%	⚠ Surcharg	DS6-14

DP6	
HGL Up	9.50'
HGL Down	9.29'
EGL Up	10.40'
EGL Down	10.19'
Invert Up	7.79'
Invert Down	7.57'

JM-6A	
Structure Type	<none>
Rim Elevation	17.98'
HGL	10.74'
EGL	10.74'



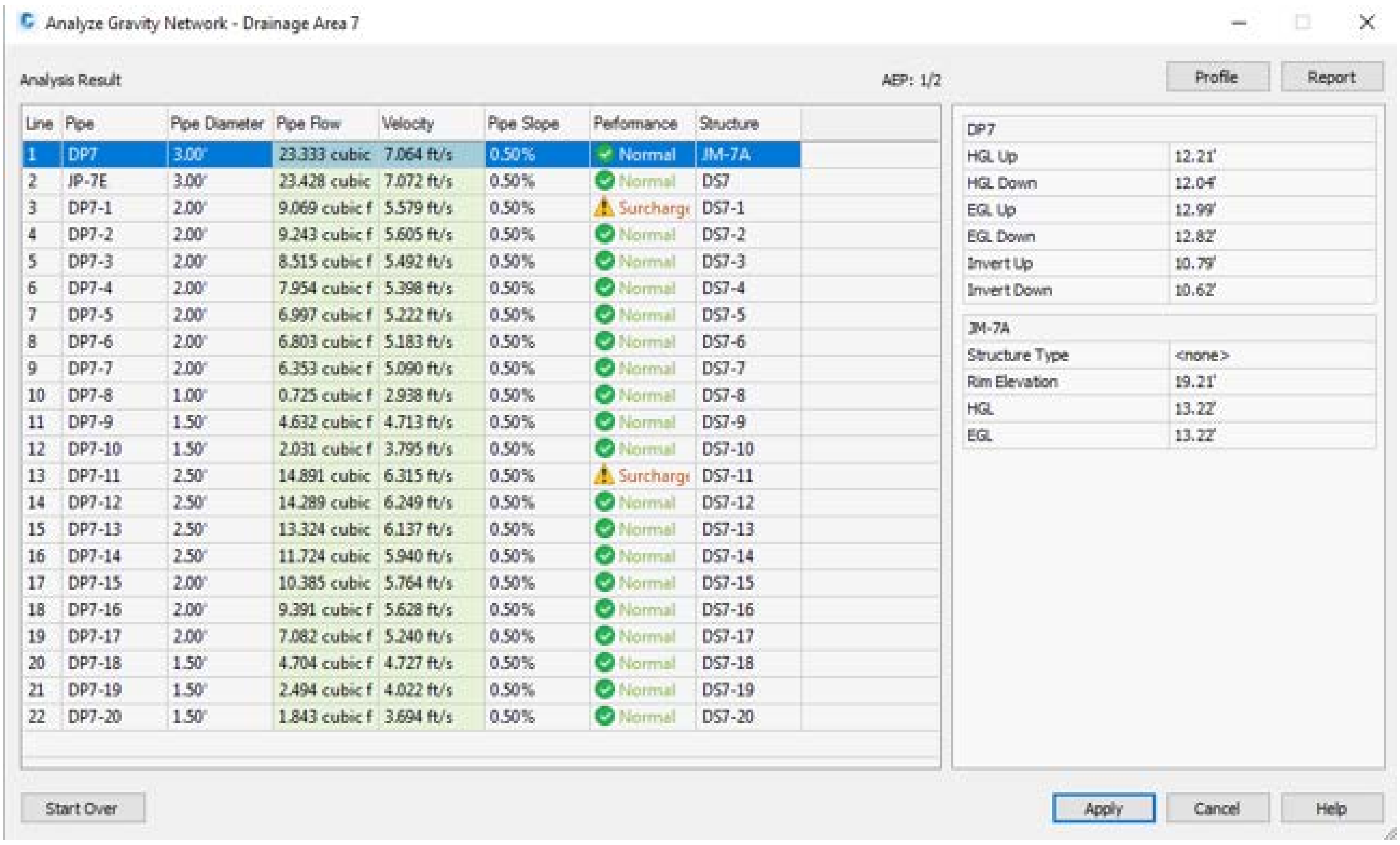
Analysis Result

AEP: 1/10

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Perfomance	Structure
1	DP6	3.00'	49.209 cubic	8.233 ft/s	0.50%	✓ Normal	JM-6A
2	JP-6E	3.00'	49.287 cubic	8.234 ft/s	0.50%	✓ Normal	DS6
3	DP6-1	3.00'	47.358 cubic	8.208 ft/s	0.50%	⚠ Surcharg	DS6-1
4	DP6-2	3.00'	43.904 cubic	8.127 ft/s	0.50%	⚠ Surcharg	DS6-2
5	DP6-3	3.00'	40.609 cubic	8.021 ft/s	0.50%	⚠ Surcharg	DS6-3
6	DP6-4	3.00'	40.336 cubic	8.011 ft/s	0.50%	⚠ Surcharg	DS6-4
7	DP6-13	1.50'	3.675 cubic f	9.579 ft/s	4.14%	⚠ Surcharg	DS6-13
8	DP6-5	3.00'	36.644 cubic	7.861 ft/s	0.50%	⚠ Surcharg	DS6-5
9	DP6-6	3.00'	32.096 cubic	7.636 ft/s	0.50%	⚠ Surcharg	DS6-6
10	DP6-7	2.50'	27.101 cubic	7.201 ft/s	0.50%	⚠ Surcharg	DS6-7
11	DP6-8	2.50'	22.273 cubic	6.944 ft/s	0.50%	⚠ Surcharg	DS6-8
12	DP6-9	2.50'	20.868 cubic	6.847 ft/s	0.50%	⚠ Surcharg	DS6-9
13	DP6-10	2.50'	18.899 cubic	6.694 ft/s	0.50%	⚠ Surcharg	DS6-10
14	DP6-11	2.00'	16.246 cubic	6.270 ft/s	0.50%	⊗ Flooded	DS6-11
15	DP6-12	1.50'	7.842 cubic f	5.438 ft/s	0.55%	⊗ Flooded	DS6-12
16	DP6-14	1.50'	2.630 cubic f	9.188 ft/s	4.84%	⚠ Surcharg	DS6-14

DP6	
HGL Up	10.15'
HGL Down	9.94'
EGL Up	11.21'
EGL Down	10.99'
Invert Up	7.79'
Invert Down	7.57'

JM-6A	
Structure Type	<none>
Rim Elevation	17.98'
HGL	11.79'
EGL	11.79'



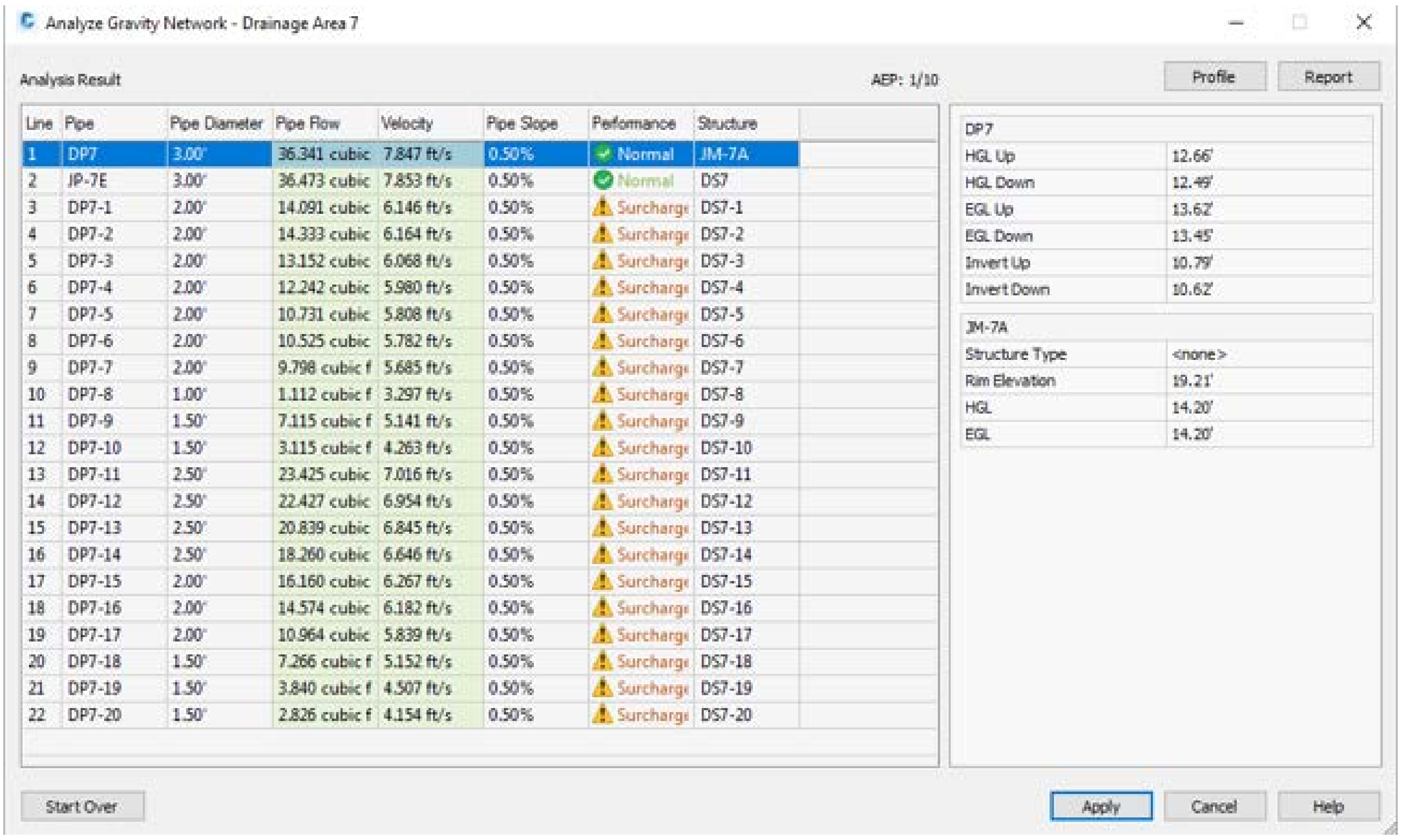
Analysis Result

AEP: 1/2

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DP7	3.00'	23.333 cubic	7.064 ft/s	0.50%	✓ Normal	JM-7A
2	JP-7E	3.00'	23.428 cubic	7.072 ft/s	0.50%	✓ Normal	DS7
3	DP7-1	2.00'	9.069 cubic f	5.579 ft/s	0.50%	⚠ Surcharg	DS7-1
4	DP7-2	2.00'	9.243 cubic f	5.605 ft/s	0.50%	✓ Normal	DS7-2
5	DP7-3	2.00'	8.515 cubic f	5.492 ft/s	0.50%	✓ Normal	DS7-3
6	DP7-4	2.00'	7.954 cubic f	5.398 ft/s	0.50%	✓ Normal	DS7-4
7	DP7-5	2.00'	6.997 cubic f	5.222 ft/s	0.50%	✓ Normal	DS7-5
8	DP7-6	2.00'	6.803 cubic f	5.183 ft/s	0.50%	✓ Normal	DS7-6
9	DP7-7	2.00'	6.353 cubic f	5.090 ft/s	0.50%	✓ Normal	DS7-7
10	DP7-8	1.00'	0.725 cubic f	2.938 ft/s	0.50%	✓ Normal	DS7-8
11	DP7-9	1.50'	4.632 cubic f	4.713 ft/s	0.50%	✓ Normal	DS7-9
12	DP7-10	1.50'	2.031 cubic f	3.795 ft/s	0.50%	✓ Normal	DS7-10
13	DP7-11	2.50'	14.891 cubic	6.315 ft/s	0.50%	⚠ Surcharg	DS7-11
14	DP7-12	2.50'	14.289 cubic	6.249 ft/s	0.50%	✓ Normal	DS7-12
15	DP7-13	2.50'	13.324 cubic	6.137 ft/s	0.50%	✓ Normal	DS7-13
16	DP7-14	2.50'	11.724 cubic	5.940 ft/s	0.50%	✓ Normal	DS7-14
17	DP7-15	2.00'	10.385 cubic	5.764 ft/s	0.50%	✓ Normal	DS7-15
18	DP7-16	2.00'	9.391 cubic f	5.628 ft/s	0.50%	✓ Normal	DS7-16
19	DP7-17	2.00'	7.082 cubic f	5.240 ft/s	0.50%	✓ Normal	DS7-17
20	DP7-18	1.50'	4.704 cubic f	4.727 ft/s	0.50%	✓ Normal	DS7-18
21	DP7-19	1.50'	2.494 cubic f	4.022 ft/s	0.50%	✓ Normal	DS7-19
22	DP7-20	1.50'	1.843 cubic f	3.694 ft/s	0.50%	✓ Normal	DS7-20

DP7	
HGL Up	12.21'
HGL Down	12.04'
EGL Up	12.99'
EGL Down	12.82'
Invert Up	10.79'
Invert Down	10.62'

JM-7A	
Structure Type	<none>
Rim Elevation	19.21'
HGL	13.22'
EGL	13.22'



Analysis Result

AEP: 1/10

Line	Pipe	Pipe Diameter	Pipe Flow	Velocity	Pipe Slope	Performance	Structure
1	DP7	3.00'	36.341 cubic	7.847 ft/s	0.50%	✓ Normal	JM-7A
2	JP-7E	3.00'	36.473 cubic	7.853 ft/s	0.50%	✓ Normal	DS7
3	DP7-1	2.00'	14.091 cubic	6.146 ft/s	0.50%	⚠ Surcharg	DS7-1
4	DP7-2	2.00'	14.333 cubic	6.164 ft/s	0.50%	⚠ Surcharg	DS7-2
5	DP7-3	2.00'	13.152 cubic	6.068 ft/s	0.50%	⚠ Surcharg	DS7-3
6	DP7-4	2.00'	12.242 cubic	5.980 ft/s	0.50%	⚠ Surcharg	DS7-4
7	DP7-5	2.00'	10.731 cubic	5.808 ft/s	0.50%	⚠ Surcharg	DS7-5
8	DP7-6	2.00'	10.525 cubic	5.782 ft/s	0.50%	⚠ Surcharg	DS7-6
9	DP7-7	2.00'	9.798 cubic f	5.685 ft/s	0.50%	⚠ Surcharg	DS7-7
10	DP7-8	1.00'	1.112 cubic f	3.297 ft/s	0.50%	⚠ Surcharg	DS7-8
11	DP7-9	1.50'	7.115 cubic f	5.141 ft/s	0.50%	⚠ Surcharg	DS7-9
12	DP7-10	1.50'	3.115 cubic f	4.263 ft/s	0.50%	⚠ Surcharg	DS7-10
13	DP7-11	2.50'	23.425 cubic	7.016 ft/s	0.50%	⚠ Surcharg	DS7-11
14	DP7-12	2.50'	22.427 cubic	6.954 ft/s	0.50%	⚠ Surcharg	DS7-12
15	DP7-13	2.50'	20.839 cubic	6.845 ft/s	0.50%	⚠ Surcharg	DS7-13
16	DP7-14	2.50'	18.260 cubic	6.646 ft/s	0.50%	⚠ Surcharg	DS7-14
17	DP7-15	2.00'	16.160 cubic	6.267 ft/s	0.50%	⚠ Surcharg	DS7-15
18	DP7-16	2.00'	14.574 cubic	6.182 ft/s	0.50%	⚠ Surcharg	DS7-16
19	DP7-17	2.00'	10.964 cubic	5.839 ft/s	0.50%	⚠ Surcharg	DS7-17
20	DP7-18	1.50'	7.266 cubic f	5.152 ft/s	0.50%	⚠ Surcharg	DS7-18
21	DP7-19	1.50'	3.840 cubic f	4.507 ft/s	0.50%	⚠ Surcharg	DS7-19
22	DP7-20	1.50'	2.826 cubic f	4.154 ft/s	0.50%	⚠ Surcharg	DS7-20

DP7	
HGL Up	12.66'
HGL Down	12.49'
EGL Up	13.62'
EGL Down	13.45'
Invert Up	10.79'
Invert Down	10.62'

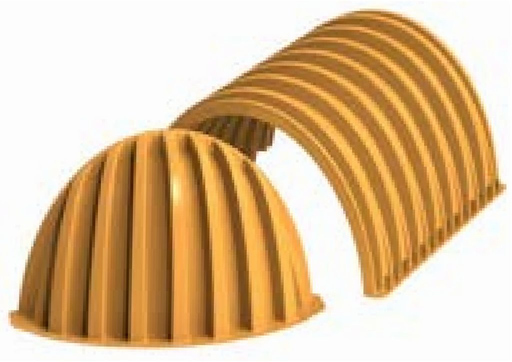
JM-7A	
Structure Type	<none>
Rim Elevation	19.21'
HGL	14.20'
EGL	14.20'

Appendix D

Stormwater Practice Specifications

StormTech® MC-3500 Chamber

Designed to meet the most stringent industry performance standards for superior structural integrity while providing designers with a cost-effective method to save valuable land and protect water resources. The StormTech system is designed primarily to be used under parking lots, thus maximizing land usage for private (commercial) and public applications. StormTech chambers can also be used in conjunction with Green Infrastructure, thus enhancing the performance and extending the service life of these practices.



Nominal Chamber Specifications

(not to scale)

Size (L x W x H)
90” x 77” x 45”
2286 mm x 1956 mm x 1143 mm

Chamber Storage
109.9 ft^3 (3.11 m^3)

Min. Installed Storage*
175.0 ft^3 (4.96 m^3)

Weight
134 lbs (60.8 kg)

Shipping
15 chambers/pallet
7 end caps/pallet
7 pallets/truck

*Assumes a minimum of 12” (300 mm) of stone above, 9” (230 mm) of stone below chambers, 6” (150 mm) of stone between chambers/ end caps and 40% stone porosity.

Nominal End Cap Specifications

(not to scale)

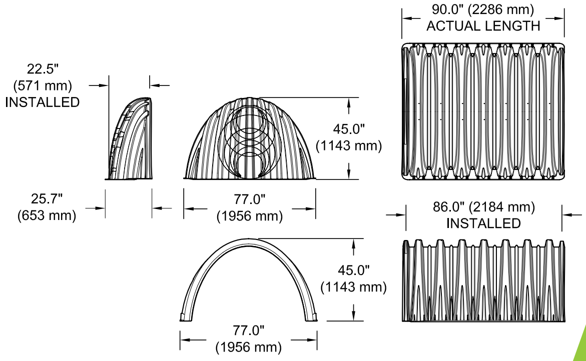
Size (L x W x H)
26.5” x 71” x 45.1”
673 mm x 1803 mm x 1145 mm

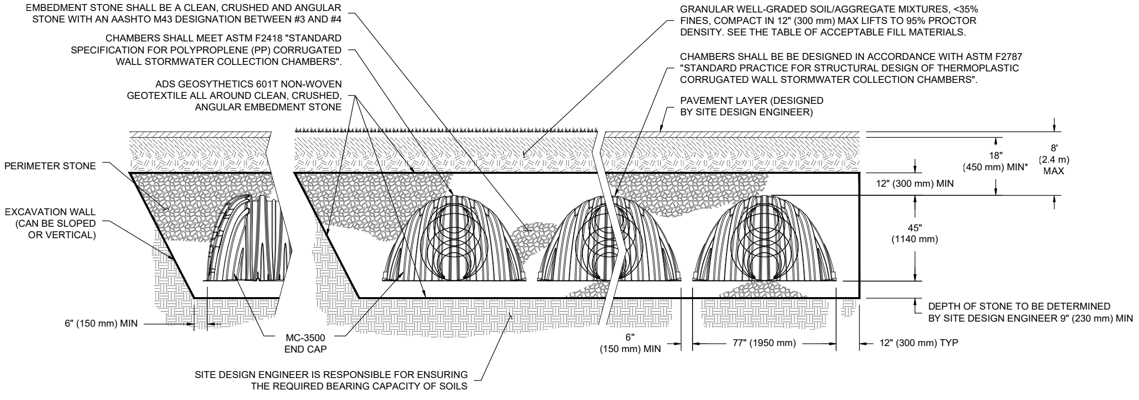
End Cap Storage
14.9 ft^3 (0.42 m^3)

Min. Installed Storage*
45.1 ft^3 (1.28 m^3)

Weight
49 lbs (22.2 kg)

*Assumes a minimum of 12” (300 mm) of stone above, 9” (230 mm) of stone below, 6” (150 mm) of stone perimeter, 6” (150 mm) of stone between chambers/ end caps and 40% stone porosity.





*MINIMUM COVER TO BOTTOM OF FLEXIBLE PAVEMENT. FOR UNPAVED INSTALLATIONS WHERE RUTTING FROM VEHICLES MAY OCCUR, INCREASE COVER TO 24" (600 mm).

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StormTech MC-3500 Specifications

Storage Volume Per Chamber

	Bare Chamber Storage ft^3 (m^3)	Chamber and Stone Foundation Depth in. (mm)			
		9 in (230 mm)	12 in (300 mm)	15 in (375 mm)	18 in (450 mm)
Chamber	109.9 (3.11)	175.0 (4.96)	179.9 (5.09)	184.9 (5.24)	189.9 (5.38)
End Cap	14.9 (0.42)	45.1 (1.28)	46.6 (1.32)	48.3 (1.37)	49.9 (1.41)

Note: Assumes 6" (150 mm) row spacing, 40% stone porosity, 12" (300 mm) stone above and includes the bare chamber/end cap volume.

Amount of Stone Per Chamber

English Tons (yds^3)	Stone Foundation Depth			
	9 in	12 in	15 in	18 in
Chamber	8.5 (6.0)	9.1 (6.5)	9.7 (6.9)	10.4 (7.4)
End Cap	3.9 (2.8)	4.1 (2.9)	4.3 (3.1)	4.5 (3.2)
Metric Kilograms (m^3)	**230 mm**	**300 mm**	**375 mm**	**450 mm**
Chamber	7711 (4.6)	8255 (5.0)	8800 (5.3)	9435 (5.7)
End Cap	3538 (2.1)	3719 (2.2)	3901 (2.4)	4082 (2.5)

Note: Assumes 12" (300 mm) of stone above and 6" (150 mm) row spacing and 6" (150 mm) of perimeter stone in front of end caps.

Volume Excavation Per Chamber yd^3 (m^3)

	Stone Foundation Depth			
	9 in (230 mm)	12 in (300 mm)	15 in (375mm)	18 in (450 mm)
Chamber	11.9 (9.1)	12.4 (9.5)	12.8 (9.8)	13.3 (10.2)
End Cap	4.0 (3.1)	4.1 (3.3)	4.3 (3.3)	4.4 (3.4)

Note: Assumes 6" (150 mm) of separation between chamber rows and 24" (600 mm) of cover. The volume of excavation will vary as depth of cover increases.

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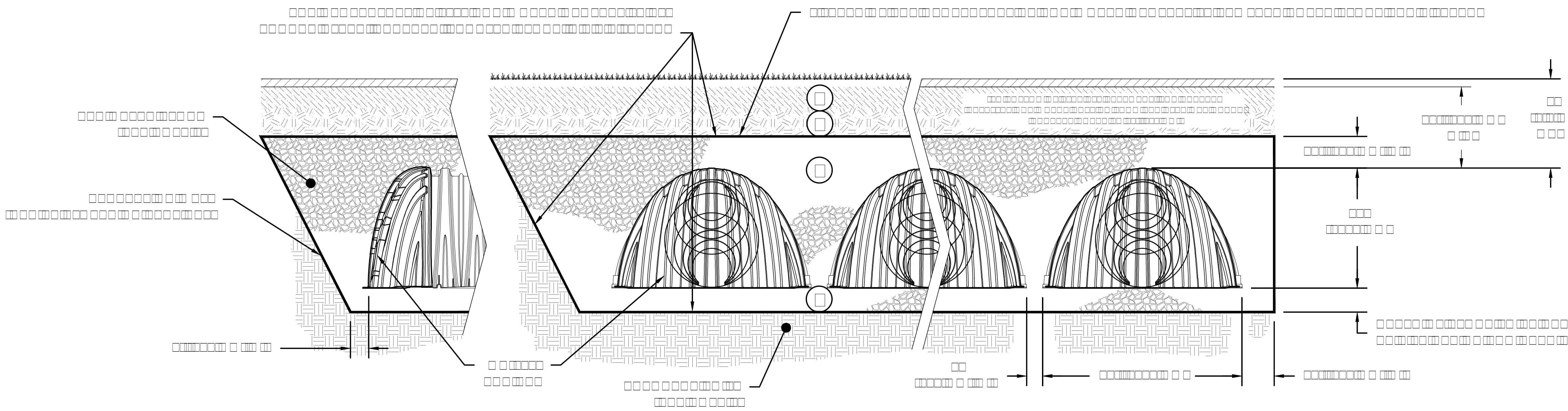
ACCEPTABLE FILL MATERIALS: STORMTECH MC-3500 CHAMBER SYSTEMS

[illegible]		[illegible]	[illegible]	[illegible]
[illegible]	**FINAL FILL:** [illegible]	[illegible]	[illegible]	[illegible]
[illegible]	**INITIAL FILL:** [illegible]	[illegible]	[illegible]	[illegible]
[illegible]	**EMBEDMENT STONE:** [illegible]	[illegible]	[illegible]	[illegible]
[illegible]	**FOUNDATION STONE:** [illegible]	[illegible]	[illegible]	[illegible]

[illegible]



[illegible]

NOTES:

[illegible]

- [illegible]
- TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 3".
- [illegible] AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.

DATE	DRWN	CHKD	DESCRIPTION

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MC-3500 TECHNICAL SPECIFICATION

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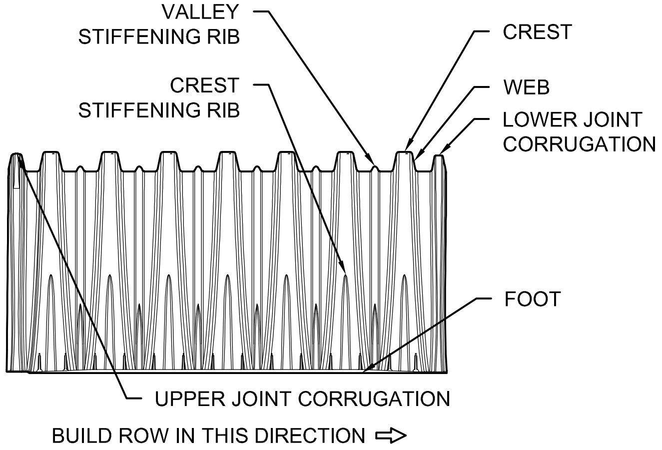
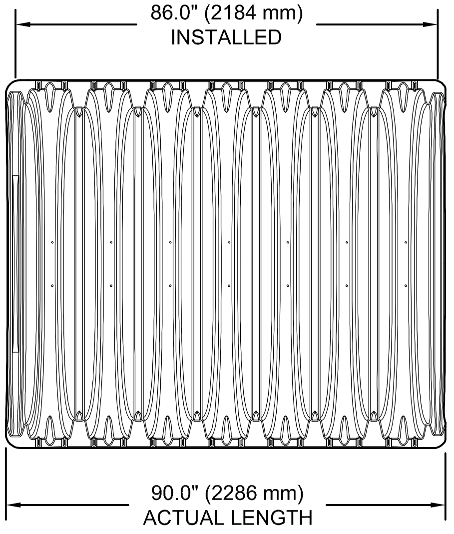
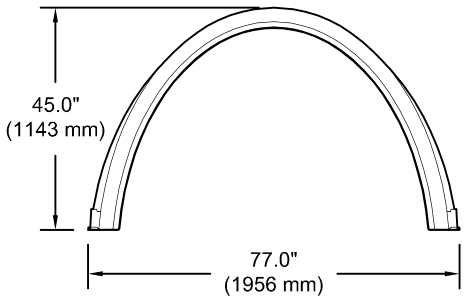
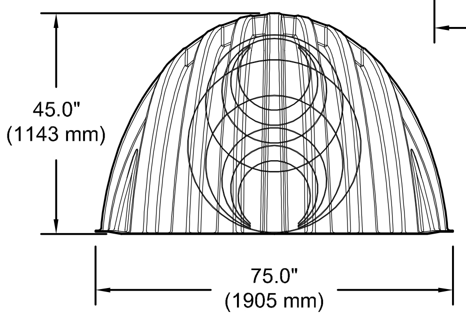
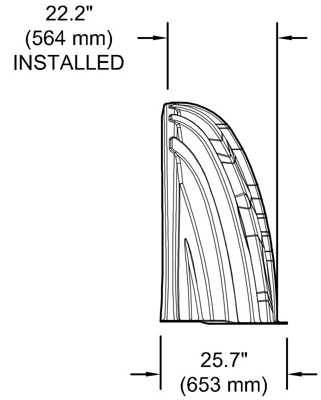










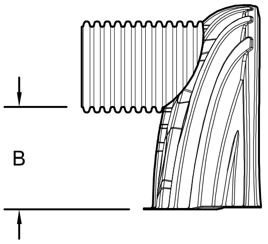
NOMINAL CHAMBER SPECIFICATIONS

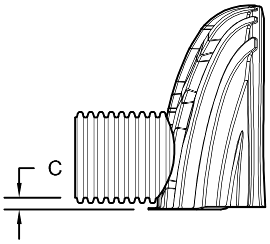
SIZE (W X H X INSTALLED LENGTH)	77.0" X 45.0" X 86.0"	(1956 mm X 1143 mm X 2184 mm)
CHAMBER STORAGE	109.9 CUBIC FEET	(3.11 m³)
MINIMUM INSTALLED STORAGE*	175.0 CUBIC FEET	(4.96 m³)
WEIGHT	134 lbs.	(60.8 kg)

NOMINAL END CAP SPECIFICATIONS

SIZE (W X H X INSTALLED LENGTH)	75.0" X 45.0" X 22.2"	(1905 mm X 1143 mm X 564 mm)
END CAP STORAGE	14.9 CUBIC FEET	(0.42 m³)
MINIMUM INSTALLED STORAGE*	45.1 CUBIC FEET	(1.28 m³)
WEIGHT	49 lbs.	(22.2 kg)

*ASSUMES 12" (305 mm) STONE ABOVE, 9" (229 mm) STONE FOUNDATION, 6" (152 mm) OF STONE BETWEEN CHAMBERS, 6" (152 mm) STONE PERIMETER IN FRONT OF END CAPS AND 40% STONE POROSITY



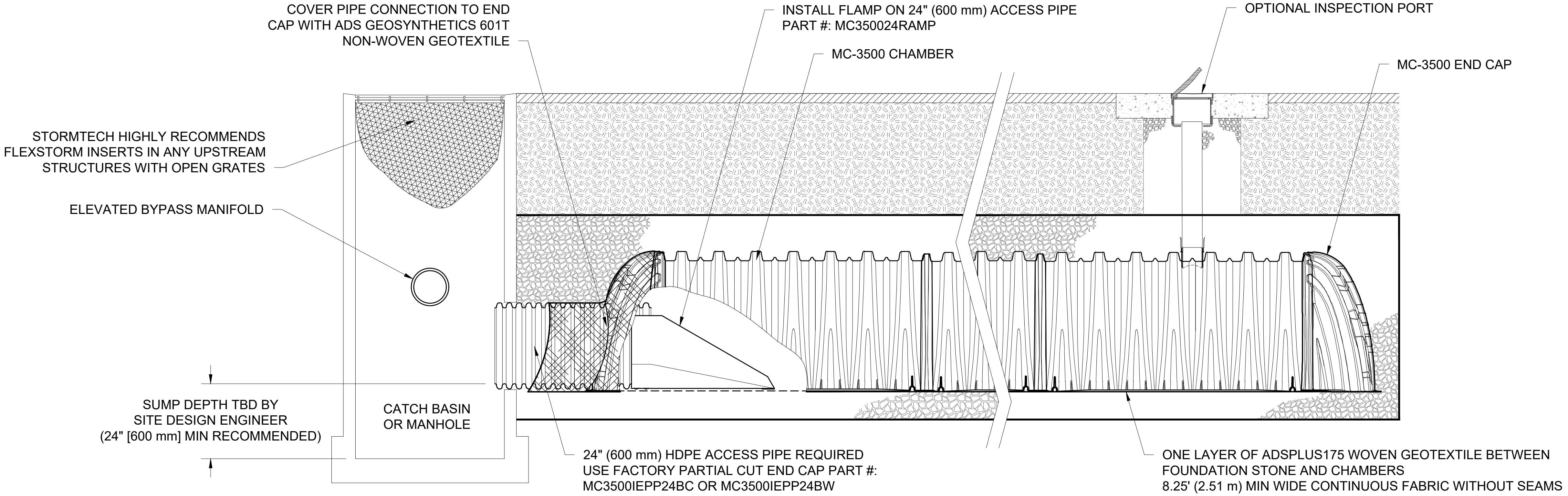


STUBS AT BOTTOM OF END CAP FOR PART NUMBERS ENDING WITH "B"
STUBS AT TOP OF END CAP FOR PART NUMBERS ENDING WITH "T"
END CAPS WITH A WELDED CROWN PLATE END WITH "C"
END CAPS WITH A PREFABRICATED WELDED STUB END WITH "W"

PART #	STUB	B	C
MC3500IEPP06T	6" (150 mm)	33.21" (844 mm)	---
MC3500IEPP06B		---	0.66" (17 mm)
MC3500IEPP08T	8" (200 mm)	31.16" (791 mm)	---
MC3500IEPP08B		---	0.81" (21 mm)
MC3500IEPP10T	10" (250 mm)	29.04" (738 mm)	---
MC3500IEPP10B		---	0.93" (24 mm)
MC3500IEPP12T	12" (300 mm)	26.36" (670 mm)	---
MC3500IEPP12B		---	1.35" (34 mm)
MC3500IEPP15T	15" (375 mm)	23.39" (594 mm)	---
MC3500IEPP15B		---	1.50" (38 mm)
MC3500IEPP18TC	18" (450 mm)	20.03" (509 mm)	---
MC3500IEPP18TW			
MC3500IEPP18BC		---	1.77" (45 mm)
MC3500IEPP18BW			
MC3500IEPP24TC	24" (600 mm)	14.48" (368 mm)	---
MC3500IEPP24TW			
MC3500IEPP24BC		---	2.06" (52 mm)
MC3500IEPP24BW			
MC3500IEPP30BC	30" (750 mm)	---	2.75" (70 mm)

NOTE: ALL DIMENSIONS ARE NOMINAL

CUSTOM PRECORED INVERTS ARE AVAILABLE UPON REQUEST. INVENTORIED MANIFOLDS INCLUDE 12-24" (300-600 mm) SIZE ON SIZE AND 15-48" (375-1200 mm) ECCENTRIC MANIFOLDS. CUSTOM INVERT LOCATIONS ON THE MC-3500 END CAP CUT IN THE FIELD ARE NOT RECOMMENDED FOR PIPE SIZES GREATER THAN 10" (250 mm). THE INVERT LOCATION IN COLUMN 'B' ARE THE HIGHEST POSSIBLE FOR THE PIPE SIZE.



MC-3500 ISOLATOR ROW PLUS DETAIL

NTS

INSPECTION & MAINTENANCE

STEP 1) INSPECT ISOLATOR ROW PLUS FOR SEDIMENT
- A. INSPECTION PORTS (IF PRESENT)
  - A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
  - A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
  - A.3. USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
  - A.4. LOWER A CAMERA INTO ISOLATOR ROW PLUS FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
  - A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- B. ALL ISOLATOR PLUS ROWS
  - B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW PLUS
  - B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW PLUS THROUGH OUTLET PIPE
    - i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
    - ii) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
  - B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.

STEP 2) CLEAN OUT ISOLATOR ROW PLUS USING THE JETVAC PROCESS
- A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45" (1.1 m) OR MORE IS PREFERRED
- B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
- C. VACUUM STRUCTURE SUMP AS REQUIRED

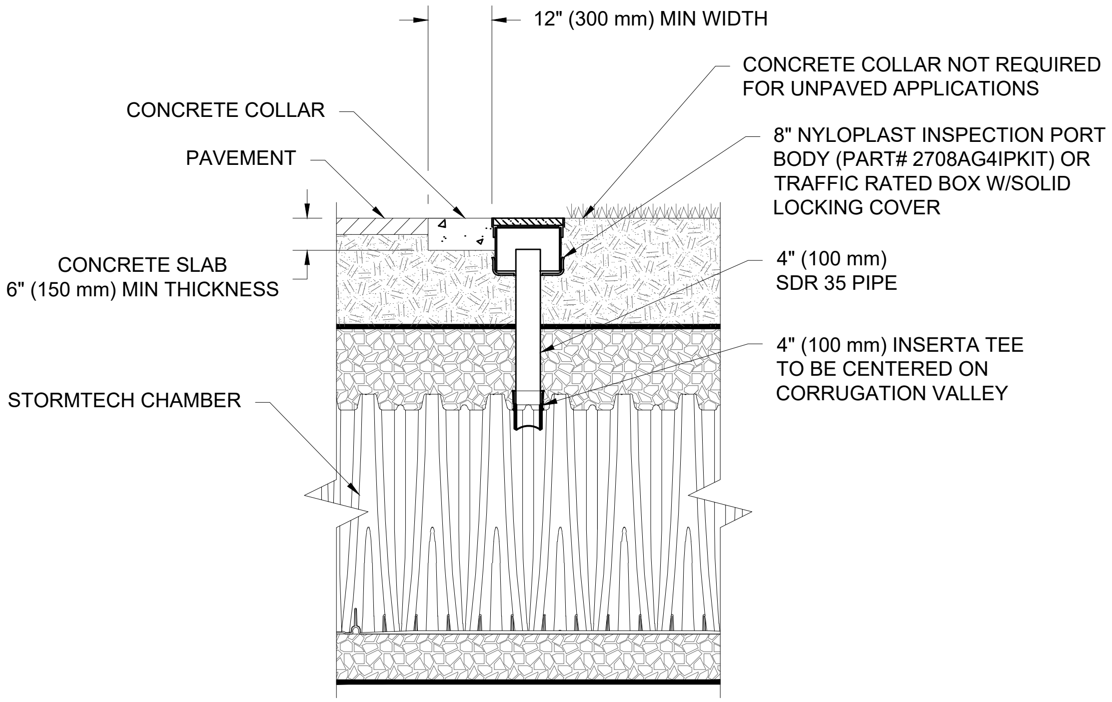
STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.

STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

NOTES

1. INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.

2. CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.



NOTE:
INSPECTION PORTS MAY BE CONNECTED THROUGH ANY CHAMBER CORRUGATION VALLEY.

4" PVC INSPECTION PORT DETAIL
(MC SERIES CHAMBER)

NTS

MC-3500
ISOLATOR ROW PLUS DETAILS

DATE: 08/26/20
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PROJECT #: ----
CHECKED: ALI

DATE	DRWN	CHKD	DESCRIPTION

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1 OF 1

MC-4500 CHAMBER

Designed to meet the most stringent industry performance standards for superior structural integrity while providing designers with a cost-effective method to save valuable land and protect water resources. The StormTech system is designed primarily to be used under parking lots, thus maximizing land usage for private (commercial) and public applications. StormTech chambers can also be used in conjunction with Green Infrastructure, thus enhancing the performance and extending the service life of these practices.

STORMTECH MC-4500 CHAMBER

(not to scale)

Nominal Chamber Specifications

Size (L x W x H)
52” x 100” x 60”
1321 mm x 2540 mm x 1524 mm

Chamber Storage
106.5 ft^3 (3.01 m^3)

Min. Installed Storage*
162.6 ft^3 (4.60 m^3)

Weight
Nominal 125 lbs (56.7 kg)

Shipping
7 chambers/pallet
5 end caps/pallet
11 pallets/truck

*Assumes a minimum of 12” (300 mm) of stone above, 9” (230 mm) of stone below chambers, 9” (230 mm) of stone between chambers/end caps and 40% stone porosity.

STORMTECH MC-4500 END CAP

(not to scale)

Nominal End Cap Specifications

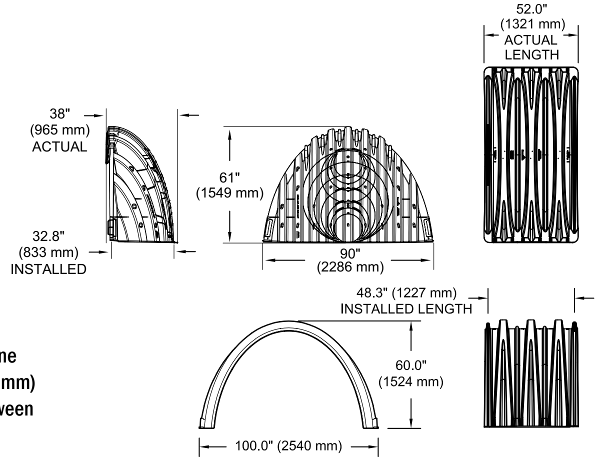
Size (L x W x H)
38” x 90” x 61”
965 mm x 2286 mm x 1549 mm

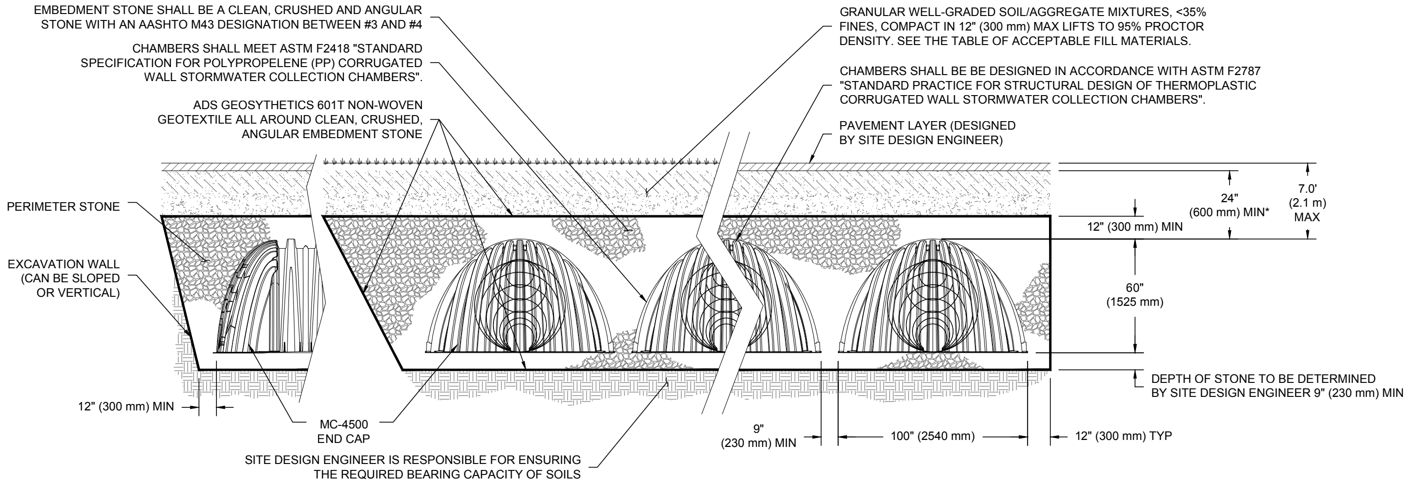
End Cap Storage
39.5 ft^3 (1.12 m^3)

Min. Installed Storage*
115.3 ft^3 (3.26 m^3)

Weight
Nominal 90.0 lbs (40.8 kg)

*Assumes a minimum of 12” (300 mm) of stone above, 9” (230 mm) of stone below, 12” (300 mm) of stone perimeter, 9” (230 mm) of stone between chambers/end caps and 40% stone porosity.





*MINIMUM COVER TO BOTTOM OF FLEXIBLE PAVEMENT. FOR UNPAVED INSTALLATIONS WHERE RUTTING FROM VEHICLES MAY OCCUR, INCREASE COVER TO 30" (750 mm).

MC-4500 CHAMBER SPECIFICATIONS

STORAGE VOLUME PER CHAMBER FT^3 (M^3)

	Bare Chamber Storage ft^3 (m^3)	Chamber and Stone Foundation Depth in. (mm)			
		9" (230 mm)	12" (300 mm)	15" (375 mm)	18" (450 mm)
MC-4500 Chamber	106.5 (3.01)	162.6 (4.60)	166.3 (4.71)	169.9 (4.81)	173.6 (4.91)
MC-4500 End Cap	39.5 (1.12)	115.3 (3.26)	118.6 (3.36)	121.9 (3.45)	125.2 (3.54)

Note: Assumes 9" (230 mm) row spacing, 40% stone porosity, 12" (300 mm) stone above and includes the bare chamber/end cap volume. End cap volume assumes 12" (300 mm) stone perimeter in front of end cap.

AMOUNT OF STONE PER CHAMBER

ENGLISH TONS (yds^3)	Stone Foundation Depth			
	9"	12"	15"	18"
MC-4500 Chamber	7.4 (5.2)	7.8 (5.5)	8.3 (5.9)	8.8 (6.2)
MC-4500 End Cap	9.8 (7.0)	10.2 (7.3)	10.6 (7.6)	11.1 (7.9)
METRIC KILOGRAMS (m^3)	230 mm	300 mm	375 mm	450 mm
MC-4500 Chamber	6713 (4.0)	7076 (4.2)	7529 (4.5)	7983 (4.7)
MC-4500 End Cap	8890 (5.3)	9253 (5.5)	9616 (5.8)	10069 (6.0)

Note: Assumes 12" (300 mm) of stone above and 9" (230 mm) row spacing and 12" (300 mm) of perimeter stone in front of end caps.

VOLUME EXCAVATION PER CHAMBER YD^3 (M^3)

	Stone Foundation Depth			
	9" (230 mm)	12" (300 mm)	15" (375mm)	18" (450 mm)
MC-4500 Chamber	10.5 (8.0)	10.8 (8.3)	11.2 (8.5)	11.5 (8.8)
MC-4500 End Cap	9.7 (7.4)	10.0 (7.6)	10.3 (7.9)	10.6 (8.1)

Note: Assumes 9" (230 mm) of separation between chamber rows, 12" (300 mm) of perimeter in front of the end caps, and 24" (600 mm) of cover. The volume of excavation will vary as depth of cover increases.

Working on a project?
Visit us at www.stormtech.com
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For more information on the StormTech MC-4500 Chamber and other ADS products, please contact our Customer Service Representatives at 1-800-821-6710

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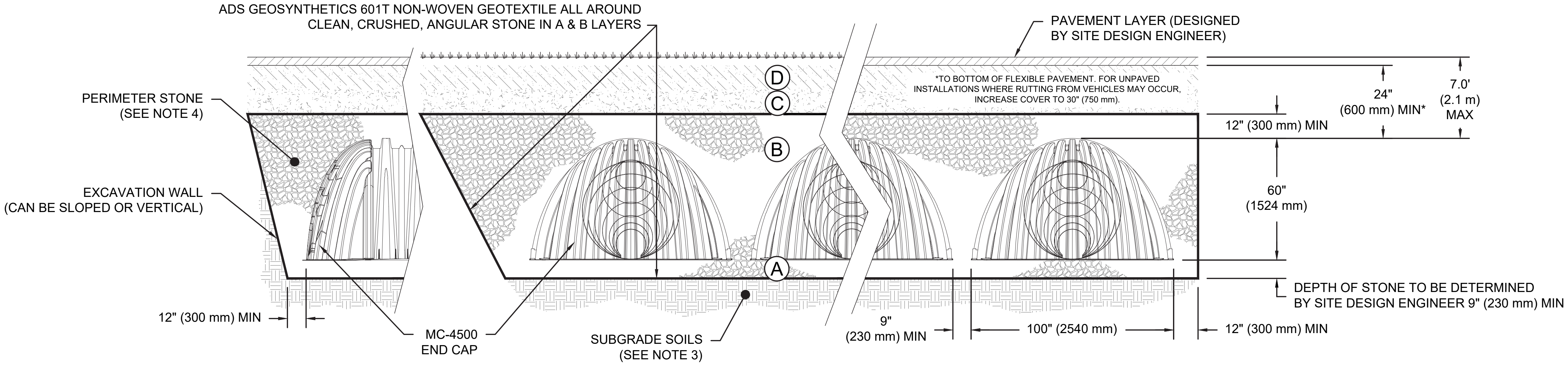
ACCEPTABLE FILL MATERIALS: STORMTECH MC-4500 CHAMBER SYSTEMS

MATERIAL LOCATION		DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	**FINAL FILL:** FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER	ANY SOIL/ROCK MATERIALS, NATIVE SOILS, OR PER ENGINEER'S PLANS. CHECK PLANS FOR PAVEMENT SUBGRADE REQUIREMENTS.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
C	**INITIAL FILL:** FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 24" (600 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	GRANULAR WELL-GRADED SOIL/AGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE. MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER.	AASHTO M145[1] A-1, A-2-4, A-3 OR AASHTO M43[1] 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 24" (600 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 12" (300 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS.
B	**EMBEDMENT STONE:** FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE.	CLEAN, CRUSHED, ANGULAR STONE	AASHTO M43[1] 3, 4	NO COMPACTION REQUIRED.
A	**FOUNDATION STONE:** FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	CLEAN, CRUSHED, ANGULAR STONE	AASHTO M43[1] 3, 4	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE.[2,3]

PLEASE NOTE:

1. THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".
2. STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 9" (230 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR.
3. WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGN LOAD CONDITIONS, A FLAT SURFACE MAY BE ACHIEVED BY RAKING OR DRAGGING WITHOUT COMPACTION EQUIPMENT. FOR SPECIAL LOAD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS.
4. ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.



*FOR COVER DEPTHS GREATER THAN 7.0' (2.1 m) PLEASE CONTACT STORMTECH

NOTES:

1. CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2418-16a, "STANDARD SPECIFICATION FOR POLYPROPYLENE (PP) CORRUGATED WALL STORMWATER COLLECTION CHAMBERS" CHAMBER CLASSIFICATION 60x101
2. MC-4500 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
3. THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS.
4. PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
5. REQUIREMENTS FOR HANDLING AND INSTALLATION:
   - TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS.
   - TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 3".
   - TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN SECTION 6.2.8 OF ASTM F2418 SHALL BE GREATER THAN OR EQUAL TO 500 LBS/IN/IN. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.

MC-4500
STANDARD CROSS SECTION

DATE:	05-10-19	DRAWN:	KR
PROJECT #:		CHECKED:	KR

DATE	DRWN	CHKD	DESCRIPTION

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MC-4500 TECHNICAL SPECIFICATION

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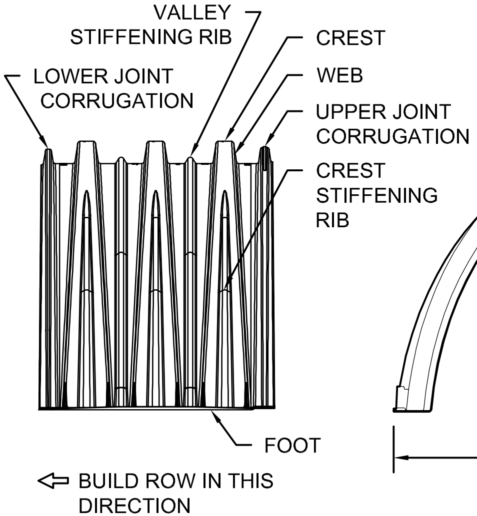
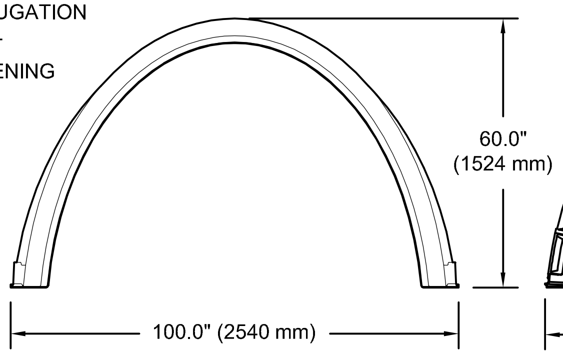
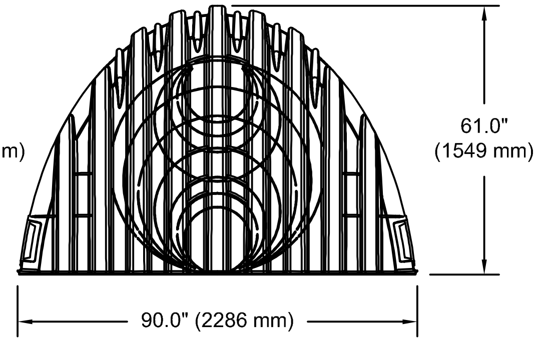
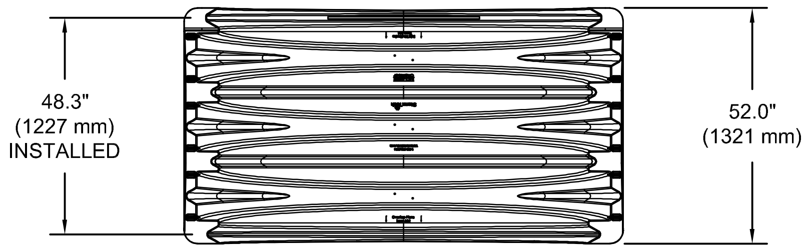
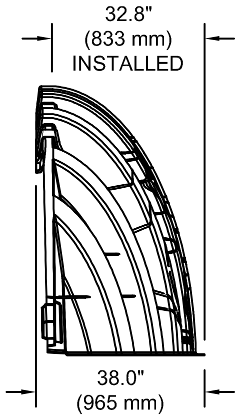










NOMINAL CHAMBER SPECIFICATIONS

SIZE (W X H X INSTALLED LENGTH)	100.0" X 60.0" X 48.3"	(2540 mm X 1524 mm X 1227 mm)
CHAMBER STORAGE	106.5 CUBIC FEET	(3.01 m³)
MINIMUM INSTALLED STORAGE*	162.6 CUBIC FEET	(4.60 m³)
WEIGHT (NOMINAL)	125.0 lbs.	(56.7 kg)

NOMINAL END CAP SPECIFICATIONS

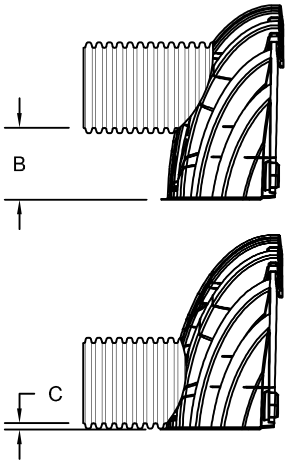
SIZE (W X H X INSTALLED LENGTH)	90.0" X 61.0" X 32.8"	(2286 mm X 1549 mm X 833 mm)
END CAP STORAGE	39.5 CUBIC FEET	(1.12 m³)
MINIMUM INSTALLED STORAGE*	115.3 CUBIC FEET	(3.26 m³)
WEIGHT (NOMINAL)	90 lbs.	(40.8 kg)

*ASSUMES 12" (305 mm) STONE ABOVE, 9" (229 mm) STONE FOUNDATION AND BETWEEN CHAMBERS, 12" (305 mm) STONE PERIMETER IN FRONT OF END CAPS AND 40% STONE POROSITY.

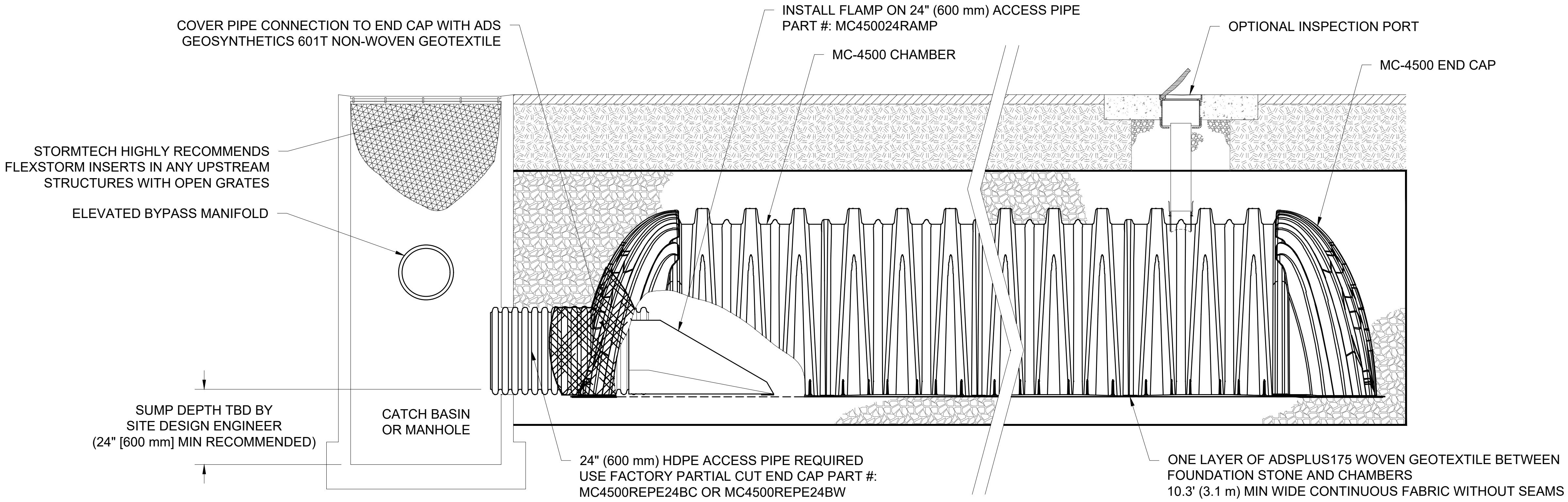
PARTIAL CUT HOLES AT BOTTOM OF END CAP FOR PART NUMBERS ENDING WITH "B"
PARTIAL CUT HOLES AT TOP OF END CAP FOR PART NUMBERS ENDING WITH "T"
END CAPS WITH A PREFABRICATED WELDED STUB END WITH "W"

PART #	STUB	B	C
MC4500IEPP06T	6" (150 mm)	42.54" (1081 mm)	---
MC4500IEPP06B		---	0.86" (22 mm)
MC4500IEPP08T	8" (200 mm)	40.50" (1029 mm)	---
MC4500IEPP08B		---	1.01" (26 mm)
MC4500IEPP10T	10" (250 mm)	38.37" (975 mm)	---
MC4500IEPP10B		---	1.33" (34 mm)
MC4500IEPP12T	12" (300 mm)	35.69" (907 mm)	---
MC4500IEPP12B		---	1.55" (39 mm)
MC4500IEPP15T	15" (375 mm)	32.72" (831 mm)	---
MC4500IEPP15B		---	1.70" (43 mm)
MC4500IEPP18T	18" (450 mm)	29.36" (746 mm)	---
MC4500IEPP18TW			
MC4500IEPP18B		---	1.97" (50 mm)
MC4500IEPP18BW			
MC4500IEPP24T	24" (600 mm)	23.05" (585 mm)	---
MC4500IEPP24TW			
MC4500IEPP24B		---	2.26" (57 mm)
MC4500IEPP24BW			
MC4500IEPP30BW	30" (750 mm)	---	2.95" (75 mm)
MC4500IEPP36BW	36" (900 mm)	---	3.25" (83 mm)
MC4500IEPP42BW	42" (1050 mm)	---	3.55" (90 mm)

NOTE: ALL DIMENSIONS ARE NOMINAL



CUSTOM PRECORED INVERTS ARE AVAILABLE UPON REQUEST. INVENTORIED MANIFOLDS INCLUDE 12-24" (300-600 mm) SIZE ON SIZE AND 15-48" (375-1200 mm) ECCENTRIC MANIFOLDS. CUSTOM INVERT LOCATIONS ON THE MC-4500 END CAP CUT IN THE FIELD ARE NOT RECOMMENDED FOR PIPE SIZES GREATER THAN 10" (250 mm). THE INVERT LOCATION IN COLUMN 'B' ARE THE HIGHEST POSSIBLE FOR THE PIPE SIZE.



MC-4500 ISOLATOR ROW PLUS DETAIL

NTS

INSPECTION & MAINTENANCE

STEP 1) INSPECT ISOLATOR ROW PLUS FOR SEDIMENT
- A. INSPECTION PORTS (IF PRESENT)
  - A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN
  - A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
  - A.3. USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
  - A.4. LOWER A CAMERA INTO ISOLATOR ROW PLUS FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL)
  - A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- B. ALL ISOLATOR PLUS ROWS
  - B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW PLUS
  - B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW PLUS THROUGH OUTLET PIPE
    - i) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY
    - ii) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE
  - B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.

STEP 2) CLEAN OUT ISOLATOR ROW PLUS USING THE JETVAC PROCESS
- A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45" (1.1 m) OR MORE IS PREFERRED
- B. APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN
- C. VACUUM STRUCTURE SUMP AS REQUIRED

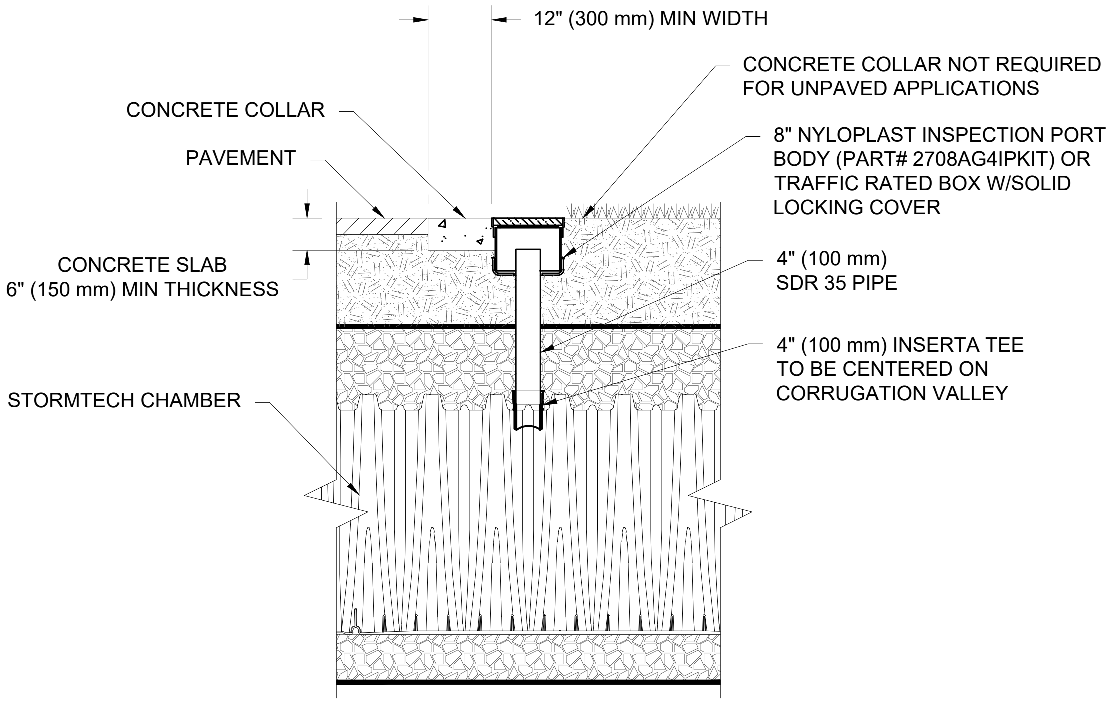
STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.

STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

NOTES

1. INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.

2. CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.



NOTE:
INSPECTION PORTS MAY BE CONNECTED THROUGH ANY CHAMBER CORRUGATION VALLEY.

4" PVC INSPECTION PORT DETAIL
(MC SERIES CHAMBER)

NTS

MC-4500
ISOLATOR ROW PLUS DETAILS

DATE: 08/26/20
DRAWN: ALI
PROJECT #: ----
CHECKED: ALI

DATE	DRWN	CHKD	DESCRIPTION

StormTech®
Detention•Retention•Water Quality
520 CROMWELL AVENUE | ROCKY HILL | CT | 06067
860-529-8188 | 888-892-2694 | WWW.STORMTECH.COM

ADS®
ADVANCED DRAINAGE SYSTEMS, INC.
4640 TRUEMAN BLVD
HILLIARD, OH 43026

SHEET 1 OF 1

THIS DRAWING HAS BEEN PREPARED BASED ON INFORMATION PROVIDED TO ADS UNDER THE DIRECTION OF THE SITE DESIGN ENGINEER OR OTHER PROJECT REPRESENTATIVE. THE SITE DESIGN ENGINEER SHALL REVIEW THIS DRAWING PRIOR TO CONSTRUCTION. IT IS THE ULTIMATE RESPONSIBILITY OF THE SITE DESIGN ENGINEER TO ENSURE THAT THE PRODUCT(S) DEPICTED AND ALL ASSOCIATED DETAILS MEET ALL APPLICABLE LAWS, REGULATIONS, AND PROJECT REQUIREMENTS.

CONTECH®
ENGINEERED SOLUTIONS

Jellyfish® Filter
Stormwater Treatment

The experts you need to solve your stormwater challenges

Contech is the leader in stormwater solutions, helping engineers, contractors and owners with infrastructure and land development projects throughout North America.

With our responsive team of stormwater experts, local regulatory expertise and flexible solutions, Contech is the trusted partner you can count on for stormwater management solutions.

Your Contech Team

STORMWATER CONSULTANT

It's my job to recommend the best solution to meet permitting requirements.

STORMWATER DESIGN ENGINEER

I work with consultants to design the best approved solution to meet your project's needs.

REGULATORY MANAGER

I understand the local stormwater regulations and what solutions will be approved.

SALES ENGINEER

I make sure our solutions meet the needs of the contractor during construction.

Contech is your partner in stormwater management solutions

Setting new standards in Stormwater Treatment – Jellyfish® Filter

The Jellyfish Filter has been tested in the field and laboratory, and has received approval from numerous stormwater regulatory agencies.

The Jellyfish Filter is a stormwater quality treatment technology featuring high flow pretreatment and membrane filtration in a compact stand-alone system. Jellyfish removes floatables, trash, oil, debris, TSS, fine silt-sized particles, and a high percentage of particulate-bound pollutants; including phosphorus, nitrogen, metals and hydrocarbons. The high surface area membrane cartridges, combined with up-flow hydraulics, frequent, passive backwashing, and rinseable/reusable cartridges ensure long-lasting performance.

Jellyfish® Filter

How the Jellyfish® Filter Treats Stormwater

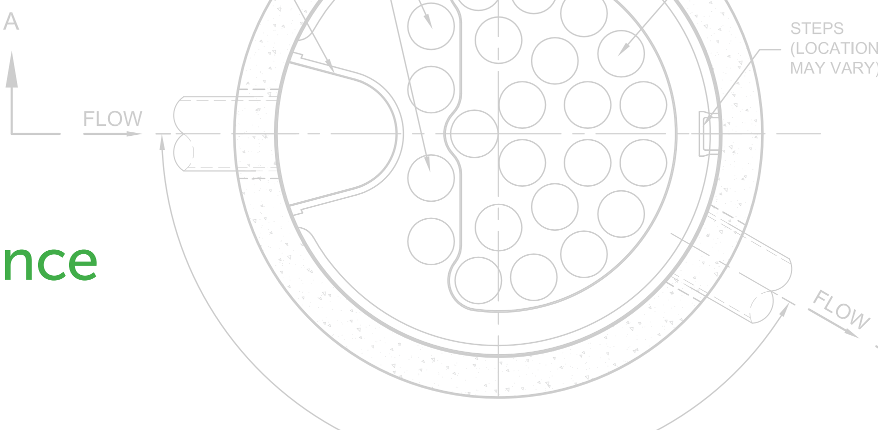
Tested in the field and laboratory ...

- Stormwater enters the Jellyfish through the inlet pipe and traps floating pollutants behind the maintenance access wall and below the cartridge deck.
- Water is conveyed below the cartridge deck where a separation skirt around the cartridges isolates oil, trash and debris outside the filtration zone.
- Water is directed to the filtration zone and up through the top of the cartridge where it exits via the outlet pipe.
- The membrane filters provide a very large surface area to effectively remove fine sand and silt-sized particles, and a high percentage of particulate-bound pollutants such as nitrogen, phosphorus, metals, and hydrocarbons while ensuring long-lasting treatment.
- As influent flow subsides, the water in the backwash pool flows back into the lower chamber. This passive backwash extends cartridge life.
- The draindown cartridge(s) located outside the backwash pool enables water levels to balance.

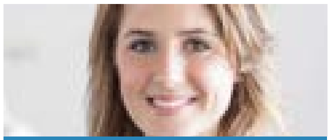
Jellyfish® Filter
Standard Configuration

Learn More:
www.ContechES.com/jellyfish

Pretreat bioretention or infiltration with Jellyfish to extend service life.



Jellyfish® Filter Performance Testing Results



APPLICATION TIPS

- The Peak Diversion Jellyfish provides treatment and high-flow bypass in one structure, eliminating the need for a separate bypass structure.
- LID and GI are complemented by filtration solutions, as they help keep sites free from fine sediments that can impede performance, remove unsightly trash, and provide a single point of maintenance.
- Selecting a filter with a long maintenance cycle and low maintenance cost will result in healthy waterways and happy property owners.

The pleated tentacles of the Jellyfish® Filter provide a large surface area for pollutant removal.

POLLUTANT OF CONCERN	% REMOVAL
Total Trash	99%
Total Suspended Solids (TSS)	89%
Total Phosphorus (TP)	59%
Total Nitrogen (TN)	51%
Total Copper (TCu)	> 50%
Total Zinc (TZn)	> 50%

Sources:
TARP II Field Study – 2012 JF 4-2-1 Configuration
MRDC Floatables Testing – 2008 JF6-6-1 Configuration

Jellyfish® Filter Features and Benefits

FEATURE	BENEFITS
High surface area membrane filtration	Low flux rate promotes cake filtration and slows membrane occlusion
High design treatment flow rate per cartridge (up to 80 gpm (5 L/s))	Compact system with a small footprint, lower construction cost
Low driving head (typically 18 inches or less (457 mm))	Design flexibility, lower construction cost
Lightweight cartridges with passive backwash	Easy maintenance and low life-cycle cost

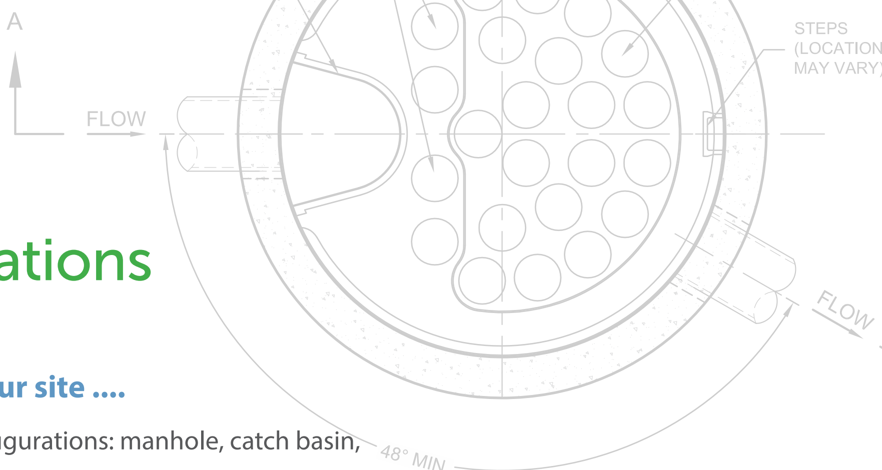
The Jellyfish Filter can be configured in a manhole, catch basin, or vault.

Select Jellyfish® Filter Certifications and Verifications

The Jellyfish Filter has been reviewed by numerous state and federal programs, including:

- Washington State Department of Ecology (TAPE) GULD – BASIC, Phosphorus
- Virginia Department of Environmental Quality (VA DEQ)
- Texas Commission of Environmental Quality (TCEQ)
- Canada ISO 14034 Environmental Management – Environmental Technology Verification (ETV)
- Philadelphia Water District (PWD)
- Maryland Department of the Environment (MD DOE)

Field tested and performance verified

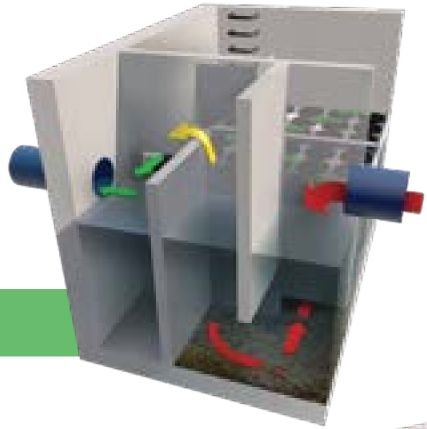


Jellyfish® Filter Configurations

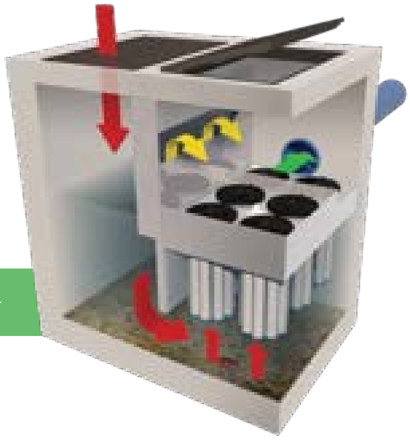
Multiple system configurations to optimize your site ....

The Jellyfish Filter can be manufactured in a variety of configurations: manhole, catch basin, vault, fiberglass tank, or custom configurations. Typically, 18 inches (457 mm) of driving head is designed into the system. For low drop sites, the designed driving head can be less.


Manhole


Vault


Curb Inlet


Grated Inlet

Jellyfish® Filter Maintenance

- Jellyfish Filter cartridges are light weight and reusable
- Maintenance of the filter cartridges is performed by removing, rinsing and reusing the cartridge tentacles.
- Vacuum extraction of captured pollutants in the sump is recommended at the same time.
- Full cartridge replacement intervals differ by site due to varying pollutant loading and type, and maintenance frequency. Replacement is anticipated every 2-5 years.
- Contech® has created a network of Certified Maintenance Providers to provide maintenance on stormwater BMP's.


The Jellyfish® Filter tentacle is light and easy to clean.

A partner you can rely on

STORMWATER SOLUTIONS

PIPE SOLUTIONS

STRUCTURES SOLUTIONS

Few companies offer the wide range of high-quality stormwater resources you can find with us — state-of-the-art products, decades of expertise, and all the maintenance support you need to operate your system cost-effectively.

THE CONTECH WAY

Contech® Engineered Solutions provides innovative, cost-effective site solutions to engineers, contractors, and developers on projects across North America. Our portfolio includes bridges, drainage, erosion control, retaining wall, sanitary sewer and stormwater management products.

TAKE THE NEXT STEP

For more information: www.ContechES.com

CONTECH® ENGINEERED SOLUTIONS

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Appendix E

NRCS Soils Report

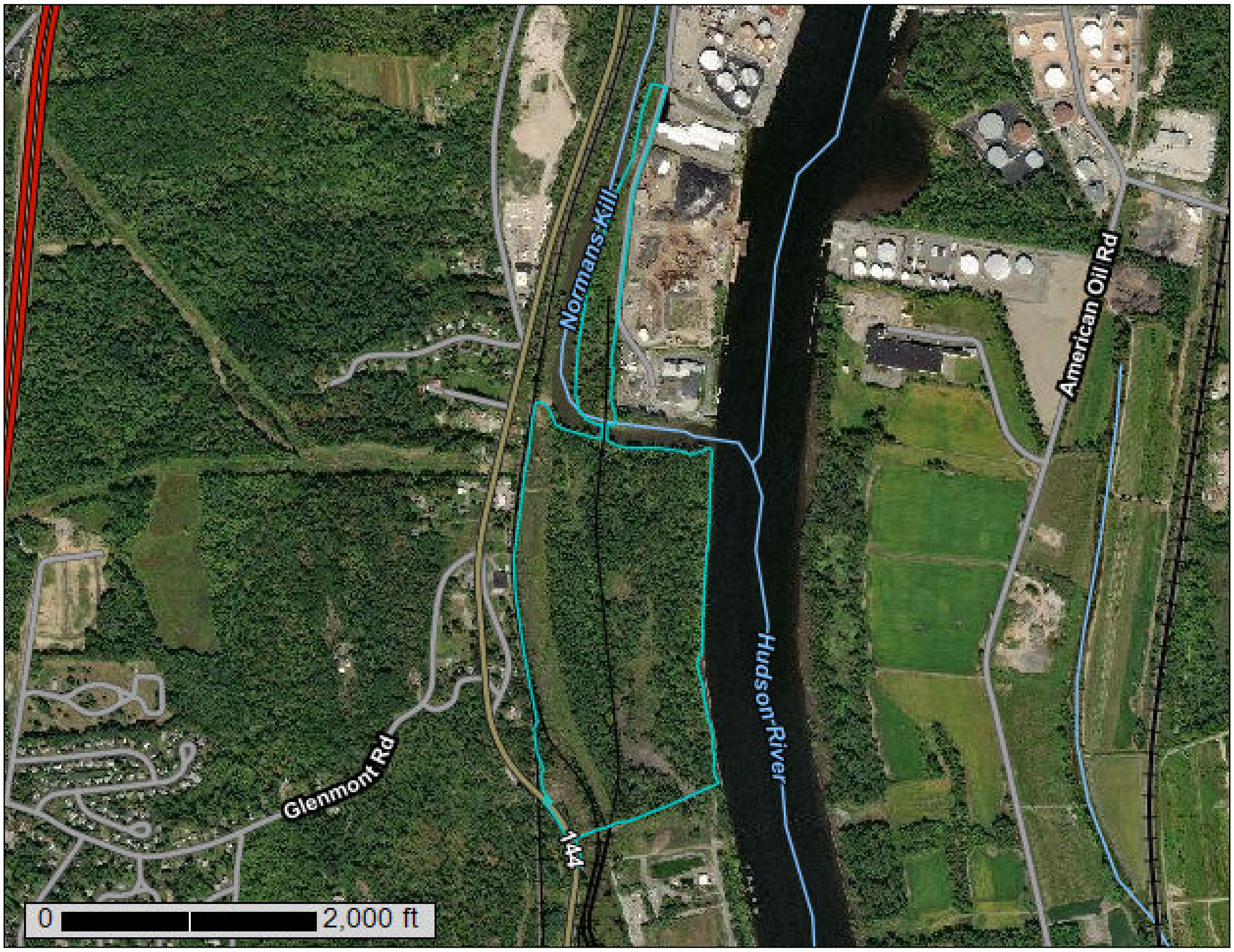
United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for
Albany County, New York



January 20, 2022

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

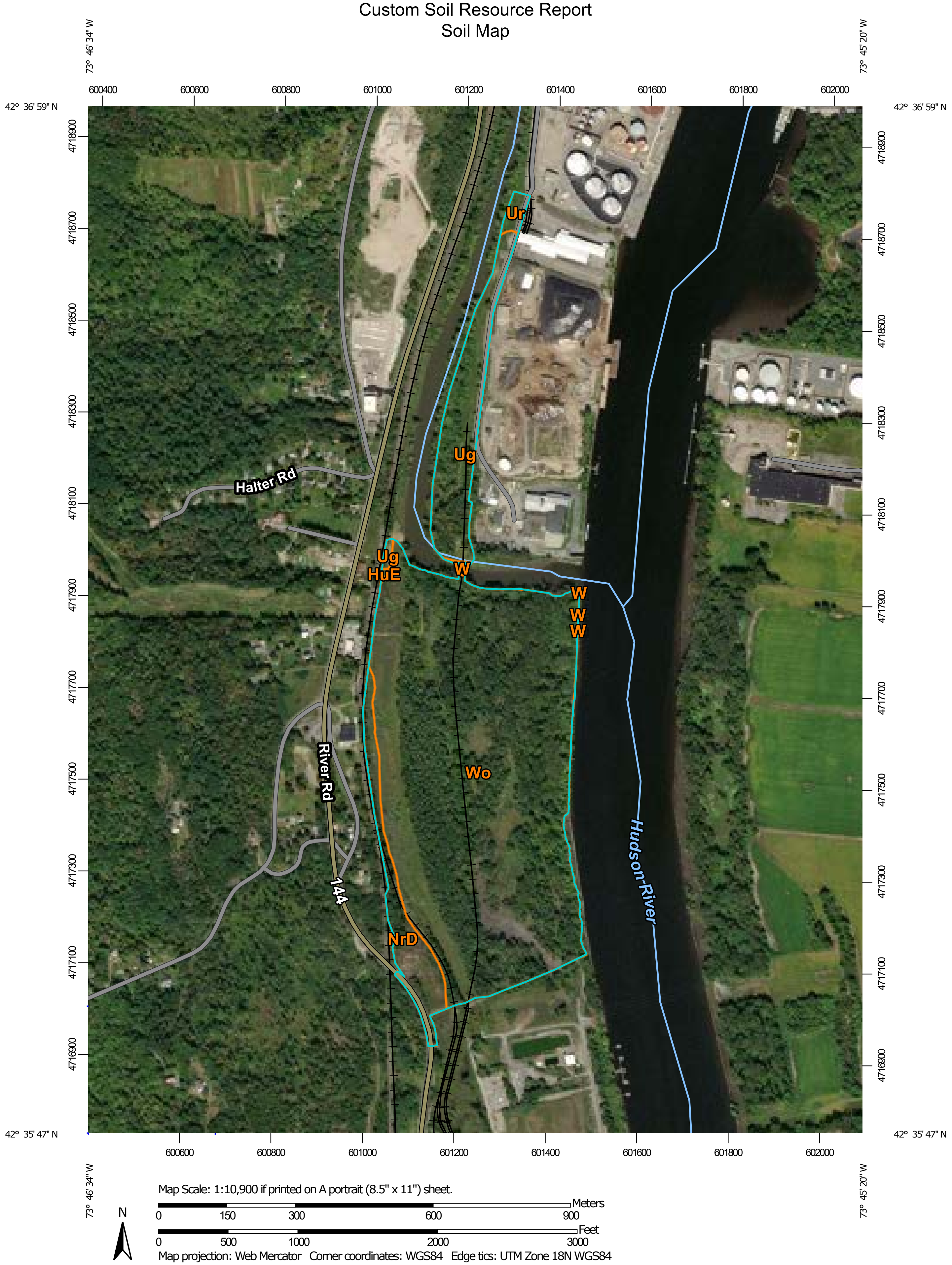
Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Soil Map



Map Scale: 1:10,900 if printed on A portrait (8.5" x 11") sheet.

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 18N WGS84

MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons

Soil Map Unit Lines

Soil Map Unit Points

Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Slide or Slip

Sodic Spot

Spoil Area

Stony Spot

Very Stony Spot

Wet Spot

Other

Special Line Features

Water Features

Streams and Canals

Transportation

Rails

Interstate Highways

US Routes

Major Roads

Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15,800.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Albany County, New York
Survey Area Data: Version 19, Aug 29, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 1, 2014—Sep 22, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
HuE	Hudson silt loam, 25 to 45 percent slopes	0.1	0.1%
NrD	Nassau very channery silt loam, hilly, very rocky	7.2	6.7%
Ug	Udorthents, loamy	11.6	10.7%
Ur	Urban land	0.8	0.8%
W	Water	0.1	0.1%
Wo	Wayland soils complex, non-calcareous substratum, 0 to 3 percent slopes, frequently flooded	88.7	81.7%
Totals for Area of Interest		**108.6**	**100.0%**

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Albany County, New York

HuE—Hudson silt loam, 25 to 45 percent slopes

Map Unit Setting

National map unit symbol: 9pg8
Elevation: 300 to 1,800 feet
Mean annual precipitation: 36 to 41 inches
Mean annual air temperature: 45 to 48 degrees F
Frost-free period: 100 to 170 days
Farmland classification: Not prime farmland

Map Unit Composition

Hudson and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Hudson

Setting

Landform: Lake plains
Landform position (two-dimensional): Summit
Landform position (three-dimensional): Riser
Down-slope shape: Concave
Across-slope shape: Convex
Parent material: Clayey and silty glaciolacustrine deposits

Typical profile

H1 - 0 to 11 inches: silt loam
H2 - 11 to 16 inches: silty clay loam
H3 - 16 to 31 inches: silty clay
H4 - 31 to 60 inches: clay

Properties and qualities

Slope: 25 to 45 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Moderately well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)
Depth to water table: About 18 to 24 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 15 percent
Available water supply, 0 to 60 inches: High (about 9.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7e
Hydrologic Soil Group: C/D
Ecological site: F144AY018NY - Moist Lake Plain
Hydric soil rating: No

Minor Components

Unnamed soils

Percent of map unit: 5 percent

Unadilla

Percent of map unit: 5 percent
Hydric soil rating: No

Colonie

Percent of map unit: 3 percent
Hydric soil rating: No

Udifluvents

Percent of map unit: 1 percent
Hydric soil rating: No

Fluvaquents

Percent of map unit: 1 percent
Landform: Flood plains
Hydric soil rating: Yes

NrD—Nassau very channery silt loam, hilly, very rocky

Map Unit Setting

National map unit symbol: 9ph1
Elevation: 600 to 1,800 feet
Mean annual precipitation: 36 to 41 inches
Mean annual air temperature: 45 to 48 degrees F
Frost-free period: 100 to 170 days
Farmland classification: Not prime farmland

Map Unit Composition

Nassau, hilly, and similar soils: 70 percent
Minor components: 30 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Nassau, Hilly

Setting

Landform: Benches, ridges, till plains
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Convex
Across-slope shape: Convex
Parent material: Channery loamy till derived mainly from local slate or shale

Typical profile

H1 - 0 to 8 inches: very channery silt loam
H2 - 8 to 16 inches: very channery silt loam
H3 - 16 to 20 inches: unweathered bedrock

Properties and qualities

Slope: 15 to 25 percent
Depth to restrictive feature: 10 to 20 inches to lithic bedrock
Drainage class: Somewhat excessively drained

Capacity of the most limiting layer to transmit water (Ksat): Very low (0.00 to 0.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Very low (about 1.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6s
Hydrologic Soil Group: D
Ecological site: F144AY033MA - Shallow Dry Till Uplands
Hydric soil rating: No

Minor Components

Rock outcrop

Percent of map unit: 9 percent
Hydric soil rating: Unranked

Manlius

Percent of map unit: 8 percent
Hydric soil rating: No

Unnamed soils

Percent of map unit: 8 percent

Lordstown

Percent of map unit: 5 percent
Hydric soil rating: No

Ug—Udorthents, loamy

Map Unit Setting

National map unit symbol: 9pj1
Elevation: 0 to 1,640 feet
Mean annual precipitation: 36 to 41 inches
Mean annual air temperature: 45 to 48 degrees F
Frost-free period: 100 to 170 days
Farmland classification: Not prime farmland

Map Unit Composition

Udorthents, loamy, and similar soils: 90 percent
Minor components: 10 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Udorthents, Loamy

Typical profile

H1 - 0 to 4 inches: loam
H2 - 4 to 70 inches: channery loam

Properties and qualities

Slope: 0 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Moderately well drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.06 to 5.95 in/hr)
Depth to water table: About 36 to 72 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 15 percent
Available water supply, 0 to 60 inches: Low (about 5.5 inches)

Minor Components

Unnamed soils

Percent of map unit: 10 percent

Ur—Urban land

Map Unit Setting

National map unit symbol: 9pj8
Mean annual precipitation: 36 to 41 inches
Mean annual air temperature: 45 to 48 degrees F
Frost-free period: 100 to 170 days
Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Urban Land

Typical profile

H1 - 0 to 6 inches: variable

Minor Components

Unnamed soils

Percent of map unit: 10 percent

Udorthents

Percent of map unit: 5 percent
Hydric soil rating: No

W—Water

Map Unit Composition

Water: 100 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Wo—Wayland soils complex, non-calcareous substratum, 0 to 3 percent slopes, frequently flooded

Map Unit Setting

National map unit symbol: 2srgt
Elevation: 160 to 1,970 feet
Mean annual precipitation: 31 to 70 inches
Mean annual air temperature: 43 to 52 degrees F
Frost-free period: 105 to 180 days
Farmland classification: Not prime farmland

Map Unit Composition

Wayland and similar soils: 60 percent
Wayland, very poorly drained, and similar soils: 30 percent
Minor components: 10 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Wayland

Setting

Landform: Flood plains
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Silty and clayey alluvium derived from interbedded sedimentary rock

Typical profile

Ap - 0 to 9 inches: silt loam
Bg - 9 to 21 inches: silt loam
Cg1 - 21 to 28 inches: silt loam
Cg2 - 28 to 47 inches: silt loam
Cg3 - 47 to 54 inches: silt loam
Cg4 - 54 to 60 inches: silt loam

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Poorly drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.14 to 14.17 in/hr)
Depth to water table: About 0 to 6 inches

Frequency of flooding: FrequentNone
Frequency of ponding: None
Calcium carbonate, maximum content: 5 percent
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Very high (about 13.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 5w
Hydrologic Soil Group: B/D
Hydric soil rating: Yes

Description of Wayland, Very Poorly Drained

Setting

Landform: Flood plains
Landform position (three-dimensional): Tread
Down-slope shape: Concave
Across-slope shape: Concave
Parent material: Silty and clayey alluvium derived from interbedded sedimentary rock

Typical profile

A - 0 to 9 inches: mucky silt loam
Bg - 9 to 21 inches: silt loam
Cg1 - 21 to 28 inches: silt loam
Cg2 - 28 to 47 inches: silt loam
Cg3 - 47 to 54 inches: silt loam
Cg4 - 54 to 60 inches: silt loam

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately low to high (0.14 to 14.17 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: NoneFrequent
Frequency of ponding: Frequent
Calcium carbonate, maximum content: 5 percent
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Very high (about 13.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 5w
Hydrologic Soil Group: B/D
Hydric soil rating: Yes

Minor Components

Holderton

Percent of map unit: 10 percent
Landform: Flood plains
Landform position (three-dimensional): Tread
Down-slope shape: Linear
Across-slope shape: Linear

Hydric soil rating: No

Soil Information for All Uses

Soil Properties and Qualities

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

Soil Qualities and Features

Soil qualities are behavior and performance attributes that are not directly measured, but are inferred from observations of dynamic conditions and from soil properties. Example soil qualities include natural drainage, and frost action. Soil features are attributes that are not directly part of the soil. Example soil features include slope and depth to restrictive layer. These features can greatly impact the use and management of the soil.

Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

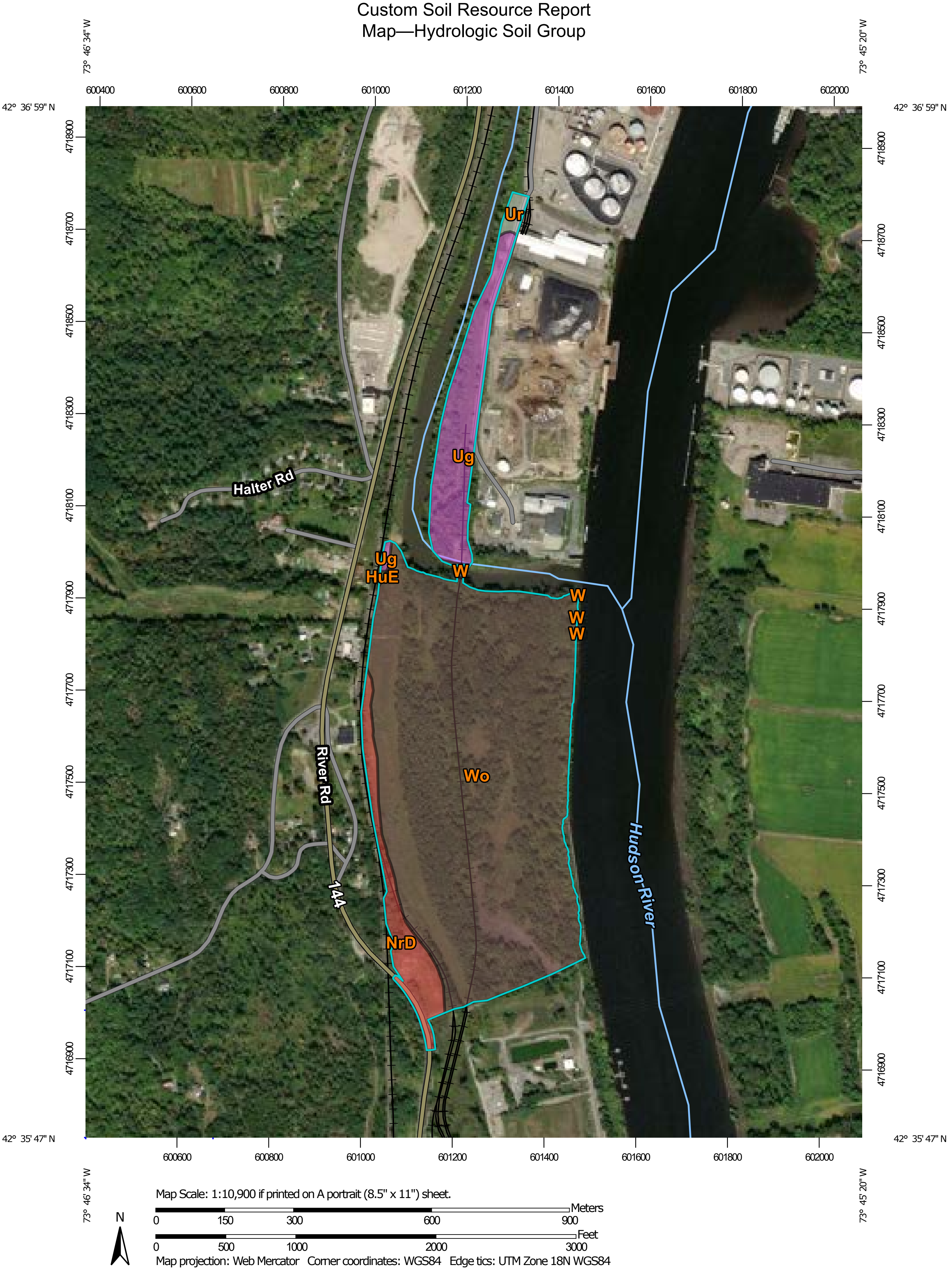
Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Custom Soil Resource Report
Map—Hydrologic Soil Group



Map Scale: 1:10,900 if printed on A portrait (8.5" x 11") sheet.

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 18N WGS84

MAP LEGEND

Area of Interest (AOI)

- ▭ Area of Interest (AOI)

Soils

Soil Rating Polygons

- 🟪 A
- 🟩 A/D
- 🟦 B
- 🟫 B/D
- 🟦 C
- 🟨 C/D
- 🟥 D
- ⬜ Not rated or not available

Soil Rating Lines

- 〰 A
- 〰 A/D
- 〰 B
- 〰 B/D
- 〰 C
- 〰 C/D
- 〰 D
- 〰 Not rated or not available

Soil Rating Points

- ▣ A
- ▣ A/D
- ▣ B
- ▣ B/D
- ▣ C
- ▣ C/D
- ▣ D
- ▣ Not rated or not available

Water Features

- 〰 Streams and Canals

Transportation

- ┼┼┼ Rails
- 〰 Interstate Highways
- 〰 US Routes
- 〰 Major Roads
- 〰 Local Roads

Background

- ▨ Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15,800.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Albany County, New York
Survey Area Data: Version 19, Aug 29, 2021

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 1, 2014—Sep 22, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Table—Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
HuE	Hudson silt loam, 25 to 45 percent slopes	C/D	0.1	0.1%
NrD	Nassau very channery silt loam, hilly, very rocky	D	7.2	6.7%
Ug	Udorthents, loamy	A	11.6	10.7%
Ur	Urban land		0.8	0.8%
W	Water		0.1	0.1%
Wo	Wayland soils complex, non-calcareous substratum, 0 to 3 percent slopes, frequently flooded	B/D	88.7	81.7%
Totals for Area of Interest			**108.6**	**100.0%**

Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

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Appendix F

Infiltration Test Results

June 7, 2022

McFarland-Johnson, Inc.
66 Railroad Place – Suite 402
Saratoga Springs, NY 12866

Attn: Mr. Steven Boisvert, P.E.
p: (518) 580-9380
e: sboisvert@mjinc.com

Re: Infiltration Testing
Proposed Marmen Manufacturing Facility
Port of Albany, NY
Terracon Project No. JB215020

Dear Mr. Boisvert:

This report presents the results of the supplemental subsurface investigation and infiltration testing program completed by Terracon at the referenced site.

A total of 21 test borings (IT-1 thru IT-15, along with IT-1A, IT-7A, IT-8A, IT-10A, IT-10B and IT-12A) were completed as part of the investigation, with their locations and depths specified by McFarland-Johnson. Individual subsurface logs for each borehole are attached herewith, along with a subsurface investigation plan(s) indicating their locations.

Infiltration tests were performed adjacent to each of these test borings and were numbered correspondingly. The tests were conducted using 4-inch diameter PVC pipe in general accord with the guidelines in Appendix D of the NYS Stormwater Management Design Manual. Results of this testing are presented for your use on the attached infiltration test data sheets, along with a tabular summary.

We appreciate the opportunity to be of service on this project. Please contact us at your convenience if you have any questions or if anything further is needed.

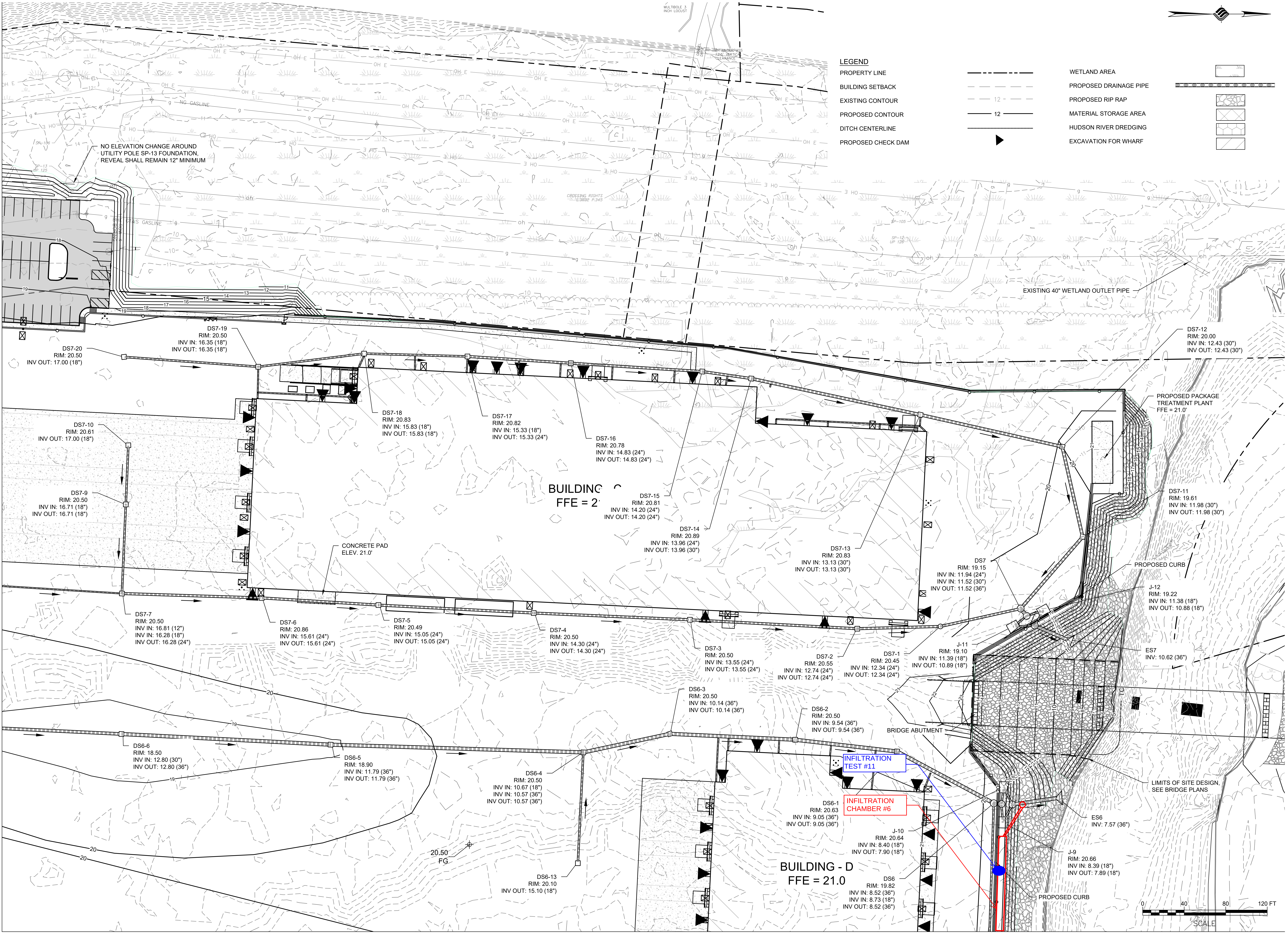
Respectfully,
Terracon Consultants – NY, Inc.

John S. Hutchison, P.E.
Senior Geotechnical Engineer

Joseph Robichaud, Jr., P.E.
Principal / Office Manager

Attachments:
- Subsurface investigation plan(s)
- Test boring logs
- Infiltration test results summary table
- Infiltration test data sheets

Terracon Consultants – NY, Inc. 30 Corporate Circle, Suite 201 Albany, NY 12203
p (518) 266-0310 f (518) 266-9238 terracon.com



McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	AS SHOWN
DATE	04/19/2022
PROJECT	18641.00

FOR REVIEW NOT FOR CONSTRUCTION

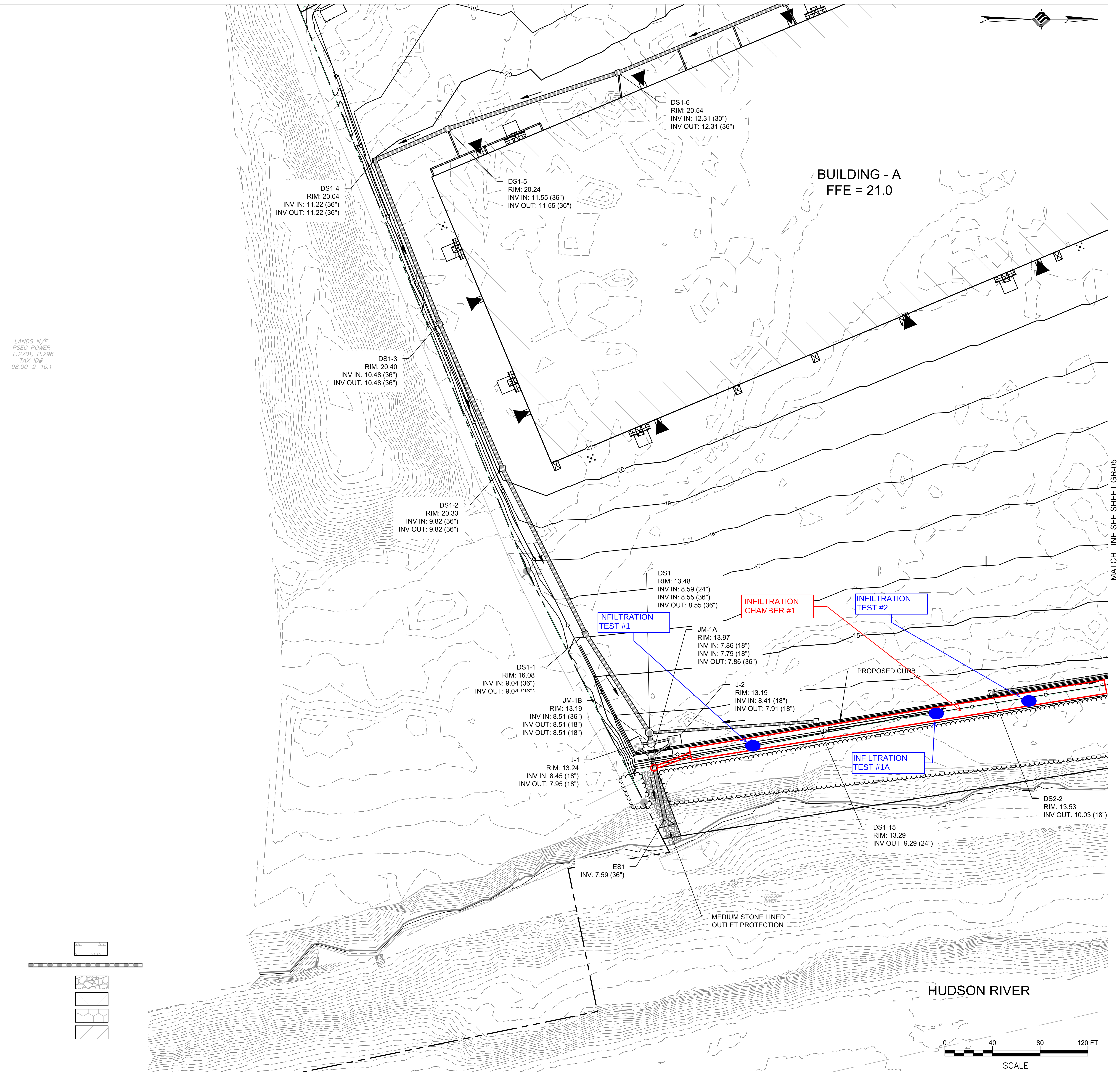
IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
GRADING & DRAINAGE

DRAWING NUMBER
GR-03

22 OF 67

MATCH LINE SEE SHEET GR-01



MATCH LINE SEE SHEET GR-05

LEGEND

PROPERTY LINE	— - - — - - —	WETLAND AREA	▭
BUILDING SETBACK	— — — —	PROPOSED DRAINAGE PIPE	▬
EXISTING CONTOUR	— — 12 — —	PROPOSED RIP RAP	▨
PROPOSED CONTOUR	—— 12 ——	MATERIAL STORAGE AREA	⊠
DITCH CENTERLINE	————	HUDSON RIVER DREDGING	▨
PROPOSED CHECK DAM	▶	EXCAVATION FOR WHARF	▨

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES\18641.00-GRAD.DWG

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE

FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	AS SHOWN
DATE	04/19/2022
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DRAWING TITLE

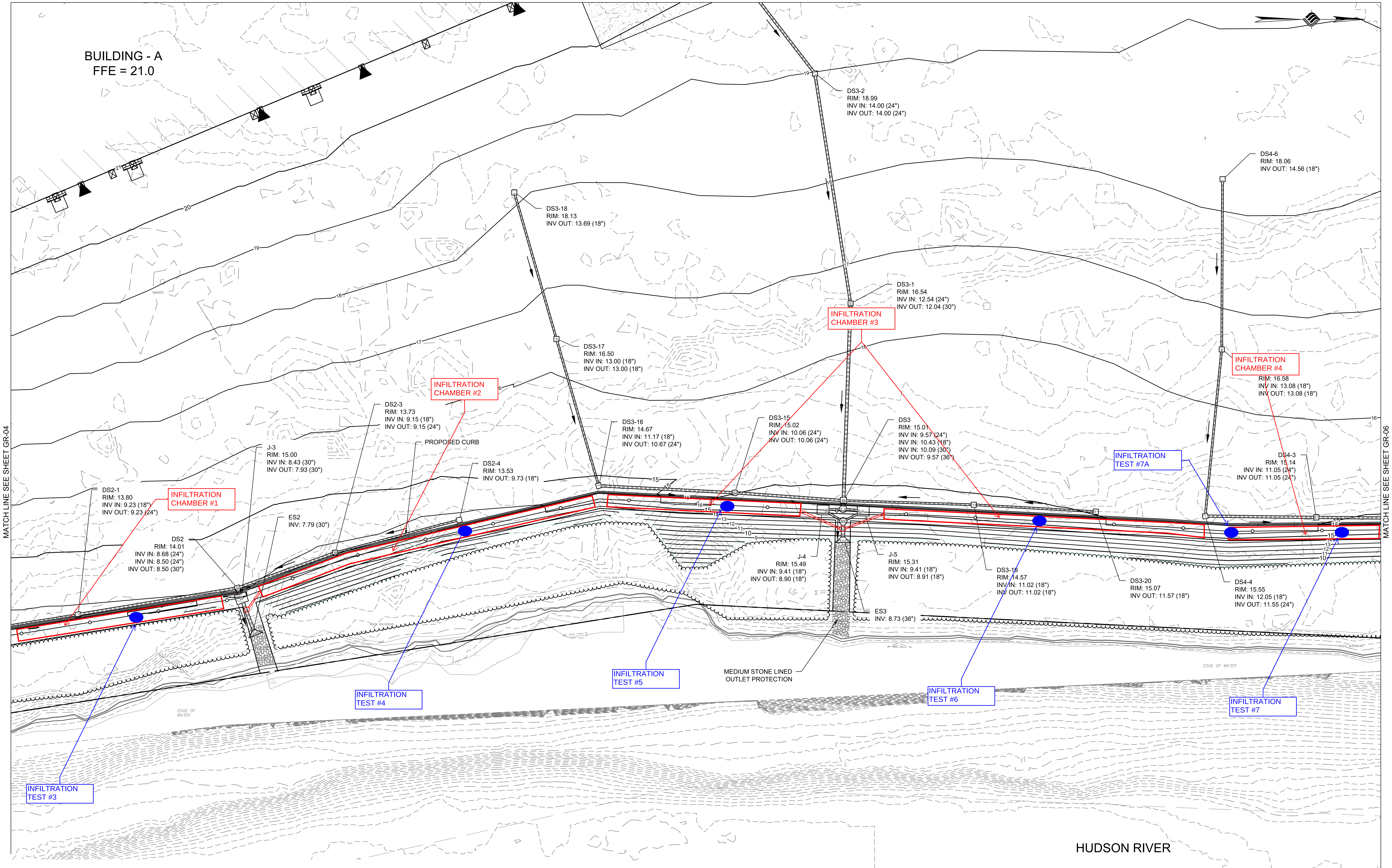
GRADING & DRAINAGE

DRAWING NUMBER

GR-04

23 OF 67

MATCH LINE SEE SHEET GR-02



LEGEND

PROPERTY LINE	—·—··—
BUILDING SETBACK	– – – –
EXISTING CONTOUR	– – 12 – –
PROPOSED CONTOUR	—— 12 ——
DITCH CENTERLINE	————
PROPOSED CHECK DAM	▶
WETLAND AREA	▨
PROPOSED DRAINAGE PIPE	▬
PROPOSED RIP RAP	▨
MATERIAL STORAGE AREA	☒
HUDSON RIVER DREDGING	▨
EXCAVATION FOR WHARF	▨

0 40 80 120 FT
SCALE

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McFarland Johnson
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SaratogaROM@mjinc.com

PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	AS SHOWN
DATE	04/19/2022
PROJECT	18641.00

FOR REVIEW NOT FOR CONSTRUCTION

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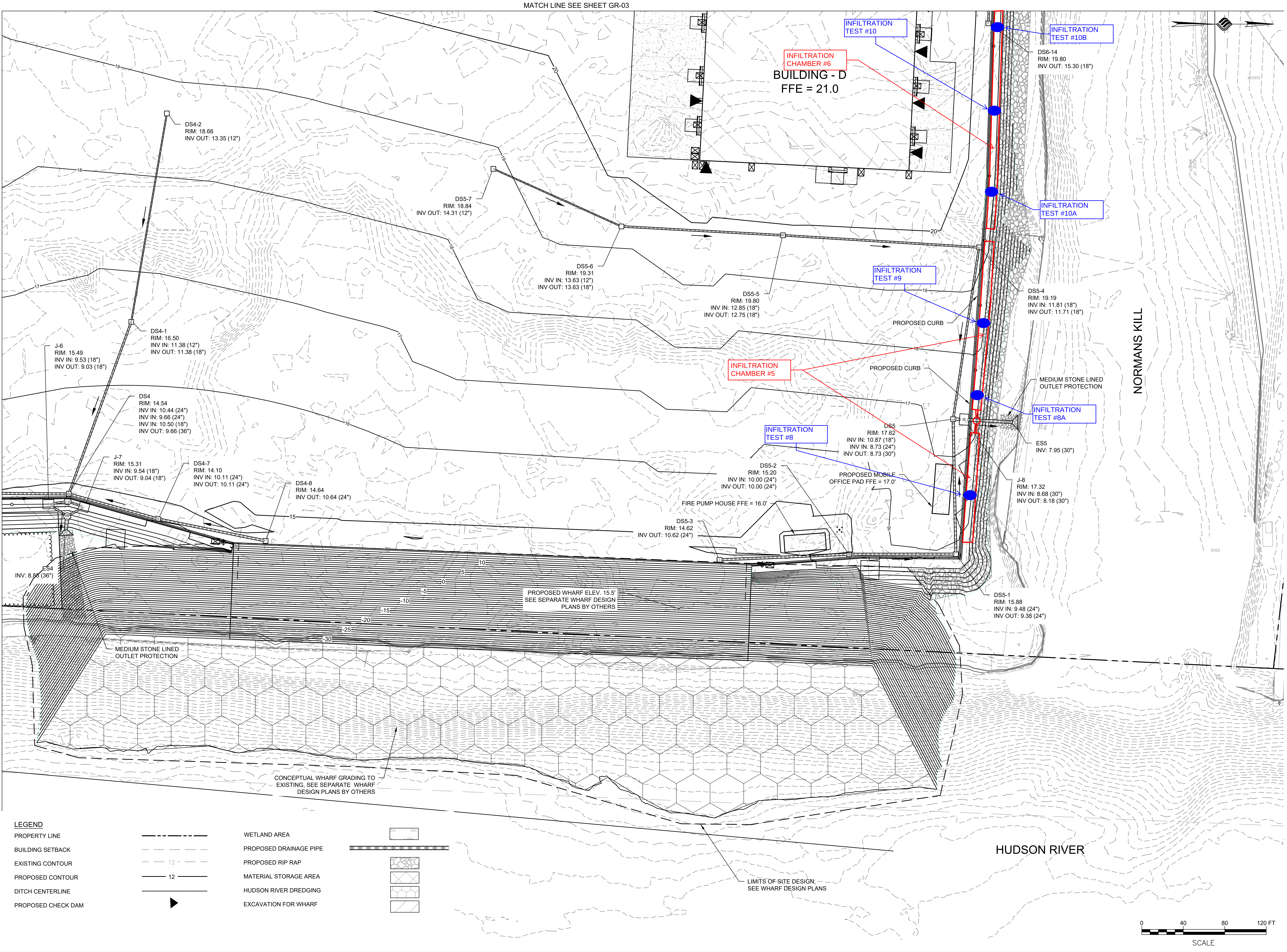
DRAWING TITLE
GRADING & DRAINAGE

DRAWING NUMBER
GR-05

24 OF 67



LEGEND

PROPERTY LINE	— - — - —	WETLAND AREA	▭
BUILDING SETBACK	— — — —	PROPOSED DRAINAGE PIPE	═══
EXISTING CONTOUR	— — 12 — —	PROPOSED RIP RAP	▨
PROPOSED CONTOUR	—— 12 ——	MATERIAL STORAGE AREA	☒
DITCH CENTERLINE	———	HUDSON RIVER DREDGING	⬡
PROPOSED CHECK DAM	▶	EXCAVATION FOR WHARF	▨

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES\18641.00-GRAD.DWG

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SaratogaROM@mjinc.com

PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	AS SHOWN
DATE	04/19/2022
PROJECT	18641.00

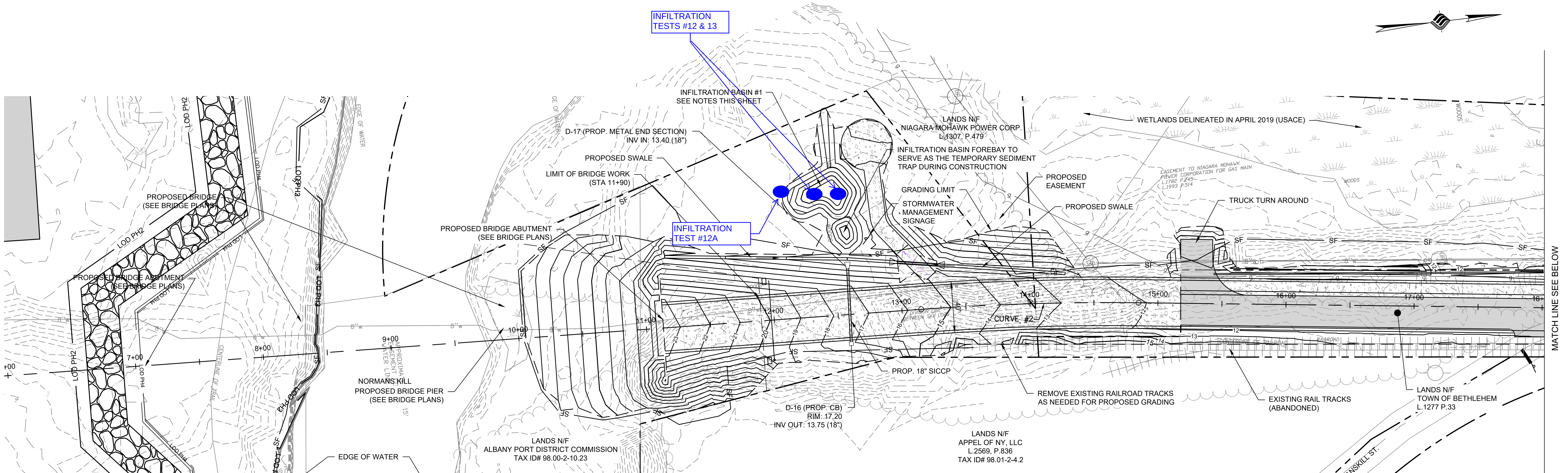
FOR REVIEW NOT FOR CONSTRUCTION

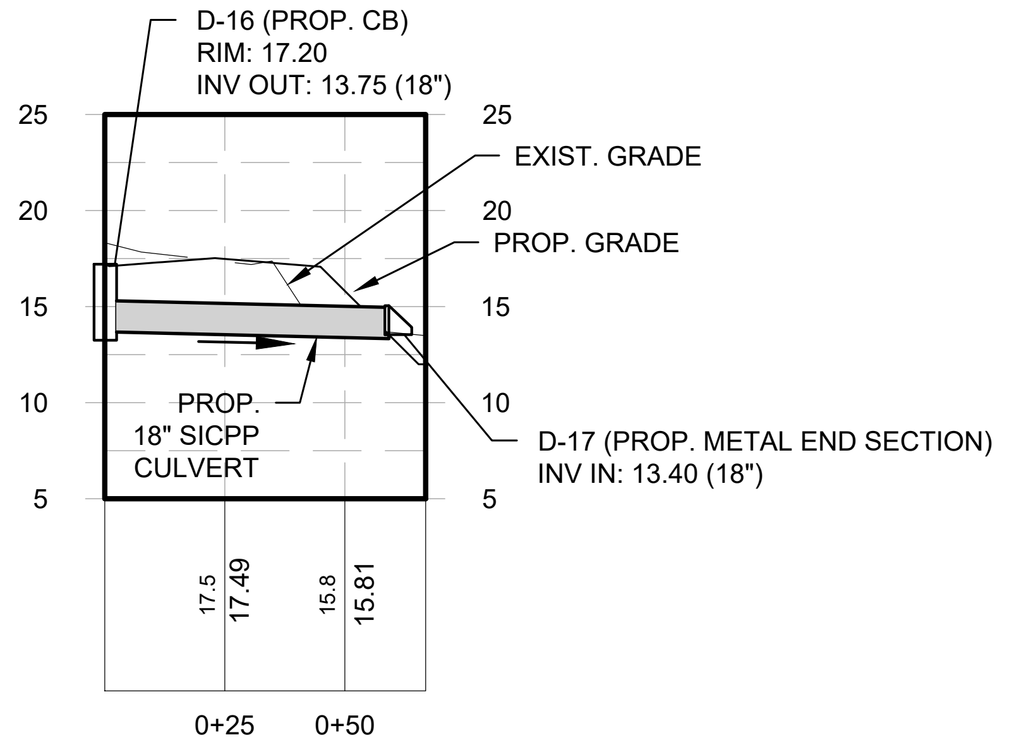
IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
GRADING &
DRAINAGE

DRAWING NUMBER
GR-06

25 OF 67





D-16 TO D-17

Horizontal Scale: 1" = 40'
Vertical Scale: 1" = 10'

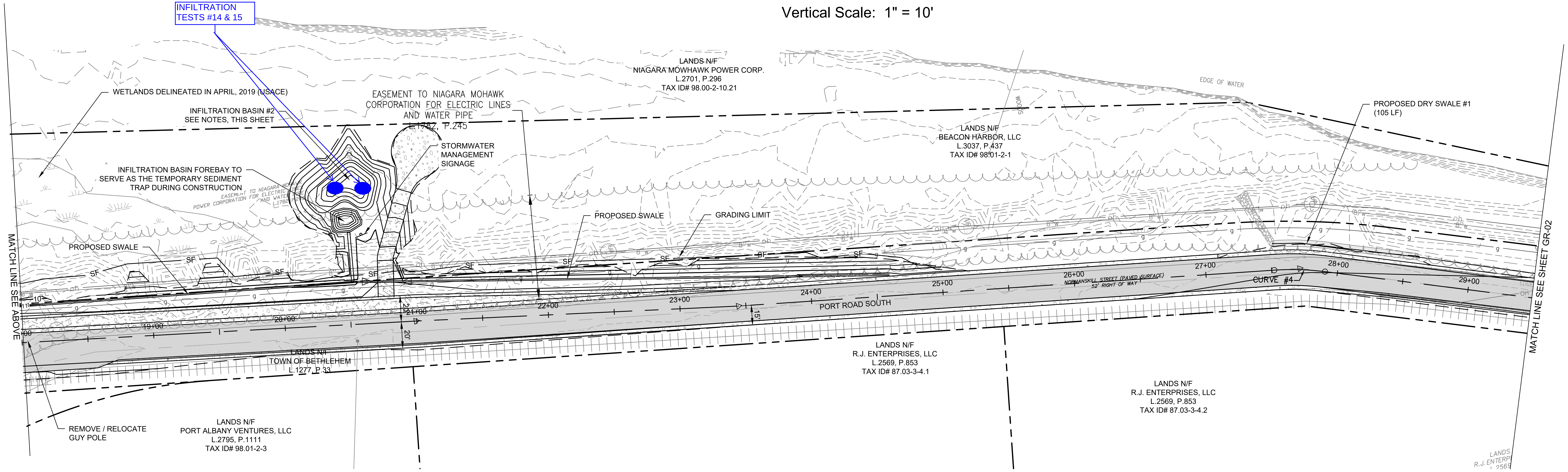
LEGEND

PROPERTY LINE	— - —
PROPOSED BOX BEAM GUIDE RAIL	o—o—o
DITCH CENTERLINE	— · · —
WETLAND AREA	▨
SECURITY FENCE	—o—
SILT FENCE	— SF —
CATCH BASIN	☐
EXISTING OVERHEAD ELECTRIC	— OH —
EXISTING GAS LINE	— G —
EXISTING WATERLINE	— W —
PROPOSED PAVEMENT	■
PROPOSED CONCRETE	▨
RIP-RAP WATER EMBANKMENT STABILIZATION	▨

NOTES:

1. UPSTREAM CONSTRUCTION SHALL BE COMPLETED AND STABILIZED BEFORE CONNECTION TO A DOWNSTREAM INFILTRATION FACILITY. A DENSE AND VIGOROUS VEGETATIVE COVER SHALL BE ESTABLISHED OVER THE CONTRIBUTING PERVIOUS DRAINAGE AREAS BEFORE RUNOFF CAN BE ACCEPTED INTO THE FACILITY.
2. PROPOSED INFILTRATION BASINS SHALL BE OWNED , OPERATED AND MAINTAINED BY THE PORT OF ALBANY AND BE ACCESSIBLE BY THE TOWN FOR INSPECTION.

0 40 80 120 FT
SCALE



McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
NORMANSKILL ST. REHAB

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	1"=40'
DATE	APRIL 2022
PROJECT	18641.00

FOR REVIEW NOT FOR CONSTRUCTION

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
GRADING, DRAINAGE, EROSION AND SEDIMENT CONTROL

DRAWING NUMBER
GR-01

10 OF 16

N:\18641.00 ALBANY PORT EXPANSION\DRAWINGS\SHEET FILES - NORMANSKILL ST\18641.00-GRAD.DWG

BORING LOG NO. IT-1

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6002° Longitude: -73.7632°

Approximate Surface Elev.: 16.6 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	DESCRIPTION	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		0.3	**TOPSOIL**	16.5+/-			☒	21	4-11-21-48 N=32		
		3.5	**FILL - SILTY SAND (SM)**, rootlets, coal, and cinders noted, black, dense to very dense	13+/-			☒	24	42-50-35-17 N=85		
			FILL - COAL ASH, wood and roots noted, black, loose to medium dense		5		☒	21	3-5-4-3 N=9		
							☒	21	3-3-2-4 N=5		
		11.0	**Becomes gray w/ crushed stone and slag**	5.5+/-			☒	22	6-6-17-37 N=23		
			SILT (ML), shells noted, gray to brown, medium stiff to stiff **Silt and clay bands noted 12-14'**		10		☒	19	17-7-9-5 N=16		
		14.0		2.5+/-			☒	19	3-3-2-1 N=5		

Boring Terminated at 14 Feet

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 10.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-11-2022	Boring Completed: 05-11-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-2

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6011° Longitude: -73.7634°

Approximate Surface Elev.: 15.3 (Ft.) +/-

DEPTH		ELEVATION (Ft.)
1.0	**TOPSOIL**	14.5+/-
2.0	**FILL - SANDY SILT (ML)**, roots noted, gray, medium stiff	13.5+/-
8.5	**FILL - SILTY SAND (SM)**, wood, brick, and coal fragments noted, gray, very loose to medium dense	7+/-
10.0	**SANDY SILT (ML)**, organics and sand seams noted, gray, loose	5.5+/-
12.0	**SILT (ML)**, gray, soft	3.5+/-

Boring Terminated at 12 Feet

SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
☒	18	2-3-4-5 N=7		
☒	17	8-8-11-13 N=19		
☒	22	6-7-7-8 N=14		
☒	15	6-3-3-2 N=6		
☒	14	1-2-2-1 N=4		
☒	24	2-1-1-2 N=2		

DEPTH (Ft.): 5, 10

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 9.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-11-2022	Boring Completed: 05-11-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-3

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6017° Longitude: -73.7636°

Approximate Surface Elev.: 10.8 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		0.5	**TOPSOIL**	10.5+/-			☒	6	1-2-2-2 N=4		
		3.0	**FILL - SILTY SAND (SM)**, wood chips and organics noted, brown, loose to medium dense	8+/-			☒	17	4-3-7-4 N=10		
			SILT (ML), mottled, occasional sand and clay bands, brown, medium stiff to stiff		5		☒	22	3-3-3-3 N=6		
		8.0		3+/-			☒	18	3-3-3-3 N=6		
			Boring Terminated at 8 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 4.5' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-12-2022	Boring Completed: 05-12-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-4

Page 1 of 1

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6022° Longitude: -73.7638°

Approximate Surface Elev.: 12.8 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	▩		**FILL - SANDY SILT WITH GRAVEL (ML)**, with clayey bands, wood, coal pieces, and roots noted, gray, soft to medium stiff				☒	14	1-1-2-2 N=3		
	▩						☒	19	2-3-4-5 N=7		
	▩	6.0		7+/-	5		☒	19	4-3-3-4 N=6		
	▨	8.0	**VARVED SILT AND CLAY (CL-ML)**, mottled, rootlets noted, orange to brown, medium stiff	5+/-			☒	17	4-4-3-3 N=7		
	▒	10.0	**SILT WITH SAND (ML)**, sand seams noted, brown, medium stiff	3+/-	10		☒	18	4-3-3-4 N=6		
			Boring Terminated at 10 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 6.5' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-11-2022

Boring Completed: 05-11-2022

Drill Rig: CME-750X

Driller: J. Lamm

Project No.: JB215020

BORING LOG NO. IT-5

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6027° Longitude: -73.7638°

Approximate Surface Elev.: 9.1 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	▨	3.0	**FILL - SANDY SILT (ML)**, pieces of wood noted, gray, soft	6+/-			☒ ☒	19 18	1-1-1-2 N=2 1-3-5-3 N=8		
	‖	4.0	**SILT (ML)**, gray, medium stiff	5+/-							
	⁙	6.0	**POORLY GRADED SAND (SP)**, brown, loose	3+/-	5		☒	19	2-1-3-3 N=4		
			Boring Terminated at 6 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 2.5' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-11-2022	Boring Completed: 05-11-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-6

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6034° Longitude: -73.7638°

Approximate Surface Elev.: 10.7 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	▨		**SILTY CLAY (CL-ML)**, rootlets noted, gray, soft to medium stiff				☒	14	1-1-2-2 N=3		
	▨	4.0	**Organics noted 2-4'**	6.5+/-			☒	19	2-3-4-3 N=7		
	░		**SILT WITH SAND (ML)**, brown, soft to medium stiff		5		☒	19	2-1-2-2 N=3		
	░	8.0	**Sand lens from 7-8'**	2.5+/-			☒	19	3-3-2-3 N=5		
			Boring Terminated at 8 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 4.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-11-2022	Boring Completed: 05-11-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-7

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6041° Longitude: -73.7637°

Approximate Surface Elev.: 9.6 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	▨	2.0	**VARVED SILT AND CLAY (CL-ML)**, rootlets noted, gray, soft	7.5+/-			☒	12	WH-1-2-2 N=3		
							☒	19	2-2-2-2 N=4		
		5.5	**SILT (ML)**, roots and rootlets noted, brown, medium stiff	4+/-	5		☒	19	2-2-2-1 N=4		
	🟨	6.0	**POORLY GRADED SAND (SP)**, brown, very loose	3.5+/-							
			Boring Terminated at 6 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
WH = Weight of Hammer
- 4" PVC infiltration test pipe set at depth of 3.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-12-2022	Boring Completed: 05-12-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-8

Page 1 of 1

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6069° Longitude: -73.7637°

Approximate Surface Elev.: 12.4 (Ft.) +/-

DEPTH		ELEVATION (Ft.)
2.5	**TOPSOIL**	10+/-
6.5	**FILL - SILTY SAND (SM)**, mottled, trace gravel, brown, medium dense	6+/-
8.0	**SILTY CLAY (CL-ML)**, rootlets noted, gray to brown, medium stiff	4.5+/-
10.0	**SILT (ML)**, gray, medium stiff	2.5+/-

Boring Terminated at 10 Feet

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	17	1-2-2-4 N=4		
		☒	19	4-6-6-6 N=12		
5		☒	12	7-11-6-6 N=17		
		☒	19	7-5-3-3 N=8		
10		☒	19	2-3-3-3 N=6		

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 6.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-10-2022	Boring Completed: 05-10-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-9

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6069° Longitude: -73.7643°

Approximate Surface Elev.: 13.6 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
			SILTY SAND (SM), rootlets and organics noted, brown, very loose to medium dense				☒	12	1-1-2-2 N=3		
							☒	19	3-3-4-4 N=7		
		5.0		8.5+/-	5		☒	18	7-6-6-7 N=12		
			SILT WITH SAND (ML), organics noted, brown, stiff **Grades to banded silt with sand**				☒	21	6-7-6-8 N=13		
		8.0		5.5+/-							
			SILT (ML), brown, medium stiff				☒	24	2-3-2-3 N=5		
		10.0		3.5+/-	10						

Boring Terminated at 10 Feet

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 7.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-12-2022	Boring Completed: 05-12-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-10

Page 1 of 1

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6070° Longitude: -73.7650°

Approximate Surface Elev.: 17.3 (Ft.) +/-

DEPTH	Description	ELEVATION (Ft.)
	No recovery 0-2' **SILT WITH SAND (ML)**, mottled, trace gravel, rootlets noted, brown, medium stiff to stiff **Silty sand seam noted 5.5-6'** **Grades to brownish gray**	
9.0		8.5+/-
	SILTY CLAY (CL-ML), sand lenses noted, gray, medium stiff	
12.0		5.5+/-
	SILT (ML), gray, medium stiff	
14.0		3.5+/-
	Boring Terminated at 14 Feet	

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
0-2		☒	0	1-2-2-2 N=4		
2-4		☒	12	2-3-2-4 N=5		
4-6		☒	22	5-5-4-6 N=9		
6-8		☒	19	5-4-5-5 N=9		
8-10		☒	12	2-3-3-4 N=6		
10-12		☒	22	3-3-5-5 N=8		
12-14		☒	24	4-4-4-2 N=8		

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 11.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-12-2022

Boring Completed: 05-12-2022

Drill Rig: CME-750X

Driller: J. Lamm

Project No.: JB215020

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 6/7/22

BORING LOG NO. IT-11

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6070° Longitude: -73.7656°

Approximate Surface Elev.: 15.4 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	DESCRIPTION	ELEVATION (Ft.)
		0.5	**TOPSOIL**	15+/-
		7.0	**SILT WITH SAND (ML)**, rootlets noted, mottled, orange to brown, soft to very stiff **Gravelly seams noted 4-7'**	8.5+/-
		12.0	**SILT (ML)**, gray to brown, medium stiff to very stiff	3.5+/-

Boring Terminated at 12 Feet

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	19	WH-1-2-2 N=3		
		☒	21	1-1-2-3 N=3		
5		☒	18	2-3-5-9 N=8		
		☒	19	12-17-18-13 N=35		
		☒	24	4-5-5-5 N=10		
10		☒	24	5-4-3-2 N=7		

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 9.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-12-2022	Boring Completed: 05-12-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-12

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan Latitude: 42.6084° Longitude: -73.7665° Approximate Surface Elev.: 14 (Ft.) +/- DEPTH	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		0.5 **TOPSOIL**	13.5+/-			☒	12	WH-2-3-4 N=5		
	▨	**FILL - SANDY SILT (ML)**, shell fragments, wood, and rootlets noted, brown, medium stiff								
				5		☒	24	3-3-4-7 N=7		
	▨	7.0	7+/-							
		FILL - SILT (ML), wood and coal fragments noted, gray, medium stiff				☒	24	4-5-3-7 N=8		
		9.0	5+/-							
		Boring Terminated at 9 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
WH = Weight of Hammer
- 4" PVC infiltration test pipe set at depth of 6.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-10-2022	Boring Completed: 05-10-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-13

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility	**CLIENT: McFarland Johnson** **Saratoga Springs, NY**
SITE: River Road **Glenmont, NY**	

LOCATION See Exploration Plan

Latitude: 42.6085° Longitude: -73.7665°

Approximate Surface Elev.: 14.6 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
			SANDY SILT (ML), trace gravel and rootlets noted, brown, medium stiff				☒	8	WH-2-4-3 N=6		
					5		☒	24	3-3-4-4 N=7		
		7.0		7.5+/-							
			SILTY SAND (SM), trace gravel and rootlets noted, gray, loose				☒	18	4-4-5-3 N=9		
		9.0		5.5+/-							
			SANDY SILT (ML), trace gravel and wood noted, gray, medium stiff		10		☒	21	1-1-3-4 N=4		
		11.0		3.5+/-							
			Boring Terminated at 11 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method: 2 1/4" ID HSA Abandonment Method: Boring backfilled with soil cuttings upon completion.	See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any). See Supporting Information for explanation of symbols and abbreviations. Approx. ground surface elev. furnished by client	Notes: Logged by: JCH WH = Weight of Hammer - 4" PVC infiltration test pipe set at depth of 6.5' alongside test boring

WATER LEVEL OBSERVATIONS	Terracon	Boring Started: 05-10-2022	Boring Completed: 05-10-2022
No free water observed	30 Corporate Cir Ste 201 Albany, NY	Drill Rig: CME-750X	Driller: J. Lamm
		Project No.: JB215020	

BORING LOG NO. IT-14

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan Latitude: 42.6106° Longitude: -73.7662° Approximate Surface Elev.: 5.8 (Ft.) +/- DEPTH	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		0.5 **TOPSOIL**	5.5+/-			✋	12			
		2.0 **SANDY SILT (ML)**, rootlets noted, brown	4+/-			✋	12			
		3.0 **SILTY CLAY (CL-ML)**, roots noted, orange to brown	3+/-							
		Boring Terminated at 3 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
Hand Auger

Abandonment Method:
Boring backfilled with excavation spoils upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 1.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-12-2022	Boring Completed: 05-12-2022
Drill Rig: Hand Auger	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-15

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan Latitude: 42.6107° Longitude: -73.7662° Approximate Surface Elev.: 5.8 (Ft.) +/- DEPTH	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		0.5 **TOPSOIL**	5.5+/-			✋	15			
		1.5 **SILT (ML)**, rootlets noted, brown	4.5+/-			✋	12			
		3.0 **SILTY CLAY (CL-ML)**, sand seams noted, brown	3+/-							
		Boring Terminated at 3 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
Hand Auger

Abandonment Method:
Boring backfilled with excavation spoils upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 1.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-12-2022	Boring Completed: 05-12-2022
Drill Rig: Hand Auger	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-1A

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6007° Longitude: -73.7634°

Approximate Surface Elev.: 14.3 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	▨	**FILL - SILTY SAND WITH GRAVEL (SM)**, black, medium dense to very dense			☒	18	2-3-11-14 N=14		
	▨	3.5 11+/-			☒	21	25-27-25-10 N=52		
	▨	**FILL - COAL ASH**, rootlets noted, black, loose to medium dense							
	▨		5		☒	5	7-5-3-3 N=8		
	▨	9.0 5.5+/-			☒	19	4-2-3-2 N=5		
	▨	**FILL - POORLY GRADED SAND (SP)**, black, very loose 11.0 3.5+/-	10		☒	19	1-1-2-1 N=3		

Boring Terminated at 11 Feet

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 8.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-17-2022	Boring Completed: 05-17-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-7A

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6039° Longitude: -73.7638°

Approximate Surface Elev.: 11.2 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		SILTY CLAY (CL-ML), gray, soft				☒	12	2-1-2-3 N=3		
		2.5	8.5+/-							
		SANDY SILT (ML), brown, medium stiff				☒	19	2-3-3-4 N=6		
				5		☒	19	2-2-3-2 N=5		
						☒	19	2-3-3-2 N=6		
		8.0	3+/-							

Boring Terminated at 8 Feet

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 5.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-17-2022	Boring Completed: 05-17-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-8A

Page 1 of 1

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6069° Longitude: -73.7641°

Approximate Surface Elev.: 14.8 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	DESCRIPTION	ELEVATION (Ft.)
	▨	4.5	**SILTY SAND (SM)**, sand seams noted, brown, very loose to loose	10.5+/-
	‖	6.0	**SILT (ML)**, mottled, brown, medium stiff	9+/-
	▨	8.0	**SILTY SAND (SM)**, gray clayey bands noted, brown, medium dense	7+/-
	‖	12.0	**SILT (ML)**, gray, soft	3+/-

Boring Terminated at 12 Feet

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	15	WH-1-1-1 N=2		
		☒	18	4-4-5-4 N=9		
5		☒	22	3-2-4-7 N=6		
		☒	22	12-14-9-10 N=23		
		☒	24	1-2-2-2 N=4		
10		☒	24	3-1-2-1 N=3		

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 7.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-16-2022	Boring Completed: 05-16-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-10A

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6070° Longitude: -73.7650°

Approximate Surface Elev.: 17.4 (Ft.) +/-

DEPTH		ELEVATION (Ft.)
0.5	**TOPSOIL**	17+/-
	SANDY SILT (ML), rootlets noted, brown, soft to stiff **Gray clayey bands 3-3.5'** **Brown sandy bands 4-6'**	
7.5		10+/-
	SILTY CLAY (CL-ML), gray, soft to stiff **Varved 7.5-8'**	
14.0	**Sand seams noted 12-14'**	3.5+/-
	Boring Terminated at 14 Feet	

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	15	WH/12"-2-3 N=2		
		☒	19	4-5-5-6 N=10		
5		☒	24	3-4-4-4 N=8		
		☒	21	5-7-6-5 N=13		
		☒	24	3-2-4-3 N=6		
10		☒	24	1-1-2-1 N=3		
		☒	24	1-2-2-2 N=4		

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 11.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-16-2022	Boring Completed: 05-16-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

BORING LOG NO. IT-10B

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6070° Longitude: -73.7653°

Approximate Surface Elev.: 16.7 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		No recovery 0-2'				☒	0	WH-1-2-3 N=3		
	▨	**<u>VARVED SILT AND CLAY (CL-ML)</u>**, rootlets noted, gray, soft to stiff				☒	14	4-4-5-5 N=9		
	▨			5						
	▨	6.0	10.5+/-							
	▦	**<u>SANDY SILT (ML)</u>**, brown, stiff to very stiff				☒	21	5-4-5-6 N=9		
	▦					☒	22	4-6-13-15 N=19		
	▦	10.5	6+/-	10		☒	22	13-9-6-6 N=15		
		<u>SILT (ML)</u>, mottled, brown, stiff				☒	24	4-5-4-3 N=9		
		13.0	3.5+/-							

Boring Terminated at 13 Feet

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
2 1/4" ID HSA

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 9.5' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-16-2022	Boring Completed: 05-16-2022
Drill Rig: CME-750X	Driller: J. Lamm
Project No.: JB215020	

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 6/7/22

BORING LOG NO. IT-12A

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6083° Longitude: -73.7665°

Approximate Surface Elev.: 12.7 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		0.5	**TOPSOIL**	12+/-							
		2.0	**FILL - SILT WITH SAND (ML)**, organics and roots noted, brown	10.5+/-			✋				
		4.0	**FILL - SANDY SILT (ML)**, brick noted, orange to brown	8.5+/-			✋				
		8.0	**SILTY SAND (SM)**, mottled, shell fragments and rootlets noted, orange to brown	4.5+/-	5		✋ ✋				
			Boring Terminated at 8 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
Hand Auger

Abandonment Method:
Boring backfilled with excavation spoils upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Approx. ground surface elev. furnished by client

Notes:

Logged by: JCH
- 4" PVC infiltration test pipe set at depth of 4.0' alongside test boring

WATER LEVEL OBSERVATIONS

No free water observed

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 05-17-2022	Boring Completed: 05-17-2022
Drill Rig: Hand Auger	Driller: J. Lamm
Project No.: JB215020	

Port of Albany Expansion Site
Infiltration Test Summary Table

Test #	Location				EG Elev. * (ft)	Test Depth (ft)	Test Elev.	Test Date	Results ** (in/hr)
	Northing	Easting	Lat	Long					
IT-1	1373126.03	690476.39	N042° 36' 00.83"	W073° 45' 47.69"	16.6	10.0	6.6	5/12/2022	1.7
IT-1A	1373285.45	690441.73	N042° 36' 02.41"	W073° 45' 48.14"	14.3	8.0	6.3	5/18/2022	1.7
IT-2	1373428.1	690427.42	N042° 36' 03.82"	W073° 45' 48.31"	15.3	9.0	6.3	5/12/2022	0
IT-3	1373669.92	690372.95	N042° 36' 06.21"	W073° 45' 49.01"	10.8	4.5	6.3	5/13/2022	0.2
IT-4	1373828.59	690333.82	N042° 36' 07.78"	W073° 45' 49.51"	12.8	6.5	6.3	5/12/2022	0.2
IT-5	1374016.28	690316.2	N042° 36' 09.64"	W073° 45' 49.73"	9.1	2.5	6.6	5/12/2022	-0.2
IT-6	1374286.56	690329.4	N042° 36' 12.31"	W073° 45' 49.52"	10.7	4.0	6.7	5/13/2022	0
IT-7	1374550.28	690339.68	N042° 36' 14.91"	W073° 45' 49.35"	9.6	3.0	6.6	5/13/2022	1.2
IT-7A	1374449.39	690328.11	N042° 36' 13.92"	W073° 45' 49.52"	11.2	5.0	6.2	5/18/2022	0.5
IT-8	1375558.9	690328.24	N042° 36' 24.88"	W073° 45' 49.39"	12.4	6.0	6.4	5/12/2022	2.6
IT-8A	1375555.8	690230.63	N042° 36' 24.86"	W073° 45' 50.69"	14.8	7.0	7.8	5/18/2022	0.4
IT-9	1375569.02	690181.72	N042° 36' 24.99"	W073° 45' 51.35"	13.6	7.0	6.6	5/13/2022	0.1
IT-10	1375581.89	689989.53	N042° 36' 25.13"	W073° 45' 53.91"	17.3	11.0	6.3	5/13/2022	1.2
IT-10A	1375579.21	690067.6	N042° 36' 25.10"	W073° 45' 52.87"	17.4	11.0	6.4	5/18/2022	0
IT-10B	1375586.34	689908.31	N042° 36' 25.18"	W073° 45' 55.00"	16.7	9.5	7.2	5/18/2022	0
IT-11	1375587.41	689824.53	N042° 36' 25.20"	W073° 45' 56.12"	15.4	9.0	6.4	5/12/2022	0.1
IT-12	1376092.03	689570.93	N042° 36' 30.21"	W073° 45' 59.45"	14.0	6.0	8.0	5/13/2022	0.5
IT-12A	1376065.55	689567.77	N042° 36' 29.95"	W073° 45' 59.50"	12.7	4.0	8.7	5/18/2022	8.6
IT-13	1376115.27	689572.99	N042° 36' 30.44"	W073° 45' 59.42"	14.6	6.5	8.1	5/13/2022	0.1
IT-14	1376905.51	689652.3	N042° 36' 38.24"	W073° 45' 58.27"	5.8	1.0	4.8	5/13/2022	12.4
IT-15	1376922.65	689655.08	N042° 36' 38.41"	W073° 45' 58.23"	5.8	1.0	4.8	5/13/2022	>24

* Note EG elevations may have changed from the tree clearing work.

** Represents result of final trial at each location.

INFILTRATION TEST RESULTS

PROJECT: Proposed Marmen Manufacturing Facility

PROJECT NO. JB215020

PROJECT LOCATION: t/o Bethlehem, NY

TEST DATE: May 12 and 13, 2022

WEATHER: M. Sunny 70°-80° F

TESTER: J. Hutchison, et al.

Test Location	Test Depth (feet)	Trial No.	Water Drop (ft)	Elapsed Time (min)	Infiltration Rate (inches/hour)
IT-1	10.0	1	0.12	60	1.4
		2	0.12	60	1.4
		3	0.15	60	1.8
		4	0.14	60	1.7
		NOTE: Infiltration rate during final trial run = 1.7 inches per hour			
IT-2	9.0	1	0.07	60	0.8
		2	0.00	60	0.0
		3	-	-	-
		4	-	-	-
		NOTE: Infiltration rate during final trial run = 0.0 inches per hour			
IT-3	4.5	1	0.02	60	0.2
		2	-	-	-
		3	-	-	-
		4	-	-	-
		NOTE: Infiltration rate during final trial run = 0.2 inches per hour			

Notes:

1) Testing was conducted in general accord with the “Infiltration Testing Requirements” outlined in Appendix D of the New York State Stormwater Management Design Manual.
2) Infiltration tests were located alongside companion test borings numbered correspondingly.

SOIL CLASSIFICATION AT TEST DEPTH

Test Location IT-1 – Fill (coal ash w/ crushed stone and slag)
Test Location IT-2 – Sandy silt (ML)
Test Location IT-3 – Silt (ML)

INFILTRATION TEST RESULTS

PROJECT: Proposed Marmen Manufacturing Facility

PROJECT NO. JB215020

PROJECT LOCATION: t/o Bethlehem, NY

TEST DATE: May 12 and 13, 2022

WEATHER: M. Sunny 70°-80° F

TESTER: J. Hutchison, et al.

Test Location	Test Depth (feet)	Trial No.	Water Drop (ft)	Elapsed Time (min)	Infiltration Rate (inches/hour)
IT-4	6.5	1	0.06	60	0.7
		2	0.02	60	0.2
		3	-	-	-
		4	-	-	-
		NOTE: Infiltration rate during final trial run = 0.2 inches per hour			
IT-5	2.5	1	0.02	60	0.8
		2	-0.02	60	-0.2
		3	-	-	-
		4	-	-	-
		NOTE: Infiltration rate during final trial run = -0.2 inches per hour			
IT-6	4.0	1	0.00	60	0.0
		2	-	-	-
		3	-	-	-
		4	-	-	-
		NOTE: Infiltration rate during final trial run = 0.0 inches per hour			

Notes:

1) Testing was conducted in general accord with the “Infiltration Testing Requirements” outlined in Appendix D of the New York State Stormwater Management Design Manual.
2) Infiltration tests were located alongside companion test borings numbered correspondingly.

SOIL CLASSIFICATION AT TEST DEPTH

Test Location IT-4 – Silt and clay (CL-ML)
Test Location IT-5 – Silt (ML)
Test Location IT-6 – Silt w/ sand (ML)

INFILTRATION TEST RESULTS

PROJECT: Proposed Marmen Manufacturing Facility	**PROJECT NO.** JB215020
PROJECT LOCATION: t/o Bethlehem, NY	**TEST DATE:** May 12 and 13, 2022
WEATHER: M. Sunny 70°-80° F	**TESTER:** J. Hutchison, et al.

Test Location	Test Depth (feet)	Trial No.	Water Drop (ft)	Elapsed Time (min)	Infiltration Rate (inches/hour)
IT-7	3.0	1	0.14	60	1.7
		2	0.22	60	2.6
		3	0.10	60	1.2
		4	-	-	-
		NOTE: Infiltration rate during final trial run = 1.2 inches per hour			
IT-8	6.0	1	0.29	60	3.5
		2	0.23	60	2.8
		3	0.23	60	2.8
		4	0.22	60	2.6
		NOTE: Infiltration rate during final trial run = 2.6 inches per hour			
IT-9	7.0	1	0.01	60	0.1
		2	0.01	60	0.1
		3	0.01	60	0.1
		4	0.01	60	0.1
		NOTE: Infiltration rate during final trial run = 0.1 inches per hour			

Notes:

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2) Infiltration tests were located alongside companion test borings numbered correspondingly.

SOIL CLASSIFICATION AT TEST DEPTH

Test Location IT-7 – Silt (ML)
Test Location IT-8 – Silty sand (SM)
Test Location IT-9 – Silt with sand (ML)

APPENDIX B-3D

RFP # 2025-12

BEACON ISLAND PHASE 3
Packaged Wastewater Treatment Plant
and
Fire Pump House and Marine Inlet

REFERENCE DOCUMENTS (4 of 5)

Reference Documents

Beacon Island Phase 3 Program

- NYSDEC Article 11 and 15 Permits. Dated 11/10/22 - Page 1
- State Pollutant Discharge Elimination System (SPDES) Permit. Dated 10/1/23 - Page 21
- Stormwater Pollution Protection Plan (SWPPP). Dated 6/20/22 - Page 59
- Soil Management Plan. Dated 10/23/22 - Page 564
- Landfill Closure Certification Report. Dated 10/21/24 - Page 634
- Geotechnical Engineering Report. Dated 2/2/2023 - Page 659
- Army Corps of Engineers Permit. Date 4/10/23 - Page 779
- Community Air Monitoring Plan (CAMP). Dated 10/23/22 – Page 806

INFILTRATION TEST RESULTS

PROJECT: Proposed Marmen Manufacturing Facility	**PROJECT NO.** JB215020
PROJECT LOCATION: t/o Bethlehem, NY	**TEST DATE:** May 12 and 13, 2022
WEATHER: M. Sunny 70°-80° F	**TESTER:** J. Hutchison, et al.

Test Location	**Test Depth (feet)**	**Trial No.**	**Water Drop (ft)**	**Elapsed Time (min)**	**Infiltration Rate (inches/hour)**
IT-10	11.0	1	0.28	60	3.4
		2	0.07	60	0.8
		3	0.10	60	1.2
		4	0.10	60	1.2
		NOTE: Infiltration rate during final trial run = 1.2 inches per hour			
IT-11	9.0	1	0.02	60	0.2
		2	0.01	60	0.1
		3	-	-	-
		4	-	-	-
		NOTE: Infiltration rate during final trial run = 0.1 inches per hour			
IT-12	6.0	1	0.04	60	0.5
		2	0.02	60	0.2
		3	0.04	60	0.5
		4	-	-	-
		NOTE: Infiltration rate during final trial run = 0.5 inches per hour			

Notes:

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2) Infiltration tests were located alongside companion test borings numbered correspondingly.

SOIL CLASSIFICATION AT TEST DEPTH

Test Location IT-10 – Silty clay (CL-ML)
Test Location IT-11 – Silt (ML)
Test Location IT-12 – Fill (sandy silt)

Terracon

INFILTRATION TEST RESULTS

PROJECT: Proposed Marmen Manufacturing Facility

PROJECT NO. JB215020

PROJECT LOCATION: t/o Bethlehem, NY

TEST DATE: May 12 and 13, 2022

WEATHER: M. Sunny 70°-80° F

TESTER: J. Hutchison, et al.

Test Location	Test Depth (feet)	Trial No.	Water Drop (ft)	Elapsed Time (min)	Infiltration Rate (inches/hour)
IT-13	6.5	1	0.08	60	1.0
		2	0.00	60	0.0
		3	0.01	60	0.1
		4	-	-	-
		NOTE: Infiltration rate during final trial run = 0.1 inches per hour			
IT-14	1.0	1	1.93	52	> 24
		2	1.93	56	> 24
		3	1.93	64	21.8
		4	1.03	60	12.4
		NOTE: Infiltration rate during final trial run = 12.4 inches per hour			
IT-15	1.0	1	1.80	4	> 24
		2	1.80	7	> 24
		3	1.80	7	> 24
		4	1.80	8	> 24
		NOTE: Infiltration rate during final trial run > 24 inches per hour			

Notes:

1) Testing was conducted in general accord with the “Infiltration Testing Requirements” outlined in Appendix D of the New York State Stormwater Management Design Manual.
2) Infiltration tests were located alongside companion test borings numbered correspondingly.

SOIL CLASSIFICATION AT TEST DEPTH

Test Location IT-13 – Sandy silt (ML)
Test Location IT-14 – Sandy silt (ML)
Test Location IT-15 – Silt (ML)

INFILTRATION TEST RESULTS

PROJECT: Proposed Marmen Manufacturing Facility

PROJECT NO. JB215020

PROJECT LOCATION: t/o Bethlehem, NY

TEST DATE: May 18, 2022

WEATHER: P. Sunny 68° F

TESTER: J. Lamm, et al.

Test Location	Test Depth (feet)	Trial No.	Water Drop (ft)	Elapsed Time (min)	Infiltration Rate (inches/hour)
IT-1A	8.0	1	0.10	60	1.2
		2	0.12	60	1.4
		3	0.15	60	1.8
		4	0.14	60	1.7
		NOTE: Infiltration rate during final trial run = 1.7 inches per hour			
IT-7A	5.0	1	0.04	60	0.5
		2	0.02	60	0.2
		3	0.06	60	0.7
		4	0.04	60	0.5
		NOTE: Infiltration rate during final trial run = 0.5 inches per hour			
IT-8A	7.0	1	0.02	60	0.2
		2	0.02	60	0.2
		3	0.02	60	0.2
		4	0.03	60	0.4
		NOTE: Infiltration rate during final trial run = 0.4 inches per hour			

Notes:

1) Testing was conducted in general accord with the “Infiltration Testing Requirements” outlined in Appendix D of the New York State Stormwater Management Design Manual.
2) Infiltration tests were located alongside companion test borings numbered correspondingly.

SOIL CLASSIFICATION AT TEST DEPTH

Test Location IT-1A – Fill (coal ash)
Test Location IT-7A – Sandy silt (ML)
Test Location IT-8A – Silty sand (SM)

INFILTRATION TEST RESULTS

PROJECT: Proposed Marmen Manufacturing Facility	**PROJECT NO.** JB215020
PROJECT LOCATION: t/o Bethlehem, NY	**TEST DATE:** May 18, 2022
WEATHER: P. Sunny 68° F	**TESTER:** J. Lamm, et al.

Test Location	Test Depth (feet)	Trial No.	Water Drop (ft)	Elapsed Time (min)	Infiltration Rate (inches/hour)
IT-10A	11.0	1	0.00	60	0.0
		2	0.00	60	0.0
		3	0.00	60	0.0
		4	-	-	-
		NOTE: Infiltration rate during final trial run = 0.0 inches per hour			
IT-10B	9.5	1	0.03	60	0.4
		2	0.00	60	0.0
		3	0.00	60	0.0
		4	-	-	-
		NOTE: Infiltration rate during final trial run = 0.0 inches per hour			
IT-12A	4.0	1	0.52	60	6.2
		2	0.54	60	6.5
		3	0.56	60	6.7
		4	0.72	60	8.6
		NOTE: Infiltration rate during final trial run = 8.6 inches per hour			

Notes:

1) Testing was conducted in general accord with the “Infiltration Testing Requirements” outlined in Appendix D of the New York State Stormwater Management Design Manual.
2) Infiltration tests were located alongside companion test borings numbered correspondingly.

SOIL CLASSIFICATION AT TEST DEPTH

Test Location IT-10A – Silty clay (CL-ML)
Test Location IT-10B – Sandy silt (ML)
Test Location IT-12A – Silty sand (SM)

APPENDIX D

MAINTENANCE INSPECTION CHECKLISTS

Stormwater Pond/Wetland Operation, Maintenance and Management Inspection Checklist

Project ____________________

Location: ____________________

Site Status: ____________________

Date: ____________________

Time: ____________________

Inspector: ____________________

Maintenance Item	**Satisfactory/ Unsatisfactory**	**Comments**
1. Embankment and emergency spillway (Annual, After Major Storms)		
1. Vegetation and ground cover adequate		
2. Embankment erosion		
3. Animal burrows		
4. Unauthorized planting		
5. Cracking, bulging, or sliding of dam		
a. Upstream face		
b. Downstream face		
c. At or beyond toe		
downstream		
upstream		
d. Emergency spillway		
6.Pond, toe & chimney drains clear and functioning		
7.Seeps/leaks on downstream face		
8.Slope protection or riprap failure		
9. Vertical/horizontal alignment of top of dam “As-Built”		

Maintenance Item	**Satisfactory/ Unsatisfactory**	**Comments**
10. Emergency spillway clear of obstructions and debris		
11. Other (specify)		
2. Riser and principal spillway (Annual)		
Type: Reinforced concrete ______ Corrugated pipe _______ Masonry _______ 1. Low flow orifice obstructed		
2. Low flow trash rack. a. Debris removal necessary		
b. Corrosion control		
3. Weir trash rack maintenance a. Debris removal necessary		
b. corrosion control		
4. Excessive sediment accumulation insider riser		
5. Concrete/masonry condition riser and barrels a. cracks or displacement		
b. Minor spalling (<1")		
c. Major spalling (rebars exposed)		
d. Joint failures		
e. Water tightness		
6. Metal pipe condition		
7. Control valve a. Operational/exercised		
b. Chained and locked		
8. Pond drain valve a. Operational/exercised		
b. Chained and locked		
9. Outfall channels functioning		
10. Other (specify)		

Maintenance Item	**Satisfactory/ Unsatisfactory**	**Comments**
3. Permanent Pool (Wet Ponds) (monthly)		
1. Undesirable vegetative growth		
2. Floating or floatable debris removal required		
3. Visible pollution		
4. Shoreline problem		
5. Other (specify)		
4. Sediment Forebays		
1.Sedimentation noted		
2. Sediment cleanout when depth < 50% design depth		
5. Dry Pond Areas		
1. Vegetation adequate		
2. Undesirable vegetative growth		
3. Undesirable woody vegetation		
4. Low flow channels clear of obstructions		
5. Standing water or wet spots		
6. Sediment and / or trash accumulation		
7. Other (specify)		
6. Condition of Outfalls (Annual , After Major Storms)		
1. Riprap failures		
2. Slope erosion		
3. Storm drain pipes		
4.Endwalls / Headwalls		
5. Other (specify)		
7. Other (Monthly)		
1. Encroachment on pond, wetland or easement area		

Maintenance Item	Satisfactory/ Unsatisfactory	Comments
2. Complaints from residents		
3.Aesthetics a. Grass growing required		
b. Graffiti removal needed		
c. Other (specify)		
4. Conditions of maintenance access routes.		
5. Signs of hydrocarbon build-up		
6. Any public hazards (specify)		
8. Wetland Vegetation (Annual)		
1. Vegetation healthy and growing Wetland maintaining 50% surface area coverage of wetland plants after the second growing season. (If unsatisfactory, reinforcement plantings needed)		
2. Dominant wetland plants: Survival of desired wetland plant species Distribution according to landscaping plan?		
3. Evidence of invasive species		
4. Maintenance of adequate water depths for desired wetland plant species		
5. Harvesting of emergent plantings needed		
6. Have sediment accumulations reduced pool volume significantly or are plants “choked” with sediment		
7. Eutrophication level of the wetland.		
8. Other (specify)		

Comments:

Actions to be Taken:

Infiltration Trench Operation, Maintenance, and Management Inspection Checklist

Project:
Location:
Site Status:

Date:

Time:

Inspector:

MAINTENANCE ITEM	**SATISFACTORY / UNSATISFACTORY**	**COMMENTS**
1. Debris Cleanout (Monthly)		
Trench surface clear of debris		
Inflow pipes clear of debris		
Overflow spillway clear of debris		
Inlet area clear of debris		
2. Sediment Traps or Forebays (Annual)		
Obviously trapping sediment		
Greater than 50% of storage volume remaining		
3. Dewatering (Monthly)		
Trench dewaters between storms		
4. Sediment Cleanout of Trench (Annual)		
No evidence of sedimentation in trench		
Sediment accumulation doesn't yet require cleanout		
5. Inlets (Annual)		

MAINTENANCE ITEM	SATISFACTORY / UNSATISFACTORY	COMMENTS
Good condition		
No evidence of erosion		
6. Outlet/Overflow Spillway (Annual)		
Good condition, no need for repair		
No evidence of erosion		
7. Aggregate Repairs (Annual)		
Surface of aggregate clean		
Top layer of stone does not need replacement		
Trench does not need rehabilitation		

Comments:

Actions to be Taken:

Sand/Organic Filter Operation, Maintenance and Management Inspection Checklist

Project:
Location:
Site Status:

Date:

Time:

Inspector:

MAINTENANCE ITEM	SATISFACTORY / UNSATISFACTORY	COMMENTS
1. Debris Cleanout (Monthly)		
Contributing areas clean of debris		
Filtration facility clean of debris		
Inlet and outlets clear of debris		
2. Oil and Grease (Monthly)		
No evidence of filter surface clogging		
Activities in drainage area minimize oil and grease entry		
3. Vegetation (Monthly)		
Contributing drainage area stabilized		
No evidence of erosion		
Area mowed and clipping removed		
4. Water Retention Where Required (Monthly)		
Water holding chambers at normal pool		
No evidence of leakage		
5. Sediment Deposition (Annual)		

MAINTENANCE ITEM	SATISFACTORY / UNSATISFACTORY	COMMENTS
Filter chamber free of sediments		
Sedimentation chamber not more than half full of sediments		
6. Structural Components (Annual)		
No evidence of structural deterioration		
Any grates are in good condition		
No evidence of spalling or cracking of structural parts		
7. Outlet/Overflow Spillway (Annual)		
Good condition, no need for repairs		
No evidence of erosion (if draining into a natural channel)		
8. Overall Function of Facility (Annual)		
Evidence of flow bypassing facility		
No noticeable odors outside of facility		

Comments:

Actions to be Taken:

Open Channel Operation, Maintenance, and Management Inspection Checklist

Project:
Location:
Site Status:

Date:

Time:

Inspector:

MAINTENANCE ITEM	SATISFACTORY/ UNSATISFACTORY	COMMENTS
1. Debris Cleanout (Monthly)		
Contributing areas clean of debris		
2. Check Dams or Energy Dissipators (Annual, After Major Storms)		
No evidence of flow going around structures		
No evidence of erosion at downstream toe		
Soil permeability		
Groundwater / bedrock		
3. Vegetation (Monthly)		
Mowing done when needed		
Minimum mowing depth not exceeded		
No evidence of erosion		
Fertilized per specification		
4. Dewatering (Monthly)		
Dewaters between storms		

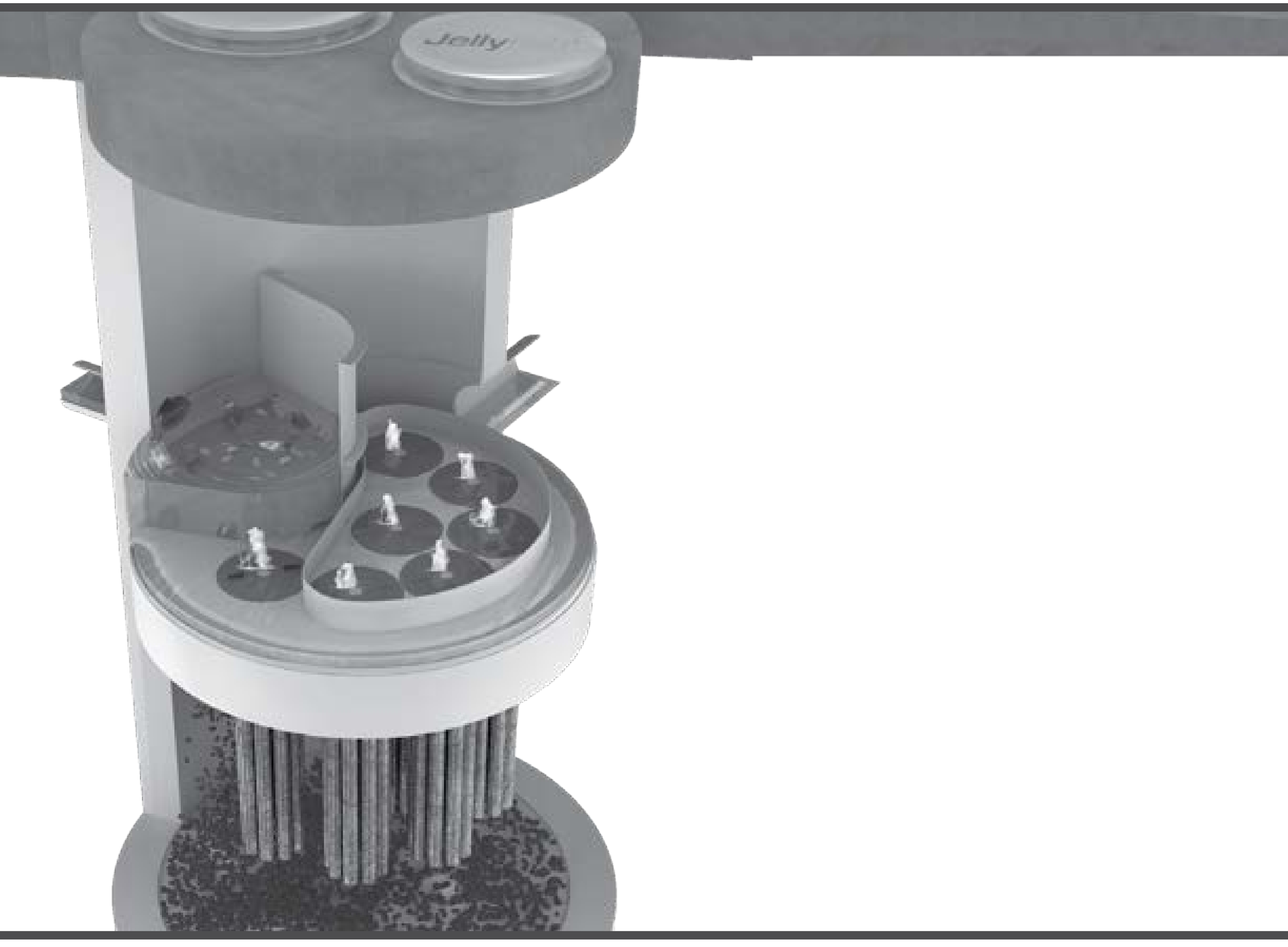
MAINTENANCE ITEM	SATISFACTORY/ UNSATISFACTORY	COMMENTS
5. Sediment deposition (Annual)		
Clean of sediment		
6. Outlet/Overflow Spillway (Annual)		
Good condition, no need for repairs		
No evidence of erosion		

Comments:

Actions to be Taken:

CONTECH®
ENGINEERED SOLUTIONS

Jellyfish® Filter Maintenance Guide



Jellyfish®

JELLYFISH® FILTER
INSPECTION & MAINTENANCE GUIDE

Jellyfish units are often just one of many structures in a more comprehensive stormwater drainage and treatment system.

In order for maintenance of the Jellyfish filter to be successful, it is imperative that all other components be properly maintained. The maintenance and repair of upstream facilities should be carried out prior to Jellyfish maintenance activities.

In addition to considering upstream facilities, it is also important to correct any problems identified in the drainage area. Drainage area concerns may include: erosion problems, heavy oil loading, and discharges of inappropriate materials.

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Inspection Process 7

1.0 Inspection and Maintenance Overview

The primary purpose of the Jellyfish® Filter is to capture and remove pollutants from stormwater runoff. As with any filtration system, these pollutants must be removed to maintain the filter's maximum treatment performance. Regular inspection and maintenance are required to insure proper functioning of the system.

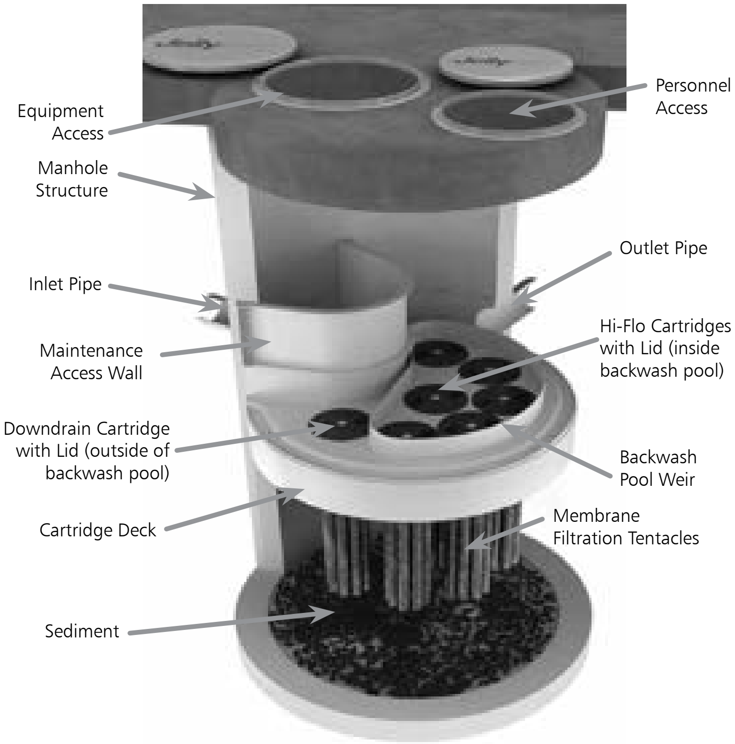
Maintenance frequencies and requirements are site specific and vary depending on pollutant loading. Additional maintenance activities may be required in the event of non-storm event runoff, such as base-flow or seasonal flow, an upstream chemical spill or due to excessive sediment loading from site erosion or extreme runoff events. It is a good practice to inspect the system after major storm events.

Inspection activities are typically conducted from surface observations and include:

- Observe if standing water is present
- Observe if there is any physical damage to the deck or cartridge lids
- Observe the amount of debris in the Maintenance Access Wall (MAW) or inlet bay for vault systems

Maintenance activities include:

- Removal of oil, floatable trash and debris
- Removal of collected sediments
- Rinsing and re-installing the filter cartridges
- Replace filter cartridge tentacles, as needed



Note: Separator Skirt not shown

2.0 Inspection Timing

Inspection of the Jellyfish Filter is key in determining the maintenance requirements for, and to develop a history of, the site's pollutant loading characteristics. In general, inspections should be performed at the times indicated below; *or per the approved project stormwater quality documents (if applicable), whichever is more frequent*.

1. A minimum of quarterly inspections during the first year of operation to assess the sediment and floatable pollutant accumulation, and to ensure proper functioning of the system.
2. Inspection frequency in subsequent years is based on the inspection and maintenance plan developed in the first year of operation. Minimum frequency should be once per year.
3. Inspection is recommended after each major storm event.
4. Inspection is required immediately after an upstream oil, fuel or other chemical spill.

3.0 Inspection Procedure

The following procedure is recommended when performing inspections:

1. Provide traffic control measures as necessary.
2. Inspect the MAW or inlet bay for floatable pollutants such as trash, debris, and oil sheen.
3. Measure oil and sediment depth in several locations, by lowering a sediment probe until contact is made with the floor of the structure. Record sediment depth, and presences of any oil layers.
4. Inspect cartridge lids. Missing or damaged cartridge lids to be replaced.
5. Inspect the MAW (where appropriate), cartridge deck and receptacles, and backwash pool weir, for damaged or broken components.

3.1 Dry weather inspections

- Inspect the cartridge deck for standing water, and/or sediment on the deck.
- No standing water under normal operating conditions.
- Standing water inside the backwash pool, but not outside the backwash pool indicates, that the filter cartridges need to be rinsed.

Inspection Utilizing Sediment Probe

- Standing water outside the backwash pool is not anticipated and may indicate a backwater condition caused by high water elevation in the receiving water body, or possibly a blockage in downstream infrastructure.
- Any appreciable sediment (≥1/16”) accumulated on the deck surface should be removed.

3.2 Wet weather inspections

- Observe the rate and movement of water in the unit. Note the depth of water above deck elevation within the MAW or inlet bay.
- Less than 6 inches, flow should be exiting the cartridge lids of each of the draindown cartridges (i.e. cartridges located outside the backwash pool).
- Greater than 6 inches, flow should be exiting the cartridge lids of each of the draindown cartridges and each of the hi-flo cartridges (i.e. cartridges located inside the backwash pool), and water should be overflowing the backwash pool weir.
- 18 inches or greater and relatively little flow is exiting the cartridge lids and outlet pipe, this condition indicates that the filter cartridges need to be rinsed.

4.0 Maintenance Requirements

Required maintenance for the Jellyfish Filter is based upon results of the most recent inspection, historical maintenance records, or the site specific water quality management plan; whichever is more frequent. In general, maintenance requires some combination of the following:

1. Sediment removal for depths reaching 12 inches or greater, or within 3 years of the most recent sediment cleaning, whichever occurs sooner.
2. Floatable trash, debris, and oil removal.
3. Deck cleaned and free from sediment.
4. Filter cartridges rinsed and re-installed as required by the most recent inspection results, or within 12 months of the most recent filter rinsing, whichever occurs sooner.
5. Replace tentacles if rinsing does not restore adequate hydraulic capacity, remove accumulated sediment, or if damaged or missing. It is recommended that tentacles should remain in service no longer than 5 years before replacement.
6. Damaged or missing cartridge deck components must be repaired or replaced as indicated by results of the most recent inspection.
7. The unit must be cleaned out and filter cartridges inspected immediately after an upstream oil, fuel, or chemical spill. Filter cartridge tentacles should be replaced if damaged or compromised by the spill.

5.0 Maintenance Procedure

The following procedures are recommended when maintaining the Jellyfish Filter:

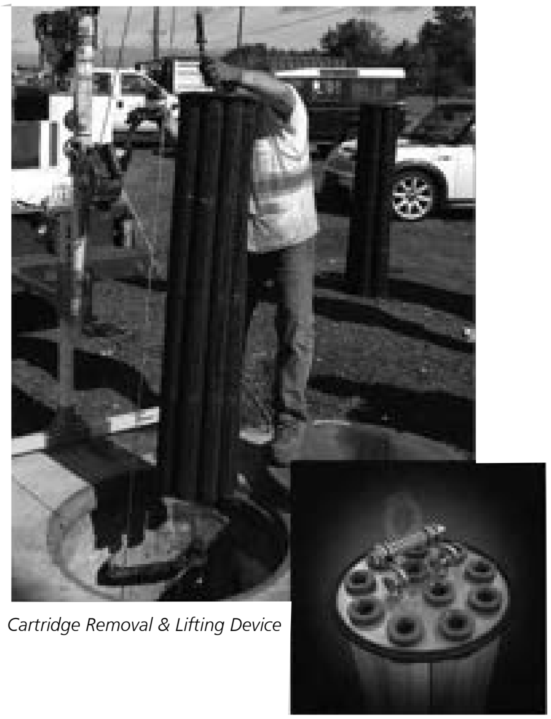
1. Provide traffic control measures as necessary.
2. Open all covers and hatches. Use ventilation equipment as required, according to confined space entry procedures. ***Caution: Dropping objects onto the cartridge deck may cause damage***.
3. Perform Inspection Procedure prior to maintenance activity.
4. To access the cartridge deck for filter cartridge service, descend into the structure and step directly onto the deck. Caution: Do not step onto the maintenance access wall (MAW) or backwash pool weir, as damage may result. Note that the cartridge deck may be slippery.
5. Maximum weight of maintenance crew and equipment on the cartridge deck not to exceed 450 lbs.

5.1 Filter Cartridge Removal

1. Remove a cartridge lid.
2. Remove cartridges from the deck using the lifting loops in the cartridge head plate. Rope or a lifting device (available from Contech) should be used. ***Caution: Should a snag occur, do not force the cartridge upward as damage to the tentacles may result. Wet cartridges typically weigh between 100 and 125 lbs***.
3. Replace and secure the cartridge lid on the exposed empty receptacle as a safety precaution. Contech does not recommend exposing more than one empty cartridge receptacle at a time.

5.2 Filter Cartridge Rinsing

1. Remove all 11 tentacles from the cartridge head plate. Take care not to lose or damage the O-ring seal as well as the plastic threaded nut and connector.



Cartridge Removal & Lifting Device

2. Position tentacles in a container (or over the MAW), with the threaded connector (open end) facing down, so rinse water is flushed through the membrane and captured in the container.
3. Using the Jellyfish rinse tool (available from Contech) or a low-pressure garden hose sprayer, direct water spray onto the tentacle membrane, sweeping from top to bottom along the length of the tentacle. Rinse until all sediment is removed from the membrane. ***Caution: Do not use a high pressure sprayer or focused stream of water on the membrane. Excessive water pressure may damage the membrane***.

4. Collected rinse water is typically removed by vacuum hose.
5. Reassemble cartridges as detailed later in this document. Reuse O-rings and nuts, ensuring proper placement on each tentacle.

5.3 Sediment and Flotables Extraction

1. Perform vacuum cleaning of the Jellyfish Filter only after filter cartridges have been removed from the system. Access the lower chamber for vacuum cleaning only through the maintenance access wall (MAW) opening. Be careful not to damage the flexible plastic separator skirt that is attached to the underside of the deck on manhole systems. Do not lower the vacuum wand through a cartridge receptacle, as damage to the receptacle will result.
2. Vacuum floatable trash, debris, and oil, from the MAW opening or inlet bay. Alternatively, floatable solids may be removed by a net or skimmer.

Vacuuming Sump Through MAW

3. Pressure wash cartridge deck and receptacles to remove all sediment and debris. Sediment should be rinsed into the sump area. Take care not to flush rinse water into the outlet pipe.
4. Remove water from the sump area. Vacuum or pump equipment should only be introduced through the MAW or inlet bay.
5. Remove the sediment from the bottom of the unit through the MAW or inlet bay opening.

Vacuuming Sump Through MAW

6. For larger diameter Jellyfish Filter manholes (≥8-ft) and some vaults complete sediment removal may be facilitated by removing a cartridge lid from an empty receptacle and inserting a jetting wand (not a vacuum wand) through the receptacle. Use the sprayer to rinse loosened sediment toward the vacuum hose in the MAW opening, being careful not to damage the receptacle.

5.4 Filter Cartridge Reinstallation and Replacement

1. Cartridges should be installed after the deck has been cleaned. It is important that the receptacle surfaces be free from grit and debris.
2. Remove cartridge lid from deck and carefully lower the filter cartridge into the receptacle until head plate gasket is seated squarely in receptacle. ***Caution: Do not force the cartridge downward; damage may occur.***
3. Replace the cartridge lid and check to see that both male threads are properly seated before rotating approximately 1/3 of a full rotation until firmly seated. Use of an approved rim gasket lubricant may facilitate installation. See next page for additional details.
4. If rinsing is ineffective in removing sediment from the tentacles, or if tentacles are damaged, provisions must be made to replace the spent or damaged tentacles with new tentacles. Contact Contech to order replacement tentacles.

5.5 Chemical Spills

Caution: If a chemical spill has been captured, do not attempt maintenance. Immediately contact the local hazard response agency and contact Contech.

5.6 Material Disposal

The accumulated sediment found in stormwater treatment and conveyance systems must be handled and disposed of in accordance with regulatory protocols. It is possible for sediments to contain measurable concentrations of heavy metals and organic chemicals (such as pesticides and petroleum products). Areas with the greatest potential for high pollutant loading include industrial areas and heavily traveled roads. Sediments and water must be disposed of in accordance with all applicable waste disposal regulations. When scheduling maintenance, consideration must be made for the disposal of solid and liquid wastes. This typically requires coordination with a local landfill for solid waste disposal. For liquid waste disposal a number of options are available including a municipal vacuum truck decant facility, local waste water treatment plant or on-site treatment and discharge.

Jellyfish Filter Components & Filter Cartridge Assembly and Installation

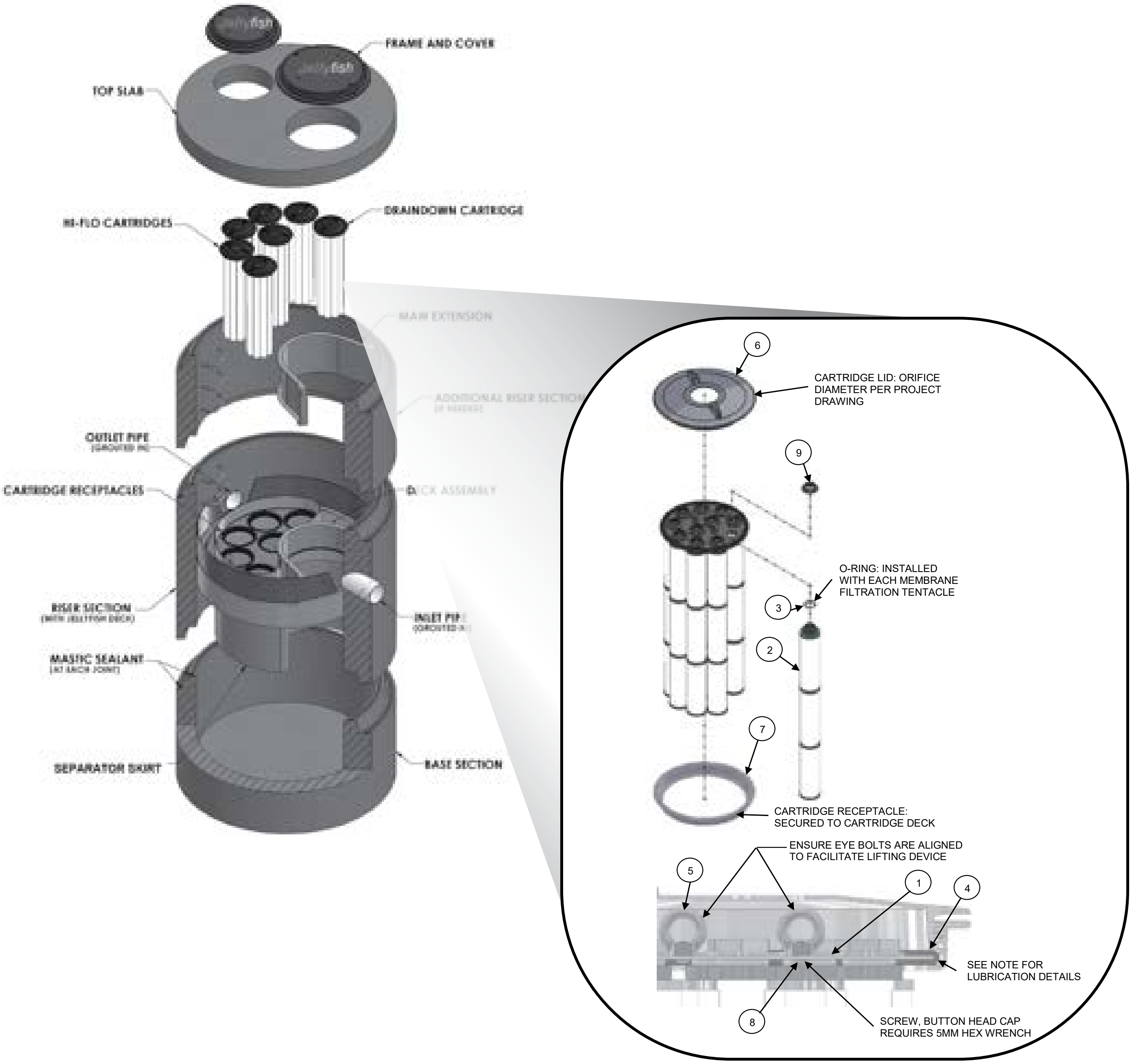


TABLE 1: BOM

ITEM NO.	DESCRIPTION
1	JF HEAD PLATE
2	JF TENTACLE
3	JF O-RING
4	JF HEAD PLATE GASKET
5	JF CARTRIDGE EYELET
6	JF 14IN COVER
7	JF RECEPTACLE
8	BUTTON HEAD CAP SCREW M6X14MM SS
9	JF CARTRIDGE NUT

TABLE 2: APPROVED GASKET LUBRICANTS

PART NO.	MFR	DESCRIPTION
78713	LA-CO	LUBRI-JOINT
40501	HERCULES	DUCK BUTTER
30600	OATEY	PIPE LUBRICANT
PSLUBXL1Q	PROSELECT	PIPE JOINT LUBRICANT

NOTES:

Head Plate Gasket Installation:

Install Head Plate Gasket (Item 4) onto the Head Plate (Item 1) and liberally apply a lubricant from Table 2: Approved Gasket Lubricants onto the gasket where it contacts the Receptacle (Item 7) and Cartridge Lide (ITem 6). Follow Lubricant manufacturer's instructions.

Lid Assembly:

Rotate Cartridge Lid counter-clockwise until both male threads drop down and properly seat. Then rotate Cartridge Lid clock-wise approximately one-third of a full rotation until Cartridge Lid is firmly secured, creating a watertight seal.

Jellyfish Filter Inspection and Maintenance Log

Owner:				Jellyfish Model No:		
Location:				GPS Coordinates:		
Land Use:	Commercial:		Industrial:		Service Station:	
Roadway/Highway:			Airport:		Residential:	

Date/Time:						
Inspector:						
Maintenance Contractor:						
Visible Oil Present: (Y/N)						
Oil Quantity Removed:						
Floatable Debris Present: (Y/N)						
Floatable Debris Removed: (Y/N)						
Water Depth in Backwash Pool						
Draindown Cartridges externally rinsed and recommissioned: (Y/N)						
New tentacles put on Draindown Cartridges: (Y/N)						
Hi-Flo Cartridges externally rinsed and recommissioned: (Y/N)						
New tentacles put on Hi-Flo Cartridges: (Y/N)						
Sediment Depth Measured: (Y/N)						
Sediment Depth (inches or mm):						
Sediment Removed: (Y/N)						
Cartridge Lids intact: (Y/N)						
Observed Damage:						
Comments:						

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- Drawings and specifications are available at www.conteches.com/jellyfish.
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Isolator® Row Plus
O&M Manual

ADS StormTech™

The Isolator® Row Plus

Introduction

An important component of any Stormwater Pollution Prevention Plan is inspection and maintenance. The StormTech Isolator Row Plus is a technique to inexpensively enhance Total Suspended Solids (TSS) and Total Phosphorus (TP) removal with easy access for inspection and maintenance.

The Isolator Row Plus

The Isolator Row Plus is a row of StormTech chambers, either SC-160, SC-310, SC-310-3, SC-740, DC-780, MC-3500 or MC-7200 models, that is surrounded with filter fabric and connected to a closely located manhole for easy access. The fabric-wrapped chambers provide for sediment settling and filtration as stormwater rises in the Isolator Row Plus and passes through the filter fabric. The open bottom chambers and perforated sidewalls (SC-310, SC- 310-3 and SC-740 models) allow stormwater to flow both vertically and horizontally out of the chambers. Sediments are captured in the Isolator Row Plus protecting the adjacent stone and chambers storage areas from sediment accumulation.

ADS geotextile fabric is placed between the stone and the Isolator Row Plus chambers. The woven geotextile provides a media for stormwater filtration, a durable surface for maintenance, prevents scour of the underlying stone and remains intact during high pressure jetting. A non-woven fabric is placed over the chambers to provide a filter media for flows passing through the chamber's sidewall. The non-woven fabric is not required over the SC-160, DC-780, MC-3500 or MC-7200 models as these chambers do not have perforated side walls.

The Isolator Row Plus is designed to capture the "first flush" runoff and offers the versatility to be sized on a volume basis or a flow-rate basis. An upstream manhole provides access to the Isolator Row Plus and includes a high/low concept such that stormwater flow rates or volumes that exceed the capacity of the Isolator Row Plus bypass through a manifold to the other chambers. This is achieved with an elevated bypass manifold or a high-flow weir. This creates a differential between the Isolator Row Plus row of chambers and the manifold to the rest of the system, thus allowing for settlement time in the Isolator Row Plus. After Stormwater flows through the Isolator Row Plus and into the rest of the chamber system it is either exfiltrated into the soils below or passed at a controlled rate through an outlet manifold and outlet control structure.

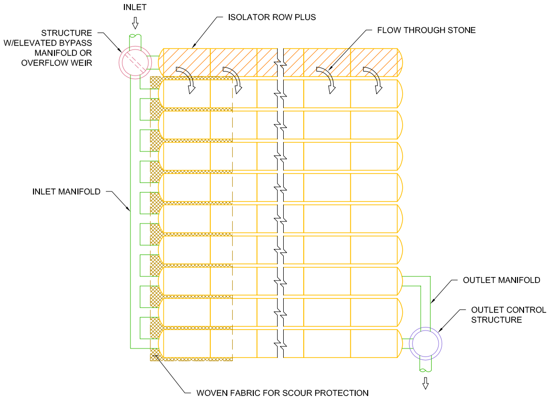
The Isolator Row FLAMP™ (patent pending) is a flared end ramp apparatus attached to the inlet pipe on the inside of the chamber end cap. The FLAMP provides a smooth transition from pipe invert to fabric bottom. It is configured to improve chamber function performance by enhancing outflow of solid debris that would otherwise collect at the chamber's end. It also serves to improve the fluid and solid flow into the access pipe during maintenance and cleaning and to guide cleaning and inspection equipment back into the inlet pipe when complete.

The Isolator Row Plus may be part of a treatment train system. The treatment train design and pretreatment device selection by the design engineer is often driven by regulatory requirements. Whether pretreatment is used or not, StormTech recommend using the Isolator Row Plus to minimize maintenance requirements and maintenance costs.

Note: *See the StormTech Design Manual for detailed information on designing inlets for a StormTech system, including the Isolator Row Plus.*

Looking down the Isolator Row PLUS from the manhole opening, ADS PLUS Fabric is shown between the chamber and stone base.

StormTech Isolator Row PLUS with Overflow Spillway (not to scale)



Isolator Row Plus Inspection/Maintenance

Inspection

The frequency of inspection and maintenance varies by location. A routine inspection schedule needs to be established for each individual location based upon site specific variables. The type of land use (i.e. industrial, commercial, residential), anticipated pollutant load, percent imperviousness, climate, etc. all play a critical role in determining the actual frequency of inspection and maintenance practices.

At a minimum, StormTech recommends annual inspections. Initially, the Isolator Row Plus should be inspected every 6 months for the first year of operation. For subsequent years, the inspection should be adjusted based upon previous observation of sediment deposition.

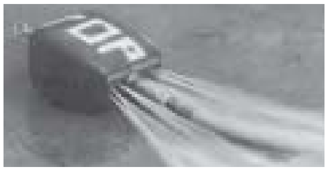
The Isolator Row Plus incorporates a combination of standard manhole(s) and strategically located inspection ports (as needed). The inspection ports allow for easy access to the system from the surface, eliminating the need to perform a confined space entry for inspection purposes.

If upon visual inspection it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3 inches throughout the length of the Isolator Row Plus, clean-out should be performed.

Maintenance

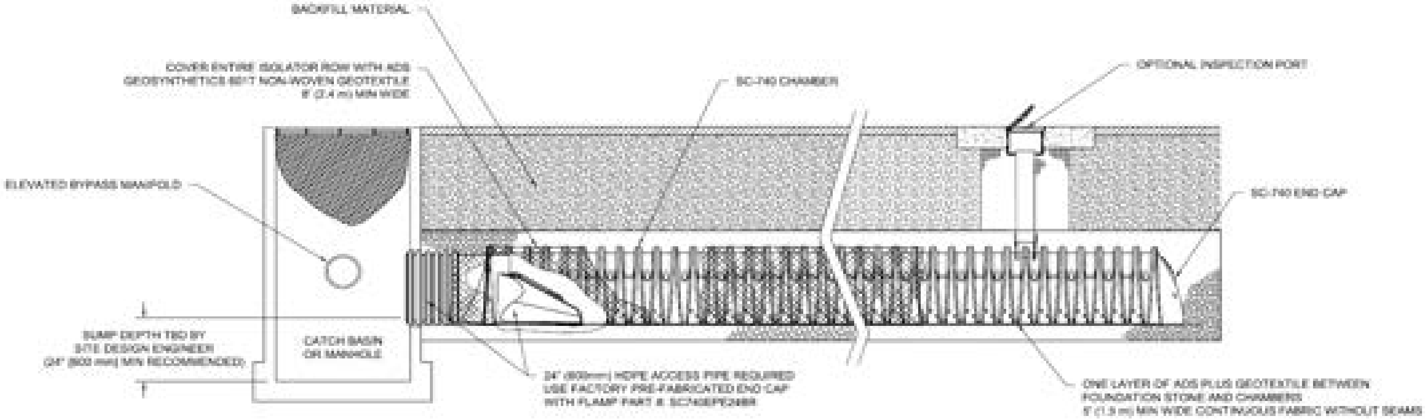
The Isolator Row Plus was designed to reduce the cost of periodic maintenance. By "isolating" sediments to just one row, costs are dramatically reduced by eliminating the need to clean out each row of the entire storage bed. If inspection indicates the potential need for maintenance, access is provided via a manhole(s) located on the end(s) of the row for cleanout. If entry into the manhole is required, please follow local and OSHA rules for a confined space entries.

Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high pressure water nozzle to propel itself down the Isolator Row Plus while scouring and suspending sediments. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45" are best. StormTech recommends a maximum nozzle pressure of 2000 psi be utilized during cleaning. JetVac reels can vary in length. For ease of maintenance, ADS recommends Isolator Row Plus lengths up to 200' (61 m). **The JetVac process shall only be performed on StormTech Isolator Row Plus that have ADS Plus Fabric (as specified by StormTech) over their angular base stone.**



StormTech Isolator Row PLUS (*not to scale*)

Note: *Non-woven fabric is only required over the inlet pipe connection into the end cap for SC-160LP, DC-780, MC-3500 and MC-7200 chamber models and is not required over the entire Isolator Row PLUS.*



Isolator Row Plus Step By Step Maintenance Procedures

Step 1

Inspect Isolator Row Plus for sediment.

- A) Inspection ports (if present)
    - i. Remove lid from floor box frame
    - ii. Remove cap from inspection riser
    - iii. Using a flashlight and stadia rod,measure depth of sediment and record results on maintenance log.
    - iv. If sediment is at or above 3 inch depth, proceed to Step 2. If not, proceed to Step 3.
- B) All Isolator Row Plus
    - i. Remove cover from manhole at upstream end of Isolator Row Plus
    - ii. Using a flashlight, inspect down Isolator Row Plus through outlet pipe
        1. Mirrors on poles or cameras may be used to avoid a confined space entry
        2. Follow OSHA regulations for confined space entry if entering manhole
    - iii. If sediment is at or above the lower row of sidewall holes (approximately 3 inches), proceed to Step 2.
      If not, proceed to Step 3.

Step 2

Clean out Isolator Row Plus using the JetVac process.

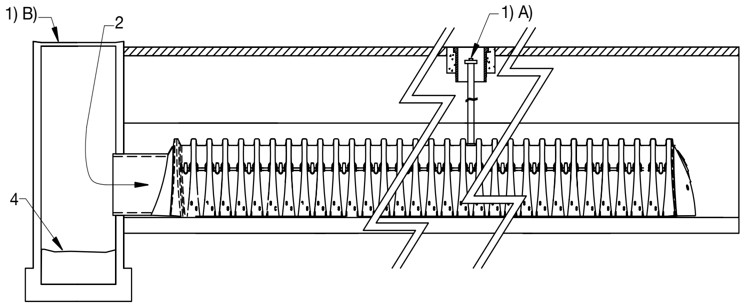
- A) A fixed floor cleaning nozzle with rear facing nozzle spread of 45 inches or more is preferable
- B) Apply multiple passes of JetVac until backflush water is clean
- C) Vacuum manhole sump as required

Step 3

Replace all caps, lids and covers, record observations and actions.

Step 4

Inspect & clean catch basins and manholes upstream of the StormTech system.



Sample Maintenance Log

Date	Stadia Rod Readings		Sedi-ment Depth (1)–(2)	Observations/Actions	Inspector
	Fixed point to chamber bottom (1)	Fixed point to top of sediment (2)			
3/15/11	6.3 ft	none		New installation. Fixed point is CI frame at grade	DJM
9/24/11		6.2	0.1 ft	Some grit felt	SM
6/20/13		5.8	0.5 ft	Mucky feel, debris visible in manhole and in Isolator Row PLUS, maintenance due	NV
7/7/13	6.3 ft		0	System jetted and vacuumed	DJM

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5.1.6 Soil Restoration

Description

Soil Restoration is a required practice applied across areas of a development site where soils have been disturbed and will be vegetated in order to recover the original properties and porosity of the soil. Healthy soil is vital to a sustainable environment and landscape. A deep, well drained soil, rich in organic matter, absorbs rainwater, helps prevent flooding and soil erosion, filters out water pollutants, and promotes vigorous plant growth that requires less irrigation, pesticides, and fertilizer.

Soil Restoration is applied in the cleanup, restoration, and landscaping phase of construction followed by the permanent establishment of an appropriate, deep-rooted groundcover to help maintain the restored soil structure. Soil restoration includes mechanical decompaction, compost amendment, or both.

Many runoff reduction practices need Soil Restoration measures applied over and adjacent to the practice to achieve runoff reduction performance. (See typical compacted soil in Figure 5.15). Consult individual profile sheets for specific design criteria.

Figure 5.14 Shows typical compacted soils that nearly reach the bulk density of concrete (Schueler et al 2000)

Key Benefits

- More marketable buildings and landscapes
- Less stormwater runoff, better water quality
- Healthier, aesthetically pleasing landscapes
- Increased porosity on redevelopment sites where impervious cover is converted to pervious
- Achieves performance standards on runoff reduction practices
- Decreases runoff volume generated and lowers the demand on runoff control structures
- Enhances direct groundwater recharge
- Promotes successful long-term revegetation by restoring soil organic matter, permeability, drainage and water holding capacity for healthy root system development of trees, shrubs and deep-rooted ground covers, minimizing lawn chemical requirements, plant drowning during wet periods, and burnout during dry periods

Typical Perceived Obstacles and Realities

- Higher cost due to soil restoration- *application of soil de-compaction and enhancement may have additional initial cost; however, they provide benefit in reducing the need for conveyance structures.*
- Space constraints and obstruction for use of equipment - *post construction space may limit the ability of some of the de-compaction equipment, however, alternative equipment and sensible planning help overcome this obstacle.*

Discussion

Tilling exposes compacted soil devoid of oxygen to air and recreates temporary air space. In addition, research has shown that the incorporation of organic compost, can greatly improve temporary water storage in the soil and subsequent runoff reduction through infiltration and evapotranspiration.

Soils that have a permanent high water table close to the surface (0-12 inches), either influenced by a clay or other highly impervious layer of material, may have bulk densities so naturally high that compaction has little added impact on infiltration (Lacey 2008). However, these soils will still benefit from the addition of compost. The water holding capacity, penetration, structural stability, and fertility of clay soils were improved with compost mixing (Avnimelech and Cohen 1988).

Table 5.3 describes various soil disturbance activities related to land development, soil types and the requirements for soil restoration for each activity. Soil Restoration or modification of curve numbers is a required practice. Restoration is applied across areas of a development site where soils have been compacted and will be vegetated according to the criteria defined in Table 5.3. If Soil Restoration is not applied according to these criteria, designers are required to:

a) Increase the calculated WQv by factoring in the compacted areas that have not been kept as impervious cover (including areas of cut or fill, heavy traffic areas on site, or Impervious Cover reduction in redevelopment projects unless aeration or full soil restoration is applied, per Table 5.3).

b) Change by one level the post-construction hydrologic soil group (HSG) to a less permeable group than the original condition. This is applied to all volumetric and discharge rate control computations.

<table>
<tr><th colspan="4">Table 5.3 Soil Restoration Requirements</th></tr>
<tr><th>Type of Soil Disturbance</th><th colspan="2">Soil Restoration Requirement</th><th>Comments/Examples</th></tr>
<tr><td>No soil disturbance</td><td colspan="2">Restoration not permitted</td><td>Preservation of Natural Features</td></tr>
<tr><td>Minimal soil disturbance</td><td colspan="2">Restoration not required</td><td>Clearing and grubbing</td></tr>
<tr><td rowspan="2">Areas where topsoil is stripped only - no change in grade</td><td>HSG A &B</td><td>HSG C&D</td><td rowspan="2">Protect area from any ongoing construction activities.</td></tr>
<tr><td>apply 6 inches of topsoil</td><td>Aerate* and apply 6 inches of topsoil</td></tr>
<tr><td rowspan="2">Areas of cut or fill</td><td>HSG A &B</td><td>HSG C & D</td><td rowspan="2"></td></tr>
<tr><td>Aerate and apply 6 inches of topsoil</td><td>Apply full Soil Restoration **</td></tr>
<tr><td>Heavy traffic areas on site (especially in a zone 5-25 feet around buildings but not within a 5 foot perimeter around foundation walls)</td><td colspan="2">Apply full Soil Restoration (de-compaction and compost enhancement)</td><td></td></tr>
<tr><td>Areas where Runoff Reduction and/or Infiltration practices are applied</td><td colspan="2">Restoration not required, but may be applied to enhance the reduction specified for appropriate practices.</td><td>Keep construction equipment from crossing these areas. To protect newly installed practice from any ongoing construction activities construct a single phase operation fence area</td></tr>
<tr><td>Redevelopment projects</td><td colspan="2">Soil Restoration is required on redevelopment projects in areas where existing impervious area will be converted to pervious area.</td><td></td></tr>
</table>

*Aeration includes the use of machines such as tractor-drawn implements with coulters making a narrow slit in the soil, a roller with many spikes making indentations in the soil, or prongs which function like a mini-subsoiler.

** Per “Deep Ripping and De-compaction, DEC 2008”.

Using this Practice

During periods of relatively low to moderate subsoil moisture, the disturbed subsoils are returned to rough grade and the following Soil Restoration steps applied:

1) Apply 3 inches of compost over subsoil

2) Till compost into subsoil to a depth of at least 12 inches using a cat-mounted ripper, tractor-mounted disc, or tiller, mixing, and circulating air and compost into subsoils
3) Rock-pick until uplifted stone/rock materials of four inches and larger size are cleaned off the site
4) Apply topsoil to a depth of 6 inches
5) Vegetate as required by approved plan.

Figure 5.15 Soil aerator implement

At the end of the project an inspector should be able to push a 3/8” metal bar 12 inches into the soil just with body weight. Figures 5.16 and 5.17 show two attachments used for soil decompaction. Tilling (step 2 above) should not be performed within the drip line of any existing trees or over utility installations that are within 24 inches of the surface.

COMPOST SPECIFICATIONS

Compost shall be aged, from plant derived materials, free of viable weed seeds, have no visible free water or dust produced when handling, pass through a half inch screen and have a pH suitable to grow desired plants.

Maintenance

A simple maintenance agreement should identify where Soil Restoration is applied, where newly restored areas are/cannot be cleared, who the responsible parties are to ensure that routine vegetation improvements are made (i.e., thinning, invasive plant removal, etc.). Soil compost amendments within a filter strip or grass channel should be located in public right of way, or within a dedicated stormwater or drainage easement.

Figure 5.16 Soil aerator implement

First year maintenance operations includes:

- Initial inspections for the first six months (once after each storm greater than half- inch)

- Reseeding to repair bare or eroding areas to assure grass stabilization
- Water once every three days for first month, and then provide a half inch of water per week during first year. Irrigation plan may be adjusted according to the rain event.
- Fertilization may be needed in the fall after the first growing season to increase plant vigor
- Ongoing Maintenance:

  Two points help ensure lasting results of decompaction:

  1) Planting the appropriate ground cover with deep roots to maintain the soil structure
  2) Keeping the site free of vehicular and foot traffic or other weight loads. Consider pedestrian footpaths. (Sometimes it may be necessary to de-thatch the turf every few years)

References/Further Resources

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APPENDIX E

SPDES PERMIT

NEW YORK STATE OF OPPORTUNITY | Department of Environmental Conservation

NEW YORK STATE
DEPARTMENT OF ENVIRONMENTAL CONSERVATION

SPDES GENERAL PERMIT
FOR STORMWATER DISCHARGES

From

CONSTRUCTION ACTIVITY

Permit No. GP- 0-20-001

Issued Pursuant to Article 17, Titles 7, 8 and Article 70

of the Environmental Conservation Law

Effective Date: January 29, 2020

Expiration Date: January 28, 2025

John J. Ferguson

Chief Permit Administrator

Authorized Signature

1-23-20

Date

Address:
NYS DEC
Division of Environmental Permits
625 Broadway, 4th Floor
Albany, N.Y. 12233-1750

PREFACE

Pursuant to Section 402 of the Clean Water Act (“CWA”), stormwater *discharges* from certain *construction activities* are unlawful unless they are authorized by a *National Pollutant Discharge Elimination System* ” permit or by a state permit program. New York administers the approved State Pollutant Discharge Elimination System (SPDES) program with permits issued in accordance with the New York State Environmental Conservation Law (ECL) Article 17, Titles 7, 8 and Article 70.

An *owner or operator* of a *construction activity* that is eligible for coverage under this permit must obtain coverage prior to the *commencement of construction activity*. Activities that fit the definition of “*construction activity*”, as defined under 40 CFR 122.26(b)(14)(x), (15)(i), and (15)(ii), constitute construction of a *point source* and therefore, pursuant to ECL section 17-0505 and 17-0701, the *owner or operator* must have coverage under a SPDES permit prior to *commencing construction activity*. The *owner or operator* cannot wait until there is an actual *discharge* from the *construction site* to obtain permit coverage.

***Note: The italicized words/phrases within this permit are defined in Appendix A.**

NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION SPDES GENERAL PERMIT FOR STORMWATER DISCHARGES FROM CONSTRUCTION ACTIVITIES

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Part 1. PERMIT COVERAGE AND LIMITATIONS

A. Permit Application

This permit authorizes stormwater *discharges* to *surface waters of the State* from the following *construction activities* identified within 40 CFR Parts 122.26(b)(14)(x), 122.26(b)(15)(i) and 122.26(b)(15)(ii), provided all of the eligibility provisions of this permit are met:

1. *Construction activities* involving soil disturbances of one (1) or more acres; including disturbances of less than one acre that are part of a *larger common plan of development or sale* that will ultimately disturb one or more acres of land; excluding *routine maintenance activity* that is performed to maintain the original line and grade, hydraulic capacity or original purpose of a facility;

2. *Construction activities* involving soil disturbances of less than one (1) acre where the Department has determined that a *SPDES* permit is required for stormwater *discharges* based on the potential for contribution to a violation of a *water quality standard* or for significant contribution of *pollutants* to *surface waters of the State.*

3. *Construction activities* located in the watershed(s) identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.

B. Effluent Limitations Applicable to Discharges from Construction Activities

*Discharge*s authorized by this permit must achieve, at a minimum, the effluent limitations in Part I.B.1. (a) – (f) of this permit. These limitations represent the degree of effluent reduction attainable by the application of best practicable technology currently available.

1. Erosion and Sediment Control Requirements - The *owner or operator* must select, design, install, implement and maintain control measures to *minimize* the *discharge* of *pollutants* and prevent a violation of the *water quality standards*. The selection, design, installation, implementation, and maintenance of these control measures must meet the non-numeric effluent limitations in Part I.B.1.(a) – (f) of this permit and be in accordance with the New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016, using sound engineering judgment. Where control measures are not designed in conformance with the design criteria included in the technical standard, the *owner or operator* must include in the *Stormwater Pollution Prevention Plan* (“SWPPP”) the reason(s) for the

deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.

a. **Erosion and Sediment Controls.** Design, install and maintain effective erosion and sediment controls to *minimize* the *discharge* of *pollutants* and prevent a violation of the *water quality standards*. At a minimum, such controls must be designed, installed and maintained to:

   (i) *Minimize* soil erosion through application of runoff control and soil stabilization control measure to *minimize pollutant discharges*;

   (ii) Control stormwater *discharge*s, including both peak flowrates and total stormwater volume, to *minimize* channel and *streambank* erosion and scour in the immediate vicinity of the *discharge* points;

   (iii) *Minimize* the amount of soil exposed during *construction activity*;

   (iv) *Minimize* the disturbance of *steep slopes*;

   (v) *Minimize* sediment *discharge*s from the site;

   (vi) Provide and maintain *natural buffer*s around surface waters, direct stormwater to vegetated areas and maximize stormwater infiltration to reduce *pollutant discharge*s, unless *infeasible*;

   (vii) *Minimize* soil compaction. Minimizing soil compaction is not required where the intended function of a specific area of the site dictates that it be compacted;

   (viii) Unless *infeasible*, preserve a sufficient amount of topsoil to complete soil restoration and establish a uniform, dense vegetative cover; and

   (ix) *Minimize* dust. On areas of exposed soil, *minimize* dust through the appropriate application of water or other dust suppression techniques to control the generation of pollutants that could be discharged from the site.

b. **Soil Stabilization**. In areas where soil disturbance activity has temporarily or permanently ceased, the application of soil stabilization measures must be initiated by the end of the next business day and completed within fourteen (14) days from the date the current soil disturbance activity ceased. For construction sites that *directly discharge* to one of the 303(d) segments listed in Appendix E or is located in one of the watersheds listed in Appendix C, the application of soil stabilization measures must be initiated by the end of the next business day and completed within seven (7) days from the date the current soil disturbance activity ceased. See Appendix A for definition of *Temporarily Ceased*.

c. **Dewatering**. *Discharge*s from *dewatering* activities, including *discharge*s from *dewatering* of trenches and excavations, must be managed by appropriate control measures.

d. **Pollution Prevention Measures**. Design, install, implement, and maintain effective pollution prevention measures to *minimize* the *discharge* of *pollutants* and prevent a violation of the *water quality standards*. At a minimum, such measures must be designed, installed, implemented and maintained to:

(i) *Minimize* the *discharge* of *pollutants* from equipment and vehicle washing, wheel wash water, and other wash waters. This applies to washing operations that use clean water only. Soaps, detergents and solvents cannot be used;

(ii) *Minimize* the exposure of building materials, building products, construction wastes, trash, landscape materials, fertilizers, pesticides, herbicides, detergents, sanitary waste, hazardous and toxic waste, and other materials present on the site to precipitation and to stormwater. Minimization of exposure is not required in cases where the exposure to precipitation and to stormwater will not result in a *discharge* of *pollutants*, or where exposure of a specific material or product poses little risk of stormwater contamination (such as final products and materials intended for outdoor use) ; and

(iii) Prevent the *discharge* of *pollutants* from spills and leaks and implement chemical spill and leak prevention and response procedures.

e. **Prohibited *Discharge*s.** The following *discharge*s are prohibited:

(i) Wastewater from washout of concrete;

(ii) Wastewater from washout and cleanout of stucco, paint, form release oils, curing compounds and other construction materials;

(iii) Fuels, oils, or other *pollutants* used in vehicle and equipment operation and maintenance;

(iv) Soaps or solvents used in vehicle and equipment washing; and

(v) Toxic or hazardous substances from a spill or other release.

f. Surface Outlets. When discharging from basins and impoundments, the outlets shall be designed, constructed and maintained in such a manner that sediment does not leave the basin or impoundment and that erosion at or below the outlet does not occur.

C. Post-construction Stormwater Management Practice Requirements

1. The *owner or operator* of a *construction activity* that requires post-construction stormwater management practices pursuant to Part III.C. of this permit must select, design, install, and maintain the practices to meet the *performance criteria* in the New York State Stormwater Management Design Manual ("Design Manual"), dated January 2015, using sound engineering judgment. Where post-construction stormwater management practices ("SMPs") are not designed in conformance with the *performance criteria* in the Design Manual, the *owner or operator* must include in the SWPPP the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.

2. The *owner or operator* of a *construction activity* that requires post-construction stormwater management practices pursuant to Part III.C. of this permit must design the practices to meet the applicable *sizing criteria* in Part I.C.2.a., b., c. or d. of this permit.

a. Sizing Criteria for New Development

(i) Runoff Reduction Volume ("RRv"): Reduce the total Water Quality Volume ("WQv") by application of RR techniques and standard SMPs with RRv capacity. The total WQv shall be calculated in accordance with the criteria in Section 4.2 of the Design Manual.

(ii) Minimum RRv and Treatment of Remaining Total WQv: Construction activities that cannot meet the criteria in Part I.C.2.a.(i) of this permit due to site limitations shall direct runoff from all newly constructed impervious areas to a RR technique or standard SMP with RRv capacity unless infeasible. The specific site limitations that prevent the reduction of 100% of the WQv shall be documented in the SWPPP.

For each impervious area that is not directed to a RR technique or standard SMP with RRv capacity, the SWPPP must include documentation which demonstrates that all options were considered and for each option explains why it is considered infeasible.

In no case shall the runoff reduction achieved from the newly constructed impervious areas be less than the Minimum RRv as calculated using the criteria in Section 4.3 of the Design Manual. The remaining portion of the total WQv that cannot be reduced shall be treated by application of standard SMPs.

(iii) Channel Protection Volume (“Cpv”): Provide 24 hour extended detention of the post-developed 1-year, 24-hour storm event; remaining after runoff reduction. The Cpv requirement does not apply when:
  (1) Reduction of the entire Cpv is achieved by application of runoff reduction techniques or infiltration systems, or
  (2) The site discharges directly to tidal waters, or fifth order or larger streams.

(iv) *Overbank* Flood Control Criteria (“Qp”): Requires storage to attenuate the post-development 10-year, 24-hour peak discharge rate (Qp) to predevelopment rates. The Qp requirement does not apply when:
  (1) the site discharges directly to tidal waters or fifth order or larger streams, or
  (2) A downstream analysis reveals that *overbank* control is not required.

(v) Extreme Flood Control Criteria (“Qf”): Requires storage to attenuate the post-development 100-year, 24-hour peak discharge rate (Qf) to predevelopment rates. The Qf requirement does not apply when:
  (1) the site discharges directly to tidal waters or fifth order or larger streams, or
  (2) A downstream analysis reveals that *overbank* control is not required.

b. *Sizing Criteria* for *New Development* in Enhanced Phosphorus Removal Watershed

(i) Runoff Reduction Volume (RRv): Reduce the total Water Quality Volume (WQv) by application of RR techniques and standard SMPs with RRv capacity. The total WQv is the runoff volume from the 1-year, 24 hour design storm over the post-developed watershed and shall be calculated in accordance with the criteria in Section 10.3 of the Design Manual.

(ii) Minimum RRv and Treatment of Remaining Total WQv: *Construction activities* that cannot meet the criteria in Part I.C.2.b.(i) of this permit due to *site limitations* shall direct runoff from all newly constructed *impervious areas* to a RR technique or standard SMP with RRv capacity unless *infeasible*. The specific *site limitations* that prevent the reduction of 100% of the WQv shall be documented in the SWPPP. For each *impervious area* that is not directed to a RR technique or standard SMP with RRv capacity, the SWPPP must include documentation which demonstrates that all options were considered and for each option explains why it is considered *infeasible.*

In no case shall the runoff reduction achieved from the newly constructed *impervious areas* be less than the Minimum RRv as calculated using the criteria in Section 10.3 of the Design Manual. The remaining portion of the total WQv that cannot be reduced shall be treated by application of standard SMPs.

(iii) Channel Protection Volume (Cpv): Provide 24 hour extended detention of the post-developed 1-year, 24-hour storm event; remaining after runoff reduction. The Cpv requirement does not apply when:
(1) Reduction of the entire Cpv is achieved by application of runoff reduction techniques or infiltration systems, or
(2) The site *discharge*s directly to tidal waters, or fifth order or larger streams.

(iv) *Overbank* Flood Control Criteria (Qp): Requires storage to attenuate the post-development 10-year, 24-hour peak *discharge* rate (Qp) to predevelopment rates. The Qp requirement does not apply when:
(1) the site *discharge*s directly to tidal waters or fifth order or larger streams, or
(2) A downstream analysis reveals that *overbank* control is not required.

(v) Extreme Flood Control Criteria (Qf): Requires storage to attenuate the post-development 100-year, 24-hour peak *discharge* rate (Qf) to predevelopment rates. The Qf requirement does not apply when:
(1) the site *discharge*s directly to tidal waters or fifth order or larger streams, or
(2) A downstream analysis reveals that *overbank* control is not required.

c. Sizing Criteria for Redevelopment Activity

(i) Water Quality Volume (WQv): The WQv treatment objective for *redevelopment activity* shall be addressed by one of the following options. *Redevelopment activities* located in an Enhanced Phosphorus Removal Watershed (see Part III.B.3. and Appendix C of this permit) shall calculate the WQv in accordance with Section 10.3 of the Design Manual. All other *redevelopment activities* shall calculate the WQv in accordance with Section 4.2 of the Design Manual.

(1) Reduce the existing *impervious cover* by a minimum of 25% of the total disturbed, *impervious area*. The Soil Restoration criteria in Section 5.1.6 of the Design Manual must be applied to all newly created pervious areas, or

(2) Capture and treat a minimum of 25% of the WQv from the disturbed, *impervious area* by the application of standard SMPs; or reduce 25% of the WQv from the disturbed, *impervious area* by the application of RR techniques or standard SMPs with RRv capacity., or

(3) Capture and treat a minimum of 75% of the WQv from the disturbed, *impervious area* as well as any additional runoff from tributary areas by application of the alternative practices discussed in Sections 9.3 and 9.4 of the Design Manual., or

(4) Application of a combination of 1, 2 and 3 above that provide a weighted average of at least two of the above methods. Application of this method shall be in accordance with the criteria in Section 9.2.1(B) (IV) of the Design Manual.

If there is an existing post-construction stormwater management practice located on the site that captures and treats runoff from the *impervious area* that is being disturbed, the WQv treatment option selected must, at a minimum, provide treatment equal to the treatment that was being provided by the existing practice(s) if that treatment is greater than the treatment required by options 1 – 4 above.

(ii) Channel Protection Volume (Cpv): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site.

(iii) *Overbank* Flood Control Criteria (Qp): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site.

(iv) Extreme Flood Control Criteria (Qf): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site

d. Sizing Criteria for Combination of Redevelopment Activity and New Development

Construction projects that include both New Development and Redevelopment Activity shall provide post-construction stormwater management controls that meet the sizing criteria calculated as an aggregate of the Sizing Criteria in Part I.C.2.a. or b. of this permit for the New Development portion of the project and Part I.C.2.c of this permit for Redevelopment Activity portion of the project.

D. Maintaining Water Quality

The Department expects that compliance with the conditions of this permit will control *discharge*s necessary to meet applicable *water quality standards*. It shall be a violation of the *ECL* for any discharge to either cause or contribute to a violation of *water quality standards* as contained in Parts 700 through 705 of Title 6 of the Official Compilation of Codes, Rules and Regulations of the State of New York, such as:

1. There shall be no increase in turbidity that will cause a substantial visible contrast to natural conditions;

2. There shall be no increase in suspended, colloidal or settleable solids that will cause deposition or impair the waters for their best usages; and

3. There shall be no residue from oil and floating substances, nor visible oil film, nor globules of grease.

If there is evidence indicating that the stormwater *discharge*s authorized by this permit are causing, have the reasonable potential to cause, or are contributing to a violation of the *water quality standards*; the *owner or operator* must take appropriate corrective action in accordance with Part IV.C.5. of this general permit and document in accordance with Part IV.C.4. of this general permit. To address the *water quality standard* violation the *owner or operator* may need to provide additional information, include and implement appropriate controls in the SWPPP to correct the problem, or obtain an individual SPDES permit.

If there is evidence indicating that despite compliance with the terms and conditions of this general permit it is demonstrated that the stormwater *discharge*s authorized by this permit are causing or contributing to a violation of *water quality standards*, or if the Department determines that a modification of the permit is necessary to prevent a violation of *water quality standards*, the authorized *discharge*s will no longer be eligible for coverage under this permit. The Department may require the *owner or operator* to obtain an individual SPDES permit to continue discharging.

E. Eligibility Under This General Permit

1. This permit may authorize all *discharges* of stormwater from *construction activity* to *surface waters of the State* and *groundwaters* except for ineligible *discharges* identified under subparagraph F. of this Part.

2. Except for non-stormwater *discharges* explicitly listed in the next paragraph, this permit only authorizes stormwater *discharge*s; including stormwater runoff, snowmelt runoff, and surface runoff and drainage, from *construction activities*.

3. Notwithstanding paragraphs E.1 and E.2 above, the following non-stormwater discharges are authorized by this permit: those listed in 6 NYCRR 750-1.2(a)(29)(vi), with the following exception: "Discharges from firefighting activities are authorized only when the firefighting activities are emergencies/unplanned"; waters to which other components have not been added that are used to control dust in accordance with the SWPPP; and uncontaminated *discharge*s from *construction site* de-watering operations. All non-stormwater discharges must be identified in the SWPPP. Under all circumstances, the *owner or operator* must still comply with *water quality standards* in Part I.D of this permit.

4. The *owner or operator* must maintain permit eligibility to *discharge* under this permit. Any *discharges* that are not compliant with the eligibility conditions of this permit are not authorized by the permit and the *owner or operator* must either apply for a separate permit to cover those ineligible *discharges* or take steps necessary to make the *discharge* eligible for coverage.

F. Activities Which Are Ineligible for Coverage Under This General Permit

All of the following are **<u>not</u>** authorized by this permit:

1. *Discharge*s after *construction activities* have been completed and the site has undergone *final stabilization*;

2. *Discharge*s that are mixed with sources of non-stormwater other than those expressly authorized under subsection E.3. of this Part and identified in the SWPPP required by this permit;

3. *Discharges* that are required to obtain an individual SPDES permit or another SPDES general permit pursuant to Part VII.K. of this permit;

4. *Construction activities* or *discharges* from *construction activities* that may adversely affect an *endangered or threatened species* unless the *owner or*

operator has obtained a permit issued pursuant to 6 NYCRR Part 182 for the project or the Department has issued a letter of non-jurisdiction for the project. All documentation necessary to demonstrate eligibility shall be maintained on site in accordance with Part II.D.2 of this permit;

5. *Discharges* which either cause or contribute to a violation of *water quality standards* adopted pursuant to the *ECL* and its accompanying regulations;

6. *Construction activities* for residential, commercial and institutional projects:

    a. Where the *discharge*s from the *construction activities* are tributary to waters of the state classified as AA or AA-s; and

    b. Which are undertaken on land with no existing *impervious cover*; and

    c. Which disturb one (1) or more acres of land designated on the current United States Department of Agriculture (“USDA”) Soil Survey as Soil Slope Phase “D”, (provided the map unit name is inclusive of slopes greater than 25%), or Soil Slope Phase “E” or “F” (regardless of the map unit name), or a combination of the three designations.

7. *Construction activities* for linear transportation projects and linear utility projects:

    a. Where the *discharge*s from the *construction activities* are tributary to waters of the state classified as AA or AA-s; and

    b. Which are undertaken on land with no existing *impervious cover*; and

    c. Which disturb two (2) or more acres of land designated on the current USDA Soil Survey as Soil Slope Phase “D” (provided the map unit name is inclusive of slopes greater than 25%), or Soil Slope Phase “E” or “F” (regardless of the map unit name), or a combination of the three designations.

8. *Construction activities* that have the potential to affect an *historic property*, unless there is documentation that such impacts have been resolved. The following documentation necessary to demonstrate eligibility with this requirement shall be maintained on site in accordance with Part II.D.2 of this permit and made available to the Department in accordance with Part VII.F of this permit:

   a. Documentation that the *construction activity* is not within an archeologically sensitive area indicated on the sensitivity map, and that the *construction activity* is not located on or immediately adjacent to a property listed or determined to be eligible for listing on the National or State Registers of Historic Places, and that there is no new permanent building on the *construction site* within the following distances from a building, structure, or object that is more than 50 years old, or if there is such a new permanent building on the *construction site* within those parameters that NYS Office of Parks, Recreation and Historic Preservation (OPRHP), a Historic Preservation Commission of a Certified Local Government, or a qualified preservation professional has determined that the building, structure, or object more than 50 years old is not historically/archeologically significant.

      - 1-5 acres of disturbance - 20 feet
      - 5-20 acres of disturbance - 50 feet
      - 20+ acres of disturbance - 100 feet, or

   b. DEC consultation form sent to OPRHP, and copied to the NYS DEC Agency Historic Preservation Officer (APO), and
      (i) the State Environmental Quality Review (SEQR) Environmental Assessment Form (EAF) with a negative declaration or the Findings Statement, with documentation of OPRHP's agreement with the resolution; or
      (ii) documentation from OPRHP that the *construction activity* will result in No Impact; or
      (iii) documentation from OPRHP providing a determination of No Adverse Impact; or
      (iv) a Letter of Resolution signed by the owner/operator, OPRHP and the DEC APO which allows for this *construction activity* to be eligible for coverage under the general permit in terms of the State Historic Preservation Act (SHPA); or

   c. Documentation of satisfactory compliance with Section 106 of the National Historic Preservation Act for a coterminous project area:

  - (i) No Affect
  - (ii) No Adverse Affect
  - (iii) Executed Memorandum of Agreement, or

d. Documentation that:

(i) SHPA Section 14.09 has been completed by NYS DEC or another state agency.

9. *Discharge*s from *construction activities* that are subject to an existing SPDES individual or general permit where a SPDES permit for *construction activity* has been terminated or denied; or where the *owner or operator* has failed to renew an expired individual permit.

Part II. PERMIT COVERAGE

A. How to Obtain Coverage

1. An *owner or operator* of a *construction activity* that is not subject to the requirements of a regulated, traditional land use control MS4 must first prepare a SWPPP in accordance with all applicable requirements of this permit and then submit a completed Notice of Intent (NOI) to the Department to be authorized to discharge under this permit.

2. An *owner or operator* of a *construction activity* that is subject to the requirements of a *regulated, traditional land use control MS4* must first prepare a SWPPP in accordance with all applicable requirements of this permit and then have the SWPPP reviewed and accepted by the *regulated, traditional land use control MS4* prior to submitting the NOI to the Department. The *owner or operator* shall have the “MS4 SWPPP Acceptance” form signed in accordance with Part VII.H., and then submit that form along with a completed NOI to the Department.

3. The requirement for an *owner or operator* to have its SWPPP reviewed and accepted by the *regulated, traditional land use control MS4* prior to submitting the NOI to the Department does not apply to an *owner or operator* that is obtaining permit coverage in accordance with the requirements in Part II.F. (Change of *Owner or Operator*) or where the *owner or operator* of the *construction activity* is the *regulated, traditional land use control MS4* . This exemption does not apply to *construction activities* subject to the New York City Administrative Code.

B. Notice of Intent (NOI) Submittal

1. Prior to December 21, 2020, an owner or operator shall use either the electronic (eNOI) or paper version of the NOI that the Department prepared. Both versions of the NOI are located on the Department’s website (http://www.dec.ny.gov/). The paper version of the NOI shall be signed in accordance with Part VII.H. of this permit and submitted to the following address:

   NOTICE OF INTENT
   NYS DEC, Bureau of Water Permits
   625 Broadway, 4th Floor
   Albany, New York 12233-3505

2. Beginning December 21, 2020 and in accordance with EPA’s 2015 NPDES Electronic Reporting Rule (40 CFR Part 127), the *owner or operator* must submit the NOI electronically using the *Department’s* online NOI.

3. The *owner or operator* shall have the SWPPP preparer sign the “SWPPP Preparer Certification” statement on the NOI prior to submitting the form to the Department.

4. As of the date the NOI is submitted to the Department, the *owner or operator* shall make the NOI and SWPPP available for review and copying in accordance with the requirements in Part VII.F. of this permit.

C. Permit Authorization

1. An *owner or operator* shall not *commence construction activity* until their authorization to *discharge* under this permit goes into effect.

2. Authorization to *discharge* under this permit will be effective when the *owner or operator* has satisfied all of the following criteria:

   a. project review pursuant to the State Environmental Quality Review Act (“SEQRA”) have been satisfied, when SEQRA is applicable. See the Department’s website (http://www.dec.ny.gov/) for more information,

   b. where required, all necessary Department permits subject to the *Uniform Procedures Act (“UPA”)* (see 6 NYCRR Part 621), or the equivalent from another New York State agency, have been obtained, unless otherwise notified by the Department pursuant to 6 NYCRR 621.3(a)(4). *Owners or operators* of *construction activities* that are required to obtain *UPA* permits must submit a preliminary SWPPP to the appropriate DEC Permit Administrator at the Regional Office listed in Appendix F at the time all other necessary *UPA* permit applications are submitted. The preliminary SWPPP must include sufficient information to demonstrate that the *construction activity* qualifies for authorization under this permit,

c. the final SWPPP has been prepared, and

d. a complete NOI has been submitted to the Department in accordance with the requirements of this permit.

3. An *owner or operator* that has satisfied the requirements of Part II.C.2 above will be authorized to *discharge* stormwater from their *construction activity* in accordance with the following schedule:

   a. For *construction activities* that are not subject to the requirements of a *regulated, traditional land use control MS4*:

      (i) Five (5) business days from the date the Department receives a complete electronic version of the NOI (eNOI) for *construction activities* with a SWPPP that has been prepared in conformance with the design criteria in the technical standard referenced in Part III.B.1 and the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C.; or

      (ii) Sixty (60) business days from the date the Department receives a complete NOI (electronic or paper version) for *construction activities* with a SWPPP that has not been prepared in conformance with the design criteria in technical standard referenced in Part III.B.1. or, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C., the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, or;

      (iii) Ten (10) business days from the date the Department receives a complete paper version of the NOI for *construction activities* with a SWPPP that has been prepared in conformance with the design criteria in the technical standard referenced in Part III.B.1 and the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C.

b. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4*:

  (i) Five (5) business days from the date the Department receives both a complete electronic version of the NOI (eNOI) and signed "*MS4* SWPPP Acceptance" form, or

  (ii) Ten (10) business days from the date the Department receives both a complete paper version of the NOI and signed "MS4 SWPPP Acceptance" form.

4. Coverage under this permit authorizes stormwater *discharges* from only those areas of disturbance that are identified in the NOI. If an *owner or operator* wishes to have stormwater *discharges* from future or additional areas of disturbance authorized, they must submit a new NOI that addresses that phase of the development, unless otherwise notified by the Department. The *owner or operator* shall not *commence construction activity* on the future or additional areas until their authorization to *discharge* under this permit goes into effect in accordance with Part II.C. of this permit.

D. General Requirements For Owners or Operators With Permit Coverage

1. The *owner or operator* shall ensure that the provisions of the SWPPP are implemented from the *commencement of construction activity* until all areas of disturbance have achieved *final stabilization* and the Notice of Termination ("NOT") has been submitted to the Department in accordance with Part V. of this permit. This includes any changes made to the SWPPP pursuant to Part III.A.4. of this permit.

2. The *owner or operator* shall maintain a copy of the General Permit (GP-0-20-001), NOI, *NOI Acknowledgment Letter*, SWPPP, MS4 SWPPP Acceptance form, inspection reports, responsible contractor's or subcontractor's certification statement (see Part III.A.6.), and all documentation necessary to demonstrate eligibility with this permit at the *construction site* until all disturbed areas have achieved *final stabilization* and the NOT has been submitted to the Department. The documents must be maintained in a secure location, such as a job trailer, on-site construction office, or mailbox with lock. The secure location must be accessible during normal business hours to an individual performing a compliance inspection.

3. The *owner or operator* of a *construction activity* shall not disturb greater than five (5) acres of soil at any one time without prior written authorization from the Department or, in areas under the jurisdiction of a *regulated, traditional land* *use control MS4,* the *regulated, traditional land use control MS4* (provided the *regulated, traditional land use control MS4* is not the *owner or operator* of the *construction activity*). At a minimum, the *owner or operator* must comply with the following requirements in order to be authorized to disturb greater than five (5) acres of soil at any one time:

a. The *owner or operator* shall have a *qualified inspector* conduct **at least** two (2) site inspections in accordance with Part IV.C. of this permit every seven (7) calendar days, for as long as greater than five (5) acres of soil remain disturbed. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.

b. In areas where soil disturbance activity has temporarily or permanently ceased, the application of soil stabilization measures must be initiated by the end of the next business day and completed within seven (7) days from the date the current soil disturbance activity ceased. The soil stabilization measures selected shall be in conformance with the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016.

c. The *owner or operator* shall prepare a phasing plan that defines maximum disturbed area per phase and shows required cuts and fills.

d. The *owner or operator* shall install any additional site-specific practices needed to protect water quality.

e. The *owner or operator* shall include the requirements above in their SWPPP.

4. In accordance with statute, regulations, and the terms and conditions of this permit, the Department may suspend or revoke an *owner's or operator's* coverage under this permit at any time if the Department determines that the SWPPP does not meet the permit requirements or consistent with Part VII.K..

5. Upon a finding of significant non-compliance with the practices described in the SWPPP or violation of this permit, the Department may order an immediate stop to all activity at the site until the non-compliance is remedied. The stop work order shall be in writing, describe the non-compliance in detail, and be sent to the *owner or operator*.

6. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4*, the *owner or operator* shall notify the

regulated, traditional land use control MS4 in writing of any planned amendments or modifications to the post-construction stormwater management practice component of the SWPPP required by Part III.A. 4. and 5. of this permit. Unless otherwise notified by the *regulated, traditional land use control MS4*, the *owner or operator* shall have the SWPPP amendments or modifications reviewed and accepted by the *regulated, traditional land use control MS4* prior to commencing construction of the post-construction stormwater management practice.

E. Permit Coverage for Discharges Authorized Under GP-0-15-002

1. Upon renewal of SPDES General Permit for Stormwater Discharges from *Construction Activity* (Permit No. GP-0-15-002), an *owner or operator* of *a construction activity* with coverage under GP-0-15-002, as of the effective date of GP- 0-20-001, shall be authorized to *discharge* in accordance with GP- 0-20-001, unless otherwise notified by the Department.

   An *owner or operator* may continue to implement the technical/design components of the post-construction stormwater management controls provided that such design was done in conformance with the technical standards in place at the time of initial project authorization. However, they must comply with the other, non-design provisions of GP-0-20-001.

F. Change of Owner or Operator

1. When property ownership changes or when there is a change in operational control over the construction plans and specifications, the original *owner or operator* must notify the new *owner or operator*, in writing, of the requirement to obtain permit coverage by submitting a NOI with the Department. For *construction activities* subject to the requirements of a *regulated, traditional land use control MS4,* the original *owner or operator* must also notify the MS4, in writing, of the change in ownership at least 30 calendar days prior to the change in ownership.

2. Once the new *owner or operator* obtains permit coverage, the original *owner or operator* shall then submit a completed NOT with the name and permit identification number of the new *owner or operator* to the Department at the address in Part II.B.1. of this permit. If the original *owner or operator* maintains ownership of a portion of the *construction activity* and will disturb soil, they must maintain their coverage under the permit.

3. Permit coverage for the new *owner or operator* will be effective as of the date the Department receives a complete NOI, provided the original *owner or*

operator was not subject to a sixty (60) business day authorization period that has not expired as of the date the Department receives the NOI from the new *owner or operator*.

Part III. STORMWATER POLLUTION PREVENTION PLAN (SWPPP)

A. General SWPPP Requirements

1. A SWPPP shall be prepared and implemented by the *owner or operator* of each *construction activity* covered by this permit. The SWPPP must document the selection, design, installation, implementation and maintenance of the control measures and practices that will be used to meet the effluent limitations in Part I.B. of this permit and where applicable, the post-construction stormwater management practice requirements in Part I.C. of this permit. The SWPPP shall be prepared prior to the submittal of the NOI. The NOI shall be submitted to the Department prior to the *commencement of construction activity*. A copy of the completed, final NOI shall be included in the SWPPP.

2. The SWPPP shall describe the erosion and sediment control practices and where required, post-construction stormwater management practices that will be used and/or constructed to reduce the *pollutants* in stormwater *discharges* and to assure compliance with the terms and conditions of this permit. In addition, the SWPPP shall identify potential sources of pollution which may reasonably be expected to affect the quality of stormwater *discharges*.

3. All SWPPPs that require the post-construction stormwater management practice component shall be prepared by a *qualified professional* that is knowledgeable in the principles and practices of stormwater management and treatment.

4. The *owner or operator* must keep the SWPPP current so that it at all times accurately documents the erosion and sediment controls practices that are being used or will be used during construction, and all post-construction stormwater management practices that will be constructed on the site. At a minimum, the *owner or operator* shall amend the SWPPP, including construction drawings:

    a. whenever the current provisions prove to be ineffective in minimizing *pollutants* in stormwater *discharges* from the site;

b. whenever there is a change in design, construction, or operation at the *construction site* that has or could have an effect on the *discharge* of *pollutants*;

c. to address issues or deficiencies identified during an inspection by the *qualified inspector,* the Department or other regulatory authority; and

d. to document the final construction conditions.

5. The Department may notify the *owner or operator* at any time that the SWPPP does not meet one or more of the minimum requirements of this permit. The notification shall be in writing and identify the provisions of the SWPPP that require modification. Within fourteen (14) calendar days of such notification, or as otherwise indicated by the Department, the *owner or operator* shall make the required changes to the SWPPP and submit written notification to the Department that the changes have been made. If the *owner or operator* does not respond to the Department's comments in the specified time frame, the Department may suspend the *owner's or operator's* coverage under this permit or require the *owner or operator* to obtain coverage under an individual SPDES permit in accordance with Part II.D.4. of this permit.

6. Prior to the *commencement of construction activity*, the *owner or operator* must identify the contractor(s) and subcontractor(s) that will be responsible for installing, constructing, repairing, replacing, inspecting and maintaining the erosion and sediment control practices included in the SWPPP; and the contractor(s) and subcontractor(s) that will be responsible for constructing the post-construction stormwater management practices included in the SWPPP. The *owner or operator* shall have each of the contractors and subcontractors identify at least one person from their company that will be responsible for implementation of the SWPPP. This person shall be known as the *trained contractor.* The *owner or operator* shall ensure that at least one *trained contractor* is on site on a daily basis when soil disturbance activities are being performed.

   The *owner or operator* shall have each of the contractors and subcontractors identified above sign a copy of the following certification statement below before they commence any *construction activity*:

   > "I hereby certify under penalty of law that I understand and agree to comply with the terms and conditions of the SWPPP and agree to implement any corrective actions identified by the *qualified inspector* during a site inspection. I also understand that the *owner or operator* must comply with

> the terms and conditions of the most current version of the New York State Pollutant Discharge Elimination System ("SPDES") general permit for stormwater *discharge*s from *construction activities* and that it is unlawful for any person to cause or contribute to a violation of *water quality standards*. Furthermore, I am aware that there are significant penalties for submitting false information, that I do not believe to be true, including the possibility of fine and imprisonment for knowing violations"

In addition to providing the certification statement above, the certification page must also identify the specific elements of the SWPPP that each contractor and subcontractor will be responsible for and include the name and title of the person providing the signature; the name and title of the *trained contractor* responsible for SWPPP implementation; the name, address and telephone number of the contracting firm; the address (or other identifying description) of the site; and the date the certification statement is signed. The *owner or operator* shall attach the certification statement(s) to the copy of the SWPPP that is maintained at the *construction site*. If new or additional contractors are hired to implement measures identified in the SWPPP after construction has commenced, they must also sign the certification statement and provide the information listed above.

7. For projects where the Department requests a copy of the SWPPP or inspection reports, the *owner or operator* shall submit the documents in both electronic (PDF only) and paper format within five (5) business days, unless otherwise notified by the Department.

B. Required SWPPP Contents

1. Erosion and sediment control component - All SWPPPs prepared pursuant to this permit shall include erosion and sediment control practices designed in conformance with the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016. Where erosion and sediment control practices are not designed in conformance with the design criteria included in the technical standard, the *owner or operator* must demonstrate *equivalence* to the technical standard. At a minimum, the erosion and sediment control component of the SWPPP shall include the following:

   a. Background information about the scope of the project, including the location, type and size of project

b. A site map/construction drawing(s) for the project, including a general location map. At a minimum, the site map shall show the total site area; all improvements; areas of disturbance; areas that will not be disturbed; existing vegetation; on-site and adjacent off-site surface water(s); floodplain/floodway boundaries; wetlands and drainage patterns that could be affected by the *construction activity*; existing and final contours ; locations of different soil types with boundaries; material, waste, borrow or equipment storage areas located on adjacent properties; and location(s) of the stormwater *discharge*(s);

c. A description of the soil(s) present at the site, including an identification of the Hydrologic Soil Group (HSG);

d. A construction phasing plan and sequence of operations describing the intended order of *construction activities*, including clearing and grubbing, excavation and grading, utility and infrastructure installation and any other activity at the site that results in soil disturbance;

e. A description of the minimum erosion and sediment control practices to be installed or implemented for each *construction activity* that will result in soil disturbance. Include a schedule that identifies the timing of initial placement or implementation of each erosion and sediment control practice and the minimum time frames that each practice should remain in place or be implemented;

f. A temporary and permanent soil stabilization plan that meets the requirements of this general permit and the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016, for each stage of the project, including initial land clearing and grubbing to project completion and achievement of *final stabilization*;

g. A site map/construction drawing(s) showing the specific location(s), size(s), and length(s) of each erosion and sediment control practice;

h. The dimensions, material specifications, installation details, and operation and maintenance requirements for all erosion and sediment control practices. Include the location and sizing of any temporary sediment basins and structural practices that will be used to divert flows from exposed soils;

i. A maintenance inspection schedule for the contractor(s) identified in Part III.A.6. of this permit, to ensure continuous and effective operation of the erosion and sediment control practices. The maintenance inspection schedule shall be in accordance with the requirements in the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016;

j. A description of the pollution prevention measures that will be used to control litter, construction chemicals and construction debris from becoming a *pollutant* source in the stormwater *discharges*;

k. A description and location of any stormwater *discharges* associated with industrial activity other than construction at the site, including, but not limited to, stormwater *discharges* from asphalt plants and concrete plants located on the *construction site*; and

l. Identification of any elements of the design that are not in conformance with the design criteria in the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016. Include the reason for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.

2. Post-construction stormwater management practice component – The *owner or operator* of any construction project identified in Table 2 of Appendix B as needing post-construction stormwater management practices shall prepare a SWPPP that includes practices designed in conformance with the applicable *sizing criteria* in Part I.C.2.a., c. or d. of this permit and the *performance criteria* in the technical standard, New York State Stormwater Management Design Manual dated January 2015

   Where post-construction stormwater management practices are not designed in conformance with the *performance criteria* in the technical standard, the *owner or operator* must include in the SWPPP the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.

   The post-construction stormwater management practice component of the SWPPP shall include the following:

   a. Identification of all post-construction stormwater management practices to be constructed as part of the project. Include the dimensions, material specifications and installation details for each post-construction stormwater management practice;

b. A site map/construction drawing(s) showing the specific location and size of each post-construction stormwater management practice;

c. A Stormwater Modeling and Analysis Report that includes:
   (i) Map(s) showing pre-development conditions, including watershed/subcatchments boundaries, flow paths/routing, and design points;

   (ii) Map(s) showing post-development conditions, including watershed/subcatchments boundaries, flow paths/routing, design points and post-construction stormwater management practices;

   (iii) Results of stormwater modeling (i.e. hydrology and hydraulic analysis) for the required storm events. Include supporting calculations (model runs), methodology, and a summary table that compares pre and post-development runoff rates and volumes for the different storm events;

   (iv) Summary table, with supporting calculations, which demonstrates that each post-construction stormwater management practice has been designed in conformance with the *sizing criteria* included in the Design Manual;

   (v) Identification of any *sizing criteria* that is not required based on the requirements included in Part I.C. of this permit; and

   (vi) Identification of any elements of the design that are not in conformance with the *performance criteria* in the Design Manual. Include the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the Design Manual;

d. Soil testing results and locations (test pits, borings);

e. Infiltration test results, when required; and

f. An operations and maintenance plan that includes inspection and maintenance schedules and actions to ensure continuous and effective operation of each post-construction stormwater management practice. The plan shall identify the entity that will be responsible for the long term operation and maintenance of each practice.

3. Enhanced Phosphorus Removal Standards - All construction projects identified in Table 2 of Appendix B that are located in the watersheds identified in Appendix C shall prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the applicable *sizing criteria* in Part I.C.2. b., c. or d. of this permit and the *performance criteria,* Enhanced Phosphorus Removal Standards included in the Design Manual. At a minimum, the post-construction stormwater management practice component of the SWPPP shall include items 2.a - 2.f. above.

C. Required SWPPP Components by Project Type

Unless otherwise notified by the Department, *owners or operators* of *construction activities* identified in Table 1 of Appendix B are required to prepare a SWPPP that only includes erosion and sediment control practices designed in conformance with Part III.B.1 of this permit. *Owners or operators* of the *construction activities* identified in Table 2 of Appendix B shall prepare a SWPPP that also includes post-construction stormwater management practices designed in conformance with Part III.B.2 or 3 of this permit.

Part IV. INSPECTION AND MAINTENANCE REQUIREMENTS

A. General Construction Site Inspection and Maintenance Requirements

1. The *owner or operator* must ensure that all erosion and sediment control practices (including pollution prevention measures) and all post-construction stormwater management practices identified in the SWPPP are inspected and maintained in accordance with Part IV.B. and C. of this permit.

2. The terms of this permit shall not be construed to prohibit the State of New York from exercising any authority pursuant to the ECL, common law or federal law, or prohibit New York State from taking any measures, whether civil or criminal, to prevent violations of the laws of the State of New York or protect the public health and safety and/or the environment.

B. Contractor Maintenance Inspection Requirements

1. The *owner or operator* of each *construction activity* identified in Tables 1 and 2 of Appendix B shall have a *trained contractor* inspect the erosion and sediment control practices and pollution prevention measures being implemented within the active work area daily to ensure that they are being maintained in effective operating condition at all times. If deficiencies are identified, the contractor shall

begin implementing corrective actions within one business day and shall complete the corrective actions in a reasonable time frame.

2. For construction sites where soil disturbance activities have been temporarily suspended (e.g. winter shutdown) and *temporary stabilization* measures have been applied to all disturbed areas, the *trained contractor* can stop conducting the maintenance inspections. The *trained contractor* shall begin conducting the maintenance inspections in accordance with Part IV.B.1. of this permit as soon as soil disturbance activities resume.

3. For construction sites where soil disturbance activities have been shut down with partial project completion, the *trained contractor* can stop conducting the maintenance inspections if all areas disturbed as of the project shutdown date have achieved *final stabilization* and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational.

C. Qualified Inspector Inspection Requirements

The *owner or operator* shall have a *qualified inspector* conduct site inspections in conformance with the following requirements:

[Note: The *trained contractor* identified in Part III.A.6. and IV.B. of this permit **cannot** conduct the *qualified inspector* site inspections unless they meet the *qualified inspector* qualifications included in Appendix A. In order to perform these inspections, the *trained contractor* would have to be a:

- licensed Professional Engineer,
- Certified Professional in Erosion and Sediment Control (CPESC),
- New York State Erosion and Sediment Control Certificate Program holder
- Registered Landscape Architect, or
- someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity].

1. A *qualified inspector* shall conduct site inspections for all *construction activities* identified in Tables 1 and 2 of Appendix B, with the exception of:

    a. the construction of a single family residential subdivision with 25% or less *impervious cover* at total site build-out that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres and is not located in one of the watersheds listed in Appendix C and not directly discharging to one of the 303(d) segments listed in Appendix E;

b. the construction of a single family home that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres and is not located in one of the watersheds listed in Appendix C and not directly discharging to one of the 303(d) segments listed in Appendix E;

c. construction on agricultural property that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres; and

d. *construction activities* located in the watersheds identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.

2. Unless otherwise notified by the Department, the *qualified inspector* shall conduct site inspections in accordance with the following timetable:

a. For construction sites where soil disturbance activities are on-going, the *qualified inspector* shall conduct a site inspection at least once every seven (7) calendar days.

b. For construction sites where soil disturbance activities are on-going and the *owner or operator* has received authorization in accordance with Part II.D.3 to disturb greater than five (5) acres of soil at any one time, the *qualified inspector* shall conduct at least two (2) site inspections every seven (7) calendar days. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.

c. For construction sites where soil disturbance activities have been temporarily suspended (e.g. winter shutdown) and *temporary stabilization* measures have been applied to all disturbed areas, the *qualified inspector* shall conduct a site inspection at least once every thirty (30) calendar days. The *owner or operator* shall notify the DOW Water (SPDES) Program contact at the Regional Office (see contact information in Appendix F) or, in areas under the jurisdiction of a *regulated, traditional land use control MS4,* the *regulated, traditional land use control MS4* (provided the *regulated, traditional land use control MS4* is not the *owner or operator* of the *construction activity*) in writing prior to reducing the frequency of inspections.

d. For construction sites where soil disturbance activities have been shut down with partial project completion, the *qualified inspector* can stop conducting inspections if all areas disturbed as of the project shutdown date have achieved *final stabilization* and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational. The *owner or operator* shall notify the DOW Water (SPDES) Program contact at the Regional Office (see contact information in Appendix F) or, in areas under the jurisdiction of a *regulated, traditional land use control MS4,* the *regulated, traditional land use control MS4* (provided the *regulated, traditional land use control MS4* is not the *owner or operator* of the *construction activity*) in writing prior to the shutdown. If soil disturbance activities are not resumed within 2 years from the date of shutdown, the *owner or operator* shall have the *qualified inspector* perform a final inspection and certify that all disturbed areas have achieved *final stabilization,* and all temporary, structural erosion and sediment control measures have been removed*;* and that all post-construction stormwater management practices have been constructed in conformance with the SWPPP by signing the "*Final Stabilization*" and "Post-Construction Stormwater Management Practice" certification statements on the NOT. The *owner or operator* shall then submit the completed NOT form to the address in Part II.B.1 of this permit.

e. For construction sites that directly *discharge* to one of the 303(d) segments listed in Appendix E or is located in one of the watersheds listed in Appendix C, the *qualified inspector* shall conduct at least two (2) site inspections every seven (7) calendar days. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.

3. At a minimum, the *qualified inspector* shall inspect all erosion and sediment control practices and pollution prevention measures to ensure integrity and effectiveness, all post-construction stormwater management practices under construction to ensure that they are constructed in conformance with the SWPPP, all areas of disturbance that have not achieved *final stabilization,* all points of *discharge* to natural surface waterbodies located within, or immediately adjacent to, the property boundaries of the *construction site*, and all points of *discharge* from the *construction site*.

4. The *qualified inspector* shall prepare an inspection report subsequent to each and every inspection. At a minimum, the inspection report shall include and/or address the following:

a. Date and time of inspection;

b. Name and title of person(s) performing inspection;

c. A description of the weather and soil conditions (e.g. dry, wet, saturated) at the time of the inspection;

d. A description of the condition of the runoff at all points of *discharge* from the *construction site*. This shall include identification of any *discharges* of sediment from the *construction site*. Include *discharges* from conveyance systems (i.e. pipes, culverts, ditches, etc.) and overland flow;

e. A description of the condition of all natural surface waterbodies located within, or immediately adjacent to, the property boundaries of the *construction site* which receive runoff from disturbed areas. This shall include identification of any *discharges* of sediment to the surface waterbody;

f. Identification of all erosion and sediment control practices and pollution prevention measures that need repair or maintenance;

g. Identification of all erosion and sediment control practices and pollution prevention measures that were not installed properly or are not functioning as designed and need to be reinstalled or replaced;

h. Description and sketch of areas with active soil disturbance activity, areas that have been disturbed but are inactive at the time of the inspection, and areas that have been stabilized (temporary and/or final) since the last inspection;

i. Current phase of construction of all post-construction stormwater management practices and identification of all construction that is not in conformance with the SWPPP and technical standards;

j. Corrective action(s) that must be taken to install, repair, replace or maintain erosion and sediment control practices and pollution prevention measures; and to correct deficiencies identified with the construction of the post-construction stormwater management practice(s);

k. Identification and status of all corrective actions that were required by previous inspection; and

l. Digital photographs, with date stamp, that clearly show the condition of all practices that have been identified as needing corrective actions. The *qualified inspector* shall attach paper color copies of the digital photographs to the inspection report being maintained onsite within seven (7) calendar days of the date of the inspection. The *qualified inspector* shall also take digital photographs, with date stamp, that clearly show the condition of the practice(s) after the corrective action has been completed. The *qualified inspector* shall attach paper color copies of the digital photographs to the inspection report that documents the completion of the corrective action work within seven (7) calendar days of that inspection.

5. Within one business day of the completion of an inspection, the *qualified inspector* shall notify the *owner or operator* and appropriate contractor or subcontractor identified in Part III.A.6. of this permit of any corrective actions that need to be taken. The contractor or subcontractor shall begin implementing the corrective actions within one business day of this notification and shall complete the corrective actions in a reasonable time frame.

6. All inspection reports shall be signed by the *qualified inspector.* Pursuant to Part II.D.2. of this permit, the inspection reports shall be maintained on site with the SWPPP.

Part V. TERMINATION OF PERMIT COVERAGE

A. Termination of Permit Coverage

1. An *owner or operator* that is eligible to terminate coverage under this permit must submit a completed NOT form to the address in Part II.B.1 of this permit. The NOT form shall be one which is associated with this permit, signed in accordance with Part VII.H of this permit.

2. An *owner or operator* may terminate coverage when one or more the following conditions have been met:

   a. Total project completion - All *construction activity* identified in the SWPPP has been completed; <u>and</u> all areas of disturbance have achieved *final stabilization*; <u>and</u> all temporary, structural erosion and sediment control measures have been removed; <u>and</u> all post-construction stormwater management practices have been constructed in conformance with the SWPPP and are operational;

b. Planned shutdown with partial project completion - All soil disturbance activities have ceased; and all areas disturbed as of the project shutdown date have achieved *final stabilization*; and all temporary, structural erosion and sediment control measures have been removed; and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational;

c. A new *owner or operator* has obtained coverage under this permit in accordance with Part II.F. of this permit.

d. The *owner or operator* obtains coverage under an alternative SPDES general permit or an individual SPDES permit.

3. For *construction activities* meeting subdivision 2a. or 2b. of this Part, the *owner or operator* shall have the *qualified inspector* perform a final site inspection prior to submitting the NOT. The *qualified inspector* shall, by signing the "*Final Stabilization*" and "Post-Construction Stormwater Management Practice certification statements on the NOT, certify that all the requirements in Part V.A.2.a. or b. of this permit have been achieved.

4. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4* and meet subdivision 2a. or 2b. of this Part, the *owner or operator* shall have the *regulated, traditional land use control MS4* sign the "MS4 Acceptance" statement on the NOT in accordance with the requirements in Part VII.H. of this permit. The *regulated, traditional land use control MS4* official, by signing this statement, has determined that it is acceptable for the *owner or operator* to submit the NOT in accordance with the requirements of this Part. The *regulated, traditional land use control MS4* can make this determination by performing a final site inspection themselves or by accepting the *qualified inspector*'s final site inspection certification(s) required in Part V.A.3. of this permit.

5. For *construction activities* that require post-construction stormwater management practices and meet subdivision 2a. of this Part, the *owner or operator* must, prior to submitting the NOT, ensure one of the following:

a. the post-construction stormwater management practice(s) and any right-of-way(s) needed to maintain such practice(s) have been deeded to the municipality in which the practice(s) is located,

b. an executed maintenance agreement is in place with the municipality that will maintain the post-construction stormwater management practice(s),

c. for post-construction stormwater management practices that are privately owned, the *owner or operator* has a mechanism in place that requires operation and maintenance of the practice(s) in accordance with the operation and maintenance plan, such as a deed covenant in the *owner or operator's* deed of record,

d. for post-construction stormwater management practices that are owned by a public or private institution (e.g. school, university, hospital), government agency or authority, or public utility; the *owner or operator* has policy and procedures in place that ensures operation and maintenance of the practices in accordance with the operation and maintenance plan.

Part VI. REPORTING AND RETENTION RECORDS

A. Record Retention

The *owner or operator* shall retain a copy of the NOI, NOI Acknowledgment Letter, SWPPP, MS4 SWPPP Acceptance form and any inspection reports that were prepared in conjunction with this permit for a period of at least five (5) years from the date that the Department receives a complete NOT submitted in accordance with Part V. of this general permit.

B. Addresses

With the exception of the NOI, NOT, and MS4 SWPPP Acceptance form (which must be submitted to the address referenced in Part II.B.1 of this permit), all written correspondence requested by the Department, including individual permit applications, shall be sent to the address of the appropriate DOW Water (SPDES) Program contact at the Regional Office listed in Appendix F.

Part VII. STANDARD PERMIT CONDITIONS

A. Duty to Comply

The *owner or operator* must comply with all conditions of this permit. All contractors and subcontractors associated with the project must comply with the terms of the SWPPP. Any non-compliance with this permit constitutes a violation of the Clean Water

Act (CWA) and the ECL and is grounds for an enforcement action against the *owner or operator* and/or the contractor/subcontractor; permit revocation, suspension or modification; or denial of a permit renewal application. Upon a finding of significant non-compliance with this permit or the applicable SWPPP, the Department may order an immediate stop to all *construction activity* at the site until the non-compliance is remedied. The stop work order shall be in writing, shall describe the non-compliance in detail, and shall be sent to the *owner or operator*.

If any human remains or archaeological remains are encountered during excavation, the *owner or operator* must immediately cease, or cause to cease, all *construction activity* in the area of the remains and notify the appropriate Regional Water Engineer (RWE). *Construction activity* shall not resume until written permission to do so has been received from the RWE.

B. Continuation of the Expired General Permit

This permit expires five (5) years from the effective date. If a new general permit is not issued prior to the expiration of this general permit, an *owner or operator* with coverage under this permit may continue to operate and *discharge* in accordance with the terms and conditions of this general permit, if it is extended pursuant to the State Administrative Procedure Act and 6 NYCRR Part 621, until a new general permit is issued.

C. Enforcement

Failure of the *owner or operator,* its contractors, subcontractors, agents and/or assigns to strictly adhere to any of the permit requirements contained herein shall constitute a violation of this permit. There are substantial criminal, civil, and administrative penalties associated with violating the provisions of this permit. Fines of up to $37,500 per day for each violation and imprisonment for up to fifteen (15) years may be assessed depending upon the nature and degree of the offense.

D. Need to Halt or Reduce Activity Not a Defense

It shall not be a defense for an *owner or operator* in an enforcement action that it would have been necessary to halt or reduce the *construction activity* in order to maintain compliance with the conditions of this permit.

E. Duty to Mitigate

The *owner or operator* and its contractors and subcontractors shall take all reasonable steps to *minimize* or prevent any *discharge* in violation of this permit which has a reasonable likelihood of adversely affecting human health or the environment.

F. Duty to Provide Information

The *owner or operator* shall furnish to the Department, within a reasonable specified time period of a written request, all documentation necessary to demonstrate eligibility and any information to determine compliance with this permit or to determine whether cause exists for modifying or revoking this permit, or suspending or denying coverage under this permit, in accordance with the terms and conditions of this permit. The NOI, SWPPP and inspection reports required by this permit are public documents that the *owner or operator* must make available for review and copying by any person within five (5) business days of the *owner or operator* receiving a written request by any such person to review these documents. Copying of documents will be done at the requester's expense.

G. Other Information

When the *owner or operator* becomes aware that they failed to submit any relevant facts, or submitted incorrect information in the NOI or in any of the documents required by this permit , or have made substantive revisions to the SWPPP (e.g. the scope of the project changes significantly, the type of post-construction stormwater management practice(s) changes, there is a reduction in the sizing of the post-construction stormwater management practice, or there is an increase in the disturbance area or *impervious area*), which were not reflected in the original NOI submitted to the Department, they shall promptly submit such facts or information to the Department using the contact information in Part II.A. of this permit. Failure of the *owner or operator* to correct or supplement any relevant facts within five (5) business days of becoming aware of the deficiency shall constitute a violation of this permit.

H. Signatory Requirements

1. All NOIs and NOTs shall be signed as follows:

    a. For a corporation these forms shall be signed by a responsible corporate officer. For the purpose of this section, a responsible corporate officer means:

(i) a president, secretary, treasurer, or vice-president of the corporation in charge of a principal business function, or any other person who performs similar policy or decision-making functions for the corporation; or

(ii) the manager of one or more manufacturing, production or operating facilities, provided the manager is authorized to make management decisions which govern the operation of the regulated facility including having the explicit or implicit duty of making major capital investment recommendations, and initiating and directing other comprehensive measures to assure long term environmental compliance with environmental laws and regulations; the manager can ensure that the necessary systems are established or actions taken to gather complete and accurate information for permit application requirements; and where authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures;

b. For a partnership or sole proprietorship these forms shall be signed by a general partner or the proprietor, respectively; or

c. For a municipality, State, Federal, or other public agency these forms shall be signed by either a principal executive officer or ranking elected official. For purposes of this section, a principal executive officer of a Federal agency includes:

(i) the chief executive officer of the agency, or

(ii) a senior executive officer having responsibility for the overall operations of a principal geographic unit of the agency (e.g., Regional Administrators of EPA).

2. The SWPPP and other information requested by the Department shall be signed by a person described in Part VII.H.1. of this permit or by a duly authorized representative of that person. A person is a duly authorized representative only if:

a. The authorization is made in writing by a person described in Part VII.H.1. of this permit;

b. The authorization specifies either an individual or a position having responsibility for the overall operation of the regulated facility or activity, such as the position of plant manager, operator of a well or a well field, superintendent, position of *equivalent* responsibility, or an individual or position having overall responsibility for environmental matters for the company. (A duly authorized representative may thus be either a named individual or any individual occupying a named position) and,

   c. The written authorization shall include the name, title and signature of the authorized representative and be attached to the SWPPP.

3. All inspection reports shall be signed by the *qualified inspector* that performs the inspection.

4. The MS4 SWPPP Acceptance form shall be signed by the principal executive officer or ranking elected official from the *regulated, traditional land use control MS4,* or by a duly authorized representative of that person.

   It shall constitute a permit violation if an incorrect and/or improper signatory authorizes any required forms, SWPPP and/or inspection reports.

I. Property Rights

The issuance of this permit does not convey any property rights of any sort, nor any exclusive privileges, nor does it authorize any injury to private property nor any invasion of personal rights, nor any infringement of Federal, State or local laws or regulations. *Owners or operators* must obtain any applicable conveyances, easements, licenses and/or access to real property prior to *commencing construction activity*.

J. Severability

The provisions of this permit are severable, and if any provision of this permit, or the application of any provision of this permit to any circumstance, is held invalid, the application of such provision to other circumstances, and the remainder of this permit shall not be affected thereby.

K. Requirement to Obtain Coverage Under an Alternative Permit

1. The Department may require any owner or operator authorized by this permit to apply for and/or obtain either an individual SPDES permit or another SPDES general permit. When the Department requires any discharger authorized by a general permit to apply for an individual SPDES permit, it shall notify the discharger in writing that a permit application is required. This notice shall

include a brief statement of the reasons for this decision, an application form, a statement setting a time frame for the owner or operator to file the application for an individual SPDES permit, and a deadline, not sooner than 180 days from owner or operator receipt of the notification letter, whereby the authorization to discharge under this general permit shall be terminated. Applications must be submitted to the appropriate Permit Administrator at the Regional Office. The Department may grant additional time upon demonstration, to the satisfaction of the Department, that additional time to apply for an alternative authorization is necessary or where the Department has not provided a permit determination in accordance with Part 621 of this Title.

2. When an individual SPDES permit is issued to a discharger authorized to *discharge* under a general SPDES permit for the same *discharge*(s), the general permit authorization for outfalls authorized under the individual SPDES permit is automatically terminated on the effective date of the individual permit unless termination is earlier in accordance with 6 NYCRR Part 750.

L. Proper Operation and Maintenance

The *owner or operator* shall at all times properly operate and maintain all facilities and systems of treatment and control (and related appurtenances) which are installed or used by the *owner or operator* to achieve compliance with the conditions of this permit and with the requirements of the SWPPP.

M. Inspection and Entry

The *owner or operator* shall allow an authorized representative of the Department, EPA, applicable county health department, or, in the case of a *construction site* which *discharges* through an *MS4*, an authorized representative of the *MS4* receiving the discharge, upon the presentation of credentials and other documents as may be required by law, to:

1. Enter upon the owner’s or operator's premises where a regulated facility or activity is located or conducted or where records must be kept under the conditions of this permit;

2. Have access to and copy at reasonable times, any records that must be kept under the conditions of this permit; and

3. Inspect at reasonable times any facilities or equipment (including monitoring and control equipment), practices or operations regulated or required by this permit.

4. Sample or monitor at reasonable times, for purposes of assuring permit compliance or as otherwise authorized by the Act or ECL, any substances or parameters at any location.

N. Permit Actions

This permit may, at any time, be modified, suspended, revoked, or renewed by the Department in accordance with 6 NYCRR Part 621. The filing of a request by the *owner or operator* for a permit modification, revocation and reissuance, termination, a notification of planned changes or anticipated noncompliance does not limit, diminish and/or stay compliance with any terms of this permit.

O. Definitions

Definitions of key terms are included in Appendix A of this permit.

P. Re-Opener Clause

1. If there is evidence indicating potential or realized impacts on water quality due to any stormwater discharge associated with construction activity covered by this permit, the owner or operator of such discharge may be required to obtain an individual permit or alternative general permit in accordance with Part VII.K. of this permit or the permit may be modified to include different limitations and/or requirements.

2. Any Department initiated permit modification, suspension or revocation will be conducted in accordance with 6 NYCRR Part 621, 6 NYCRR 750-1.18, and 6 NYCRR 750-1.20.

Q. Penalties for Falsification of Forms and Reports

In accordance with 6NYCRR Part 750-2.4 and 750-2.5, any person who knowingly makes any false material statement, representation, or certification in any application, record, report or other document filed or required to be maintained under this permit, including reports of compliance or noncompliance shall, upon conviction, be punished in accordance with ECL §71-1933 and or Articles 175 and 210 of the New York State Penal Law.

R. Other Permits

Nothing in this permit relieves the *owner or operator* from a requirement to obtain any other permits required by law.

APPENDIX A – Acronyms and Definitions

Acronyms

APO – Agency Preservation Officer
BMP – Best Management Practice
CPESC – Certified Professional in Erosion and Sediment Control
Cpv – Channel Protection Volume
CWA – Clean Water Act (or the Federal Water Pollution Control Act, 33 U.S.C. §1251 et seq)
DOW – Division of Water
EAF – Environmental Assessment Form
ECL - Environmental Conservation Law
EPA – U. S. Environmental Protection Agency
HSG – Hydrologic Soil Group
MS4 – Municipal Separate Storm Sewer System
NOI – Notice of Intent
NOT – Notice of Termination
NPDES – National Pollutant Discharge Elimination System
OPRHP – Office of Parks, Recreation and Historic Places
Qf – Extreme Flood
Qp – Overbank Flood
RRv – Runoff Reduction Volume
RWE – Regional Water Engineer
SEQR – State Environmental Quality Review
SEQRA - State Environmental Quality Review Act
SHPA – State Historic Preservation Act
SPDES – State Pollutant Discharge Elimination System
SWPPP – Stormwater Pollution Prevention Plan
TMDL – Total Maximum Daily Load
UPA – Uniform Procedures Act
USDA – United States Department of Agriculture
WQv – Water Quality Volume

Definitions

All definitions in this section are solely for the purposes of this permit.

Agricultural Building – a structure designed and constructed to house farm implements, hay, grain, poultry, livestock or other horticultural products; excluding any structure designed, constructed or used, in whole or in part, for human habitation, as a place of employment where agricultural products are processed, treated or packaged, or as a place used by the public.

Agricultural Property –means the land for construction of a barn, *agricultural building*, silo, stockyard, pen or other structural practices identified in Table II in the "Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State" prepared by the Department in cooperation with agencies of New York Nonpoint Source Coordinating Committee (dated June 2007).

Alter Hydrology from Pre to Post-Development Conditions - means the post-development peak flow rate(s) has increased by more than 5% of the pre-developed condition for the design storm of interest (e.g. 10 yr and 100 yr).

Combined Sewer - means a sewer that is designed to collect and convey both "sewage" and "stormwater".

Commence (Commencement of) Construction Activities - means the initial disturbance of soils associated with clearing, grading or excavation activities; or other construction related activities that disturb or expose soils such as demolition, stockpiling of fill material, and the initial installation of erosion and sediment control practices required in the SWPPP. See definition for "*Construction Activity(ies)*" also.

Construction Activity(ies) - means any clearing, grading, excavation, filling, demolition or stockpiling activities that result in soil disturbance. Clearing activities can include, but are not limited to, logging equipment operation, the cutting and skidding of trees, stump removal and/or brush root removal. Construction activity does not include routine maintenance that is performed to maintain the original line and grade, hydraulic capacity, or original purpose of a facility.

Construction Site – means the land area where *construction activity(ies)* will occur. See definition for "*Commence (Commencement of) Construction Activities"* and "*Larger Common Plan of Development or Sale" also.*

Dewatering – means the act of draining rainwater and/or groundwater from building foundations, vaults or excavations/trenches.

Direct Discharge (to a specific surface waterbody) - means that runoff flows from a *construction site* by overland flow and the first point of discharge is the specific surface waterbody, or runoff flows from a *construction site* to a separate storm sewer system and the first point of discharge from the separate storm sewer system is the specific surface waterbody.

Discharge(s) - means any addition of any pollutant to waters of the State through an outlet or *point source*.

Embankment –means an earthen or rock slope that supports a road/highway.

Endangered or Threatened Species – see 6 NYCRR Part 182 of the Department's rules and regulations for definition of terms and requirements.

Environmental Conservation Law (ECL) - means chapter 43-B of the Consolidated Laws of the State of New York, entitled the Environmental Conservation Law.

Equivalent (Equivalence) – means that the practice or measure meets all the performance, longevity, maintenance, and safety objectives of the technical standard and will provide an equal or greater degree of water quality protection.

Final Stabilization - means that all soil disturbance activities have ceased and a uniform, perennial vegetative cover with a density of eighty (80) percent over the entire pervious surface has been established; or other equivalent stabilization measures, such as permanent landscape mulches, rock rip-rap or washed/crushed stone have been applied on all disturbed areas that are not covered by permanent structures, concrete or pavement.

General SPDES permit - means a SPDES permit issued pursuant to 6 NYCRR Part 750-1.21 and Section 70-0117 of the ECL authorizing a category of discharges.

Groundwater(s) - means waters in the saturated zone. The saturated zone is a subsurface zone in which all the interstices are filled with water under pressure greater than that of the atmosphere. Although the zone may contain gas-filled interstices or interstices filled with fluids other than water, it is still considered saturated.

Historic Property – means any building, structure, site, object or district that is listed on the State or National Registers of Historic Places or is determined to be eligible for listing on the State or National Registers of Historic Places.

Impervious Area (Cover) - means all impermeable surfaces that cannot effectively infiltrate rainfall. This includes paved, concrete and gravel surfaces (i.e. parking lots, driveways, roads, runways and sidewalks); building rooftops and miscellaneous impermeable structures such as patios, pools, and sheds.

Infeasible – means not technologically possible, or not economically practicable and achievable in light of best industry practices.

Larger Common Plan of Development or Sale - means a contiguous area where multiple separate and distinct *construction activities* are occurring, or will occur, under one plan. The term “plan” in “larger common plan of development or sale” is broadly defined as any announcement or piece of documentation (including a sign, public notice or hearing, marketing plan, advertisement, drawing, permit application, State Environmental Quality Review Act (SEQRA) environmental assessment form or other documents, zoning request, computer design, etc.) or physical demarcation (including boundary signs, lot stakes, surveyor markings, etc.) indicating that *construction activities* may occur on a specific plot.

For discrete construction projects that are located within a larger common plan of development or sale that are at least 1/4 mile apart, each project can be treated as a separate plan of development or sale provided any interconnecting road, pipeline or utility project that is part of the same “common plan” is not concurrently being disturbed.

Minimize – means reduce and/or eliminate to the extent achievable using control measures (including best management practices) that are technologically available and economically practicable and achievable in light of best industry practices.

Municipal Separate Storm Sewer (MS4) - a conveyance or system of conveyances (including roads with drainage systems, municipal streets, catch basins, curbs, gutters, ditches, man-made channels, or storm drains):

(i) Owned or operated by a State, city, town, borough, county, parish, district, association, or other public body (created by or pursuant to State law) having jurisdiction over disposal of sewage, industrial wastes, stormwater, or other wastes, including special districts under State law such as a sewer district, flood control district or drainage district, or similar entity, or an Indian tribe or an authorized Indian tribal organization, or a designated and approved management agency under section 208 of the CWA that discharges to surface waters of the State*;*
(ii) Designed or used for collecting or conveying stormwater;
(iii) Which is not a *combined sewer*; and
(iv) Which is not part of a Publicly Owned Treatment Works (POTW) as defined at 40 CFR 122.2.

National Pollutant Discharge Elimination System (NPDES) - means the national system for the issuance of wastewater and stormwater permits under the Federal Water Pollution Control Act (Clean Water Act).

Natural Buffer –means an undisturbed area with natural cover running along a surface water (e.g. wetland, stream, river, lake, etc.).

New Development – means any land disturbance that does not meet the definition of Redevelopment Activity included in this appendix.

New York State Erosion and Sediment Control Certificate Program – a certificate program that establishes and maintains a process to identify and recognize individuals who are capable of developing, designing, inspecting and maintaining erosion and sediment control plans on projects that disturb soils in New York State. The certificate program is administered by the New York State Conservation District Employees Association.

NOI Acknowledgment Letter - means the letter that the Department sends to an owner or operator to acknowledge the Department's receipt and acceptance of a complete Notice of Intent. This letter documents the owner's or operator's authorization to discharge in accordance with the general permit for stormwater discharges from *construction activity*.

Nonpoint Source - means any source of water pollution or pollutants which is not a discrete conveyance or *point source* permitted pursuant to Title 7 or 8 of Article 17 of the Environmental Conservation Law (see ECL Section 17-1403).

Overbank –means flow events that exceed the capacity of the stream channel and spill out into the adjacent floodplain.

Owner or Operator - means the person, persons or legal entity which owns or leases the property on which the *construction activity* is occurring; an entity that has operational control over the construction plans and specifications, including the ability to make modifications to the plans and specifications; and/or an entity that has day-to-day operational control of those activities at a project that are necessary to ensure compliance with the permit conditions.

Performance Criteria – means the design criteria listed under the "Required Elements" sections in Chapters 5, 6 and 10 of the technical standard, New York State Stormwater Management Design Manual, dated January 2015. It does not include the Sizing Criteria (i.e. WQv, RRv, Cpv, Qp and Qf) in Part I.C.2. of the permit.

Point Source - means any discernible, confined and discrete conveyance, including but not limited to any pipe, ditch, channel, tunnel, conduit, well, discrete fissure, container, rolling stock, concentrated animal feeding operation, vessel or other floating craft, or landfill leachate collection system from which *pollutants* are or may be discharged.

Pollutant - means dredged spoil, filter backwash, solid waste, incinerator residue, sewage, garbage, sewage sludge, munitions, chemical wastes, biological materials, radioactive materials, heat, wrecked or discarded equipment, rock, sand and industrial, municipal, agricultural waste and ballast discharged into water; which may cause or might reasonably be expected to cause pollution of the waters of the state in contravention of the standards or guidance values adopted as provided in 6 NYCRR Parts 700 et seq .

Qualified Inspector - means a person that is knowledgeable in the principles and practices of erosion and sediment control, such as a licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder or other Department endorsed individual(s).

It can also mean someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided that person has training in the principles and practices of erosion and sediment control. Training in the principles and practices of erosion and sediment control means that the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect has received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect shall receive four (4) hours of training every three (3) years.

It can also mean a person that meets the *Qualified Professional* qualifications in addition to the *Qualified Inspector* qualifications.

Note: Inspections of any post-construction stormwater management practices that include structural components, such as a dam for an impoundment, shall be performed by a licensed Professional Engineer.

Qualified Professional - means a person that is knowledgeable in the principles and practices of stormwater management and treatment, such as a licensed Professional Engineer, Registered Landscape Architect or other Department endorsed individual(s). Individuals preparing SWPPPs that require the post-construction stormwater management practice component must have an understanding of the principles of hydrology, water quality management practice design, water quantity control design, and, in many cases, the principles of hydraulics. All components of the SWPPP that involve the practice of engineering, as defined by the NYS Education Law (see Article 145), shall be prepared by, or under the direct supervision of, a professional engineer licensed to practice in the State of New York.

Redevelopment Activity(ies) – means the disturbance and reconstruction of existing impervious area, including impervious areas that were removed from a project site within five (5) years of preliminary project plan submission to the local government (i.e. site plan, subdivision, etc.).

Regulated, Traditional Land Use Control MS4 - means a city, town or village with land use control authority that is authorized to discharge under New York State DEC's

SPDES General Permit For Stormwater Discharges from Municipal Separate Stormwater Sewer Systems (MS4s) or the City of New York's Individual SPDES Permit for their Municipal Separate Storm Sewer Systems (NY-0287890).

Routine Maintenance Activity - means *construction activity* that is performed to maintain the original line and grade, hydraulic capacity, or original purpose of a facility, including, but not limited to:

- Re-grading of gravel roads or parking lots,
- Cleaning and shaping of existing roadside ditches and culverts that maintains the approximate original line and grade, and hydraulic capacity of the ditch,
- Cleaning and shaping of existing roadside ditches that does not maintain the approximate original grade, hydraulic capacity and purpose of the ditch if the changes to the line and grade, hydraulic capacity or purpose of the ditch are installed to improve water quality and quantity controls (e.g. installing grass lined ditch),
- Placement of aggregate shoulder backing that stabilizes the transition between the road shoulder and the ditch or *embankment*,
- Full depth milling and filling of existing asphalt pavements, replacement of concrete pavement slabs, and similar work that does not expose soil or disturb the bottom six (6) inches of subbase material,
- Long-term use of equipment storage areas at or near highway maintenance facilities,
- Removal of sediment from the edge of the highway to restore a previously existing sheet-flow drainage connection from the highway surface to the highway ditch or *embankment*,
- Existing use of Canal Corp owned upland disposal sites for the canal, and
- Replacement of curbs, gutters, sidewalks and guide rail posts.

Site limitations – means site conditions that prevent the use of an infiltration technique and or infiltration of the total WQv. Typical site limitations include: seasonal high groundwater, shallow depth to bedrock, and soils with an infiltration rate less than 0.5 inches/hour. The existence of site limitations shall be confirmed and documented using actual field testing (i.e. test pits, soil borings, and infiltration test) or using information from the most current United States Department of Agriculture (USDA) Soil Survey for the County where the project is located.

Sizing Criteria – means the criteria included in Part I.C.2 of the permit that are used to size post-construction stormwater management control practices. The criteria include; Water Quality Volume (WQv), Runoff Reduction Volume (RRv), Channel Protection Volume (Cpv), *Overbank* Flood (Qp), and Extreme Flood (Qf).

State Pollutant Discharge Elimination System (SPDES) - means the system established pursuant to Article 17 of the ECL and 6 NYCRR Part 750 for issuance of permits authorizing discharges to the waters of the state.

Steep Slope – means land area designated on the current United States Department of Agriculture ("USDA") Soil Survey as Soil Slope Phase "D", (provided the map unit name is inclusive of slopes greater than 25%) , or Soil Slope Phase E or F, (regardless of the map unit name), or a combination of the three designations.

Streambank – as used in this permit, means the terrain alongside the bed of a creek or stream. The bank consists of the sides of the channel, between which the flow is confined.

Stormwater Pollution Prevention Plan (SWPPP) – means a project specific report, including construction drawings, that among other things: describes the construction activity(ies), identifies the potential sources of pollution at the *construction site*; describes and shows the stormwater controls that will be used to control the pollutants (i.e. erosion and sediment controls; for many projects, includes post-construction stormwater management controls); and identifies procedures the *owner or operator* will implement to comply with the terms and conditions of the permit. See Part III of the permit for a complete description of the information that must be included in the SWPPP.

Surface Waters of the State - shall be construed to include lakes, bays, sounds, ponds, impounding reservoirs, springs, rivers, streams, creeks, estuaries, marshes, inlets, canals, the Atlantic ocean within the territorial seas of the state of New York and all other bodies of surface water, natural or artificial, inland or coastal, fresh or salt, public or private (except those private waters that do not combine or effect a junction with natural surface waters), which are wholly or partially within or bordering the state or within its jurisdiction. Waters of the state are further defined in 6 NYCRR Parts 800 to 941.

Temporarily Ceased – means that an existing disturbed area will not be disturbed again within 14 calendar days of the previous soil disturbance.

Temporary Stabilization - means that exposed soil has been covered with material(s) as set forth in the technical standard, New York Standards and Specifications for Erosion and Sediment Control, to prevent the exposed soil from eroding. The materials can include, but are not limited to, mulch, seed and mulch, and erosion control mats (e.g. jute twisted yarn, excelsior wood fiber mats).

Total Maximum Daily Loads (TMDLs) - A TMDL is the sum of the allowable loads of a single pollutant from all contributing point and *nonpoint source*s. It is a calculation of the maximum amount of a pollutant that a waterbody can receive on a daily basis and still meet *water quality standards*, and an allocation of that amount to the pollutant's sources. A TMDL stipulates wasteload allocations (WLAs) for *point source* discharges, load allocations (LAs) for *nonpoint source*s, and a margin of safety (MOS).

Trained Contractor - means an employee from the contracting (construction) company, identified in Part III.A.6., that has received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the *trained contractor* shall receive four (4) hours of training every three (3) years.

It can also mean an employee from the contracting (construction) company, identified in Part III.A.6., that meets the *qualified inspector* qualifications (e.g. licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder, or someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity).

The *trained contractor* is responsible for the day to day implementation of the SWPPP.

Uniform Procedures Act (UPA) Permit - means a permit required under 6 NYCRR Part 621 of the Environmental Conservation Law (ECL), Article 70.

Water Quality Standard - means such measures of purity or quality for any waters in relation to their reasonable and necessary use as promulgated in 6 NYCRR Part 700 et seq.

APPENDIX B – Required SWPPP Components by Project Type

Table 1

Construction Activities that Require the Preparation of a SWPPP That Only Includes Erosion and Sediment Controls

The following construction activities that involve soil disturbances of one (1) or more acres of land, but less than five (5) acres:

- Single family home not located in one of the watersheds listed in Appendix C or not *directly discharging* to one of the 303(d) segments listed in Appendix E
- Single family residential subdivisions with 25% or less impervious cover at total site build-out and not located in one of the watersheds listed in Appendix C and not directly discharging to one of the 303(d) segments listed in Appendix E
- Construction of a barn or other *agricultural building*, silo, stock yard or pen.

The following construction activities that involve soil disturbances between five thousand (5000) square feet and one (1) acre of land:

All construction activities located in the watersheds identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Installation of underground, linear utilities; such as gas lines, fiber-optic cable, cable TV, electric, telephone, sewer mains, and water mains
- Environmental enhancement projects, such as wetland mitigation projects, stormwater retrofits and stream restoration projects
- Pond construction
- Linear bike paths running through areas with vegetative cover, including bike paths surfaced with an impervious cover
- Cross-country ski trails and walking/hiking trails
- Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are not part of residential, commercial or institutional development;
- Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that include incidental shoulder or curb work along an existing highway to support construction of the sidewalk, bike path or walking path.
- Slope stabilization projects
- Slope flattening that changes the grade of the site, but does not significantly change the runoff characteristics

Table 1 (Continued) CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP
THAT ONLY INCLUDES EROSION AND SEDIMENT CONTROLS

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Spoil areas that will be covered with vegetation
- Vegetated open space projects (i.e. recreational parks, lawns, meadows, fields, downhill ski trails) excluding projects that *alter hydrology from pre to post development* conditions,
- Athletic fields (natural grass) that do not include the construction or reconstruction of *impervious area* and do not *alter hydrology from pre to post development* conditions
- Demolition project where vegetation will be established, and no redevelopment is planned
- Overhead electric transmission line project that does not include the construction of permanent access roads or parking areas surfaced with *impervious cover*
- Structural practices as identified in Table II in the “Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State”, excluding projects that involve soil disturbances of greater than five acres and construction activities that include the construction or reconstruction of impervious area
- Temporary access roads, median crossovers, detour roads, lanes, or other temporary impervious areas that will be restored to pre-construction conditions once the construction activity is complete

Table 2

CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT INCLUDES POST-CONSTRUCTION STORMWATER MANAGEMENT PRACTICES

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Single family home located in one of the watersheds listed in Appendix C or *directly discharging* to one of the 303(d) segments listed in Appendix E
- Single family home that disturbs five (5) or more acres of land
- Single family residential subdivisions located in one of the watersheds listed in Appendix C or *directly discharging* to one of the 303(d) segments listed in Appendix E
- Single family residential subdivisions that involve soil disturbances of between one (1) and five (5) acres of land with greater than 25% impervious cover at total site build-out
- Single family residential subdivisions that involve soil disturbances of five (5) or more acres of land, and single family residential subdivisions that involve soil disturbances of less than five (5) acres that are part of a larger common plan of development or sale that will ultimately disturb five or more acres of land
- Multi-family residential developments; includes duplexes, townhomes, condominiums, senior housing complexes, apartment complexes, and mobile home parks
- Airports
- Amusement parks
- Breweries, cideries, and wineries, including establishments constructed on agricultural land
- Campgrounds
- Cemeteries that include the construction or reconstruction of impervious area (>5% of disturbed area) or *alter the hydrology from pre to post development* conditions
- Commercial developments
- Churches and other places of worship
- Construction of a barn or other *agricultural building* (e.g. silo) and structural practices as identified in Table II in the “Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State” that include the construction or reconstruction of *impervious area*, excluding projects that involve soil disturbances of less than five acres.
- Golf courses
- Institutional development; includes hospitals, prisons, schools and colleges
- Industrial facilities; includes industrial parks
- Landfills
- Municipal facilities; includes highway garages, transfer stations, office buildings, POTW’s, water treatment plants, and water storage tanks
- Office complexes
- Playgrounds that include the construction or reconstruction of impervious area
- Sports complexes
- Racetracks; includes racetracks with earthen (dirt) surface
- Road construction or reconstruction, including roads constructed as part of the construction activities listed in Table 1

Table 2 (Continued)

CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT INCLUDES POST-CONSTRUCTION STORMWATER MANAGEMENT PRACTICES

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Parking lot construction or reconstruction, including parking lots constructed as part of the construction activities listed in Table 1
- Athletic fields (natural grass) that include the construction or reconstruction of impervious area (>5% of disturbed area) or *alter the hydrology from pre to post development* conditions
- Athletic fields with artificial turf
- Permanent access roads, parking areas, substations, compressor stations and well drilling pads, surfaced with *impervious cover,* and constructed as part of an over-head electric transmission line project, wind-power project, cell tower project, oil or gas well drilling project, sewer or water main project or other linear utility project
- Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are part of a residential, commercial or institutional development
- Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are part of a highway construction or reconstruction project
- All other construction activities that include the construction or reconstruction of *impervious area* or *alter the hydrology from pre to post development* conditions, and are not listed in Table 1

APPENDIX C – Watersheds Requiring Enhanced Phosphorus Removal

Watersheds where *owners or operators* of construction activities identified in Table 2 of Appendix B must prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the Enhanced Phosphorus Removal Standards included in the technical standard, New York State Stormwater Management Design Manual ("Design Manual").

- Entire New York City Watershed located east of the Hudson River **-** Figure 1
- Onondaga Lake Watershed **-** Figure 2
- Greenwood Lake Watershed **-**Figure 3
- Oscawana Lake Watershed – Figure 4
- Kinderhook Lake Watershed – Figure 5

Figure 1 - New York City Watershed East of the Hudson

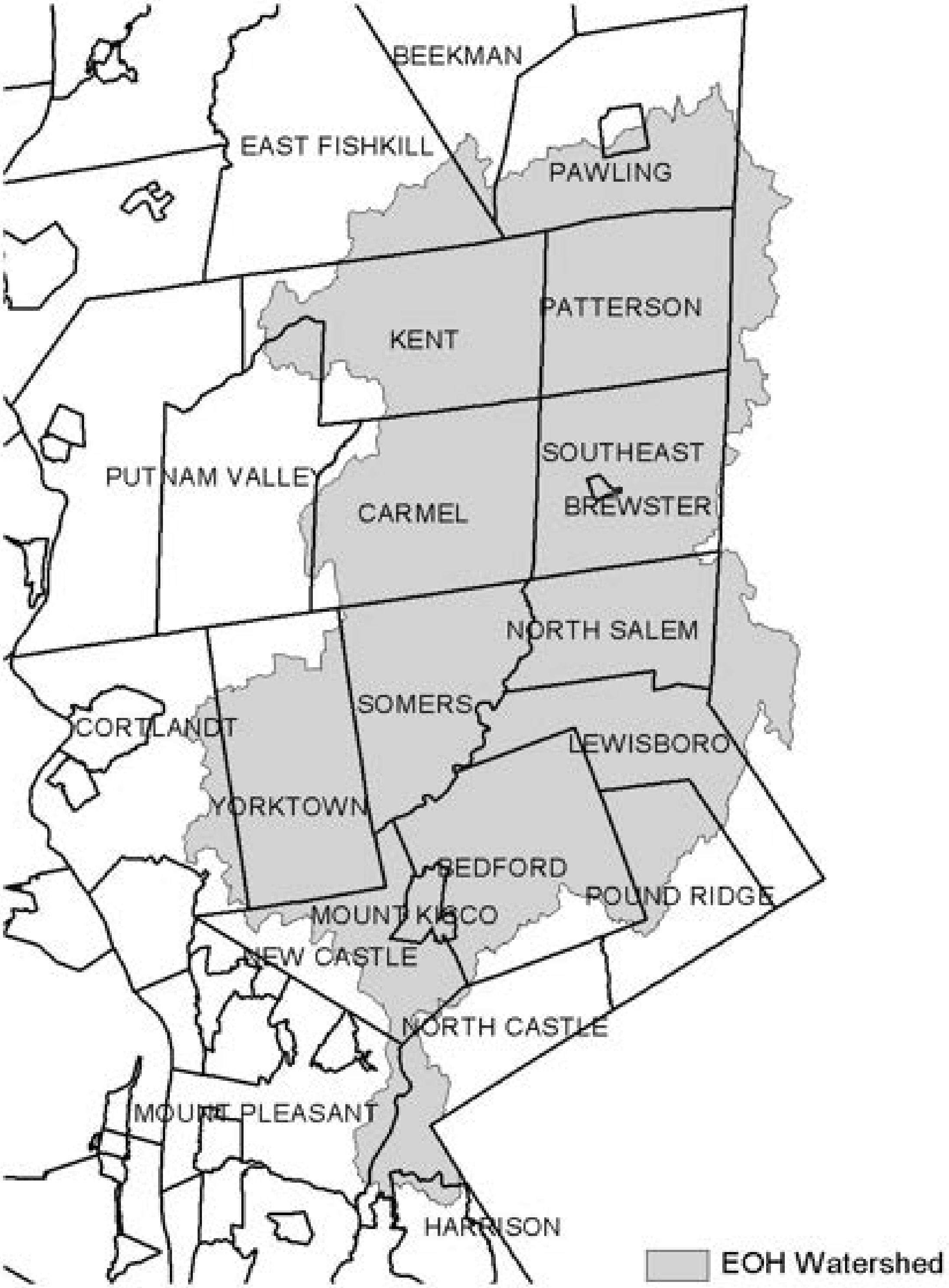


Figure 2 - Onondaga Lake Watershed

Figure 3 - Greenwood Lake Watershed

Figure 4 - Oscawana Lake Watershed

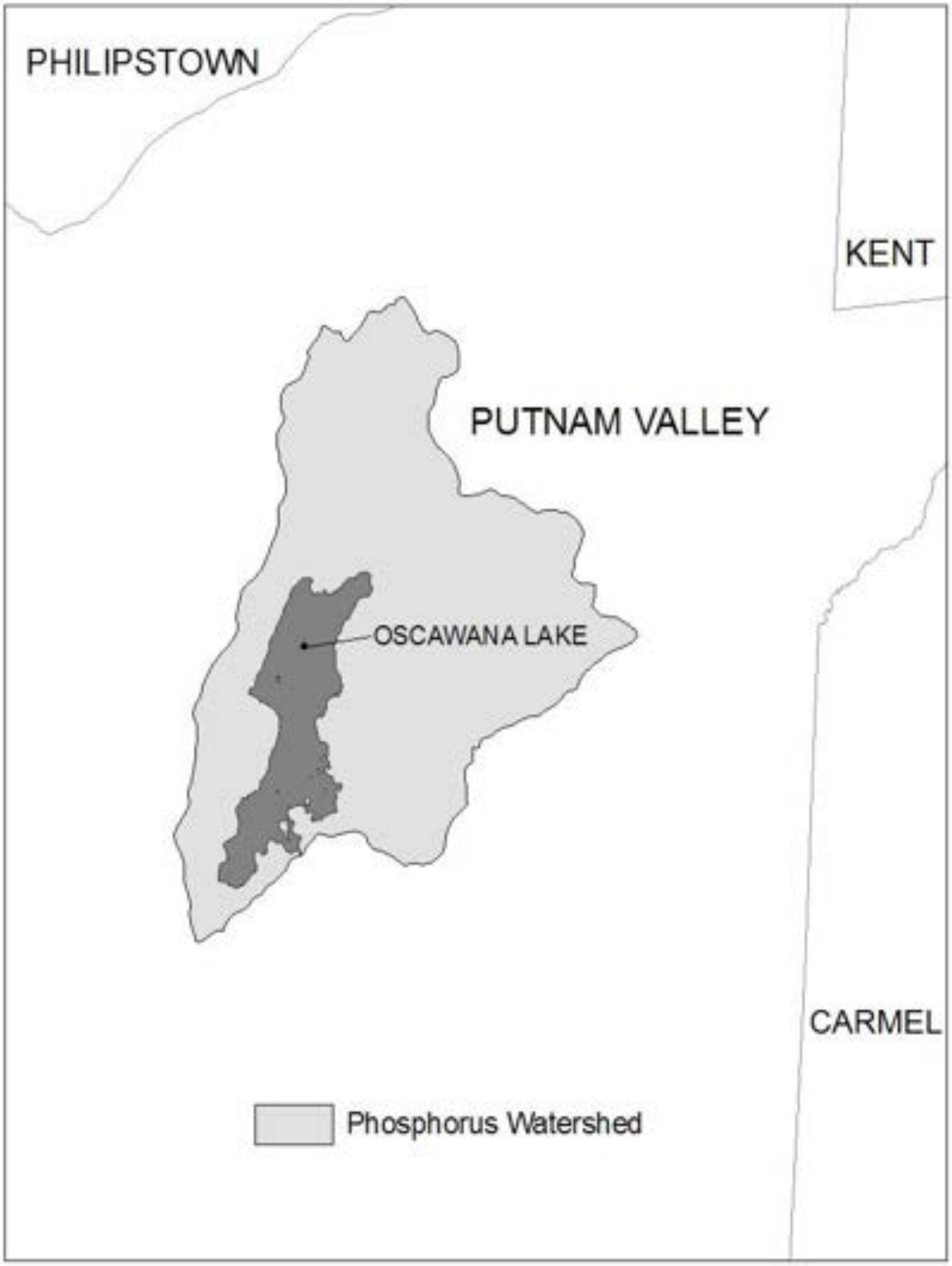
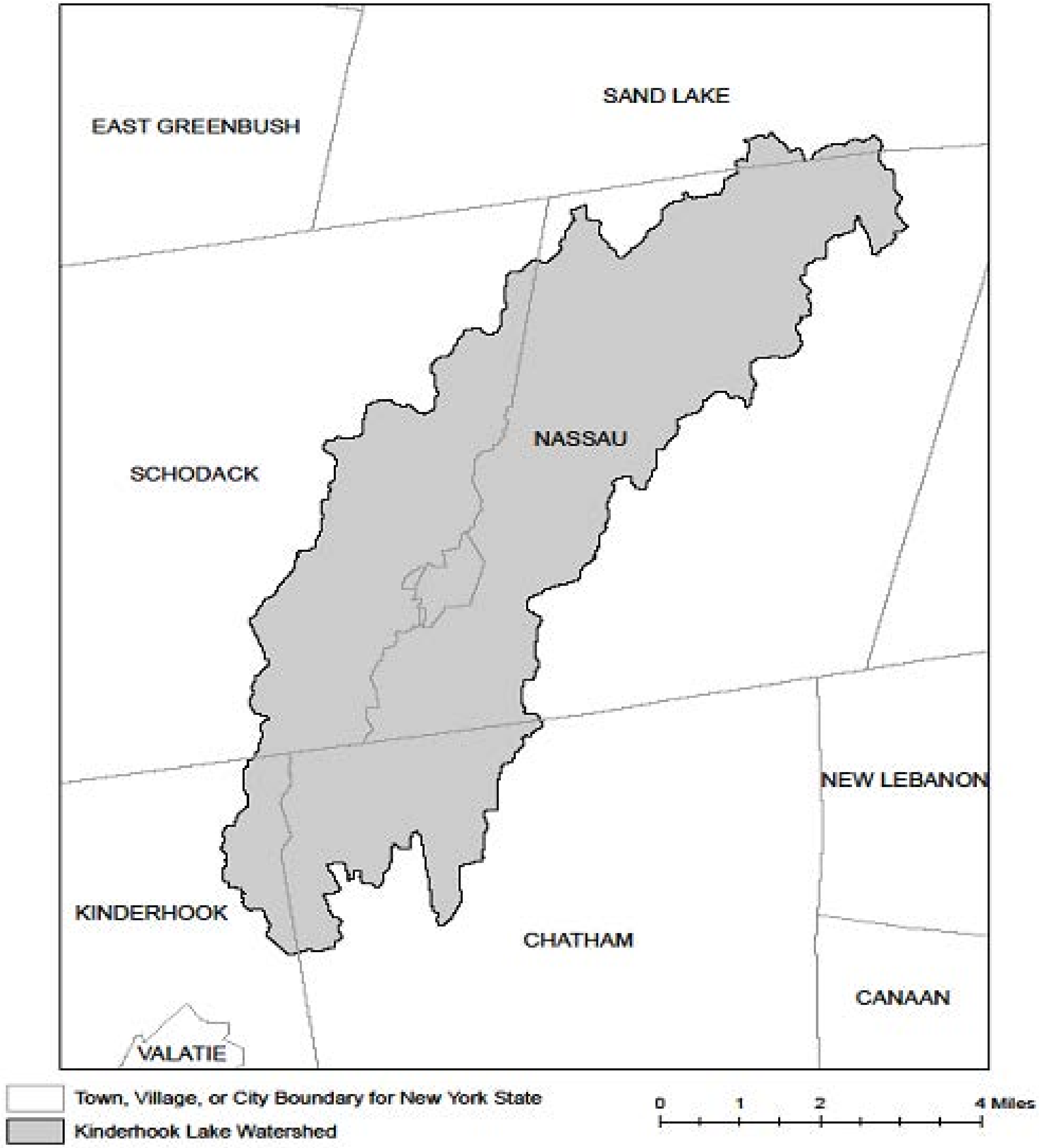


Figure 5 - Kinderhook Lake Watershed



APPENDIX D – Watersheds with Lower Disturbance Threshold

Watersheds where *owners or operators* of construction activities that involve soil disturbances between five thousand (5000) square feet and one (1) acre of land must obtain coverage under this permit.

Entire New York City Watershed that is located east of the Hudson River **-** See Figure 1 in Appendix C

APPENDIX E – 303(d) Segments Impaired by Construction Related Pollutant(s)

List of 303(d) segments impaired by pollutants related to *construction activity* (e.g. silt, sediment or nutrients). The list was developed using ”The Final New York State 2016 Section 303(d) List of Impaired Waters Requiring a TMDL/Other Strategy” dated November 2016. *Owners or operators* of single family home and single family residential subdivisions with 25% or less total impervious cover at total site build-out that involve soil disturbances of one or more acres of land, but less than 5 acres, and *directly discharge* to one of the listed segments below shall prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the New York State Stormwater Management Design Manual (“Design Manual”), dated January 2015.

COUNTY	WATERBODY	POLLUTANT
Albany	Ann Lee (Shakers) Pond, Stump Pond	Nutrients
Albany	Basic Creek Reservoir	Nutrients
Allegany	Amity Lake, Saunders Pond	Nutrients
Bronx	Long Island Sound, Bronx	Nutrients
Bronx	Van Cortlandt Lake	Nutrients
Broome	Fly Pond, Deer Lake, Sky Lake	Nutrients
Broome	Minor Tribs to Lower Susquehanna (north)	Nutrients
Broome	Whitney Point Lake/Reservoir	Nutrients
Cattaraugus	Allegheny River/Reservoir	Nutrients
Cattaraugus	Beaver (Alma) Lake	Nutrients
Cattaraugus	Case Lake	Nutrients
Cattaraugus	Linlyco/Club Pond	Nutrients
Cayuga	Duck Lake	Nutrients
Cayuga	Little Sodus Bay	Nutrients
Chautauqua	Bear Lake	Nutrients
Chautauqua	Chadakoin River and tribs	Nutrients
Chautauqua	Chautauqua Lake, North	Nutrients
Chautauqua	Chautauqua Lake, South	Nutrients
Chautauqua	Findley Lake	Nutrients
Chautauqua	Hulburt/Clymer Pond	Nutrients
Clinton	Great Chazy River, Lower, Main Stem	Silt/Sediment
Clinton	Lake Champlain, Main Lake, Middle	Nutrients
Clinton	Lake Champlain, Main Lake, North	Nutrients
Columbia	Kinderhook Lake	Nutrients
Columbia	Robinson Pond	Nutrients
Cortland	Dean Pond	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Dutchess	Fall Kill and tribs	Nutrients
Dutchess	Hillside Lake	Nutrients
Dutchess	Wappingers Lake	Nutrients
Dutchess	Wappingers Lake	Silt/Sediment
Erie	Beeman Creek and tribs	Nutrients
Erie	Ellicott Creek, Lower, and tribs	Silt/Sediment
Erie	Ellicott Creek, Lower, and tribs	Nutrients
Erie	Green Lake	Nutrients
Erie	Little Sister Creek, Lower, and tribs	Nutrients
Erie	Murder Creek, Lower, and tribs	Nutrients
Erie	Rush Creek and tribs	Nutrients
Erie	Scajaquada Creek, Lower, and tribs	Nutrients
Erie	Scajaquada Creek, Middle, and tribs	Nutrients
Erie	Scajaquada Creek, Upper, and tribs	Nutrients
Erie	South Branch Smoke Cr, Lower, and tribs	Silt/Sediment
Erie	South Branch Smoke Cr, Lower, and tribs	Nutrients
Essex	Lake Champlain, Main Lake, South	Nutrients
Essex	Lake Champlain, South Lake	Nutrients
Essex	Willsboro Bay	Nutrients
Genesee	Bigelow Creek and tribs	Nutrients
Genesee	Black Creek, Middle, and minor tribs	Nutrients
Genesee	Black Creek, Upper, and minor tribs	Nutrients
Genesee	Bowen Brook and tribs	Nutrients
Genesee	LeRoy Reservoir	Nutrients
Genesee	Oak Orchard Cr, Upper, and tribs	Nutrients
Genesee	Tonawanda Creek, Middle, Main Stem	Nutrients
Greene	Schoharie Reservoir	Silt/Sediment
Greene	Sleepy Hollow Lake	Silt/Sediment
Herkimer	Steele Creek tribs	Silt/Sediment
Herkimer	Steele Creek tribs	Nutrients
Jefferson	Moon Lake	Nutrients
Kings	Hendrix Creek	Nutrients
Kings	Prospect Park Lake	Nutrients
Lewis	Mill Creek/South Branch, and tribs	Nutrients
Livingston	Christie Creek and tribs	Nutrients
Livingston	Conesus Lake	Nutrients
Livingston	Mill Creek and minor tribs	Silt/Sediment
Monroe	Black Creek, Lower, and minor tribs	Nutrients
Monroe	Buck Pond	Nutrients
Monroe	Cranberry Pond	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Monroe	Lake Ontario Shoreline, Western	Nutrients
Monroe	Long Pond	Nutrients
Monroe	Mill Creek and tribs	Nutrients
Monroe	Mill Creek/Blue Pond Outlet and tribs	Nutrients
Monroe	Minor Tribs to Irondequoit Bay	Nutrients
Monroe	Rochester Embayment - East	Nutrients
Monroe	Rochester Embayment - West	Nutrients
Monroe	Shipbuilders Creek and tribs	Nutrients
Monroe	Thomas Creek/White Brook and tribs	Nutrients
Nassau	Beaver Lake	Nutrients
Nassau	Camaans Pond	Nutrients
Nassau	East Meadow Brook, Upper, and tribs	Silt/Sediment
Nassau	East Rockaway Channel	Nutrients
Nassau	Grant Park Pond	Nutrients
Nassau	Hempstead Bay	Nutrients
Nassau	Hempstead Lake	Nutrients
Nassau	Hewlett Bay	Nutrients
Nassau	Hog Island Channel	Nutrients
Nassau	Long Island Sound, Nassau County Waters	Nutrients
Nassau	Massapequa Creek and tribs	Nutrients
Nassau	Milburn/Parsonage Creeks, Upp, and tribs	Nutrients
Nassau	Reynolds Channel, west	Nutrients
Nassau	Tidal Tribs to Hempstead Bay	Nutrients
Nassau	Tribs (fresh) to East Bay	Nutrients
Nassau	Tribs (fresh) to East Bay	Silt/Sediment
Nassau	Tribs to Smith/Halls Ponds	Nutrients
Nassau	Woodmere Channel	Nutrients
New York	Harlem Meer	Nutrients
New York	The Lake in Central Park	Nutrients
Niagara	Bergholtz Creek and tribs	Nutrients
Niagara	Hyde Park Lake	Nutrients
Niagara	Lake Ontario Shoreline, Western	Nutrients
Niagara	Lake Ontario Shoreline, Western	Nutrients
Oneida	Ballou, Nail Creeks and tribs	Nutrients
Onondaga	Harbor Brook, Lower, and tribs	Nutrients
Onondaga	Ley Creek and tribs	Nutrients
Onondaga	Minor Tribs to Onondaga Lake	Nutrients
Onondaga	Ninemile Creek, Lower, and tribs	Nutrients
Onondaga	Onondaga Creek, Lower, and tribs	Nutrients
Onondaga	Onondaga Creek, Middle, and tribs	Nutrients

303(d) Segments Impaired by Construction Related Pollutant(s)

Onondaga	Onondaga Lake, northern end	Nutrients
Onondaga	Onondaga Lake, southern end	Nutrients
Ontario	Great Brook and minor tribs	Silt/Sediment
Ontario	Great Brook and minor tribs	Nutrients
Ontario	Hemlock Lake Outlet and minor tribs	Nutrients
Ontario	Honeoye Lake	Nutrients
Orange	Greenwood Lake	Nutrients
Orange	Monhagen Brook and tribs	Nutrients
Orange	Orange Lake	Nutrients
Orleans	Lake Ontario Shoreline, Western	Nutrients
Orleans	Lake Ontario Shoreline, Western	Nutrients
Oswego	Lake Neatahwanta	Nutrients
Oswego	Pleasant Lake	Nutrients
Putnam	Bog Brook Reservoir	Nutrients
Putnam	Boyd Corners Reservoir	Nutrients
Putnam	Croton Falls Reservoir	Nutrients
Putnam	Diverting Reservoir	Nutrients
Putnam	East Branch Reservoir	Nutrients
Putnam	Lake Carmel	Nutrients
Putnam	Middle Branch Reservoir	Nutrients
Putnam	Oscawana Lake	Nutrients
Putnam	Palmer Lake	Nutrients
Putnam	West Branch Reservoir	Nutrients
Queens	Bergen Basin	Nutrients
Queens	Flushing Creek/Bay	Nutrients
Queens	Jamaica Bay, Eastern, and tribs (Queens)	Nutrients
Queens	Kissena Lake	Nutrients
Queens	Meadow Lake	Nutrients
Queens	Willow Lake	Nutrients
Rensselaer	Nassau Lake	Nutrients
Rensselaer	Snyders Lake	Nutrients
Richmond	Grasmere Lake/Bradys Pond	Nutrients
Rockland	Congers Lake, Swartout Lake	Nutrients
Rockland	Rockland Lake	Nutrients
Saratoga	Ballston Lake	Nutrients
Saratoga	Dwaas Kill and tribs	Silt/Sediment
Saratoga	Dwaas Kill and tribs	Nutrients
Saratoga	Lake Lonely	Nutrients
Saratoga	Round Lake	Nutrients
Saratoga	Tribs to Lake Lonely	Nutrients

Schenectady	Collins Lake	Nutrients
Schenectady	Duane Lake	Nutrients
Schenectady	Mariaville Lake	Nutrients
Schoharie	Engleville Pond	Nutrients
Schoharie	Summit Lake	Nutrients
Seneca	Reeder Creek and tribs	Nutrients
St.Lawrence	Black Lake Outlet/Black Lake	Nutrients
St.Lawrence	Fish Creek and minor tribs	Nutrients
Steuben	Smith Pond	Nutrients
Suffolk	Agawam Lake	Nutrients
Suffolk	Big/Little Fresh Ponds	Nutrients
Suffolk	Canaan Lake	Silt/Sediment
Suffolk	Canaan Lake	Nutrients
Suffolk	Flanders Bay, West/Lower Sawmill Creek	Nutrients
Suffolk	Fresh Pond	Nutrients
Suffolk	Great South Bay, East	Nutrients
Suffolk	Great South Bay, Middle	Nutrients
Suffolk	Great South Bay, West	Nutrients
Suffolk	Lake Ronkonkoma	Nutrients
Suffolk	Long Island Sound, Suffolk County, West	Nutrients
Suffolk	Mattituck (Marratooka) Pond	Nutrients
Suffolk	Meetinghouse/Terrys Creeks and tribs	Nutrients
Suffolk	Mill and Seven Ponds	Nutrients
Suffolk	Millers Pond	Nutrients
Suffolk	Moriches Bay, East	Nutrients
Suffolk	Moriches Bay, West	Nutrients
Suffolk	Peconic River, Lower, and tidal tribs	Nutrients
Suffolk	Quantuck Bay	Nutrients
Suffolk	Shinnecock Bay and Inlet	Nutrients
Suffolk	Tidal tribs to West Moriches Bay	Nutrients
Sullivan	Bodine, Montgomery Lakes	Nutrients
Sullivan	Davies Lake	Nutrients
Sullivan	Evens Lake	Nutrients
Sullivan	Pleasure Lake	Nutrients
Tompkins	Cayuga Lake, Southern End	Nutrients
Tompkins	Cayuga Lake, Southern End	Silt/Sediment
Tompkins	Owasco Inlet, Upper, and tribs	Nutrients
Ulster	Ashokan Reservoir	Silt/Sediment
Ulster	Esopus Creek, Upper, and minor tribs	Silt/Sediment
Warren	Hague Brook and tribs	Silt/Sediment

303(d) Segments Impaired by Construction Related Pollutant(s)

Warren	Huddle/Finkle Brooks and tribs	Silt/Sediment
Warren	Indian Brook and tribs	Silt/Sediment
Warren	Lake George	Silt/Sediment
Warren	Tribs to L.George, Village of L George	Silt/Sediment
Washington	Cossayuna Lake	Nutrients
Washington	Lake Champlain, South Bay	Nutrients
Washington	Tribs to L.George, East Shore	Silt/Sediment
Washington	Wood Cr/Champlain Canal and minor tribs	Nutrients
Wayne	Port Bay	Nutrients
Westchester	Amawalk Reservoir	Nutrients
Westchester	Blind Brook, Upper, and tribs	Silt/Sediment
Westchester	Cross River Reservoir	Nutrients
Westchester	Lake Katonah	Nutrients
Westchester	Lake Lincolndale	Nutrients
Westchester	Lake Meahagh	Nutrients
Westchester	Lake Mohegan	Nutrients
Westchester	Lake Shenorock	Nutrients
Westchester	Long Island Sound, Westchester (East)	Nutrients
Westchester	Mamaroneck River, Lower	Silt/Sediment
Westchester	Mamaroneck River, Upper, and minor tribs	Silt/Sediment
Westchester	Muscoot/Upper New Croton Reservoir	Nutrients
Westchester	New Croton Reservoir	Nutrients
Westchester	Peach Lake	Nutrients
Westchester	Reservoir No.1 (Lake Isle)	Nutrients
Westchester	Saw Mill River, Lower, and tribs	Nutrients
Westchester	Saw Mill River, Middle, and tribs	Nutrients
Westchester	Sheldrake River and tribs	Silt/Sediment
Westchester	Sheldrake River and tribs	Nutrients
Westchester	Silver Lake	Nutrients
Westchester	Teatown Lake	Nutrients
Westchester	Titicus Reservoir	Nutrients
Westchester	Truesdale Lake	Nutrients
Westchester	Wallace Pond	Nutrients
Wyoming	Java Lake	Nutrients
Wyoming	Silver Lake	Nutrients

APPENDIX F – List of NYS DEC Regional Offices

Region	Covering the following counties:	DIVISION OF ENVIRONMENTAL PERMITS (DEP) Permit Administrators	DIVISION OF WATER (DOW) Water (SPDES) Program
1	Nassau and Suffolk	50 Circle Road Stony Brook, Ny 11790 Tel. (631) 444-0365	50 Circle Road Stony Brook, Ny 11790-3409 Tel. (631) 444-0405
2	Bronx, Kings, New York, Queens and Richmond	1 Hunters Point Plaza, 47-40 21st St. Long Island City, Ny 11101-5407 Tel. (718) 482-4997	1 Hunters Point Plaza, 47-40 21st St. Long Island City, Ny 11101-5407 Tel. (718) 482-4933
3	Dutchess, Orange, Putnam, Rockland, Sullivan, Ulster and Westchester	21 South Putt Corners Road New Paltz, Ny 12561-1696 Tel. (845) 256-3059	100 Hillside Avenue, Suite 1w White Plains, Ny 10603 Tel. (914) 428 - 2505
4	Albany, Columbia, Delaware, Greene, Montgomery, Otsego, Rensselaer, Schenectady and Schoharie	1150 North Westcott Road Schenectady, Ny 12306-2014 Tel. (518) 357-2069	1130 North Westcott Road Schenectady, Ny 12306-2014 Tel. (518) 357-2045
5	Clinton, Essex, Franklin, Fulton, Hamilton, Saratoga, Warren and Washington	1115 State Route 86, Po Box 296 Ray Brook, Ny 12977-0296 Tel. (518) 897-1234	232 Golf Course Road Warrensburg, Ny 12885-1172 Tel. (518) 623-1200
6	HERKIMER, JEFFERSON, LEWIS, ONEIDA AND ST. LAWRENCE	STATE OFFICE BUILDING 317 WASHINGTON STREET WATERTOWN, NY 13601-3787 TEL. (315) 785-2245	STATE OFFICE BUILDING 207 GENESEE STREET UTICA, NY 13501-2885 TEL. (315) 793-2554
7	BROOME, CAYUGA, CHENANGO, CORTLAND, MADISON, ONONDAGA, OSWEGO, TIOGA AND TOMPKINS	615 ERIE BLVD. WEST SYRACUSE, NY 13204-2400 TEL. (315) 426-7438	615 ERIE BLVD. WEST SYRACUSE, NY 13204-2400 TEL. (315) 426-7500
8	CHEMUNG, GENESEE, LIVINGSTON, MONROE, ONTARIO, ORLEANS, SCHUYLER, SENECA, STEUBEN, WAYNE AND YATES	6274 EAST AVON-LIMA ROADAVON, NY 14414-9519 TEL. (585) 226-2466	6274 EAST AVON-LIMA RD. AVON, NY 14414-9519 TEL. (585) 226-2466
9	ALLEGANY, CATTARAUGUS, CHAUTAUQUA, ERIE, NIAGARA AND WYOMING	270 MICHIGAN AVENUE BUFFALO, NY 14203-2999 TEL. (716) 851-7165	270 MICHIGAN AVENUE BUFFALO, NY 14203-2999 TEL. (716) 851-7070

APPENDIX F

NEW YORK STATE HISTORIC PRESERVATION OFFICE (SHPO) “NO EFFECT LETTER”

NEW YORK STATE OF OPPORTUNITY. | Parks, Recreation, and Historic Preservation

ANDREW M. CUOMO
Governor

ERIK KULLESEID
Commissioner

September 13, 2019

Mr. Andrew Dangler
USACE Update Regulatory Field Office
1 Buffington Street
Building 10, 3rd Floor North
Watervliet, NY 12819

Re: USACE
Albany Port District Commission Industrial Park Project
City of Albany, Town of Bethlehem, Albany County, NY
18PR07273

Dear Mr. Dangler:

Thank you for requesting the comments of the New York State Historic Preservation Office (SHPO). We have reviewed the visual simulation and the August 6, 2019 McFarland Johnson letter noting that the proposed building height has changed and could reach 85 feet in accordance with Section 106 of the National Historic Preservation Act of 1966. These comments are those of the SHPO and relate only to Historic/Cultural resources.

The visual simulation of the proposed building shows that the roof will be visible from the National Register eligible Papscanee Island Historic District. As noted in our November 2009 Determination of Eligibility for Papscanee Island, "Papscanee Island is historically and archaeologically significant for its association with the Upper Hudson Valley's predominate native people, the Mohican…"The rich soil along the flats and on Papscanee Island were flooded annually and generations of Mohicans cleared and cultivated these areas."

While some buildings have been introduced into the landscape, these buildings are not directly across from one of the few remaining cultivated areas on the Island. Since only the top of the building will be visible, the SHPO continues to recommend that this undertaking will have **No Adverse Effect** on historic properties with the **condition** that non-reflective, earth toned roofing materials are utilized. Maintaining a non-reflective roof will minimize any visual intrusions and help maintain the agricultural setting of the Papscanee Island Historic District.

If you have any questions, I can be reached at (518) 268-2179.

Sincerely,

Nancy Herter

Nancy Herter
Archaeology Unit Program Coordinator

NEW YORK STATE OF OPPORTUNITY. | **Parks, Recreation, and Historic Preservation**

KATHY HOCHUL
Governor

ERIK KULLESEID
Commissioner

December 9, 2021

Jordan Tate
Environmental Analyst
McFarland Johnson
60 Railroad Place
Suite 402
Saratoga Springs, NY 12866

Re: USACE
Marmen/Welcon Offshore Wind Tower Manufacturing Plant
Town of Bethlehem, Albany County, NY
21PR04693

Dear Jordan Tate:

Thank you for requesting the comments of the New York State Historic Preservation Office (SHPO). We have reviewed the provided documentation in accordance with Section 106 of the National Historic Preservation Act of 1966. These comments are those of the SHPO and relate only to Historic/Cultural resources.

SHPO has reviewed the proposed cut/fill plan and construction depths, and the visual simulation from the Papscanee Island shoreline. We have no archaeological concerns with the proposed ground disturbing activities that will occur during this project. Based on the visual simulation, the SHPO concurs with the Stockbridge Munsee Community (SMC) THPO that the Marmen/Welcon Offshore Wind Tower Manufacturing Plant will have an adverse visual effect on the National Register eligible Papscanee Island Historic District (08303.000130).

SHPO will provide additional comments once the Acoustic Noise Assessment has been completed to measure the proposed project's noise impacts at the Papscanee Island Historic District and the SMC THPO's comments regarding noise impacts have been provided.

If you have any questions, I can be reached at Jessica.Schreyer@parks.ny.gov.

Sincerely,

Jessica E. Schreyer

Jessica Schreyer
Scientist Archaeology

Stockbridge-Munsee Tribal Historic Preservation

Arvid E. Miller Library Museum
N8510 Mohheconnuck Road
Bowler, WI 54416

Extension Office
86 Spring Street
Williamstown, MA 01267

March 3, 2022

Andrew Dangler
Biologist/Senior Project Manager
Upstate New York Section
DEPARTMENT OF THE ARMY
US Army Corps of Engineers, ATTN: CENAN-OP-RU
1 Buffington St., Bldg. 10, 3rd Fl. North
Watervliet, NY 12189
Via email only

Re: Port of Albany Expansion Project, Town of Bethlehem, Albany Co., NY
SHPO #18PR07273 /21PR04693

Dear Mr. Dangler:

I am writing regarding the aforementioned project as part of our continuing Government-to-Government Section 106 consultation.

In review of the project’s designs and the additional visual and audial assessments, Stockbridge-Munsee Community feels that its cultural resource concerns have now been satisfactorily addressed.

Our determination is that the Port of Albany project will have **No Adverse Effect** to Historic Properties. No further consultation is required unless the designs should change further.

If you have any questions, please feel free to contact our office.

Respectfully,

BHartley

Bonney Hartley
Tribal Historic Preservation Manager
Stockbridge-Munsee Community

Cc: John Eddins, ACHP *Via email only*
Jessica Schreyer, NY SHPO *via email only*
David Witt, Charles Vandrei, NY DEC, *via email only*
Robert Leslie, Town of Bethlehem, *via email only*
Megan Daly, Port of Albany, *via email only*
Steve Boisfort, David Rosa, Jordan Tate- Mc Farland Johnson, *via email only*

(413) 884-6048 *Email: thpo@mohican-nsn.gov*

NEW YORK STATE OF OPPORTUNITY. | Parks, Recreation, and Historic Preservation

KATHY HOCHUL
Governor

ERIK KULLESEID
Commissioner

March 25, 2022

Jordan Tate
Environmental Analyst
McFarland Johnson
60 Railroad Place
Suite 402
Saratoga Springs, NY 12866

Re: USACE
Marmen/Welcon Offshore Wind Tower Manufacturing Plant
Town of Bethlehem, Albany County, NY
21PR04693

Dear Jordan Tate:

Thank you for requesting the comments of the New York State Historic Preservation Office (SHPO). We have reviewed the provided documentation in accordance with Section 106 of the National Historic Preservation Act of 1966. These comments are those of the SHPO and relate only to Historic/Cultural resources.

SHPO has reviewed the Noise Assessment and updated Visual Simulations (provided via email, McFarland Johnson, Inc., 2/18/22) to evaluate the project's effects on the National Register eligible Papscanee Island Historic District (08303.000130) and the Site Plan (McFarland Johnson, Inc., 3/18/22) showing the proposed 3.30-acre deed restricted area containing the vegetated area along the Hudson River shoreline. The vegetated area will provide a visual and acoustic buffer between the Papscanee Island Historic District and the proposed manufacturing plant.

We note that the Stockbridge Munsee Community has issued their opinion (letter, dated 3/2/22) that this project will have No Adverse Effect on Historic Resources.

Based on this review, it is the opinion of the SHPO that no historic properties, including archaeological and/or historic resources, will be Adversely Affected by this undertaking with the condition that a Restrictive Deed Covenant is filed to protect and maintain the vegetated buffer along the Hudson River shoreline.

If you have any questions, I can be reached at Jessica.Schreyer@parks.ny.gov.

Sincerely,

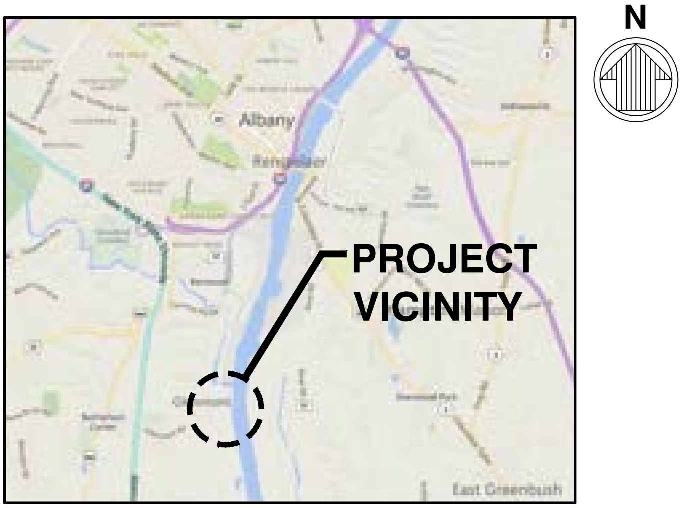
Jessica E. Schreyer

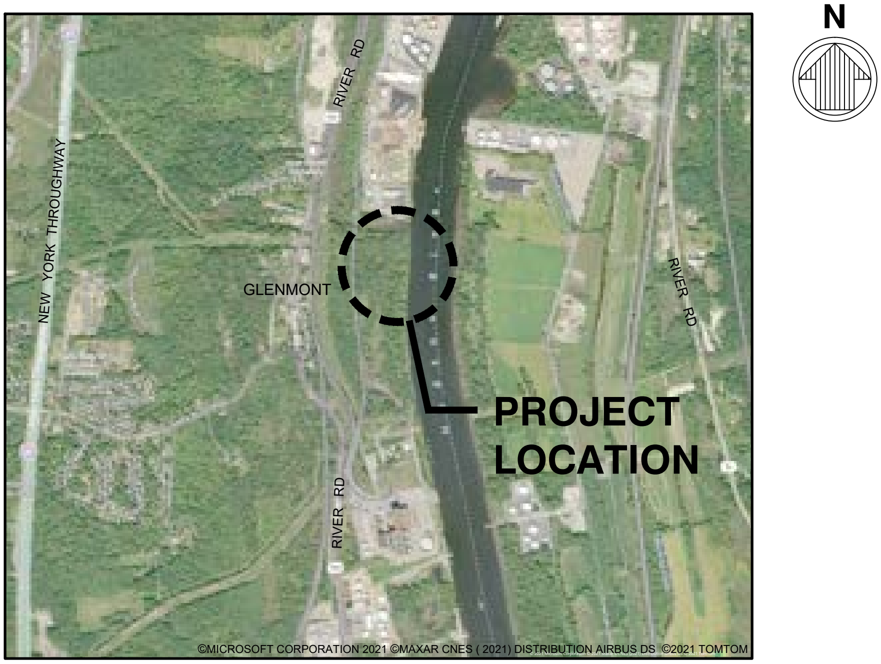
Jessica Schreyer
Scientist Archaeology

New York State Office of Parks, Recreation and Historic Preservation
Division for Historic Preservation, Peebles Island, PO Box 189, Waterford, New York 12188-0189
(518) 237-8643 • https://parks.ny.gov/shpo

APPENDIX G

DRAFT WHARF AND DREDGING E&SC PLANS





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VICINITY AND LOCATION MAP

SCALE: N.T.S.

NOTES:

1. HORIZONTAL CONTROL REFERENCED TO NORTH AMERICAN DATUM OF 1983, STATE PLANE COORDINATE SYSTEM, NEW YORK, EAST ZONE, IN FEET.

2. WATER LEVEL DATUM IS BASED ON NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88), AS FOLLOWS:

- MEAN HIGHER HIGH WATER LEVEL (MHHW) = +3.78 (NAVD88)
- MEAN HIGH WATER LEVEL (MHW) = +3.40 (NAVD88)
- MEAN TIDE LEVEL (MTL) = +0.91 (NAVD88)
- MEAN LOW WATER LEVEL (MLW) = -1.59 (NAVD88)
- MEAN LOWER LOW WATER LEVEL (MLLW) = -1.81 (NAVD88)

PURPOSE: WHARF CONSTRUCTION
PERMIT SUBMITTAL-NOT TO BE USED
FOR CONSTRUCTION
DATUM: NAVD88

m&n engineering, p.c.

OWNER/APPLICANT:
ALBANY PORT DISTRICT COMMISSION
PORT OF ALBANY

IN:	HUDSON RIVER
NEAR:	SOUTH OF ALBANY
LOCATION:	PORT OF ALBANY 106 SMITH BOULEVARD ALBANY, NEW YORK 12202

WHARF DREDGING AND CONSTRUCTION

VICINITY AND LOCATION

SHEET 1 OF 5 DATE: (REV1) 2021-10-11

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PLAN - EXISTING CONDITIONS

150' 0' 150' 300'

SCALE: 1"=300'

PURPOSE: WHARF CONSTRUCTION
PERMIT SUBMITTAL-NOT TO BE USED
FOR CONSTRUCTION
DATUM: NAVD88

m&n engineering, p.c.

OWNER/APPLICANT:
ALBANY PORT DISTRICT COMMISSION
PORT OF ALBANY

IN:	HUDSON RIVER
NEAR:	SOUTH OF ALBANY
LOCATION:	PORT OF ALBANY 106 SMITH BOULEVARD ALBANY, NEW YORK 12202

WHARF DREDGING AND CONSTRUCTION

PLAN - EXISTING CONDITIONS

SHEET 2 OF 5 DATE: (REV1) 2021-10-11

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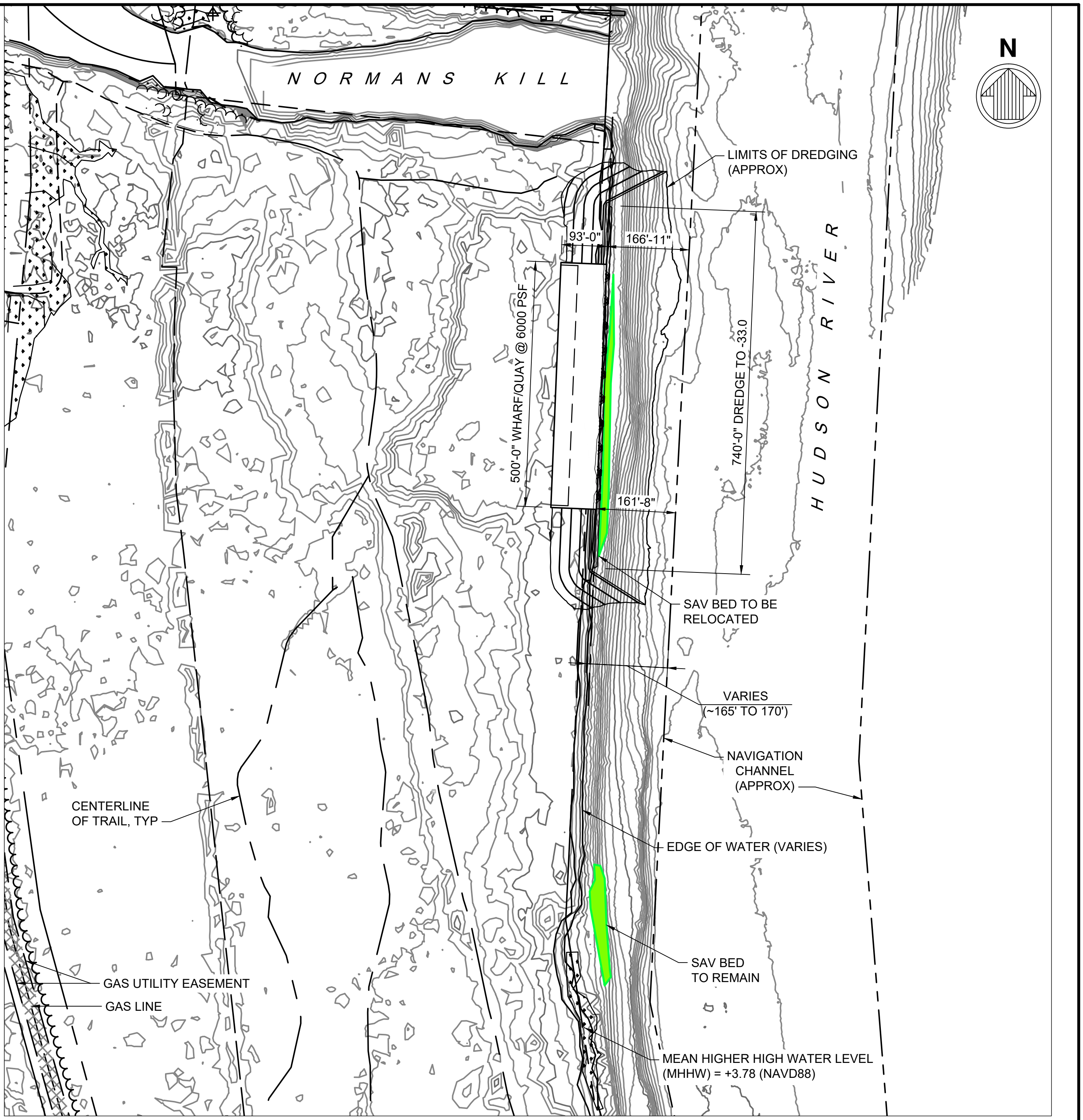

PLAN - PROPOSED CONDITIONS

150' 0' 150' 300'

SCALE: 1"=300'

PURPOSE: WHARF CONSTRUCTION
PERMIT SUBMITTAL-NOT TO BE USED
FOR CONSTRUCTION
DATUM: NAVD88

m&n engineering, p.c.

OWNER/APPLICANT:
ALBANY PORT DISTRICT COMMISSION
PORT OF ALBANY

IN:	HUDSON RIVER
NEAR:	SOUTH OF ALBANY
LOCATION:	PORT OF ALBANY 106 SMITH BOULEVARD ALBANY, NEW YORK 12202

WHARF DREDGING AND CONSTRUCTION

PLAN - PROPOSED CONDITIONS

SHEET 3 OF 5 DATE: (REV1) 2021-10-11

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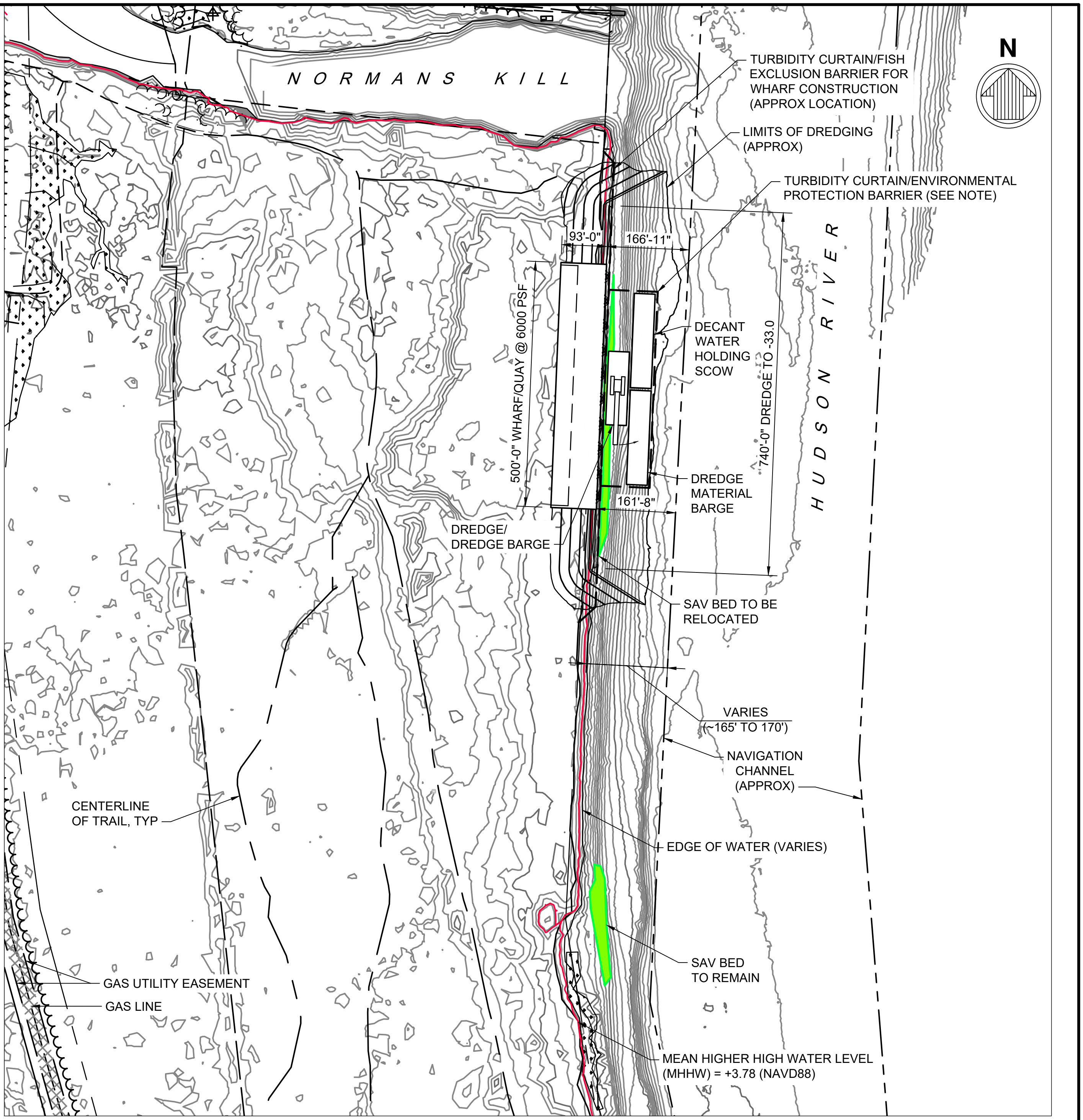

NOTE: DREDGE EQUIPMENT AND
ASSOCIATED TURBIDITY CURTAIN/
ENVIRONMENTAL PROTECTION
BARRIER LOCATIONS VARY.

PLAN - PROPOSED TEMPORARY ENVIRONMENTAL PROTECTION

150' 0' 150' 300'

SCALE: 1"=300'

PURPOSE: WHARF CONSTRUCTION
PERMIT SUBMITTAL-NOT TO BE USED
FOR CONSTRUCTION
DATUM: NAVD88

m&n engineering, p.c.

OWNER/APPLICANT:
ALBANY PORT DISTRICT COMMISSION
PORT OF ALBANY

IN: HUDSON RIVER
NEAR: SOUTH OF ALBANY
LOCATION: PORT OF ALBANY
106 SMITH BOULEVARD
ALBANY, NEW YORK 12202

WHARF DREDGING AND CONSTRUCTION

PLAN - PROPOSED TEMPORARY ENVIRONMENTAL PROTECTION

SHEET 4 OF 5 DATE: (REV1) 2021-10-11

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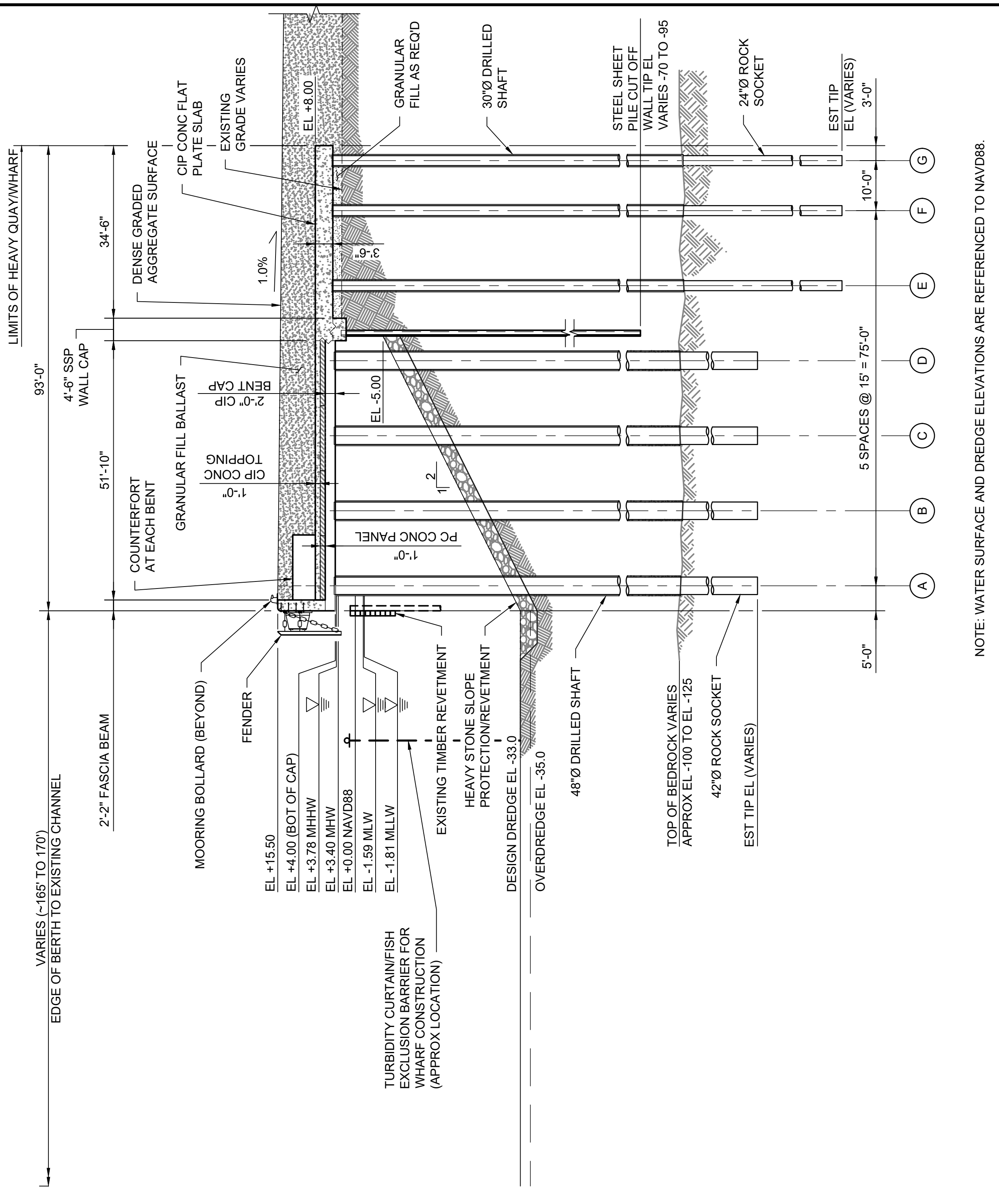

SECTION - WHARF AND DREDGING

SCALE: N.T.S.

PURPOSE: WHARF CONSTRUCTION
PERMIT SUBMITTAL-NOT TO BE USED
FOR CONSTRUCTION
DATUM: NAVD88

m&n engineering, p.c.

OWNER/APPLICANT:
ALBANY PORT DISTRICT COMMISSION
PORT OF ALBANY

IN:	HUDSON RIVER
NEAR:	SOUTH OF ALBANY
LOCATION:	PORT OF ALBANY 106 SMITH BOULEVARD ALBANY, NEW YORK 12202

WHARF DREDGING AND CONSTRUCTION

SECTION - WHARF AND DREDGING

SHEET 5 OF 5 DATE: (REV1) 2021-10-11

APPENDIX H

SOIL MANAGEMENT PLAN

ATLANTIC TESTING LABORATORIES

WBE certified company

Albany
22 Corporate Drive
Clifton Park, NY 12065
518-383-9144 (T)
atlantictesting.com

October 23, 2020

McFarland Johnson, Inc.
60 Railroad Place, Suite 402
Saratoga Springs, New York 12866

Attn: David Rosa

Re: Soil Management Plan
Port of Albany Expansion Project
Beacon Island Parcel
Bethlehem, Albany County, New York
MJ Project No. 18641.02
ATL Report No. AT5596CE-05-10-20

Ladies/Gentlemen:

Enclosed is a copy of the Soil Management Plan prepared for the referenced site. This report was completed in accordance with the standard form of agreement between McFarland Johnson, Inc., and Atlantic Testing Laboratories, Limited.

Please contact our office should you have any questions, or if we may be of further assistance.

Sincerely,
ATLANTIC TESTING LABORATORIES, Limited

Cheyenne J. Dashnaw, P.E.
Senior Engineer

CJD/cjd

Enclosures

cc: Georgie Nugent, McFarland Johnson, Inc.

Soil Management Plan

Port of Albany Expansion Project
Beacon Island Parcel
Bethlehem, Albany County, New York

WBE certified company

Prepared by:

Atlantic Testing Laboratories, Limited
22 Corporate Drive
Clifton Park, New York 12065

Prepared for:

McFarland Johnson, Inc.
60 Railroad Place, Suite 402
Saratoga Springs, New York 12866
MJ Project No. 18641.02

Albany Port District Commission
106 Smith Boulevard
Albany, New York 12202

ATL Report No. AT5596CE-05-10-20

October 23, 2020

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1.0 INTRODUCTION

1.1 Purpose

Atlantic Testing Laboratories, Limited (ATL) was retained by McFarland Johnson, Inc., on behalf of the Albany Port District Commission, to prepare a Soil Management Plan that can be used to address areas at the Beacon Island parcel that are impacted with ash material and related debris. The purpose of this Soil Management Plan is to summarize procedures to implement for planned excavation activities, installation of a soil cover system in areas of ash material or other potential impacted fill, and management of waste soil and/or groundwater. This Soil Management Plan also addresses protocol for monitoring and sampling and analysis during excavation and site work, and recommendations for installation of vapor barrier systems beneath proposed buildings.

1.2 Site Description

The project site is the Beacon Island parcel located to the east of River Road (County Route 144) and along the west side of the Hudson River, in the Town of Bethlehem, Albany County, New York. The Beacon Island parcel is comprised of approximately 80 acres, and is the site of a planned expansion for the Port of Albany. A Site Location Map, showing the approximate location of the subject site, is included in Appendix A.

Information provided to ATL by McFarland Johnson, Inc., indicates that planned redevelopment for the site includes land clearing, excavation and backfill operations, dredging sediment for the area along the Hudson River, and construction of facilities to be associated with the Port of Albany.

1.3 Plan Contents and Organization

This Soil Management Plan includes an introductory section (Section 1), a summary of information obtained from prior investigations (Section 2), pertinent coordination items when work is scheduled for impacted areas (Section 4) and a description of procedures that may be warranted for various site work activities (Section 3), a description of procedures to be implemented during specific site work activities (Sections 5 through 12), and a description of reports and records that should be maintained for work completed at the subject site (Section 13). Appendices are included to provide supplemental information that is considered pertinent to the items described in the Soil Management Plan and are referenced where applicable.

This Soil Management Plan is organized in a manner to allow for site representatives to review and identify applicable measures to be implemented for different areas of work and types of work activities being performed. Section 1.4 describes different areas of work and the associated work activities that may be applicable. A Soil Management Plan Flow Chart, contained in Appendix B, is provided to outline tasks to be implemented for management of existing on-site soil and soil that may be imported for use as fill.

1.4 Applicability

1.4.1 Areas to be Developed with Buildings/Structures

Areas to be developed with buildings or other structures will require appropriate soil management procedures, in association with the excavation, backfill, and grading for the installation of foundation systems, and with the construction of slabs-on-grade for buildings. Following is a summary of the soil management procedures to be implemented, with reference to applicable sections of this Soil Management Plan.

- *Excavation for foundations in areas of known/suspect impacts from ash:* Refer to **Section 5** and **Item A** of Soil Management Plan Flow Chart (Appendix B)
- *Excavation for foundations in areas without known/suspect impacts from ash:* Refer to **Section 6** and **Item B** of Soil Management Plan Flow Chart (Appendix B)
- *Backfill in areas of construction:* Refer to **Section 11** and **Items E and F** of Soil Management Plan Flow Chart (Appendix B)
- *Construction of slabs-on-grade for buildings:* Refer to **Section 9** and **Item E** of Soil Management Plan Flow Chart (Appendix B)

1.4.2 Areas to be Developed with Asphalt/Concrete Surfaces

Areas to be developed with asphalt or concrete surfaces (e.g., driveways, parking lots, walking paths) will require appropriate soil management procedures, in association with the excavation, backfill, and grading prior to installation of the asphalt or concrete surface cover. Following is a summary of the soil management procedures to be implemented, with reference to applicable sections of this Soil Management Plan.

- *Excavation and site preparation in areas of known/suspect impacts from ash:* Refer to **Section 5** and **Item A** of Soil Management Plan Flow Chart (Appendix B)
- *Excavation and site preparation in areas without known/suspect impacts from ash:* Refer to **Section 6** and **Item B** of Soil Management Plan Flow Chart (Appendix B)
- *Backfill in areas of construction:* Refer to **Section 11** and **Items E and F** of Soil Management Plan Flow Chart (Appendix B)

1.4.3 Areas to be Developed with Lawn/Landscaping

Areas to be redeveloped with lawn or landscaping will require appropriate soil management procedures, in association with the excavation, backfill, grading, and soil cover system installation. Following is a summary of the soil management procedures to be implemented, with reference to applicable sections of this Soil Management Plan.

- *Excavation and site preparation in areas of known/suspect impacts from ash:* Refer to **Section 5** and **Item A** of Soil Management Plan Flow Chart (Appendix B)
- *Excavation and site preparation in areas without known/suspect impacts from ash:* Refer to **Section 6** and **Item B** of Soil Management Plan Flow Chart (Appendix B)
- *Backfill and soil cover system in areas of lawns and landscaping:* Refer to **Sections 8 and 11** and **Items E and F** of Soil Management Plan Flow Chart (Appendix B)

1.4.4 Areas to Remain Wooded

Areas of the subject site that are currently wooded and are planned to remain wooded will not require implementation of specific soil management procedures at this time. In the event that these areas are scheduled for redevelopment in the future, or if there are any ground intrusive activities performed, appropriate soil management procedures should then be provided.

1.4.5 Areas to be Dredged

Management of areas where sediment is planned for dredging is described in general for this Soil Management Plan; however, additional planning and coordination with the New York State Department of Environmental Conservation (NYSDEC), United States Army Corps of Engineers (USACE), and other applicable regulatory agencies will be necessary prior to performing the dredging operations and the handling and reuse or disposal of dredged sediments. Refer to **Section 10** and **Item D** of the Soil Management Plan Flow Chart (Appendix B).

2.0 BACKGROUND INFORMATION AND AREAS OF CONCERN

2.1 Summary of Previous Investigations

ATL performed sediment sampling at the subject site in June 2019 and August 2020, and a subsurface investigation and soil sampling in September 2020. Additionally, ATL was provided with a draft Phase II Environmental Site Assessment report prepared by Bergmann Associates for the Port of Albany and dated April 6, 2017.

The sediment sampling conducted by ATL in June 2019 included the advancement of 5 cores, and the sediment sampling conducted by ATL in August 2020 included the advancement of 10 cores. The cores were advanced in the areas scheduled for dredging as part of planned redevelopment for the site. Sediment samples were collected from the core locations and submitted for subsequent laboratory analysis. Laboratory analysis parameters for the June 2019 sampling event were selected to include compounds described in NYSDEC Technical and Operational Guidance Series (TOGS) 5.1.9, whereas the samples collected in August 2020 were analyzed for parameters to evaluate potential reuse options in addition to the NYSDEC TOGS 5.1.9 parameters. A complete summary of findings for the previously completed sediment sampling and analysis are provided in ATL Report No. CD4644CE-01-07-19, dated July 15, 2019, ATL Report No. CD4644CE-01-07-19 Addendum 1, dated August 2, 2019, and ATL Report No. AT5596CE-03-09-20 dated September 24, 2020.

The subsurface investigation and soil sampling conducted by ATL in September 2020 included the advancement of 45 probes. The probes were advanced in accessible areas throughout the site, to assess the presence or absence of ash material. Additionally, soil samples were collected from locations without ash material to evaluate potential reuse options for consideration during site redevelopment. The subsurface investigation identified multiple locations where ash material is present, but did not identify obvious visual or olfactive evidence of petroleum or chemical-related contamination. Soil samples were collected from areas without ash material, including 22 samples for analysis of volatile organic compounds (VOC), and 11 samples for analysis of semi-VOC, polychlorinated biphenyls (PCB), pesticides, metals, and cyanide. With a few exceptions, laboratory analysis results for the soil samples collected from areas without ash material were below 6 NYCRR Part 360 fill material pre-determined beneficial use criteria and below NYSDEC Unrestricted Use Soil Cleanup Objectives (SCO) listed in 6 NYCRR Part 375 and/or NYSDEC CP-51 document. A general summary of the findings for the subsurface investigation previously completed by ATL is provided as Table C-1 in Appendix C. A complete summary of findings is provided in ATL Report No. AT5596CE-04-10-20, dated October 22, 2020.

The Phase II ESA conducted by Bergmann Associates in February 2017 included the advancement of 12 test pits and 8 borings, and the installation of temporary monitor wells at 3 of the boring locations. Information in the draft Phase II ESA report indicates that coal ash was observed throughout the depths for 3 of the test pits and a fourth test pit exhibited the presence of railroad ties covered in a black tar-like substance at depths of 8 to 12 feet below ground surface. Of the 8 borings advanced during this investigation, 7 exhibited evidence of coal ash. A surface soil sample was collected from the initial 2 inches for each boring. Subsurface soil samples were also collected from the borings at varying depths. There were 3 temporary monitor wells installed for collection of groundwater samples. The soil and groundwater samples were laboratory analyzed for VOC, semi-VOC, cyanide, pesticides, PCB, and target compound list (TCL) metals. With the exception of metals, target compounds for the referenced analytical parameters were non-detect for each of the soil and groundwater samples. A general summary of the findings for the subsurface investigation previously completed by Bergmann Associates is provided as Tables C-2 and C-3 in Appendix C. The report prepared by Bergmann Associates and dated April 6, 2017, should be referenced for additional details pertaining to the findings of the subsurface investigation.

2.2 Known Locations of Impacted Soil

Data and information from the previous subsurface investigation activities indicates that ash material is present at the site in a widespread condition. The ash material has been predominantly observed on the west side of the subject site. No obvious visual or olfactive evidence of petroleum or chemical-related contamination was observed at the locations investigated.

The Aerial Overview of Affected Locations, contained in Appendix D, shows approximate locations for the borings, test pits, and probes previously advanced at the subject site, along with an indication of which exhibited the presence of ash material. The referenced drawing also shows an approximate delineation of areas where ash material is expected to be present, areas where ash material is not expected, and areas that are considered to potentially contain ash material. The areas shown on the drawing are based on existing available data and not intended to represent an exact delineation for the locations of ash material.

3.0 COORDINATION OF WORK

3.1 Roles and Responsibilities

The soil management procedures described herein should be coordinated and conducted by firms and individuals who are familiar with the conditions of the Soil Management Plan, have an understanding of the known or suspected conditions in different areas of the subject site, and have related experience and capabilities to implement the applicable work activities. While the Owner of the property has responsibility for the implementation of the Soil Management Plan, it is anticipated that performance of work activities associated with the Soil Management Plan would be coordinated and completed by design professionals, contractors, and environmental consultants who are retained by the Owner. The following table provides a summary of the primary roles and responsibilities for implementation of the Soil Management Plan.

Role	Responsibilities
Owner	• Retain and coordinate with Design Professionals, Contractors, and Environmental Consultants for performance of site work pursuant to conditions of the Soil Management Plan • Maintain site records and documentation for work completed pursuant to Soil Management Plan
Design Professionals	• Incorporate applicable provisions of the Soil Management Plan into design plans and specifications for planned site redevelopment • Coordinate with Owner, Contractors, and Environmental Consultants during design and construction phases to confirm work is completed as planned
Contractors	• Correspond with NYSDEC (and other regulatory agencies, as applicable) for notifications of work activities • Perform site work activities, including, but not limited to, excavation, grading, placement and compaction of backfill, dust control, groundwater management, soil cover system installation, vapor barrier system installation, construction, and waste transport and disposal
Environmental Consultants	• Provide guidance and assistance with implementation of the Soil Management Plan • Correspond with NYSDEC (and other regulatory agencies, as applicable) to discuss clarifications or modifications to conditions of the Soil Management Plan • Conduct monitoring and soil screening during performance of work activities that affect impacted or contaminated soil • Conduct air monitoring during ground intrusive activities that affect impacted or contaminated soil • Perform soil sampling and laboratory analysis for waste materials, fill materials, and excavation areas

3.2 Project Notifications

Within 14 days, and no less than 3 days, prior to commencing work activities that may affect areas of the subject site that are impacted with ash, the NYSDEC should be notified of the planned work. This notification should be performed by the Owner and/or Contractor performing the site work. The Design Professional and Environmental Consultant should also be similarly notified.

3.3 Spill Reporting/Administration

The subject site is known to have areas impacted with ash and related debris. Other types of contamination (i.e., petroleum, chemical) have not been encountered at locations of past investigations. The NYSDEC must be notified in the event that petroleum- or chemical-related contaminated soil is discovered on the project site. This notification will need to be provided directly to the NYSDEC Spill Hotline (telephone number 1-800-457-7362).

4.0 GENERAL SITE WORK AND SOIL MANAGEMENT PLAN CONDITIONS

4.1 Health and Safety

In addition to construction site health and safety, site personnel should be aware of the contaminants of concern associated with the ash material (metals) and utilize appropriate control methods, personal protective clothing, and personal protective equipment during the handling and management of impacted materials. Contractors working at the subject site should perform work pursuant to a health and safety plan that is specific to their scope of work and associated hazards or potential hazards.

4.2 Groundwater Removal and Management (If Applicable)

In the event that there is significant groundwater inflow in a zone of contaminated soil, a vacuum truck should be provided to remove the infiltrated groundwater as the excavation progresses or at the completion of excavation activities. Alternatively, groundwater can be pumped into a frac tank(s) or other appropriate receptacles and temporarily stored on-site prior to on-site treatment and discharge or transfer and disposal off-site.

4.3 Dust Control and Air Monitoring

Fugitive dust and vapors should be minimized or mitigated during the excavation and handling of contaminated soil, if encountered. In the event that particulates and/or vapors represent a potential concern for the work area and/or surrounding areas, particulates and/or vapors should be monitored during ground intrusive activities associated with contaminated soil by setting up real-time instrumentation at locations upwind and downwind of the project area. Assessment for airborne dust would be performed using particulate monitors capable of measuring particulate matter less than 10 microns (PM10). Assessment for vapors (applicable to areas where petroleum- or chemical-related contamination is encountered) would be performed using a photoionization detector (PID) to screen the ambient air for the measurable presence of VOC.

If air monitoring for particulates is conducted, an action level of 0.15 milligrams per cubic meter (mg/m^3) should be used for PM10 concentrations associated with the project work area. If screening is performed for ambient airborne VOC concentrations, an action level of 5 parts per million (ppm) should be used for the project work area. These action levels are consistent with the NYSDEC DER-10 “Technical Guidance for Site Investigation and Remediation” and the New York State Department of Health (NYSDOH) “Generic Community Air Monitoring Plan.”

In the event that the PM10 action level is exceeded for the work site (downwind monitoring station), the upwind background level should be immediately confirmed and it should be determined whether the work site (downwind) level exceeds the upwind background by greater than 0.1 mg/m^3. For any such exceedance, work activities should temporarily cease and dust suppression techniques should be implemented. Dust suppression techniques may include some or all of the following (as cited from Appendix 1B of the NYSDEC DER-10):

- Applying water on haul roads
- Wetting equipment and excavation faces
- Spraying water on buckets during excavation and dumping
- Hauling materials in properly tarped or watertight containers

- Restricting vehicle speeds to 10 mph
- Covering excavated areas and material after excavtion activity ceases
- Reducing the excavation size and/or number of excavations

In the event that the VOC action level is exceeded for the work site (downwind monitoring station), the upwind background level should be immediately confirmed and it should be determined whether the work site (downwind) level exceeds the upwind background by greater than 5 ppm. For any such exceedance, work activities should temporarily cease while monitoring continues. If the concentrations readily decrease below 5 ppm over background, work activities can resume. If the concentrations do not readily decrease and a definitive source can not be eliminated, the work site and on-site work activities would require further evaluation to determine an appropriate course of action.

4.4 Personnel and Equipment Decontamination

Equipment that is in contact with contaminated soil should be decontaminated, as necessary to prevent cross-contamination to other areas. Equipment and tools can be decontaminated by initially scrubbing the bulk material from the item, cleaning with a phosphate-free detergent and tap water wash, rinsing with tap water, and rinsing with distilled water. In order to contain decontamination liquids, a decontamination pad, of sufficient size to accommodate the affected portions of equipment, can be constructed using double layers of polyethylene sheeting as a base and a suitable material (i.e., lumber, clean soil, hay bales) for a perimeter berm. A more durable setup would be necessary if larger, heavier items need to be decontaminated. The polyethylene sheeting should be wrapped around the perimeter berm. Wastewater generated from decontamination activities shall be disposed of pursuant to applicable local, state and federal requirements.

5.0 EXCAVATION IN AREAS IMPACTED WITH ASH AND DEBRIS

5.1 Soil Removal and Stockpile

Soil that is impacted with ash and requires excavation for planned site redevelopment activities will need to be transported off-site for disposal. In consideration of the ash materials being widespread at various locations of the subject site, and the soil cover system that will be implemented as described in other sections of this Soil Management Plan, the areal extent of excavation for ash and debris wastes will be only as necessary to complete the scheduled site redevelopment. It is intended that the depths of removal be similarly limited to scheduled depths of excavations; however, if existing available information suggests that waste ash/debris exists at only a limited distance below scheduled depths of excavations, removal of the additional material should be conducted to limit the amount of material that would remain below permanent structures or site features to be constructed.

Excavated soil should be field examined for the visual and/or olfactive indicators of the presence of ash and debris materials or the potential presence of petroleum- or chemical-related impacts. In the event that petroleum- or chemical-related impacts are identified, procedures described in Section 7 should implemented.

Excavated soil should be segregated between soil that is impacted with ash and overburden soil that does not exhibit visual evidence of these wastes. The impacted soil should be stockpiled or directly loaded for subsequent transport and disposal. The overburden soil can be processed for reuse on-site, provided applicable conditions are satisfied (reference Section 11).

For impacted soil that is stockpiled on-site, the selected location(s) should be an area not susceptible to flooding or inundation of water during precipitation events, readily accessible to equipment that will be utilized for loading and hauling the material, and located away from stormwater or site drainage components. Any contaminated soil stockpile would need to be placed on and covered with 6-mil polyethylene sheeting or other comparable impervious material that can be readily removed and disposed of. The following items should be applicable to stockpiles for contaminated soil materials.

- Polyethylene sheeting or other impervious membrane used for the base of the soil stockpile should be placed with sheets overlapping a minimum of 1 foot.
- The base of the soil stockpile should be bermed at the perimeter to contain the soil stockpile and potential runoff during precipitation events. The berm materials, which can be comprised of mounds of clean soil material, hay bales, lumber, or other readily available suitable materials, should be placed along the perimeter and wrapped with the polyethylene sheeting or other impervious membrane that is used for the base of the soil stockpile. To minimize extraneous handling of materials and the size of the completed soil stockpile area, the berm perimeter can initially be constructed along 2 sides, and the remaining 2 sides can be constructed after all soil material is placed in the stockpile or temporarily bermed at the end of each workday.
- The height and slopes of the soil stockpile should be limited such that slope stability is not compromised during storage or the loading process.
- The soil stockpile should be covered with the polyethylene sheeting upon placement of all impacted soil material or at the end of each workday. Seams should be overlapped a minimum of 1 foot. The stockpile cover should be sufficiently weighted to contain the stored soil and resist damage from wind. Materials used to weigh down and stabilize the stockpile cover should consist of readily available materials that will not tend to damage the cover upon placement (e.g., clean soil material, sand bags, tires).
- Any temporary on-site soil stockpiles should be periodically inspected to ensure that material continues to be contained and is not released to the surrounding environment. The temporary on-site soil stockpiles will need to be properly protected and maintained until removal and off-site disposal. Polyethylene sheeting should be repaired or replaced as needed.
- Water from precipitation events that ponds on the surface of the stockpile cover should be removed upon discovery. The ponded water can be discharged on-site provided there is no contact with the petroleum-contaminated soil, and provided such activities are compliant with any stormwater discharge permits that may be applicable for the site or active construction work. Water that contacts the petroleum-impacted soil shall be properly containerized and managed as impacted waste water.
- Stockpiles with impacted soil should not remain on-site in excess of 60 days.

5.2 Excavation Monitoring and Soil Sampling

A representative of the Environmental Consultant should be on-site during excavation activities in the known or suspect areas of ash materials, to examine exposed soil for the presence of ash. The on-site representative should assist with determinations for the segregation of soil material that is considered relatively clean overburden to be reused and soil material that would be classified as contaminated for off-site disposal. The on-site representative would also be available to provide guidance relative to the management of the contaminated subsurface materials.

Since it is planned to manage ash materials in-place for various locations at the subject site, soil samples are not proposed for excavation area(s) where waste ash/debris is removed, unless other potential contaminants of concern are encountered or suspected. If soil samples are to be collected from these excavation areas, the Environmental Consultant should coordinate with the NYSDEC to confirm analytical parameters, sampling locations, and quantity of samples.

5.3 Soil Reuse or Disposal

Overburden soil scheduled for reuse should be managed pursuant to procedures described in Section 11. Waste soil materials scheduled for disposal should be managed pursuant to procedures described in Section 12.

6.0 EXCAVATION IN NON-IMPACTED AREAS

6.1 Soil Removal and Stockpile

Soil that is excavated from areas without known or suspect impacts from ash can be removed and handled pursuant to routine construction and site work methods. Contractors performing the excavation work should be cognizant of the potential for impacted soil and should visually monitor the soil as removed to determine if it may be potentially affected. If ash material is identified, the excavation and soil management procedures should transition to the methods described in Section 5. In the event that petroleum- or chemical-related impacts are identified, procedures described in Section 7 should implemented.

6.2 Excavation Monitoring

As indicated in Section 6.1, a representative of the Contractor should examine exposed soil for visual and/or olfactive indicators of potential contamination. If suspect impacted materials are encountered, a representative of the Environmental Consultant should be on-site for further assessment and monitoring in the affected area.

Unless suspect contamination is encountered, soil samples are not proposed for the excavations. If soil samples are to be collected from these excavation areas, the Environmental Consultant should coordinate with the NYSDEC to confirm analytical parameters, sampling locations, and quantity of samples.

6.3 Soil Reuse or Disposal

The excavated soil that is scheduled for reuse should be managed pursuant to procedures described in Section 11. In the event that the soil materials need to be disposed of, transport and disposal should be performed pursuant to procedures described in Section 12.

7.0 EXCAVATION IN AREAS OF PETROLEUM/CHEMICAL SPILLS

7.1 Soil Removal and Stockpile

If petroleum- or chemical-related contamination is encountered during site work, a spill will need to be reported (reference Section 3.3) and contaminated soil will need to be removed for disposal. The areal extent and depths of excavation for material affected by a spill should be inclusive of the entirety of the contaminated material, if feasible and practical. If the affected materials cannot be completely removed, an alternate approach to site remediation should be coordinated through the NYSDEC.

Excavated soil should be field examined for the visual and/or olfactive indicators of the petroleum- or chemical-related impacts, and field screened for the measurable presence of VOC with a photoionization detector (PID), equipped with the 10.6 eV lamp. In general, soil exhibiting obvious visual or olfactive evidence of contamination and/or greater than 10 ppm via ambient PID screening should be removed from the excavation, and processed for subsequent disposal. Overburden soil that does not exhibit these characteristics should be stockpiled on-site for subsequent sampling and evaluation of reuse options.

For petroleum-contaminated soil that is stockpiled on-site, the selected location(s) should be an area not susceptible to flooding or inundation of water during precipitation events, readily accessible to equipment that will be utilized for loading and hauling the material, and located away from stormwater or site drainage components. Any contaminated soil stockpile would need to be placed on and covered with 6-mil polyethylene sheeting or other comparable impervious material that can be readily removed and disposed of. The following items should be applicable to stockpiles for contaminated soil materials.

- Polyethylene sheeting or other impervious membrane used for the base of the soil stockpile should be placed with sheets overlapping a minimum of 1 foot.
- The base of the soil stockpile should be bermed at the perimeter to contain the soil stockpile and potential runoff during precipitation events. The berm materials, which can be comprised of mounds of clean soil material, hay bales, lumber, or other readily available suitable materials, should be placed along the perimeter and wrapped with the polyethylene sheeting or other impervious membrane that is used for the base of the soil stockpile. To minimize extraneous handling of materials and the size of the completed soil stockpile area, the berm perimeter can initially be constructed along 2 sides, and the remaining 2 sides can be constructed after all soil material is placed in the stockpile or temporarily bermed at the end of each workday.
- The height and slopes of the soil stockpile should be limited such that slope stability is not compromised during storage or the loading process.
- The soil stockpile should be covered with the polyethylene sheeting upon placement of all impacted soil material or at the end of each workday. Seams should be overlapped a minimum of 1 foot. The stockpile cover should be sufficiently weighted to contain the stored soil and resist damage from wind. Materials used to weigh down and stabilize the stockpile cover should consist of readily available materials that will not tend to damage the cover upon placement (e.g., clean soil material, sand bags, tires).
- Any temporary on-site soil stockpiles should be periodically inspected to ensure that material continues to be contained and is not released to the surrounding environment. The temporary on-site soil stockpiles will need to be properly

protected and maintained until removal and off-site disposal. Polyethylene sheeting should be repaired or replaced as needed.
- Water from precipitation events that ponds on the surface of the stockpile cover should be removed upon discovery. The ponded water can be discharged on-site provided there is no contact with the petroleum-contaminated soil, and provided such activities are compliant with any stormwater discharge permits that may be applicable for the site or active construction work. Water that contacts the petroleum-impacted soil shall be properly containerized and managed as impacted waste water.
- Stockpiles with impacted soil should not remain on-site in excess of 60 days.

7.2 Excavation Monitoring and Sampling

A representative of the Environmental Consultant should be on-site during excavation activities in the areas affected by a petroleum- or chemical-related spill, to examine exposed soil for visual and/or olfactive indicators of petroleum- or chemical-related impacts. Additionally, field screening for the measurable presence of VOC should be performed at the time of the excavation activities, using a portable PID, equipped with a 10.6 eV lamp.

The on-site representative should assist with determinations for the segregation of soil material that is considered relatively clean overburden to be reused and soil material that would be classified as contaminated for off-site disposal. The on-site representative would also be available to provide guidance relative to the management of the contaminated subsurface materials.

Post-excavation soil samples should be collected from the walls and floor of the excavation area(s) where petroleum- or chemical-contaminated soil is removed. The quantities of soil samples to be collected from these excavation areas should be selected pursuant to the following criteria:

- For excavations with a perimeter of less than 20 feet, 1 bottom and 1 sidewall sample should be collected.
- For excavations with a perimeter between 20 and 300 feet, samples from sidewalls should be collected at a frequency of 1 per 30 linear feet and samples from the bottom should be collected at a frequency of 1 per 900 square feet.
- For excavations with a perimeter of greater than 300 linear feet, the quantity of samples to be collected should be coordinated through the NYSDEC, or selected pursuant to the same criteria specified for an excavation perimeter between 20 and 300 feet.

The post-excavation soil samples should be laboratory analyzed for VOC, in accordance with EPA Method 8260; and semi-VOC, in accordance with EPA Method 8270 (base/neutral extractables).

7.3 Soil Reuse or Disposal

Overburden soil scheduled for reuse should be managed pursuant to procedures described in Section 11. Waste soil materials scheduled for disposal should be managed pursuant to procedures described in Section 12.

8.0 SOIL COVER SYSTEM INSTALLATION

A soil cover should be installed in areas of the site that are impacted with ash material and will be utilized as lawn or landscaped areas. The following criteria should be applicable to the soil cover system.

- The upper 6 inches of the soil cover should be suitable to sustain growth of appropriate vegetation at the ground surface.
- A minimum of 1 foot of soil cover should be placed above the ash material.
- The upper 1 foot of the soil cover should not have concentrations of contaminants that exceed the Restricted Residential Soil Cleanup Objectives (SCO) set forth in 6 NYCRR Part 375-6.
- Fill that is placed at a depth below the upper 1 foot of soil cover should not have concentrations of contaminants that exceed the Commercial SCO set forth in 6 NYCRR Part 375-6.
- A demarcation layer should be provided between the soil cover layer and underlying impacted soil, unless approval is obtained from the NYSDEC to forego installation of a demarcation layer.
- In the event that the soil cover system is breached, penetrated, or temporarily removed, restoration to original conditions (or equivalent) should be performed.
- Areas with a soil cover should be inspected at least annually, to assess existing conditions and determine if any restoration or repairs are necessary. Inspections should also be performed after severe weather events or significant site operations that may have adversely affected the soil cover system.

9.0 VAPOR BARRIER SYSTEM INSTALLATION

A vapor barrier system could be considered as an option for buildings that are constructed at the subject site, especially for buildings that would be occupied on a routine basis. While risks with vapor migration from contaminants associated with ash material is relatively low, installation of a vapor barrier system is generally an inexpensive addition to the construction of a new building. A vapor barrier system could consist of a gas permeable layer (i.e., crushed stone) and a soil gas retarder membrane (i.e., polyethylene or polyolefin sheeting) between the gas permeable layer and concrete slab. Soil gas collector pipes could also be installed in the gas permeable layer, and established as a passive system, active system, or passive with capability to be transitioned to an active system. If the Owner opts for installation of a vapor barrier system for buildings, the vapor barrier system should be incorporated into design plans and specifications for the specific building(s) being constructed.

10.0 DREDGING OF SEDIMENT

10.1 Sediment Removal and Management

Dredging of sediment would need to be conducted pursuant to conditions of applicable permits, as determined through the joint application for permits process with state agencies and the USACE. A dredging plan should also be developed to identify the dredging methods and management options. Direct coordination with the NYSDEC and reference to NYSDEC TOGS 5.1.9 would be necessary to ensure that necessary criteria for the dredging operations are addressed.

10.2 Sediment Reuse or Disposal

6 NYCRR Part 360.12(c)(1)(iv) describes conditions for pre-determined beneficial use of navigational dredged material; however, laboratory analysis data for previously collected samples at the site are not indicative of the sediment material meeting the requisite criteria. For navigational dredged material that does not meet the pre-determined beneficial use criteria, a petition for a case-specific beneficial use determination (BUD) could be considered.

Laboratory analysis results from previously collected sediment samples have identified elevated concentrations for metals and PCB, and as such, dredged sediment (or affected portions thereof) should be disposed of at a permitted solid waste management facility. If reuse is desired for portions that may exhibit lesser contaminant concentrations, a plan would need to be developed for segregation and sampling and analysis, along with submitting the petition for a case-specific BUD.

11.0 BACKFILL AND SOIL REUSE

11.1 Suitable On-Site Soil

Laboratory analysis results for samples previously collected from locations on the subject site not impacted with ash indicate that concentrations of contaminants generally do not exceed the 6 NYCRR Part 375 Commercial SCO and the 6 NYCRR Part 360 fill material pre-determined beneficial use criteria. This material should be suitable for on-site reuse below the upper 1 foot of soil cover. Additional sampling and analysis may be needed for areas not previously investigated and for previously sampled areas where exceedances were identified relative to the 6 NYCRR Part 375 Commercial SCO and/or the 6 NYCRR Part 360 fill material pre-determined beneficial use criteria. If additional sampling and analysis is performed, the quantity of samples and analytical parameters should be selected pursuant to the Sampling and Analysis Schedule for Fill in Appendix E.

11.2 Imported Fill

Fill material may need to be imported to the site, for use as the upper 1 foot of soil cover system or for other areas specific types of fill material. Imported fill material should be sampled and analyzed prior to delivery to the site, to confirm the material satisfies criteria established for use as a soil cover (reference Section 8) or criteria for use as general fill, restricted use fill, or limited use fill per 6 NYCRR Part 360.13. The quantity of samples and analytical parameters for imported fill should be selected pursuant to the Sampling and Analysis Schedule for Fill in Appendix E.

12.0 WASTE TRANSPORT AND DISPOSAL

The Contractor should provide for loading and transporting contaminated soil to a permitted solid waste management facility. Transport of waste materials will require use of trucks with applicable permits pursuant to 6 NYCRR Part 364 criteria. The disposal of waste soil materials should be documented via waste manifests and/or copies of waste disposal receipts.

Waste characterization soil samples should be collected and laboratory analyzed from the impacted material, pursuant to requirements of the selected disposal facility. The selected disposal facility should be contacted prior to excavation work to identify applicable laboratory analysis parameters and quantity of samples, and to process waste profile documentation.

13.0 REPORTING AND RECORDKEEPING

Reports and records of site work should be maintained, as needed to document site conditions and soil management procedures that are completed. Reports and records to be maintained in association with this Soil Management Plan include, but may not be limited to, the following:

- As-built plans
- Waste manifests and/or disposal receipts for ash, soil, and groundwater
- Air monitoring data
- Excavation monitoring data
- Soil sampling and laboratory analysis data
- Site observation reports

APPENDIX A

Site Location Map

Site Location Map	Drawn by: TSP	Scale: Not to scale	Project No.: AT5596	Date: May 2020

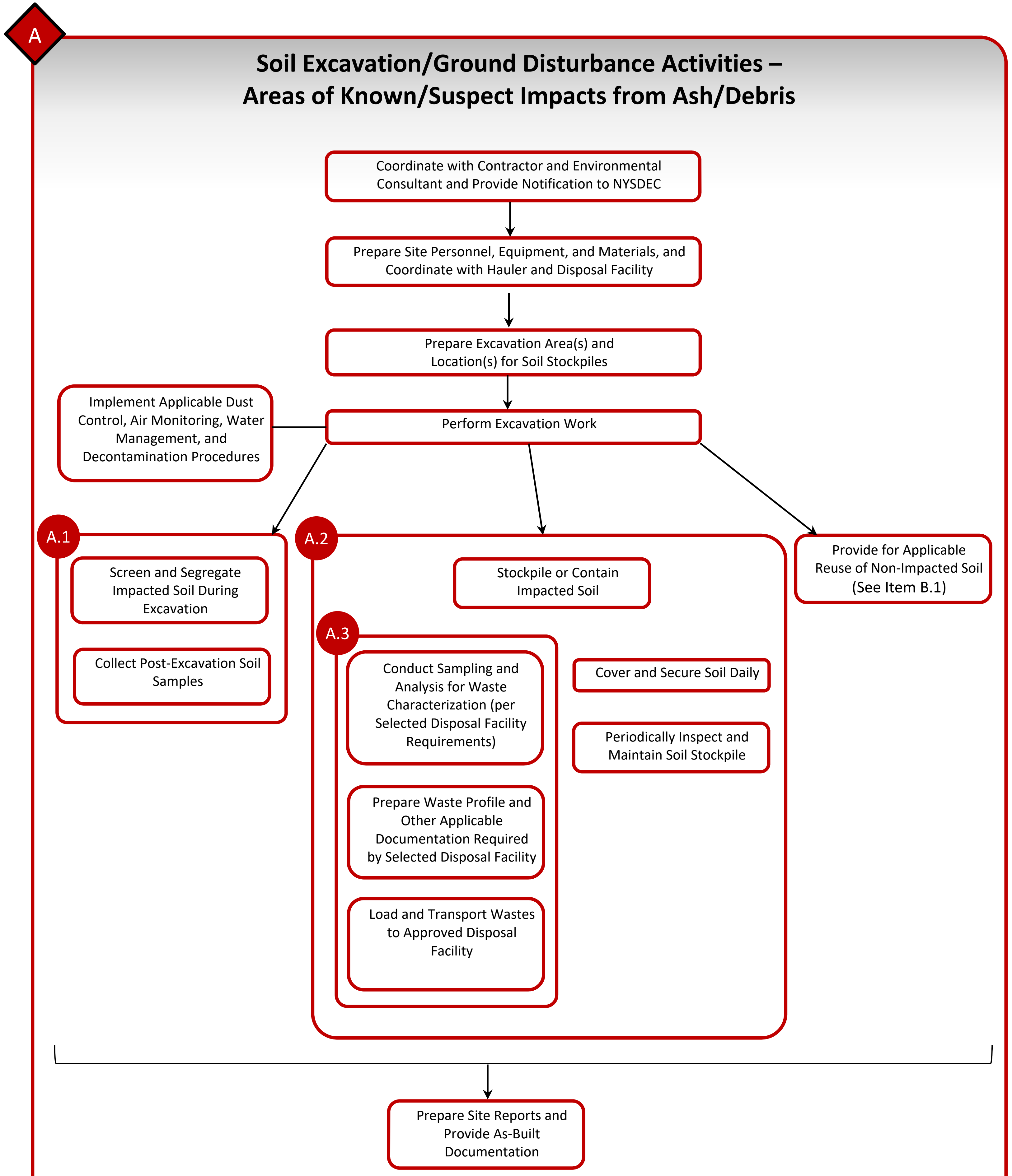
Beacon Island Parcel
Bethlehem, Albany County, New York

***ATLANTIC TESTING LABORATORIES*, Limited**

Albany, NY
Binghamton, NY
Canton, NY
Elmira, NY
Plattsburgh, NY
Poughkeepsie,
Syracuse, NY
Rochester, NY
Utica, NY
Watertown, NY

APPENDIX B

Soil Management Plan Flow Chart



Soil Excavation/Ground Disturbance Activities – Areas of Known/Suspect Impacts from Ash/Debris

Coordinate with Contractor and Environmental Consultant and Provide Notification to NYSDEC

Prepare Site Personnel, Equipment, and Materials, and Coordinate with Hauler and Disposal Facility

Prepare Excavation Area(s) and Location(s) for Soil Stockpiles

Implement Applicable Dust Control, Air Monitoring, Water Management, and Decontamination Procedures

Perform Excavation Work

A.1

Screen and Segregate Impacted Soil During Excavation

Collect Post-Excavation Soil Samples

A.2

Stockpile or Contain Impacted Soil

A.3

Conduct Sampling and Analysis for Waste Characterization (per Selected Disposal Facility Requirements)

Prepare Waste Profile and Other Applicable Documentation Required by Selected Disposal Facility

Load and Transport Wastes to Approved Disposal Facility

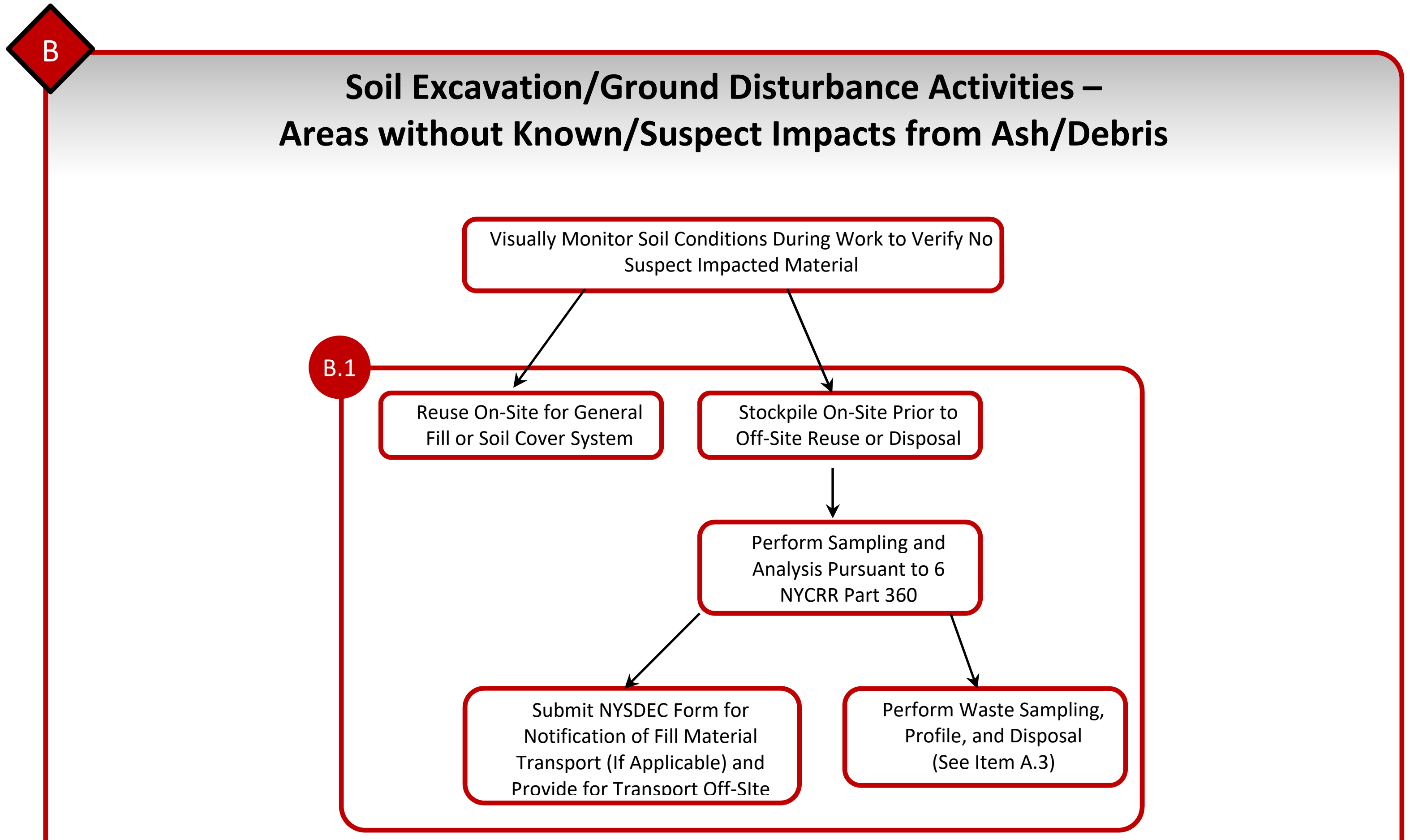
Cover and Secure Soil Daily

Periodically Inspect and Maintain Soil Stockpile

Provide for Applicable Reuse of Non-Impacted Soil (See Item B.1)

Prepare Site Reports and Provide As-Built Documentation

1. *If petroleum/chemical spill or contamination is encountered, implement soil management procedures described in Item C.*
2. *Reference Items D and E for procedures pertaining to site backfill/soil cover system and assessment for imported fill.*



Soil Excavation/Ground Disturbance Activities – Areas without Known/Suspect Impacts from Ash/Debris

Visually Monitor Soil Conditions During Work to Verify No Suspect Impacted Material

B.1

Reuse On-Site for General Fill or Soil Cover System

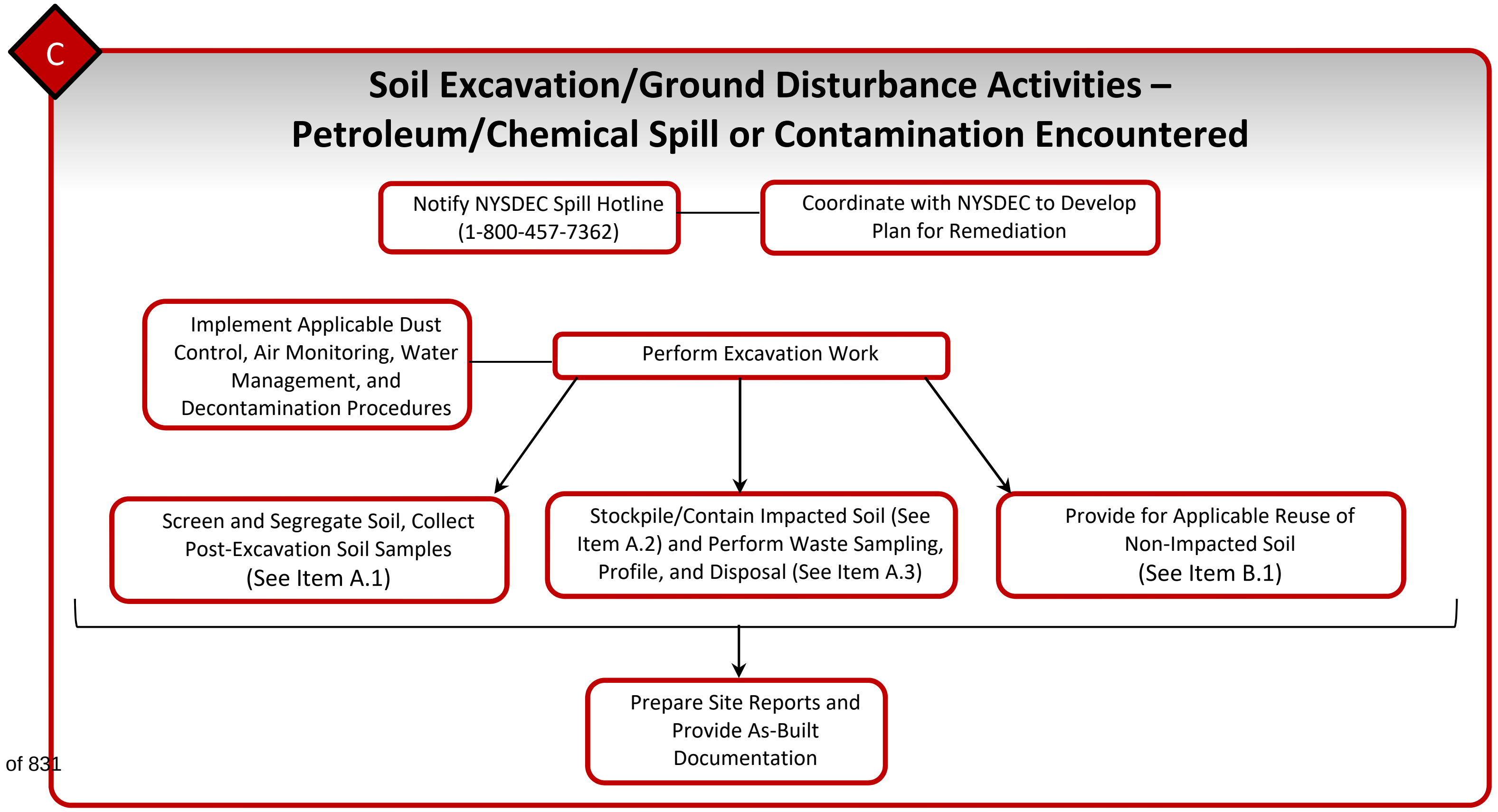
Stockpile On-Site Prior to Off-Site Reuse or Disposal

Perform Sampling and Analysis Pursuant to 6 NYCRR Part 360

Submit NYSDEC Form for Notification of Fill Material Transport (If Applicable) and Provide for Transport Off-SIte

Perform Waste Sampling, Profile, and Disposal (See Item A.3)

1. *If ash/debris material is encountered, implement soil management procedures described in Item A.*
2. *If petroleum/chemical spill or contamination is encountered, implement soil management procedures described in Item C.*
3. *Reference Items D and E for procedures pertaining to site backfill/soil cover system and assessment for imported fill.*



Soil Excavation/Ground Disturbance Activities – Petroleum/Chemical Spill or Contamination Encountered

Notify NYSDEC Spill Hotline (1-800-457-7362)

Coordinate with NYSDEC to Develop Plan for Remediation

Implement Applicable Dust Control, Air Monitoring, Water Management, and Decontamination Procedures

Perform Excavation Work

Screen and Segregate Soil, Collect Post-Excavation Soil Samples (See Item A.1)

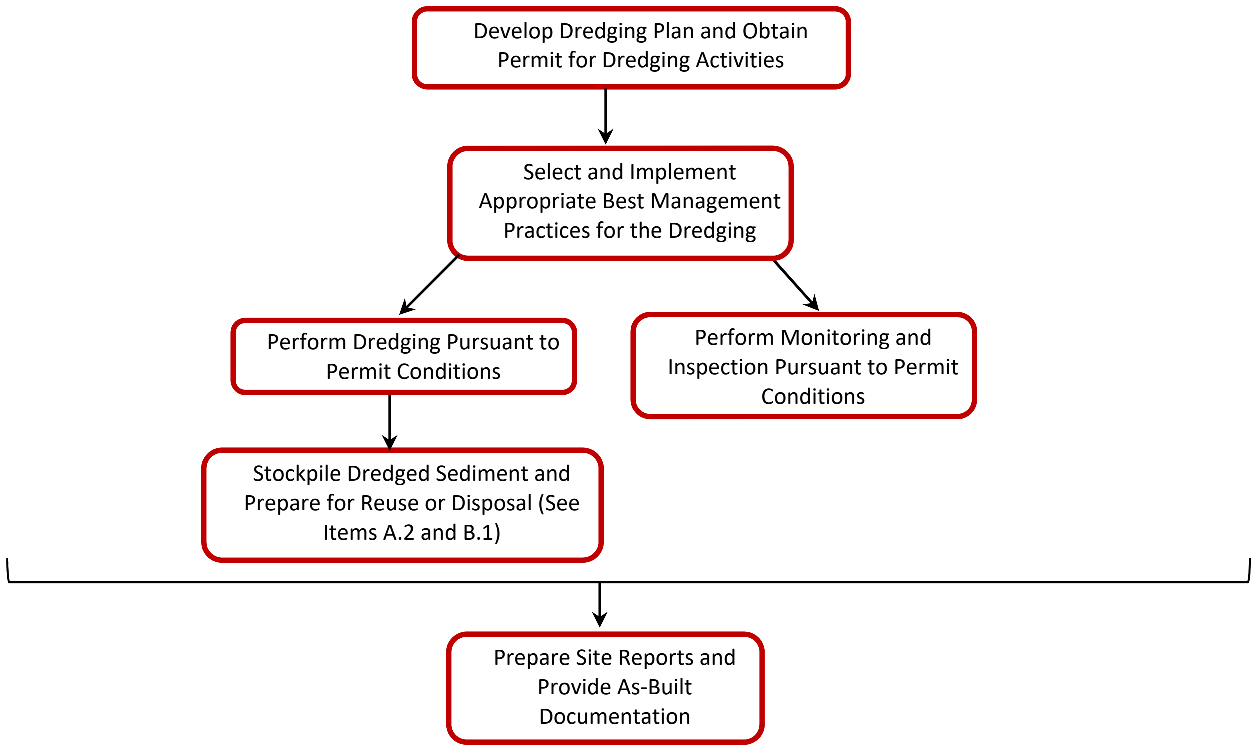
Stockpile/Contain Impacted Soil (See Item A.2) and Perform Waste Sampling, Profile, and Disposal (See Item A.3)

Provide for Applicable Reuse of Non-Impacted Soil (See Item B.1)

Prepare Site Reports and Provide As-Built Documentation

D

Sediment Dredging


Develop Dredging Plan and Obtain Permit for Dredging Activities

Select and Implement Appropriate Best Management Practices for the Dredging

Perform Dredging Pursuant to Permit Conditions

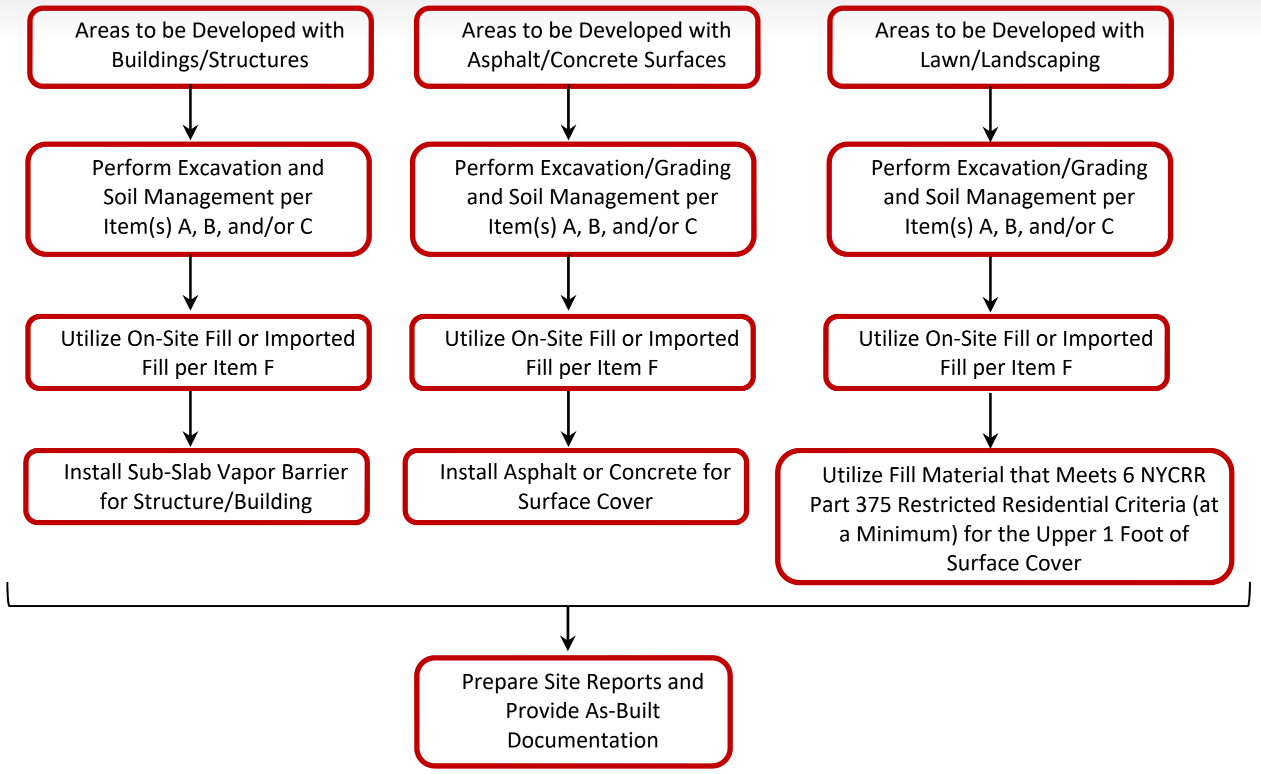
Perform Monitoring and Inspection Pursuant to Permit Conditions

Stockpile Dredged Sediment and Prepare for Reuse or Disposal (See Items A.2 and B.1)

Prepare Site Reports and Provide As-Built Documentation

E

Installation of Soil Cover System/ Ground Surface Cover Conditions


Areas to be Developed with Buildings/Structures

Perform Excavation and Soil Management per Item(s) A, B, and/or C

Utilize On-Site Fill or Imported Fill per Item F

Install Sub-Slab Vapor Barrier for Structure/Building

Areas to be Developed with Asphalt/Concrete Surfaces

Perform Excavation/Grading and Soil Management per Item(s) A, B, and/or C

Utilize On-Site Fill or Imported Fill per Item F

Install Asphalt or Concrete for Surface Cover

Areas to be Developed with Lawn/Landscaping

Perform Excavation/Grading and Soil Management per Item(s) A, B, and/or C

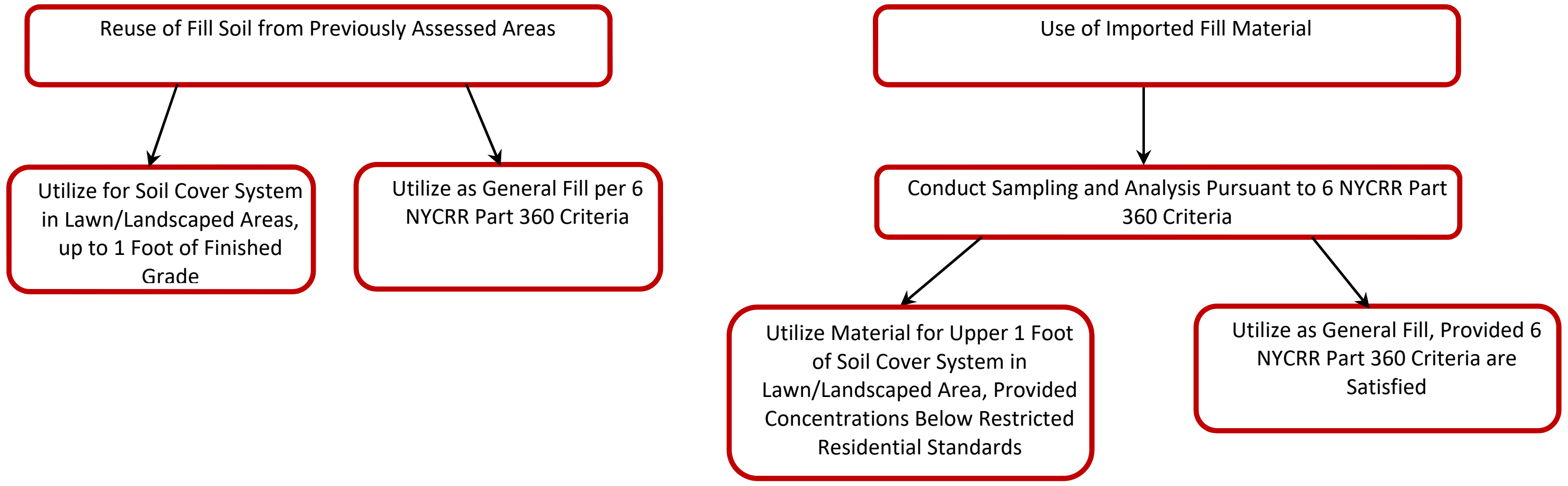
Utilize On-Site Fill or Imported Fill per Item F

Utilize Fill Material that Meets 6 NYCRR Part 375 Restricted Residential Criteria (at a Minimum) for the Upper 1 Foot of Surface Cover

Prepare Site Reports and Provide As-Built Documentation

F

Reuse of Previously Assessed On-Site Fill and Assessment and Use of Imported Fill


Reuse of Fill Soil from Previously Assessed Areas

Utilize for Soil Cover System in Lawn/Landscaped Areas, up to 1 Foot of Finished Grade

Utilize as General Fill per 6 NYCRR Part 360 Criteria

Use of Imported Fill Material

Conduct Sampling and Analysis Pursuant to 6 NYCRR Part 360 Criteria

Utilize Material for Upper 1 Foot of Soil Cover System in Lawn/Landscaped Area, Provided Concentrations Below Restricted Residential Standards

Utilize as General Fill, Provided 6 NYCRR Part 360 Criteria are Satisfied

APPENDIX C

Summary of Data from Previous Investigations

Table C-1
Summary of Information from September 2020 Investigation – Probes
(Information Available from Environmental Subsurface Investigation and Soil Sampling Report prepared by ATL and dated October 22, 2020)

Probe ID**	Depth Advanced	Coal Ash/Debris Observations	VOC Field Screening (ppm)	Samples Collected for Analysis of VOC	Compounds Exceeding 6 NYCRR Part 375 Unrestricted Use SCO	Samples Collected for Analysis of Semi-VOC, PCB, Metals, Pesticides, and Cyanide	Compounds Exceeding 6 NYCRR Part 375 Unrestricted Use SCO
B-1	20’	Coal Ash @ 0’.5 – 2’	ND				
B-2	20’	Coal Ash @ 0’ – 20’	ND				
B-3	20’	Coal Ash @ 0’ – 20’	ND				
B-4	20’	Coal Ash @ 0’ – 18’	ND				
B-5	20’		ND	Soil @ 0’ – 5’		Soil @ 0’ – 20’ (Composite with B-6 and B-8)	Iron
B-6	20’		ND	Soil @ 10’ – 15’		Soil @ 0’ – 20’ (Composite with B-5 and B-8)	See B-5
B-7	20’	Coal Ash @ 0’ – 11’	ND				
B-8	20’		ND	Soil @ 5’ – 10’		Soil @ 0’ – 20’ (Composite with B-5 and B-6)	See B-5
B-9	20’	Coal Ash @ 0.5’ – 2.5’	ND – 18.2				
B-10	20’		ND			Soil @ 0’ – 20’ (Composite with B-11 and B-12)	4,4’-DDD, Arsenic, Iron, Vanadium
B-11	20’	Coal Ash @ 8’ – 11.5’	ND	Soil @ 15’ – 20’		Soil @ 0’ – 20’ (Composite with B-10 and B-12)	See B-10

Table C-1 (continued)
Summary of Information from September 2020 Investigation – Probes
(Information Available from Environmental Subsurface Investigation and Soil Sampling Report prepared by ATL and dated October 22, 2020)

Probe ID**	Depth Advanced	Coal Ash/Debris Observations	VOC Field Screening (ppm)	Samples Collected for Analysis of VOC	Compounds Exceeding 6 NYCRR Part 375 Unrestricted Use SCO	Samples Collected for Analysis of Semi-VOC, PCB, Metals, Pesticides, and Cyanide	Compounds Exceeding 6 NYCRR Part 375 Unrestricted Use SCO
B-12	20’		ND	Soil @ 0’ – 5’		Soil @ 0’ – 20’ (Composite with B-10 and B-11)	See B-10
B-13	20’		ND	Soil @ 10’ – 15’	Acetone	Soil @ 0’ – 20’ (Composite with B-15 and B-16)	Iron, Vanadium
B-14	20’		ND				
B-15	20’		ND	Soil @ 5’ – 10’		Soil @ 0’ – 20’ (Composite with B-13 and B-16)	See B-13
B-16	20’		ND	Soil @ 15’ – 20’		Soil @ 0’ – 20’ (Composite with B-13 and B-15)	See B-13
B-17	20’		ND	Soil @ 0’ – 5’		Soil @ 0’ – 20’ (Composite with B-18 and B-19)	Iron, Vanadium
B-18	20’		ND	Soil @ 10’ – 15’		Soil @ 0’ – 20’ (Composite with B-17 and B-19)	See B-17
B-19	20’		ND	Soil @ 5’ – 10’		Soil @ 0’ – 20’ (Composite with B-17 and B-18)	See B-17
B-20	20’	Coal Ash @ 0’ – 2’	ND				
B-21	20’		ND	Soil @ 15’ – 20’		Soil @ 0’ – 20’ (Composite with B-22 and B-23)	Iron
B-22	20’		ND	Soil @ 5’ – 10’		Soil @ 0’ – 20’ (Composite with B-21 and B-23)	See B-21
B-23	20’		ND	Soil @ 15’ – 20’		Soil @ 0’ – 20’ (Composite with B-21 and B-22)	See B-21
B-24	20’		ND			Soil @ 0’ – 20’ (Composite with B-25)	Aluminum, Iron
B-25	20’		ND			Soil @ 0’ – 20’ (Composite with B-24)	See B-24
B-26	5.1’ (Refusal)		ND	Soil @ 0’ – 5’		Soil @ 0’ – 20’ (Composite with B-27)	Iron
B-27	20’		ND			Soil @ 0’ – 20’ (Composite with B-26)	See B-26
B-28	20’		ND	Soil @ 10’ – 15’		Soil @ 0’ – 20’ (Composite with B-29)	Iron

Table C-1 (continued)
Summary of Information from September 2020 Investigation – Probes
(Information Available from Environmental Subsurface Investigation and Soil Sampling Report prepared by ATL and dated October 22, 2020)

Probe ID**	Depth Advanced	Coal Ash/Debris Observations	VOC Field Screening (ppm)	Samples Collected for Analysis of VOC	Compounds Exceeding 6 NYCRR Part 375 Unrestricted Use SCO	Samples Collected for Analysis of Semi-VOC, PCB, Metals, Pesticides, and Cyanide	Compounds Exceeding 6 NYCRR Part 375 Unrestricted Use SCO
B-29	20’		ND			Soil @ 0’ – 20’ (Composite with B-28)	See B-28
B-30	10’ (Refusal)		ND	Soil @ 5’ – 10’		Soil @ 0’ – 20’ (Composite with B-31)	Iron
B-31	20’		ND	Soil @ 15’ – 20’		Soil @ 0’ – 20’ (Composite with B-30)	See B-30
B-32	20’		ND				
B-33	20’		ND				
B-34	20’		ND				
B-35	20’	Coal Ash @ 0’ – 12.5’	ND				
B-36	20’	Coal Ash @ 0.5’ – 6’	ND				
B-37	20’	Coal Ash @ 0.5’ – 7.5’	ND				
B-38	20’	Coal Ash @ 0’ – 11.5’	ND				
B-39	20’	Coal Ash @ 0’ – 15’	ND				
B-40	20’	Coal Ash @ 0.5’ – 20’	ND				
B-41	20’		ND	Soil @ 5’ – 10’		Soil @ 0’ – 20’ (Composite with B-43)	4,4’-DDE, 4,4’-DDD, Aluminum, Calcium, Iron
B-42	20’	Coal Ash @ 0’ – 8’	ND				
B-43	20’		ND	Soil @ 15’ – 20’		Soil @ 0’ – 20’ (Composite with B-41)	See B-41

Table C-1 (continued)
Summary of Information from September 2020 Investigation – Probes
(Information Available from Environmental Subsurface Investigation and Soil Sampling Report prepared by ATL and dated October 22, 2020)

Probe ID**	Depth Advanced	Coal Ash/Debris Observations	VOC Field Screening (ppm)	Samples Collected for Analysis of VOC	Compounds Exceeding 6 NYCRR Part 375 Unrestricted Use SCO	Samples Collected for Analysis of Semi-VOC, PCB, Metals, Pesticides, and Cyanide	Compounds Exceeding 6 NYCRR Part 375 Unrestricted Use SCO
B-44	20’		ND	Soil @ 0’ – 5’		Soil @ 0’ – 20’ (Composite with B-45)	Iron
B-45	20’		ND	Soil @ 10’ – 15’	Acetone	Soil @ 0’ – 20’ (Composite with B-44)	See B-44

** Approximate locations of probes are shown on the *Aerial Overview of Affected Locations* plan in Appendix D.

Table C-2
Summary of Information from February 2017 Investigation – Borings/Monitor Wells
(Information Available from Draft Phase II Environmental Site Assessment Report prepared by Bergmann Associates and dated April 6, 2017)

Boring/ Monitor Well ID**	Depth Advanced	Coal Ash/Debris Observations	VOC Field Screening (ppm)	Groundwater Observations	Samples Collected for Metals Analysis	Metals Exceeding 6 NYCRR Part 375 Unrestricted Use Soil Cleanup Objectives
B-1	100’ (environmental assessment to 12’)	Coal Ash @ 0’ – 12’	0.0	Water @ 12.9’	0” – 2” (Soil)	Arsenic, Barium, Nickel
					10’ – 12’ (Soil)	Arsenic, Barium
B-2	50’ (environmental assessment to 10’)	Coal Ash @ 0’ – 10’	0.0 – 2.9	Water @ 6’	0” – 2” (Soil)	Arsenic, Barium, Silver
					4’ – 6’ (Soil)	Arsenic, Barium, Chromium, Selenium, Silver
B-3/ MW-1	150’ (environmental assessment to 16’)	Coal Ash @ 0’ – 6’	0.0 – 13.7	Water @ 14.9’	0” – 2” (Soil)	Nickel
					2’ – 4’ (Soil)	Silver
					Groundwater (Screened @ 12’ – 22’)	Iron, Manganese
B-4/ MW-3	100’ (environmental assessment to 12’)	Coal Ash @ 0’ – 12’	0.0 – 0.9		0” – 2” (Soil)	Arsenic, Barium, Mercury
					2’ – 4’ (Soil)	Arsenic, Barium, Mercury
					Groundwater (Screened @ 5’ – 15’)	Iron, Sodium
B-5/ MW-2	50’ (environmental assessment to 14’)		2.9 – 22.9		0” – 2” (Soil)	Arsenic, Barium, Silver
					4’ – 6’ (Soil)	Arsenic, Selenium, Silver
					Groundwater (Screened @ 11’ – 21’)	Iron, Manganese
B-6	50’ (environmental assessment to 14’)	Coal Ash @ 0’ – 14’	0.0 – 0.2	Water @ 7.8’	0” – 2” (Soil)	Arsenic, Barium, Chromium, Nickel, Silver
					4’ – 6’ (Soil)	Arsenic, Barium, Nickel
B-7	50’ (environmental assessment to 20’)	Coal Ash @ 0’ – 18	0.1 – 0.2	Water @ 2.5’	0” – 2” (Soil)	Arsenic, Barium, Nickel, Silver
					1’ – 4’ (Soil)	Arsenic, Barium, Chromium, Mercury, Nickel
B-8	50’ (environmental assessment to 18’)	Coal Ash @ 6’ – 18’	0.1 – 0.6		0” – 2” (Soil)	
					6’ – 8’ (Soil)	

** Approximate locations of borings are shown on the *Aerial Overview of Affected Locations* plan in Appendix D.

Table C-3

Summary of Information from February 2017 Investigation – Test Pits
(Information Available from Draft Phase II Environmental Site Assessment Report prepared by Bergmann Associates and dated April 6, 2017)

Test Pit ID**	Depth Advanced	Coal Ash/Debris Observations	VOC Field Screening (ppm)
TP-1	12’		0.0 – 0.1
TP-2	12’		0.0 – 0.1
TP-3	12’		0.0 – 0.2
TP-4	12’		0.1 – 0.2
TP-5	12’		0.0 – 0.1
TP-6	12’		0.0
TP-7	12’		0.0
TP-8	12’	Railroad ties covered in black tar-like substance @ 8’ – 12’	0.0 – 10.1
TP-9	10’		0.0
TP-10	12’	Coal Ash @ 0’ – 12’	0.1
TP-11	12’	Coal Ash @ 0’ – 12’	0.0 – 0.1
TP-12	12’	Coal Ash @ 0’ – 12’	0.1

** Approximate locations of test pits are shown on the *Aerial Overview of Affected Locations* plan in Appendix D.

APPENDIX D

Aerial Overview of Affected Locations

LEGEND :

● B-5 (02/17) [illegible]

■ TP-4 (02/17) [illegible]

🟧 [illegible]

🟨 [illegible]

🟩 [illegible]

AERIAL OVERVIEW OF
AFFECTED LOCATIONS

Beacon Island Parcel
Bethlehem, Albany County, New York

Drawn By:	Drawing:	Scale:	Project No.:	Date :
CJD	1 of 1	As Noted	AT5596	October 2020

ATLANTIC TESTING LABORATORIES, Limited

Albany, NY Binghamton, NY Canton, NY Elmira, NY Poughkeepsie, NY
Plattsburgh, NY Rochester, NY Syracuse, NY Utica, NY Watertown, NY

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APPENDIX E

Sampling and Analysis Schedule for Fill

Sampling Criteria for Fill Material per 6 NYCRR Part 360.13(e)

Soil Quantity (cubic yards)	**Number of Discrete Samples**	**Number of Composite Samples**[1]
	Volatile Organic Compounds (VOC)	Semi-VOC, Inorganics, Polychlorinated Biphenyls (PCB), and Pesticides
0 – 300	2	1
300 – 1,000	4	2
1,000 – 10,000	6	3
>10,000	2 for every additional 10,000 cubic yards	1 for every additional 10,000 cubic yards

Notes: [1] Each composite sample will be comprised of 3 to 5 discrete samples from different locations within the fill material.

APPENDIX I

SEDIMENT BASIN CALCULATIONS

TEMPORARY SEDIMENT BASIN DESIGN DATA SHEET

Computed by NSO Date 1/20/22 Checked by ____ Date ____
Project Port of Albany Basin # 1/ WQv Pond #2
Location Expansion Site Total Area draining to basin (≤50 Ac.) 5.3 Acres

BASIN SIZE DESIGN

1. Sediment storage zone volume = 1,000 cu. ft. x number of disturbed acres = 5,300 cu. ft., Top of Zone Elev. ____
2. Dewatering zone volume = 3,600 cu. ft. x number of drainage area acres = 19,080 cu. ft., Top of Zone Elev. ____
3. Length to width ratio = 3.6:1
4. A. Cleanout at 50% of sediment storage zone volume, Elev. ____
   B. Distance below top of riser ____ feet
5. Minimum surface area is larger of 0.01 $Q_{(10)}$ 0.15 or, 0.015 DA = 0.08 use 0.15 acres

DESIGN OF SPILLWAYS & ELEVATIONS

*Basin 1 (WQv Pond #2) has adequate storage and surface area to function as a temporary sediment basin. All pipes and structures within basin are to be covered with filter fabric during construction. Sediment cleanout is to take place once final stabilization is reached.

Runoff

6. $Q_{p(10)}$ = 14.96 cfs (Attach runoff computation sheets)

Pipe Spillway (Q_{ps})

7. Min. pipe spillway cap., Q_{ps} = 0.2 x ____ Drainage Area, acres = ____ cfs
   Note: If there is no emergency spillway, then required $Q_{ps} = Q_{p(10)}$ = ____ cfs.
8. H, head = ____ ft. Barrel length = ____ ft
9. Barrel: Diam. ____ inches; Q_{ps} = (Q) ____ x (cor.fac.) ____ = ____ cfs.
10. Riser: Diam. ____ inches; Length ____ ft.; h = ____ ft. Crest Elev. ____
11. Trash Rack: Diameter = ____ inches; H, height = ____ inches

Emergency Spillway Design

12. Emergency Spillway Flow, $Q_{es} = Q_p - Q_{ps}$ = ____ - ____ = ____ cfs.
13. Width ____ ft.; H_p ____ ft Crest elevation ____; Design High Water Elev. ____
    Entrance channel slope ____ % ; Top of Dam Elev. ____
    Exit channel slope ____ %

ANTI-SEEP COLLAR/SEEPAGE DIAPHRAGM DESIGN

Collars:

14. y = ____ ft.; z = ____ :1; pipe slope = ____ %, L_s = ____ ft.
    Use ____ collars, ____ - ____ inches square; projection = ____ ft.

Diaphragms:

____ width ____ ft. height ____ ft.

DEWATERING ORIFICE SIZING

(Determined from the Dewatering Device Standard)

15. Dewatering orifice diameter = ____ inches. Skimmer ____ or Riser ____ (check one)
16. Design dewatering time ____ days (Min. 2 days required)

TEMPORARY SEDIMENT BASIN DESIGN DATA SHEET

Computed by NSO Date 1/20/22 Checked by ______ Date ______
Project Port of Albany Basin # 2
Location Expansion Site Total Area draining to basin (≤50 Ac.) 15.2 Acres

BASIN SIZE DESIGN

1. Sediment storage zone volume = 1,000 cu. ft. x number of disturbed acres = 15,200 cu. ft., Top of Zone Elev. 7.5
2. Dewatering zone volume = 3,600 cu. ft. x number of drainage area acres = 54,720 cu. ft., Top of Zone Elev. 11.0
3. Length to width ratio = 9.2:1
4. A. Cleanout at 50% of sediment storage zone volume, Elev. 6.75'
   B. Distance below top of riser 4.25 feet
5. Minimum surface area is larger of 0.01 $Q_{(10)}$ 0.57 or, 0.015 DA = 0.23 use 0.57 acres

DESIGN OF SPILLWAYS & ELEVATIONS

Runoff

6. $Q_{p(10)}$ = 56.95 cfs (Attach runoff computation sheets)

Pipe Spillway (Q_{ps})

7. Min. pipe spillway cap., Q_{ps} = 0.2 x 15.2 Drainage Area, acres = 3.04 cfs
   Note: If there is no emergency spillway, then required $Q_{ps} = Q_{p(10)}$ = - cfs.
8. H, head = 5 ft. Barrel length = 115 ft
9. Barrel: Diam. 12 inches; Q_{ps} = (Q) 4.41 x (cor.fac.) 0.86 = 3.79 cfs.
10. Riser: Diam. 15 inches; Length 4.5 ft.; h = 1 ft. Crest Elev. 11.0
11. Trash Rack: Diameter = 21 inches; H, height = 7 inches

Emergency Spillway Design

12. Emergency Spillway Flow, $Q_{es} = Q_p - Q_{ps}$ = 56.95 - 3.79 = 53.16 cfs.
13. Width 15 ft.; H_p 1 ft Crest elevation 12.0 ; Design High Water Elev. 13.0
    Entrance channel slope 1 % ; Top of Dam Elev. 14.0
    Exit channel slope 3 %

ANTI-SEEP COLLAR/SEEPAGE DIAPHRAGM DESIGN

Collars:

14. y = 4.5 ft.; z = 2 :1; pipe slope = 0.5 %, L_s = 27 ft.
    Use 2 collars, 4 - 0 inches square; projection = 1.4 ft.

Diaphragms:

______ width ______ ft. height ______ ft.

DEWATERING ORIFICE SIZING

(Determined from the Dewatering Device Standard)

15. Dewatering orifice diameter = 5 inches. Skimmer x or Riser ____ (check one)
16. Design dewatering time 2 days (Min. 2 days required)

TEMPORARY SEDIMENT BASIN DESIGN DATA SHEET

Computed by NSO Date 1/20/22 Checked by ____________ Date ________
Project Port of Albany Basin # 3
Location Expansion Site Total Area draining to basin (≤50 Ac.) 33.0 Acres

BASIN SIZE DESIGN

1. Sediment storage zone volume = 1,000 cu. ft. x number of disturbed acres = 33,000 cu. ft., Top of Zone Elev. 7.5
2. Dewatering zone volume = 3,600 cu. ft. x number of drainage area acres = 118,800 cu. ft., Top of Zone Elev. 11.0
3. Length to width ratio = 11.6:1
4. A. Cleanout at 50% of sediment storage zone volume, Elev. 6.75
   B. Distance below top of riser 4.25 feet
5. Minimum surface area is larger of 0.01 $Q_{(10)}$ 1.11 or, 0.015 DA = .49 use 1.11 acres

DESIGN OF SPILLWAYS & ELEVATIONS

Runoff

6. $Q_{p(10)}$ = 110.93 cfs (Attach runoff computation sheets)

Pipe Spillway (Q_{ps})

7. Min. pipe spillway cap., Q_{ps} = 0.2 x 33 Drainage Area, acres = 6.6 cfs
   Note: If there is no emergency spillway, then required $Q_{ps} = Q_{p(10)}$ = - cfs.
8. H, head = 5 ft. Barrel length = 115 ft
9. Barrel: Diam. 15 inches; Q_{ps} = (Q) 7.78 x (cor.fac.) 0.85 = 6.61 cfs.
10. Riser: Diam. 18 inches; Length 4.5 ft.; h = 1 ft. Crest Elev. 11.0
11. Trash Rack: Diameter = 27 inches; H, height = 8 inches

Emergency Spillway Design

12. Emergency Spillway Flow, $Q_{es} = Q_p - Q_{ps}$ = 110.93 - 6.61 = 104.32 cfs.
13. Width 30 ft.; H_p 1 ft Crest elevation 12.0; Design High Water Elev. 13.0
    Entrance channel slope 1 % ; Top of Dam Elev. 14.0
    Exit channel slope 3 %

ANTI-SEEP COLLAR/SEEPAGE DIAPHRAGM DESIGN

Collars:

14. y = 4.5 ft.; z = 3 :1; pipe slope = 0.5 %, L_s = 31.5 ft.
    Use 2 collars, 4 - 3 inches square; projection = 1.6 ft.

Diaphragms:

#________ width________ ft. height ________ft.

DEWATERING ORIFICE SIZING

(Determined from the Dewatering Device Standard)

15. Dewatering orifice diameter = 7 inches. Skimmer x or Riser ____ (check one)
16. Design dewatering time 2 days (Min. 2 days required)

Summary for Subcatchment DA1: (new Subcat)

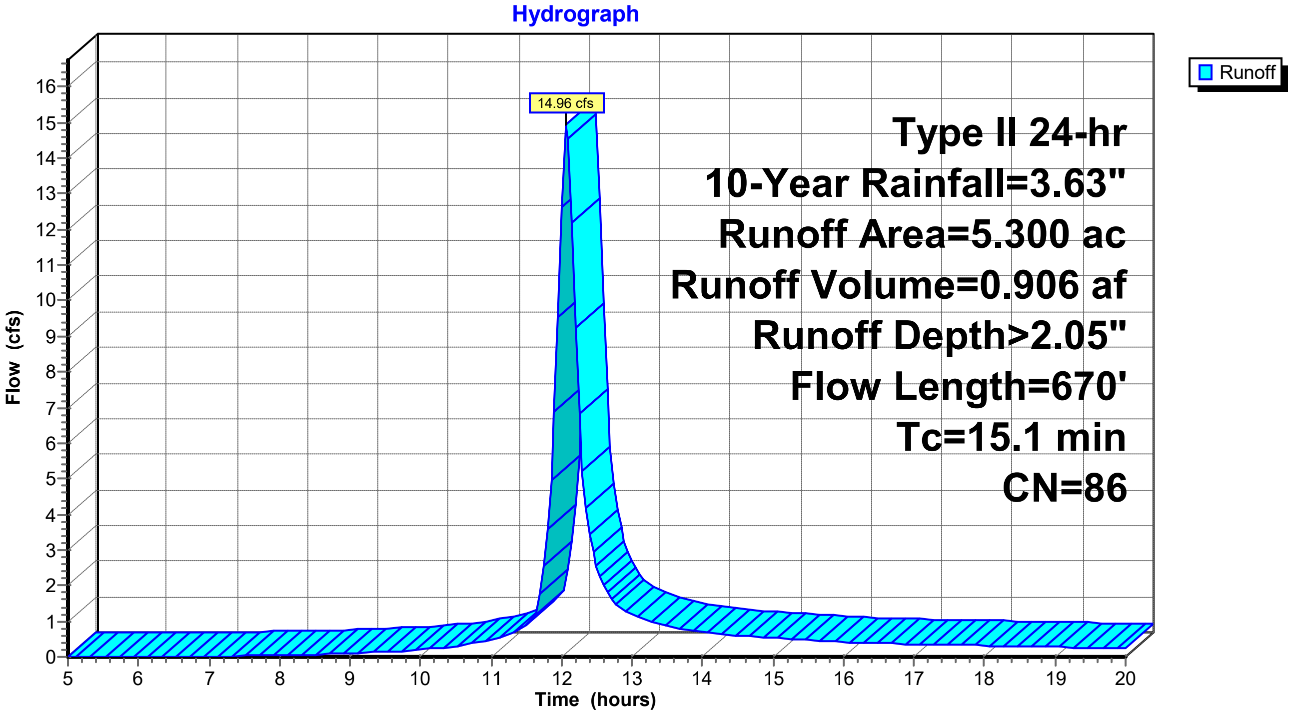
Runoff = 14.96 cfs @ 12.07 hrs, Volume= 0.906 af, Depth> 2.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

Area (ac)	CN	Description
0.850	96	Gravel surface, HSG D
2.200	80	>75% Grass cover, Good, HSG D
2.250	89	Dirt roads, HSG D
5.300	86	Weighted Average
5.300		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
3.5	150	0.0200	0.72		**Sheet Flow,** n= 0.023 P2= 2.40"
11.6	520	0.0025	0.75		**Shallow Concentrated Flow,** Grassed Waterway Kv= 15.0 fps
15.1	670	Total			

Subcatchment DA1: (new Subcat)



Summary for Subcatchment DA2: (new Subcat)

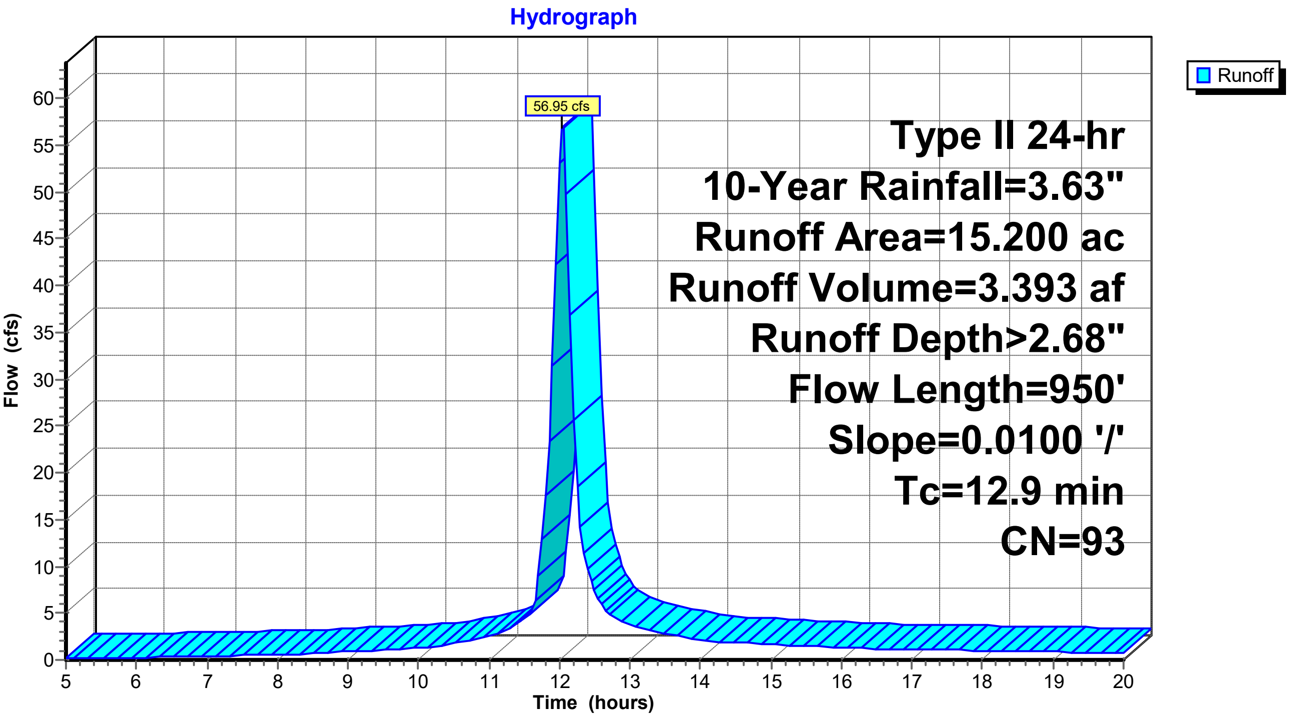
Runoff = 56.95 cfs @ 12.04 hrs, Volume= 3.393 af, Depth> 2.68"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	6.900	98	Bldg A
	8.300	89	Dirt roads, HSG D
	15.200	93	Weighted Average
	8.300		54.61% Pervious Area
	6.900		45.39% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	150	0.0100	0.54		**Sheet Flow,** n= 0.023 P2= 2.40"
8.3	800	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
12.9	950	Total			

Subcatchment DA2: (new Subcat)



Summary for Subcatchment DA3: (new Subcat)

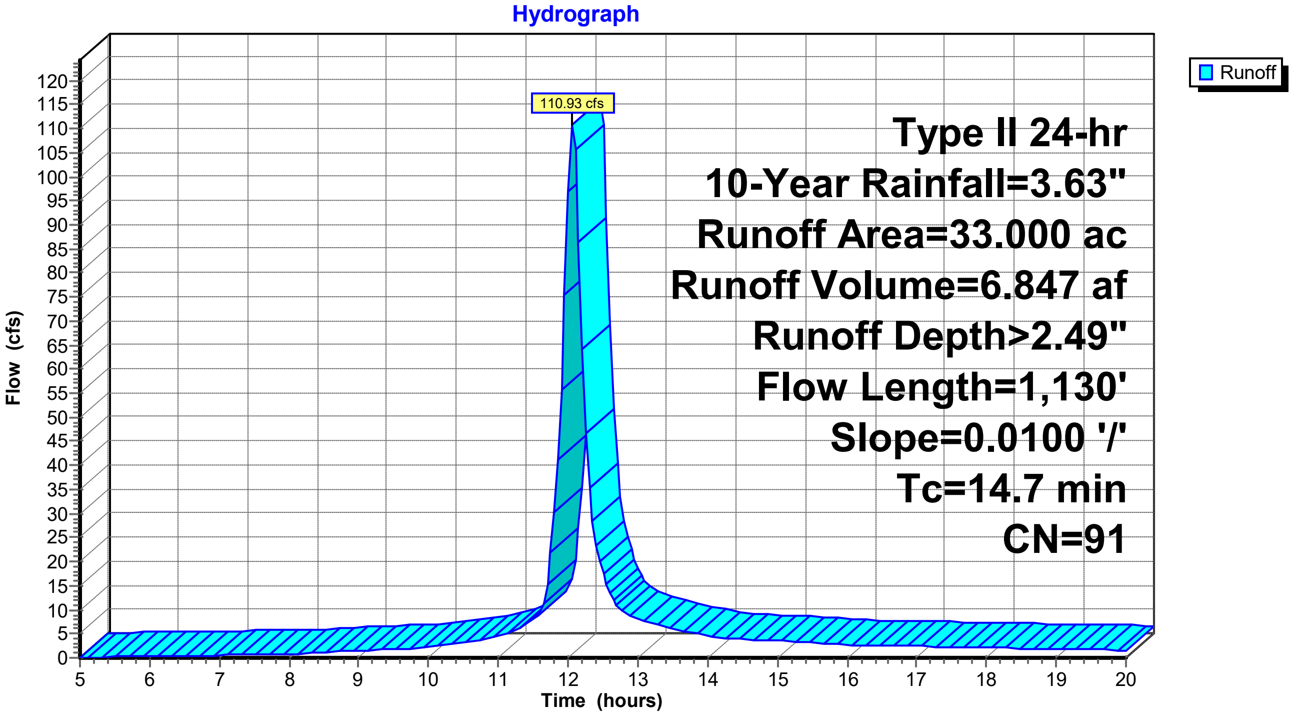
Runoff = 110.93 cfs @ 12.06 hrs, Volume= 6.847 af, Depth> 2.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-Year Rainfall=3.63"

	Area (ac)	CN	Description
*	7.000	98	Bldg B C D
	26.000	89	Dirt roads, HSG D
	33.000	91	Weighted Average
	26.000		78.79% Pervious Area
	7.000		21.21% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
4.6	150	0.0100	0.54		**Sheet Flow,** n= 0.023 P2= 2.40"
10.1	980	0.0100	1.61		**Shallow Concentrated Flow,** Unpaved Kv= 16.1 fps
14.7	1,130	Total			

Subcatchment DA3: (new Subcat)



Summary for Pond 2P: (new Pond)

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	71,786 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
6.00	9,449	976.0	0	0	9,449
7.00	11,415	988.9	10,417	10,417	11,693
7.50	12,407	995.0	5,954	16,370	12,776
11.00	19,527	1,039.0	55,416	71,786	20,738

Summary for Pond 3p: (new Pond)

Volume	Invert	Avail.Storage	Storage Description
#1	6.00'	156,085 cf	**Custom Stage Data (Irregular)** Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Perim. (feet)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)
6.00	21,679	1,328.0	0	0	21,679
6.50	23,679	1,338.0	11,336	11,336	23,903
7.00	25,694	1,347.0	12,340	23,676	25,940
7.50	27,044	1,353.0	13,183	36,859	27,395
11.00	41,606	1,419.0	119,226	156,085	42,740

APPENDIX J

DRAFT NOI

Table of Contents

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NOI for coverage under Stormwater General Permit for Construction Activity

version 1.35

(Submission #: HPE-0QXG-AE8EZ, version 1)

Details

Originally Started By	Natalie Olivieri
Alternate Identifier	Marmen-Welcon Tower Manufacturing Plant
Submission ID	HPE-0QXG-AE8EZ
Submission Reason	New
Status	Draft
Active Steps	Form Submitted

Form Input

Owner/Operator Information

Owner/Operator Name (Company/Private Owner/Municipality/Agency/Institution, etc.)
Albany Port District Commission

Owner/Operator Contact Person Last Name (NOT CONSULTANT)
Hendrick

Owner/Operator Contact Person First Name
Richard

Owner/Operator Mailing Address
106 Smith Boulevard

City
Albany

State
New York

Zip
12205

Phone
5184638763

Email
rhendrick@portofalbany.us

Federal Tax ID
14-6002520

Project Location

Project/Site Name
Marmen-Welcon Tower Manufacturing Plant

Street Address (Not P.O. Box)
309 River Road

Side of Street
East

City/Town/Village (THAT ISSUES BUILDING PERMIT)
Town of Bethlehem

State
NY

Zip
12077

DEC Region
4

County
ALBANY

Name of Nearest Cross Street
Old River Road

Distance to Nearest Cross Street (Feet)
970

Project In Relation to Cross Street
South

Tax Map Numbers Section-Block-Parcel
98.00-2-10.23

Tax Map Numbers
98.01-2-1.0

1. Coordinates

Provide the Geographic Coordinates for the project site. The two methods are:
- Navigate to the project location on the map (below) and click to place a marker and obtain the XY coordinates.
- The "Find Me" button will provide the lat/long for the person filling out this form. Then pan the map to the correct location and click the map to place a marker and obtain the XY coordinates.

Navigate to your location and click on the map to get the X,Y coordinates
42.602283629058164,-73.76555834600738

Project Details

2. What is the nature of this project?
New Construction

3. Select the predominant land use for both pre and post development conditions.

Pre-Development Existing Landuse
Forest

Post-Development Future Land Use
Industrial

3a. If Single Family Subdivision was selected in question 3, enter the number of subdivision lots.
NONE PROVIDED

---

4. In accordance with the larger common plan of development or sale, enter the total project site acreage, the acreage to be disturbed and the future impervious area (acreage)within the disturbed area.

*** ROUND TO THE NEAREST TENTH OF AN ACRE. ***

Total Site Area (acres)
108.6

Total Area to be Disturbed (acres)
72.7

Existing Impervious Area to be Disturbed (acres)
5.2

Future Impervious Area Within Disturbed Area (acres)
65.9

5. Do you plan to disturb more than 5 acres of soil at any one time?
Yes

---

6. Indicate the percentage (%) of each Hydrologic Soil Group(HSG) at the site.

A (%)
3.2

B (%)
0

C (%)
0

D (%)
96.8

7. Is this a phased project?
Yes

8. Enter the planned start and end dates of the disturbance activities.

Start Date
6/1/2022

End Date
9/1/2023

9. Identify the nearest surface waterbody(ies) to which construction site runoff will discharge.
Hudson River, Normans Kill, Wetland

9a. Type of waterbody identified in question 9?
Wetland/Federal Jurisdiction On Site (Answer 9b)
River On Site
Stream/Creek On Site

Other Waterbody Type Off Site Description
NONE PROVIDED

9b. If "wetland" was selected in 9A, how was the wetland identified?
Delineated by Consultant

10. Has the surface waterbody(ies in question 9 been identified as a 303(d) segment in Appendix E of GP-0-20-001?
No

11. Is this project located in one of the Watersheds identified in Appendix C of GP-0-20-001?
No

12. Is the project located in one of the watershed areas associated with AA and AA-S classified waters?
No

If No, skip question 13.

13. Does this construction activity disturb land with no existing impervious cover and where the Soil Slope Phase is identified as D (provided the map unit name is inclusive of slopes greater than 25%), E or F on the USDA Soil Survey?
NONE PROVIDED

If Yes, what is the acreage to be disturbed?
NONE PROVIDED

14. Will the project disturb soils within a State regulated wetland or the protected 100 foot adjacent area?
Yes

15. Does the site runoff enter a separate storm sewer system (including roadside drains, swales, ditches, culverts, etc)?
No

16. What is the name of the municipality/entity that owns the separate storm sewer system?
NONE PROVIDED

17. Does any runoff from the site enter a sewer classified as a Combined Sewer?
No

18. Will future use of this site be an agricultural property as defined by the NYS Agriculture and Markets Law?
No

19. Is this property owned by a state authority, state agency, federal government or local government?
Yes

20. Is this a remediation project being done under a Department approved work plan? (i.e. CERCLA, RCRA, Voluntary Cleanup Agreement, etc.)
No

Required SWPPP Components

21. Has the required Erosion and Sediment Control component of the SWPPP been developed in conformance with the current NYS Standards and Specifications for Erosion and Sediment Control (aka Blue Book)?
Yes

22. Does this construction activity require the development of a SWPPP that includes the post-construction stormwater management practice component (i.e. Runoff Reduction, Water Quality and Quantity Control practices/techniques)?
Yes

If you answered No in question 22, skip question 23 and the Post-construction Criteria and Post-construction SMP Identification sections.

23. Has the post-construction stormwater management practice component of the SWPPP been developed in conformance with the current NYS Stormwater Management Design Manual?
No

24. The Stormwater Pollution Prevention Plan (SWPPP) was prepared by:
Professional Engineer (P.E.)

SWPPP Preparer
Adam Frosino

Contact Name (Last, Space, First)
Frosino, Adam

Mailing Address
60 Railroad Place, Suite 402

City
Saratoga Springs

State
NY

Zip
12866

Phone
5185809380

Email
afrosino@mjinc.com

Download SWPPP Preparer Certification Form
Please take the following steps to prepare and upload your preparer certification form:

1) Click on the link below to download a blank certification form
2) The certified SWPPP preparer should sign this form
3) Scan the signed form
4) Upload the scanned document
Download SWPPP Preparer Certification Form

Please upload the SWPPP Preparer Certification

> SWPPP Preparer Cert_Signed.pdf - 05/24/2022 10:45 AM
> **Comment**
> NONE PROVIDED

Erosion & Sediment Control Criteria

25. Has a construction sequence schedule for the planned management practices been prepared?
Yes

26. Select all of the erosion and sediment control practices that will be employed on the project site:

Temporary Structural
Check Dams
Construction Road Stabilization
Dust Control
Perimeter Dike/Swale
Sediment Basin
Sediment Traps
Silt Fence
Stabilized Construction Entrance
Storm Drain Inlet Protection

Biotechnical
None

Vegetative Measures
Mulching
Seeding
Temporary Swale
Topsoiling

Permanent Structural
Lined Waterway (Rock)
Land Grading
Retaining Wall
Rock Outlet Protection
Streambank Protection

Other
NONE PROVIDED

Post-Construction Criteria

*** IMPORTANT: Completion of Questions 27-39 is not required if response to Question 22 is No.**

27. Identify all site planning practices that were used to prepare the final site plan/layout for the project.
Preservation of Undisturbed Area
Preservation of Buffers
Reduction of Clearing and Grading

27a. Indicate which of the following soil restoration criteria was used to address the requirements in Section 5.1.6("Soil Restoration") of the Design Manual (2010 version).
All disturbed areas will be restored in accordance with the Soil Restoration requirements in Table 5.3 of the Design Manual (see page 5-22).

28. Provide the total Water Quality Volume (WQv) required for this project (based on final site plan/layout). (Acre-feet)
6.29

29. Post-construction SMP Identification
Use the Post-construction SMP Identification section to identify the RR techniques (Area Reduction), RR techniques(Volume Reduction) and Standard SMPs with RRv Capacity that were used to reduce the Total WQv Required (#28).

Identify the SMPs to be used by providing the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

Note: Redevelopment projects shall use the Post-Construction SMP Identification section to identify the SMPs used to treat and/or reduce the WQv required. If runoff reduction techniques will not be used to reduce the required WQv, skip to question 33a after identifying the SMPs.

30. Indicate the Total RRv provided by the RR techniques (Area/Volume Reduction) and Standard SMPs with RRv capacity identified in question 29. (acre-feet)
1.45

31. Is the Total RRv provided (#30) greater than or equal to the total WQv required (#28)?
No

If Yes, go to question 36. If No, go to question 32.

32. Provide the Minimum RRv required based on HSG. [Minimum RRv Required = (P) (0.95) (Ai) / 12, Ai=(s) (Aic)] (acre-feet)
1.32

32a. Is the Total RRv provided (#30) greater than or equal to the Minimum RRv Required (#32)?
Yes

If Yes, go to question 33.
Note: Use the space provided in question #39 to summarize the specific site limitations and justification for not reducing 100% of WQv required (#28). A detailed evaluation of the specific site limitations and justification for not reducing 100% of the WQv required (#28) must also be included in the SWPPP.

If No, sizing criteria has not been met; therefore, NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

33. SMPs
Use the Post-construction SMP Identification section to identify the Standard SMPs and, if applicable, the Alternative SMPs to be used to treat the remaining total WQv (=Total WQv Required in #28 - Total RRv Provided in #30).

Also, provide the total impervious area that contributes runoff to each practice selected.

NOTE: Use the Post-construction SMP Identification section to identify the SMPs used on Redevelopment projects.

33a. Indicate the Total WQv provided (i.e. WQv treated) by the SMPs identified in question #33 and Standard SMPs with RRv Capacity identified in question #29. (acre-feet)
4.85

Note: For the standard SMPs with RRv capacity, the WQv provided by each practice = the WQv calculated using the contributing drainage area to the practice - provided by the practice. (See Table 3.5 in Design Manual)

34. Provide the sum of the Total RRv provided (#30) and the WQv provided (#33a).
6.31

35. Is the sum of the RRv provided (#30) and the WQv provided (#33a) greater than or equal to the total WQv required (#28)?
Yes

If Yes, go to question 36.

If No, sizing criteria has not been met; therefore, NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

36. Provide the total Channel Protection Storage Volume (CPv required and provided or select waiver (#36a), if applicable.

CPv Required (acre-feet)
0.93

CPv Provided (acre-feet)
1.17

36a. The need to provide channel protection has been waived because:
NONE PROVIDED

37. Provide the Overbank Flood (Qp) and Extreme Flood (Qf) control criteria or select waiver (#37a), if applicable.

Overbank Flood Control Criteria (Qp)

Pre-Development (CFS)
73.24

Post-Development (CFS)
12.46

Total Extreme Flood Control Criteria (Qf)

Pre-Development (CFS)
163.60

Post-Development (CFS)
30.97

37a. The need to meet the Qp and Qf criteria has been waived because:
NONE PROVIDED

38. Has a long term Operation and Maintenance Plan for the post-construction stormwater management practice(s) been developed?
Yes

If Yes, Identify the entity responsible for the long term Operation and Maintenance
Albany Port District Commission

39. Use this space to summarize the specific site limitations and justification for not reducing 100% of WQv required (#28). (See question #32a) This space can also be used for other pertinent project information.
The proposed Facility requires 85 acres of usable manufacturing and storage space along the Hudson River. This site was chosen given it is located adjacent to the existing Port of Albany and is directly on the Hudson River. However, the usable portion of the site adjacent to the Hudson River, is only 66-acre area. Therefore, the entirety of the site is needed for the OSW manufacturing process, with an ancillary receiving site located at 700 Smith Boulevard.

Responses to questions 36 and 37 are related only to Analysis Point #1A. Analysis Points #1, #2, and #3 have direct discharge to tidal waters and therefore are waived from meeting the CPv, Qp, and Qf requirements.

Post-Construction SMP Identification

Runoff Reduction (RR) Techniques, Standard Stormwater Management Practices (SMPs) and Alternative SMPs
Identify the Post-construction SMPs to be used by providing the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

RR Techniques (Area Reduction)

Round to the nearest tenth

Total Contributing Acres for Conservation of Natural Area (RR-1)
4.0

Total Contributing Impervious Acres for Conservation of Natural Area (RR-1)
0

Total Contributing Acres for Sheetflow to Riparian Buffers/Filter Strips (RR-2)
NONE PROVIDED

Total Contributing Impervious Acres for Sheetflow to Riparian Buffers/Filter Strips (RR-2)
NONE PROVIDED

Total Contributing Acres for Tree Planting/Tree Pit (RR-3)
NONE PROVIDED

Total Contributing Impervious Acres for Tree Planting/Tree Pit (RR-3)
NONE PROVIDED

Total Contributing Acres for Disconnection of Rooftop Runoff (RR-4)
NONE PROVIDED

RR Techniques (Volume Reduction)

Total Contributing Impervious Acres for Disconnection of Rooftop Runoff (RR-4)
NONE PROVIDED

Total Contributing Impervious Acres for Vegetated Swale (RR-5)
NONE PROVIDED

Total Contributing Impervious Acres for Rain Garden (RR-6)
NONE PROVIDED

Total Contributing Impervious Acres for Stormwater Planter (RR-7)
NONE PROVIDED

Total Contributing Impervious Acres for Rain Barrel/Cistern (RR-8)
NONE PROVIDED

Total Contributing Impervious Acres for Porous Pavement (RR-9)
NONE PROVIDED

Total Contributing Impervious Acres for Green Roof (RR-10)
NONE PROVIDED

Standard SMPs with RRv Capacity

Total Contributing Impervious Acres for Infiltration Trench (I-1)
NONE PROVIDED

Total Contributing Impervious Acres for Infiltration Basin (I-2)
1

Total Contributing Impervious Acres for Dry Well (I-3)
NONE PROVIDED

Total Contributing Impervious Acres for Underground Infiltration System (I-4)
23.7

Total Contributing Impervious Acres for Bioretention (F-5)
NONE PROVIDED

Total Contributing Impervious Acres for Dry Swale (O-1)
0.05

Standard SMPs

Total Contributing Impervious Acres for Micropool Extended Detention (P-1)
5.1

Total Contributing Impervious Acres for Wet Pond (P-2)
NONE PROVIDED

Total Contributing Impervious Acres for Wet Extended Detention (P-3)
NONE PROVIDED

Total Contributing Impervious Acres for Multiple Pond System (P-4)
NONE PROVIDED

Total Contributing Impervious Acres for Pocket Pond (P-5)
NONE PROVIDED

Total Contributing Impervious Acres for Surface Sand Filter (F-1)
NONE PROVIDED

Total Contributing Impervious Acres for Underground Sand Filter (F-2)
NONE PROVIDED

Total Contributing Impervious Acres for Perimeter Sand Filter (F-3)
NONE PROVIDED

Total Contributing Impervious Acres for Organic Filter (F-4)
NONE PROVIDED

Total Contributing Impervious Acres for Shallow Wetland (W-1)
NONE PROVIDED

Total Contributing Impervious Acres for Extended Detention Wetland (W-2)
NONE PROVIDED

Total Contributing Impervious Acres for Pond/Wetland System (W-3)
NONE PROVIDED

Total Contributing Impervious Acres for Pocket Wetland (W-4)
NONE PROVIDED

Total Contributing Impervious Acres for Wet Swale (O-2)
NONE PROVIDED

Alternative SMPs (DO NOT INCLUDE PRACTICES BEING USED FOR PRETREATMENT ONLY)

Total Contributing Impervious Area for Hydrodynamic
NONE PROVIDED

Total Contributing Impervious Area for Wet Vault
NONE PROVIDED

Total Contributing Impervious Area for Media Filter
NONE PROVIDED

"Other" Alternative SMP?
Contech Jellyfish Filter

Total Contributing Impervious Area for "Other"
54.7

Provide the name and manufaturer of the alternative SMPs (i.e. proprietary practice(s)) being used for WQv treatment.

Note: Redevelopment projects which do not use RR techniques, shall use questions 28, 29, 33 and 33a to provide SMPs used, total WQv required and total WQv provided for the project.

Manufacturer of Alternative SMP
Contech

Name of Alternative SMP
Jellyfish Filter

Other Permits

40. Identify other DEC permits, existing and new, that are required for this project/facility.
Endangered or Threatened Species (Incidental Take Permit)

If SPDES Multi-Sector GP, then give permit ID
NONE PROVIDED

If Other, then identify
Wetland Joint Permit, Air State Facility Permit

41. Does this project require a US Army Corps of Engineers Wetland Permit?
Yes

If "Yes," then indicate Size of Impact, in acres, to the nearest tenth
0.9

42. If this NOI is being submitted for the purpose of continuing or transferring coverage under a general permit for stormwater runoff from construction activities, please indicate the former SPDES number assigned.
NONE PROVIDED

MS4 SWPPP Acceptance

43. Is this project subject to the requirements of a regulated, traditional land use control MS4?
Yes - Please attach the MS4 Acceptance form below

If No, skip question 44

44. Has the "MS4 SWPPP Acceptance" form been signed by the principal executive officer or ranking elected official and submitted along with this NOI?
NONE PROVIDED

MS4 SWPPP Acceptance Form Download
Download form from the link below. Complete, sign, and upload.
MS4 SWPPP Acceptance Form

MS4 Acceptance Form Upload

> NONE PROVIDED
> **Comment**
> NONE PROVIDED

Owner/Operator Certification

Owner/Operator Certification Form Download

Download the certification form by clicking the link below. Complete, sign, scan, and upload the form.
Owner/Operator Certification Form (PDF, 45KB)

Upload Owner/Operator Certification Form

> POA - eNOI Owner Cert_Signed.pdf - 05/24/2022 11:52 AM
> **Comment**
> NONE PROVIDED

Attachments

Date	Attachment Name	Context	User
5/24/2022 11:52 AM	POA - eNOI Owner Cert_Signed.pdf	Attachment	Natalie Olivieri
5/24/2022 10:45 AM	SWPPP Preparer Cert_Signed.pdf	Attachment	Natalie Olivieri

Status History

	User	Processing Status
12/16/2021 1:10:48 PM	Natalie Olivieri	Draft

Processing Steps

Step Name	Assigned To/Completed By	Date Completed
Form Submitted		
Under Review	DAVID GASPER	
Under Review	Daniel von Schilgen	

APPENDIX K

UNIFORM PROCEDURES ACT EXCEPTION LETTER

NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION

Division of Environmental Permits
625 Broadway, 4th Floor, Albany, New York 12233-1750
P: (518) 402-9167 | F: (518) 402-9168 | deppermitting@dec.ny.gov
www.dec.ny.gov

March 30, 2022

VIA EMAIL

Richard Hendrick
Albany Port District Commission
106 Smith Boulevard
Albany, NY 12202
rhendrick@portofalbany.us

RE: SPDES GP-0-20-001 - Commencement of Limited Construction Activities
Port of Albany Expansion Project
Beacon Island Parcel, Bethlehem NY, Albany County
DEC # 4-0122-00322/00002

Dear Mr. Hendrick:

By letter dated March 25, 2022, Albany Port District Commission (APDC) requested to commence limited construction activities (i.e., tree clearing), and authorization to discharge pursuant to the Stormwater State Pollutant Discharge Elimination System (SPDES) General Permit for Construction Activity (GP-0-20-001), for the Port of Albany Expansion Project (Project) prior to obtaining all necessary permits from New York State Department of Environmental Conservation (NYSDEC or Department). APDC has not obtained an Air State Facility (ASF) permit, individual SPDES permit, Part 182 permit, Protection of Waters permits and Water Quality Certification (WQC) (“Uniform Procedures Act (UPA) Permits”).[1] APDC asserts in its request that good cause exists based on its committed construction schedule and specific requirements for site preparation, construction, and operational components. Pursuant to 6 NYCRR 621.3(a)(4), based on permitting considerations and the specific circumstances presented here, NYSDEC reviewed the March 25, 2022 request from APDC and determined that there is good cause to allow the commencement of limited construction activities[2] as further described herein, without all UPA Permits having been obtained.

---

[1] According to Part II(C)(1) of GP-0-20-001, an owner or operator shall not commence any construction activity in any location until the authorization to discharge under the General Permit goes into effect. Additionally, under Part II(C)(2)(b) of GP-0-20-001, authorization to discharge under the permit will be effective, among other requirements, when the owner or operator has obtained, “all necessary Department permits subject to the Uniform Procedures Act (“UPA”) (see 6 NYCRR Part 621), or the equivalent from another New York State agency” unless otherwise notified by the Department pursuant to 6 NYCRR 621.3(a)(4).

[2] Construction [Activities] are defined in GP-0-20-001 as, “any clearing, grading, excavation, filling, demolition or stockpiling activities that result in soil disturbance. Clearing activities can include, but are not limited to, logging equipment operation, the cutting and skidding of trees, stump removal and/or brush root removal. Construction activity does not include routine maintenance that is performed to maintain the original line and grade, hydraulic capacity, or original purpose of a facility.”

Demonstration of Good Cause

In evaluating whether there is good cause for NYSDEC to grant an exception to the authorization criterion contained in Part II(C)(2)(b) of SPDES GP-0-20-0001, and allow authorization to discharge and the commencement of limited construction activities prior to obtaining all other necessary UPA permits, NYSDEC has considered the following factors:

- Pursuant to UPA regulations, Section 621.3(a)(4), APDC has submitted all applications for UPA Permits for the Project.
- The requirements of the State Environmental Quality Review Act (SEQR - Article 8 of the ECL) have been satisfied,[3] as follows: (i) The Town of Bethlehem Planning Board was established as SEQR Lead Agency (Lead Agency) and a positive declaration was issued requiring a Draft Generic Environmental Impact Statement (GEIS) for the Project; (ii) a Final GEIS was accepted as complete and a SEQR Findings Statement was adopted by the Lead Agency on June 2, 2020 for the Project; (iii) the Lead Agency determined the Project may have potential to create one or more significant adverse environmental impacts not evaluated within the Final GEIS and issued a Positive Declaration requiring a Supplemental Draft EIS on July 6, 2021; and (iv) a Supplemental Final EIS was accepted as complete and a SEQR Findings Statement was adopted by the Lead Agency on March 15, 2022 for the Project.
- The requirements of the State Historic Preservation Act (SHPA),[4] have been complied with for the Project, as follows:
    - The Stockbridge Munsee Community issued their opinion in a March 2, 2022 letter that the Project will have No Adverse Effect on Historic Resources.
    - The New York State Historic Preservation Office (SHPO) issued a letter on March 25, 2022, stating that that no historic properties, including archaeological and/or historic resources, will be Adversely Affected by the Project, provided that a Restrictive Deed Covenant is filed to protect and maintain the vegetated buffer along the Hudson River shoreline.
- APDC has prepared a Stormwater Pollution Prevention Plan (SWPPP) for tree clearing activities.
- On March 29, 2022, the Town of Bethlehem as a regulated, traditional land use control Municipal Separate Stormwater Sewer Systems (MS4) accepted the SWPPP for tree clearing activities.
- APDC has indicated that to meet its contractual obligations with Equinor to be operational by December 1, 2023, tree clearing must take place by mid-April 2022 to facilitate site preparation, including implementation of a 3-month surcharge program to address poor soils and settlement concerns.
- APDC has indicated that the UPA Permits for the operational components of the Project (ASF and individual SPDES permits) require selection of manufacturing and processing equipment that must follow the Port of Albany procurement process and therefore, not all information is available prior to the construction bid. This information is required for NYSDEC to issue the ASF and individual SPDES permits.

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[3] See Part II.C(2)(a) of SPDES GP-0-20-001

[4] See Part I.F(8)(b)(iv) of SPDES GP-0-20-001

Commencement of Limited Construction Activities

In consideration of the factors identified above, NYSDEC is hereby granting a limited exception to the authorization criterion contained in Part II(C)(2)(b) of SPDES GP-0-20-001. Upon submission of a complete eNOI, APDC would be authorized to commence limited construction activities, (i.e., tree clearing), under SPDES GP-0-20-001 and pursuant to the SWPPP approved by the Town of Bethlehem on March 29, 2022, subject to the following limitations:

1. No permanent construction, including building supports and foundations, can commence until the Air State Facility permit is obtained.[5]
2. No construction of the Project’s wastewater treatment facility can commence until the individual SPDES permit is issued and plan approval for the wastewater treatment facility has been obtained from NYSDEC.
3. No excavation, fill, or dredging activities can commence in any wetlands or other waters of the U.S., including the Hudson River, Normans Kill, and federally regulated wetlands, until the following Permits are issued: Part 182, Protection of Waters, and WQC.
4. If tree clearing is conducted within federal wetlands, it must only be done using hand equipment and no stumps shall be removed.
5. No trees shall be removed within the vegetated buffer area, located along the Hudson River, in contravention of the Restrictive Deed Covenant that APDC has agreed to establish.
6. If any bats are observed flying from a tree that will be cut, work activities shall be stopped immediately and NYSDEC shall be contacted for guidance (see #5 in the following link: https://www.dec.ny.gov/animals/106090.html).
7. All other applicable terms and conditions of SPDES GP-0-20-001 are satisfied and complied with.

This exception to Part II(C)(2)(b) of SPDES GP-0-20-001 for the limited construction activities (i.e., tree clearing) is only authorized until May 15, 2022, or until NYSDEC issues all UPA Permits, whichever comes first. Please also note that granting of this exception does not guarantee issuance of the individual permits identified above as the permits are still subject to public review and comment, and other applicable provisions of 6 NYCRR Part 621. Further, this exception does not apply to construction activities that would be commenced beyond tree clearing. As a result, APDC assumes all risks in commencing construction.

Please contact me at karen.gaidasz@dec.ny.gov if you have any questions or concerns.

Sincerely,

Karen M. Gaidasz

Karen M. Gaidasz, Chief
Offshore Wind & Hydroelectric Section
Bureau of Energy Project Management

---

[5] NYCRR 201-2.1(b)(9), specifically excludes site clearing and excavation activities, it states, “*[t]he initiation of physical on-site construction activities which are of a permanent nature excluding site clearing and excavation. Such activities include, but are not limited to, installation of building supports and foundations, laying underground pipework and construction of permanent storage structures*.”

APPENDIX L

5 ACRE WAIVER REQUEST

McFarland Johnson
Innovative Solutions / Sustainable Results

60 Railroad Place, Suite 402 • Saratoga Springs, NY 12866
Phone: (518) 580-9380 • Fax: (518) 580-9383
www.mjinc.com

May 20, 2022

Paul Penman, P.E.
Town Engineer, Deputy Commissioner of Public Works
Town of Bethlehem
445 Delaware Avenue
Delmar, New York 12054

Re: SPDES General Permit for Storm Water Discharges from Construction Activity – Marmen-Welcon Tower Manufacturing Plant

Mr. Penman,

We are hereby requesting approval to disturb more than 5 acres of soil at a time for the Marmen-Welcon Tower Manufacturing Plant project.

Activities related to the above construction site will comply with all other requirements under GP-0-20-001. As such, SWPPP inspections will be conducted twice weekly while more than 5 acres of soil are disturbed.

The project has been split into five (5) phases spanning 20 months as shown on the project's Erosion and Sediment Control plans. The maximum soil disturbance at one time is anticipated to be around 12-15 acres as partial disturbance in two phases may occur simultaneously for short durations given the nature of the project. The anticipated soil disturbance areas and durations for each phase are as follows:

SOIL DISTURBANCE PHASING		
PHASE	**DISTURBANCE AREA**	**DURATION**
1	8.5 ACRES	2 MONTHS
2	11.3 ACRES	
3	11.0 ACRES MAX	6 MONTHS
3A	9.7 ACRES	
3B	9.6 ACRES	
3C	10.0 ACRES	
3D	10.1 ACRES	
3E	10.0 ACRES	
3F	3.5 ACRES	
4	NO NEW AREAS	4 MONTHS
5	5.1 ACRES	8 MONTHS

Disturbances associated with Phases 1 and 2 are expected to take place concurrently over a 2-month period. During these phases the construction entrance (including rock hammering), staging area, and site access roads will be established, as well as temporary sediment basins #1, #2, and #3. Any additional sediment controls not installed during the site tree cutting are to be installed during this phase.

During Phase 3 the site will be grubbed, and mass site grading will occur in preparation for the stone aggregate surcharging. Approximately 98,000 CY of on-site cut material will be used as fill to balance out the site. In addition, approximately 150,000-200,000 CY of suitable fill will be brought in to bring the site up to the sub-grade elevation.

The large grubbing area as well as the large amount of mass grading required resulted in breaking up Phase 3 into sub-phases of roughly 10 acres each. The sub-phases were determined based on overall project sequencing to start at the south end with Building A preparation, as well as establishing approximate areas where borrowing and fill will occur within each sub-area. Limits of disturbance to be minimized in each sub-phase by stabilizing areas within 2 days of achieving final grade. The sub-phase areas will be disturbed and stabilized in a rolling operation as the earthwork progresses from the south end of the site to the north end with disturbance ranging from 8-11 acres. The disturbance envelope ranges from 700 feet initially to 400 feet as earthwork progresses. To avoid stockpiling, available cut material from one sub-phase area that no longer requires any additional fill may be deposited and stabilized within another sub-phase area; however, the overall total disturbed area during Phase 3 shall not exceed 11.0 acres. Fine earthwork grading and subgrade preparation under the surcharge areas in accordance with the project's geotechnical report will also occur during this phase. In accordance with the SWPPP, all areas which are not active will be temporarily stabilized.

During Phase 4, approximately 700,000 CY of stone aggregate will be imported and stockpiled on the proposed building pads as a temporary building pad surcharge and will be used as the future stone yard area. It is anticipated that the subbase stone will begin to be brought into the site as the building footprint subgrade is completed for each sub-phase of Phase 3. As the stone aggregate is brought onto the site and placed in the surcharge areas it will act as permanent stabilization therefore decreasing the disturbed area. Given the volume of material to be brought into the site, this will take some time and will be completed in a rolling fashion in a similar fashion to Phase 3. The first building's surcharge import (Building A) will be completed to enable the surcharging compaction timeframe to start prior to placing the stone aggregate surcharge on the next building pad. Any peripheral subgrade areas previously disturbed or disturbed as part of the surcharge operations and no longer active will be temporarily stabilized in accordance with the SWPPP.

Phase 5 includes spreading the stone aggregate surcharge material to establish the stone yard area and all remaining construction on the project. Once the proposed drainage system and second pond have been installed and are operational, the temporary sediment basins will be removed as well as the conversion and installation of the permanent stormwater ponds. All previously disturbed areas that are to remain pervious will need to achieve final stabilization during this phase prior to issuance of the Notice of Termination.

The attached Phasing Plans demonstrates the intended construction sequence for the Marmen-Welcon Tower Manufacturing Plant project.

If you have any questions, or require anything further, please let us know. Thank you.

Sincerely,
McFarland-Johnson, Inc.

Adam J. Frosino, PE, PTOE
Project Manager

cc: Robert Leslie, Town of Bethlehem
Richard Hendrick, Port of Albany
Megan Daly, Port of Albany
Steve Boisvert, McFarland-Johnson

APPENDIX M

NATIONAL GRID ENVIRONMENTAL GUIDANCE

nationalgrid National Grid Environmental Guidance	Doc No.:	EG-501NYN
	Rev. No.:	5
	Page No.:	1 of 2
	Date:	03/05/2020

SUBJECT	REFERENCE
Release Notifications in New York North	**EP-5**

Purpose / Objective: This guidance document provides instructions for reporting a release of oil, chemical, or hazardous material.

Who: All National Grid employees and contractors working on National Grid projects or properties.

What to Do:

Report all releases to the National Grid Regional Control Center (RCC) immediately, regardless of the volume or location.

EASTERN RCC	(518) 356-6471
CENTRAL RCC	(315) 460-2796
WESTERN RCC	(716) 398-5308

Provide the following information to the RCC concerning the release:

- Name and contact information
- Time of <u>discovery</u>
- Location (be specific)
- Material and quantity
- Source (vehicle number, transformer, gas meter, HVAC unit, tank number, etc.)
- Cause (i.e., equipment malfunction, motor vehicle accident, storm, human factor, other)
- Description of impacted area and resources (land, water, air)
- Request cleanup crew

REGIONAL CONTROL CENTER:

1. Upon notification of a release, contact New York State Spill Hotline (1-800-457-7362) within 2 hours (of discovery time).

2. If release is reported at an Environmental SIR Site, contact Environmental immediately (see below).

3. Record release in the Incident Management System (IMS).

4. Contact Divisional Environmental Engineer, as applicable.

National Grid
Environmental Guidance

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Date:	**03/05/2020**

SUBJECT	**REFERENCE**
Release Notifications in New York North	**EP-5**

ENVIRONMENTAL MANAGEMENT:

1. Communicate with RCC and/or field crews regarding additional reporting requirements (e.g., releases to water and PCB releases require reporting to the National Response Center (1-800-424-8802).

2. In IMS categorize the release as Category 1 or Category 2 (EG-1502) and classify/complete/close out the incident.

3. Confirm clean up, provide additional information to state agency to close out the release as requested.

RELEASE REPORTING ON NEW YORK SIR MGP SITES

Environmental Site Investigation and Remediation has entered into Consent Orders with the New York State Department of Environmental Conservation (NYSDEC) to evaluate and, where necessary, remediate former manufactured gas plant (MGP) sites. These sites are managed by a National Grid project manager (PM). As agreed to with NYSDEC, in the event of an accidental release or spill of material at an MGP site which will require subsurface excavation work for cleanup:

- Report release using procedures above.

- The National Grid PM must be notified as soon as possible to coordinate work.

- The National Grid PM would document the release in IMS.

HELP: Contact the local Environmental Representative if you have questions.

EASTERN:	Matt Root	518-227-7508
CENTRAL:	Rich Fox	315-546-4011
WESTERN:	Lisa Montesano	716-479-5339

National Grid **Environmental Guidance**	**Doc No.:**	**EG-502NYN**
	Rev. No.:	**0**
	Page No.	**1 of 3**
	Date:	**03/06/2020**

SUBJECT	**REFERENCE**
Release Clean Up in New York North	**EP-5; Release Response**

Purpose / Objective:

The purpose of this guidance document is to provide instructions in the clean-up of oil and other chemical releases.

Who:

This guidance applies to New York North National Grid personnel who may be assigned to respond and/or clean up a release.

What To Do:

Immediate Actions:

- Secure the area
- Attempt to contain release. Utilize release kit contained on most company vehicles.
- Use absorbent/containment materials to minimize or eliminate the spread of contamination.
- Do not walk through or touch the released material; step away from the release area.
- Using physical barriers, visible warnings (i.e., caution tape, cones, etc.), or other means, restrict access to the release area. Prevent unauthorized persons from entering the area.

***Note:** Large releases should be cleaned up by qualified Hazardous Materials personnel. If the release is large and/or PCB containing, or has reached a water body, contact Regional Control or the Divisional Environmental Engineer for assistance in obtaining help with the clean-up.*

Clean up Requirements:

Assess the release. Determine material released. If hazardous material such as mercury, acid or PCB oil, contact Regional Control for help in securing Hazardous Materials personnel or release contractor.

Don personnel protective equipment (PPE) as necessary. PPE may include, but not be limited to gloves, hardhats, safety glasses, steel toed shoes, coveralls, etc. If working near roadway, appropriate cones shall be placed, and high visibility clothing shall be worn. Follow all Safety procedures.

***Note**: All releases are unique. The clean-up methodology employed will depend upon the nature of the release, the amount released, location, etc. The guidance listed below is meant to be general and not prescriptive.*

Non-PCB oil releases to land:

Non-PCB oil releases include non-PCB labeled transformers, hydraulic oil, diesel fuel and motor oil. Clean up requires removal of visible stained oil from the surface. Soil removal, use of oil sorbent pads, Speedi Dri, and use of a water-based cleaner may all be used for release clean-up.

- Remove all visible traces of oil.
- Document clean-up efforts and provide to divisional environmental engineer.
- Containerize waste in appropriate containers and label.

***Note**: NYS DEC guidance and clean up criteria vary between Regions in NYS. Effort shall be made to clean the area to as close as possible to pre-release conditions.*

Unlabeled transformer oil releases to land:

If an unlabeled transformer has released, obtain a sample of the oil from the unit and conduct a field PCB test using a CLOR-N-OIL kit. Collect an additional sample for laboratory analysis. If results reveal ≥ 50 ppm, waste clean-up debris must be handled as PCB contaminated and labeled as such until laboratory results determine otherwise. If CLOR-N-OIL results reveal < 50 ppm, the oil and waste may be handled as non-PCB until laboratory results are received. If no oil is left in the unit or a sample cannot be obtained, collect oily soil or debris and send to laboratory for analysis.

- Clean up release as noted above.
- Collect additional soil sample or wipe samples to verify clean up.
- Containerize waste and label appropriately.
- Document clean up and provide to divisional environmental engineer.
- Clean up should be completed within 48 hours. Otherwise, fully document reason for the delay.
- Clean up all reusable equipment using rags and/or cleaners as necessary.
- If PCB results confirm that the oil release was PCB contaminated or PCB oil, an EPA Identification Number may be required prior to disposal of material. Contact the Divisional Engineer for assistance.

Divisional Environmental Engineer Contacts

Western	Lisa Montesano	716-479-5339
Central	Rich Fox	315-546-4011
Eastern	Matt Root	518-227-7508

Documentation

Official record of all releases is maintained in the Incident Management System (IMS.)

nationalgrid

National Grid
Environmental Guidance

Doc No.:	**EG-502NYN**
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Date:	**03/06/2020**

SUBJECT	**REFERENCE**
Release Clean Up in New York North	**EP-5; Release Response**

Regional Control centers will initiate the IMS report upon notification from the field and document time of incident, notification from field time, regulatory notification time, release details (material released, amount, location, response measures). Regional control follows National Grid Distribution Control Center System Operations Operating Guide 3.7 "Oil Release Reporting."

If the release is of sensitive or serious nature (based on public exposure, environmental impact, or public relations issue) such as those involving bodies of water or public buildings in addition to immediate notification to the divisional engineer, notify and document in the Shift Supervisor log book.

Document remediation efforts including release area and property items/materials impacted amount of clean up necessary. Submit documentation to Divisional Environmental Engineer.

Any contact with members of the public, emergency agencies or regulatory personnel should be documented by the response crew.

Divisional Environmental Engineers shall maintain and complete the release documentation in the IMS system and ensure release is closed out.

HELP:

Contact the local Environmental Representative if you have questions.

LANDFILL CLOSURE CERTIFICATION REPORT
NYSDEC REGISTRATION NO. 01L12211

PORT OF ALBANY EXPANSION PROJECT
BEACON ISLAND PARCEL
BETHLEHEM, ALBANY COUNTY, NEW YORK

WBE certified company

PREPARED BY:

ATLANTIC TESTING LABORATORIES, LIMITED
22 Corporate Drive
Clifton Park, New York 12065

PREPARED FOR:

Albany Port District Commission
106 Smith Boulevard
Albany, New York 12202

ATL REPORT NO. AT5596CE-32-10-24

October 21, 2024

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Appendices

Appendix A - 6 NYCRR Part 360 Registration
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Appendix C - Site Inspection Forms

1.0 INTRODUCTION

Atlantic Testing Laboratories, Limited (ATL) was retained on behalf of the Albany Port District Commission to prepare a Landfill Closure Certification Report for the Beacon Island site that is impacted with coal ash material and related debris. Recent site redevelopment included landfill reclamation activities, subject to registration through the New York State Department of Environmental Conservation (NYSDEC) pursuant to 6 NYCRR Part 360-15. The site redevelopment was completed between April 2022 and October 2024. Activities related to landfill reclamation work were commenced after submission of a Landfill Reclamation Work Plan in October 2022.

2.0 SITE DESCRIPTION

2.1 Site Location

The subject site is the Beacon Island parcel located to the east of River Road (County Route 144) and along the west side of the Hudson River, in the Town of Bethlehem, Albany County, New York. The subject site is intersected by 42° 36' 11" north latitude and 73° 45' 57" west longitude. The Beacon Island parcel is comprised of approximately 80 acres, and has been redeveloped as part of an expansion for the Port of Albany.

Properties directly north of the subject site include manufacturing and processing facilities and warehouse/distribution facilities, primarily associated with the Port of Albany. Properties directly to the south include an electric power generation facility. An electric transmission line corridor is located adjacent to the west of the subject property, followed by commercial and residential properties located along River Road. The Hudson River is immediately east of the subject site. Properties in the immediate vicinity of the subject site are zoned as industrial. Residential, rural, and commercial zoned properties exist further out.

A Site Location Map, showing the location of the subject site, is attached as **Figure 1**. An Aerial View of Existing Site Conditions (Photograph Dated October 15, 2024) is provided as **Figure 2**.

2.2 Solid Waste Management Facility Registration

The Beacon Island site in Bethlehem, New York, has a current 6 NYCRR Part 360 registration under NYSDEC Registration No. 01L12211. This registration has an effective date of October 28, 2022 and expiration date of October 27, 2027. A copy of the registration is included as **Appendix A**.

3.0 LANDFILL AND CONSTRUCTION OPERATIONS

3.1 Pre-Construction Conditions

Available historical information and data from previous site investigations indicate that fill and ash materials have been placed at varying depths in areas of the subject site. Furthermore, available information indicates that placement of ash contaminated wastes ceased some time previous to 1970. Prior to landfill reclamation activities at the site, subsurface investigations were performed to assess the extents of the ash materials. **Figure 3**, Aerial Extent of Landfill Material - Pre-Construction, shows the approximate extents where ash materials were encountered prior to site redevelopment by the Port of Albany.

3.2 Landfill Reclamation Work Plan

A Landfill Reclamation Work Plan (LRWP) was prepared for the facility, to address criteria described in 6 NYCRR Part 363-11.4 for landfill reclamation activities. The LRWP included a summary of site investigations for identifying the approximate extents of landfill material and evaluating soil quality, groundwater, and soil vapor characteristics, along with feasibility study and planned reclamation work activities. The LRWP was prepared by ATL on behalf of the Port of Albany, and is referenced as ATL Report No. AT5596CE-06-10-22, dated October 20, 2022.

The primary reclamation work plan components included the following:

- Site preparation
- Disturbance of landfill materials
- Site cap
- Soil Management Plan (earthwork, staging procedures, imported fill, dust control, community air monitoring plan (CAMP), work zone air monitoring)
- Decommissioning of monitoring wells
- Stormwater and leachate management
- Closure certification report

A modification to the LRWP was submitted in December 2022 (reference ATL Report No. AT5596CE-06-10-22, dated December 1, 2022) to describe areas requiring excavation that were not described in the original work plan. This modification was provided for clarification of planned work activities, with additional portions and quantities of the coal ash materials to be relocated.

Subsequent to submitting the LRWP, NYSDEC requested a Groundwater Monitoring Plan to specify installation of multiple groundwater wells along the banks of the Hudson River. These would be used to monitor potential contaminant migration resulting from construction activities over the coal ash landfill. The Groundwater Monitoring Plan was prepared by ATL on behalf of Port of Albany (reference ATL Report No. AT5596CE-08-11-22 Revision 1, dated December 2, 2022).

Sections 3.3 through 3.9 provide further description of work that has been completed relative to the primary LRWP components.

3.3 Site Preparation

Site preparation activities during construction and redevelopment at the Beacon Island site were pertinent to ensuring the site would be suitable for installation and maintenance of a site cap over the coal ash materials. **Table 1** – Status of Site Preparation Work provides a summary of site preparation items described in the LRWP, and status of performance during construction and long-term. As indicated in **Table 1**, site preparation items were completed pursuant to conditions described in the LRWP, with some items having continued applicability through long-term operations at the site.

3.4 Disturbance of Landfill Materials

As indicated in the LRWP and subsequent modification, disturbance of landfill materials would include removal of a portion of the coal ash from approximately 6.83 acres of the site and relocation of this material to areas covering approximately 8.92 acres of the site. The estimated volume of coal ash to be relocated was 21,900 cubic yards. Disturbance of landfill materials were limited to the prescribed areas, with no other areas requiring excavation of coal ash for relocation.

Petroleum-contaminated soil was encountered at 2 locations during construction and affected materials were excavated and transported off-site for disposal at a permitted facility. One area of concern included petroleum-impacted soil discovered at the location of a monitoring well (MW-3) that was installed at the subject site in December 2022. The spill was reported under NYSDEC Spill No. 2206029, and the completed excavation for affected soil material measured approximately 25 feet in radius, with depth ranging from 4 to 8 feet. Post-excavation soil samples were collected for analysis of volatile organic compounds (VOC) and semi-VOC. A summary of the sampling and analysis results is provided in a Post-Excavation Soil Sampling and Analysis Report prepared by ATL (reference ATL Report No. AT5752CE-01-04-23, dated April 11, 2023).

The second area of concern with petroleum-contaminated soil was discovered at locations of subsurface investigation borings completed on October 4 and 5, 2022. This area was reported under NYSDEC Spill No. 2208519, and the completed excavation for affected soil material measured approximately 100 feet by 175 feet, with depths ranging between 2 to 8 feet. Post-excavation soil samples were collected for analysis of VOC and semi-VOC. A summary of the sampling and analysis results is provided in a Post-Excavation Soil Sampling and Analysis Report prepared by ATL (reference ATL Report No. AT5752CE-02-04-23 Revision 1, dated June 29, 2023).

Excavation work for the relocation of ash materials and removal of petroleum-contaminated soil for disposal was performed pursuant to conditions of the Soil Management Plan for the site (reference Section 3.6).

3.5 Site Cap

The LRWP described conditions for installation of a site cap over areas of the site where coal ash would remain and where portions of coal ash would be relocated to. The following criteria were established for the site cap.

- A minimum of 2 feet of soil cover will be placed above the ash material.
- The upper 6 inches of the soil cover will be suitable to sustain growth of appropriate vegetation at the ground surface in areas that will not be covered with impervious surfaces.
- Certain areas of the subject site will have buildings, pavement, and concrete pads that will serve as an additional layer of site cap.
- The upper 2 feet of the soil cover will not have concentrations of contaminants that exceed the Restricted Residential Soil Cleanup Objectives (SCO) set forth in 6 NYCRR Part 375-6.
- Imported fill that is placed at a depth below the upper 2 feet of soil cover will not have concentrations of contaminants that exceed the Commercial SCO set forth in 6 NYCRR Part 375-6.
- A demarcation layer will be provided between the soil cover layer and underlying impacted soil. Subsequent to site grading, and prior to placement of imported material for the site cap, a filter fabric will be placed to provide separation of these layers and will serve as the demarcation layer.

- In the event that the soil cover system is breached, penetrated, or temporarily removed, restoration to original conditions (or equivalent) will be performed.
- Areas with a soil cover will be inspected at least annually, to assess existing conditions and determine if any restoration or repairs are necessary. Inspections will also be performed after severe weather events or significant site operations that may have adversely affected the soil cover system.

The site cap has been installed during the course of construction, with fill material placed over areas containing the coal ash material and a demarcation layer (filter fabric) placed between the graded pre-existing materials and the imported fill. Section 4.2 provides additional details pertaining to as-built conditions for the site cap.

3.6 Soil Management Plan

A Soil Management Plan was prepared for the site to address areas that are impacted with ash material and related debris, in addition to describing methods to be used for the handling and management of areas impacted by petroleum/chemical spills, if encountered. The Soil Management Plan (reference ATL Report No. AT5596CE-05-10-20 Revision 2, dated October 20, 2022) was incorporated into the LRWP, with pertinent components including earthwork, staging procedures, importation of fill, dust control, community air monitoring, and work zone air monitoring. The Soil Management Plan was referenced during construction and site redevelopment work, and shall continue to be applicable for future site management and long-term maintenance and monitoring.

3.7 Decommissioning of Monitoring Wells

Prior to development of the LRWP, there were 6 pre-existing monitoring wells on the subject site. The planned redevelopment and site construction were not conducive to maintaining these monitoring wells in a stable condition for future use. As such, the pre-existing monitoring wells were decommissioned during the site redevelopment work. New monitoring wells were installed along the banks of the Hudson River to facilitate groundwater sampling and analysis prior to, during, and after construction. Reference Section 3.9 for additional details pertaining to the groundwater monitoring.

3.8 Stormwater and Leachate Management

Construction at the site included installation of closed drainage networks with ultimate discharge into the Normans Kill or Hudson River. These drainage networks were constructed as described on the project plans, SWPPP, and LRWP, and will serve to collect site stormwater and transfer away from areas with the site cap.

3.9 Groundwater Monitoring

Groundwater monitoring has been performed as part of the site construction and redevelopment. The Groundwater Monitoring Well Plan (reference ATL Report No. AT5596CE-08-11-22 Revision 1, dated December 2, 2022) specified the installation of 5 monitoring wells along the side of the site adjacent to the Hudson River. Groundwater sampling events were initiated in December 2022, prior to commencement of construction activities, and have been performed on a monthly basis through the construction work. Post-construction events are planned at a frequency of every 6 months until NYSDEC allows for discontinuation.

4.0 LANDFILL CLOSURE

4.1 Closure Methods

Closure methods were completed in general conformance with the LRWP, as described in Section 3 of this report, and were performed during the Beacon Island site construction and redevelopment project. Construction inspection and observation records, testing reports, material and product data, and other documents pertaining to the project are maintained by the Port of Albany and can be made available upon request.

4.2 As-Built Conditions

Figure 4 – Landfill Reclamation As-Built Plan provides an overview of existing site conditions, based on construction work completed for the landfill reclamation activity. **Figure 5** – Landfill Reclamation Site Cap (Gravel Thickness) shows details pertaining to the gravel thickness for the constructed site cap system. The as-built conditions generally correspond with the conditions described in the LRWP.

4.3 Inactive Facility Notification and Annual Report

6 NYCRR Part 360.21 includes conditions for providing notification to NYSDEC of closure activities, and submitting an annual report. Operations at the registered facility were limited to the landfill reclamation activities performed during construction, which pertained only to existing coal ash materials on the site. No additional wastes were received at the facility.

Appendix B includes an Inactive Solid Waste Management Facility or Activity Notification Form to serve as notification of closure and certification that the facility is no longer operational. Furthermore, pursuant to conditions stated on the form, the owner/operator relinquishes their NYSDEC registration and retains no other permit, registrations, or licenses related to the landfill reclamation activity.

It is noted that an annual report template for landfill reclamation activities is not available under Solid Waste Facilities Forms on the NYSDEC website. It is also noted that the intent of an Annual Report is to identify the quantities of materials received or processed at a solid waste management facility. In consideration of no available annual report form for landfill reclamation and the subject facility not receiving any wastes, it is not anticipated that an annual report is necessary for facility closure. If an annual report is needed or required, and an applicable annual report form is provided to the Albany Port District Commission by NYSDEC, such report can be prepared and submitted.

5.0 POST-CLOSURE OPERATIONS

5.1 Site Access/Site Security

Fencing and gates will be maintained around the perimeter of the subject site, and will serve to restrict access to authorized personnel. The general public will not have direct access to the site.

5.2 Future Site Work

Pending site work includes construction of 4 buildings, as has been incorporated into the Port of Albany's site redevelopment plans. These remaining construction activities will not include significant disturbance or alteration to the site cap or underlying coal ash materials. No other work impacting the site cap is anticipated for the foreseeable future. In the event that disturbance to the site cap materials is necessary for future site management or maintenance activities, the Soil Management Plan shall be referenced for protocol to be implemented.

5.3 Groundwater Monitoring

The Groundwater Monitoring Plan for the site includes a requirement for monthly groundwater sampling and analysis events during construction and post-construction events at a frequency of every 6 months until NYSDEC allows for discontinuation. The groundwater monitoring will continue at the stated frequencies.

5.4 Long-Term Maintenance and Monitoring

5.4.1 Site Cap Maintenance

The site cap shall be maintained intact to ensure on-site coal ash materials remain covered and there are no direct exposure risks. As specified in the Soil Management Plan for the facility, in the event that the soil cover system is breached, penetrated, or temporarily removed, restoration to original condition (or equivalent) shall be completed.

5.4.2 Annual Site Inspections

A comprehensive site-wide inspection shall be performed annually, to assess and verify the soil cover system remains in satisfactory condition. These annual inspections must be performed when the ground surface is visible (i.e., no snow cover), and shall be conducted by a representative who meets one of the following criteria:

- Qualified environmental professional per definition in 6 NYCRR Part 375
- A Professional Engineer (PE) or Professional Geologist (PG) who is licensed and registered in New York State
- A qualified person who reports directly to a PE or PG who is licensed and registered in New York State

Inspections shall be documented on a site-specific form, established for compiling sufficient information to assess the following, at a minimum:

- General site conditions at the time of the inspection
- An evaluation of the condition and continued effectiveness of the soil cover system
- Whether permanent stormwater management systems are working as designed.
- A review of site records pertinent to the soil cover system, and confirmation these are up-to-date
- Compliance with requirements of Soil Management Plan (reference LRWP)

A sample form for use during the annual inspection is included in **Appendix C**.

5.4.3 Severe Weather Event/Significant Site Operations Inspections

A comprehensive site-wide inspection shall be performed after the occurrence of a severe weather event or significant site operations, considered to have the potential for damage or disturbance to the site cap and/or stormwater management features. The site SWPPP also includes provisions for inspections after a severe weather event. The sample form included in **Appendix C** can be used for documentation of these inspections, if not otherwise covered under inspection documentation described in the SWPPP.

5.5 Recordkeeping

Records and documentation pertinent to the site construction, landfill reclamation activity, and post-closure operations should be maintained and accessible for review upon request. Following is a list of the types of records and documentation that should be maintained:

- Site investigation reports
- LRWP and modification letter
- Soil Management Plan (included with LRWP via appendix)
- Groundwater Monitoring Well Plan and groundwater sampling and analysis reports
- Landfill Closure Certification Report
- Construction inspection and observation reports
- Construction-related testing reports pertinent to components of the landfill reclamation activities (e.g., community air monitoring reports, soil sampling and analysis reports, soil compaction testing)
- Construction-related material/product data pertinent to components of the landfill reclamation activities (e.g., site cap materials, stormwater system components)
- As-built plans (included with Landfill Closure Certification Report via appendix)
- Annual site inspection and severe weather event/significant site operations inspection reports (minimum of 3 years recommended)
- Records of any repairs or modifications to site cap

TABLES

Table 1 – Status of Site Preparation Work

Table 1 – Status of Site Preparation Work

Site Preparation Activities Described in LRWP	Performance During Construction	Long-Term Status
All wooded areas on the property to be cleared and grubbed, with the exception of trees along the shoreline	Clearing and grubbing have removed wooded areas, with trees along the shoreline remaining	Trees along the shoreline are to remain for the foreseeable future. Trees are not planned for planting in other areas, and as such, should not impact the site cap and underlying coal ash materials.
Approximately 0.81 acre of wetland area to be impacted at the northwest section of the site, with mitigation measures pursuant to the Stormwater Pollution Prevention Plan (SWPPP)	Wetland impacts and mitigation completed pursuant to project plans and permits.	N/A
Rail cars and rail line to be removed from the site	Rail cars and rail line were removed	N/A
Grading to include excavation at designated areas of cuts; however, majority of site will receive fill	Site grading has been completed, with portions of coal ash relocated via planned cuts and fill	No further site excavation is currently planned. In the event that localized excavations need to be performed in the future, work shall be performed per conditions of the Soil Management Plan
Aggregate surcharging to be applied to footprints of each of 4 buildings to be constructed, in addition to a utility crossing area	Aggregate surcharging has been completed for the building footprints and utility crossing area.	Buildings are yet to be constructed. The landfill materials, with site cap, are not expected to be significantly impacted or altered during future building construction.
Entrance driveway to be constructed on the south end of the west side of the site	Entrance driveway has been constructed.	The entrance driveway will remain as permanent access to the site.
Erosion and sediment control to be implemented per SWPPP conditions	SWPPP was implemented during construction for erosion and sediment control, with routine inspections. Maintenance or modifications provided as needed.	The erosion and sediment control plan employed temporary and permanent measures. Temporary measures will not be applicable to long-term site operations. Permanent measures include riprap outlet protection, soil stabilization, check dams, and diversion dikes, and will require continued maintenance.
Site stormwater management systems to be installed pursuant to SWPPP conditions	SWPPP was implemented during construction for site stormwater management.	Permanent stormwater management features will remain at the site. Albany Port District Commission will be responsible for maintenance of post-construction stormwater management facilities, including jellyfish filter (12 units at 7 locations), stormwater ponds (2), infiltration basins (2), dry swales (2), and stormwater collection and conveyance systems.
A gated security fence to be installed around the site	An 8-foot chain link security fence, with lockable gates at entrances, was installed for the construction site.	A security fence will be maintained for the facility, which will serve to restrict access to the site.

FIGURES

Figure 1 – Site Location Map
Figure 2 – Aerial View of Existing Site Conditions (Photograph Dated October 15, 2024)
Figure 3 – Aerial Extent of Landfill Material - Pre-Construction
Figure 4 – Landfill Reclamation As-Built Plan
Figure 5 – Landfill Reclamation Site Cap (Gravel Thickness)

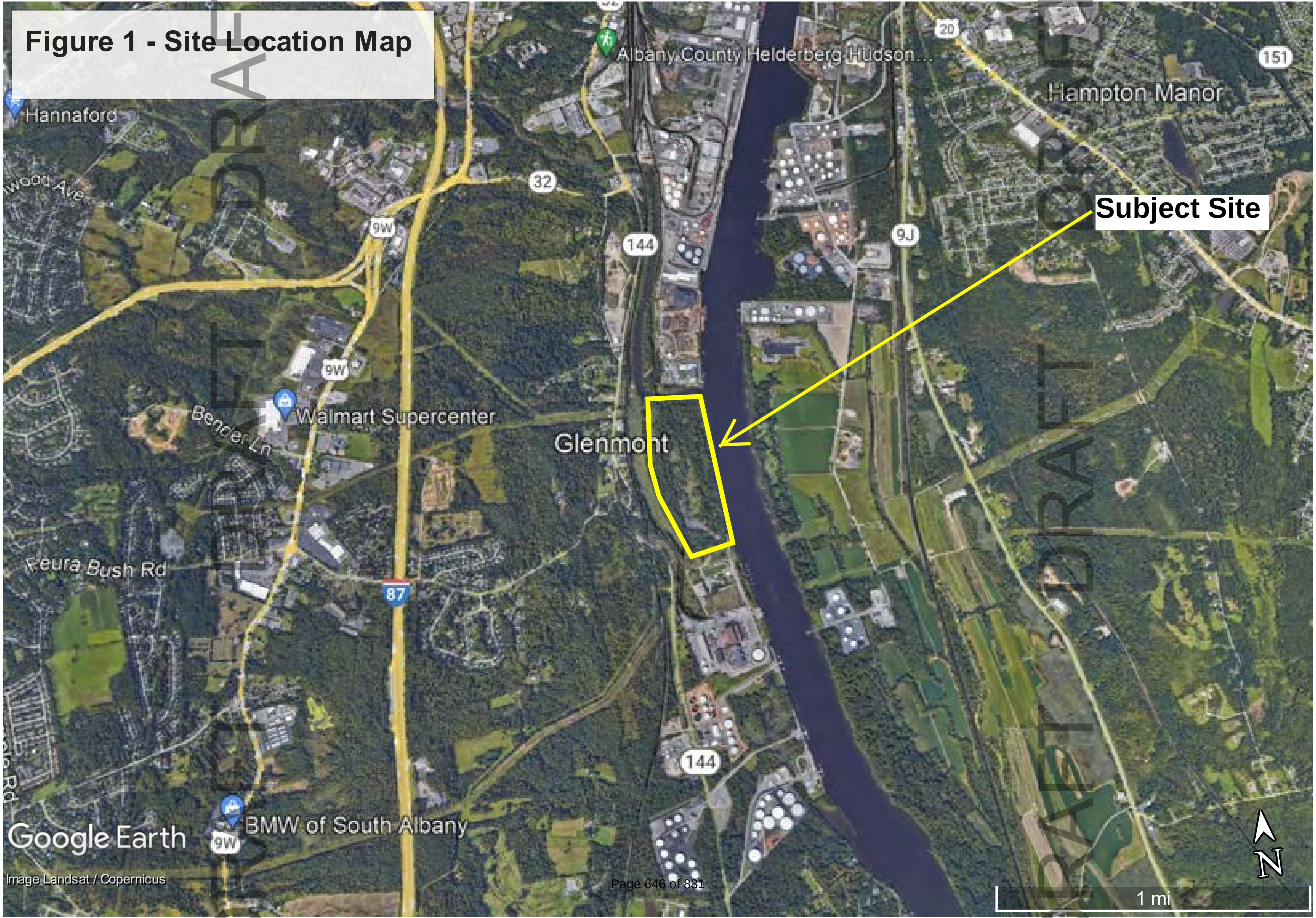


Figure 1 - Site Location Map

AERIAL SITE PHOTO FROM OCTOBER 15, 2024.

0 150 300 450 FT
SCALE

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
PERMIT FIGURE

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT EXPANSION SITE - SITE PREPARATION

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	1"=150'
DATE	OCTOBER 18, 2024
PROJECT	18641.00

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
LANDFILL RECLAMATION
AERIAL PHOTO

DRAWING NUMBER
FIGURE 2

OF

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES - SITE PREPARATION\LANDFILL REC - FIGURE 2 AERIAL.DWG

LEGEND :

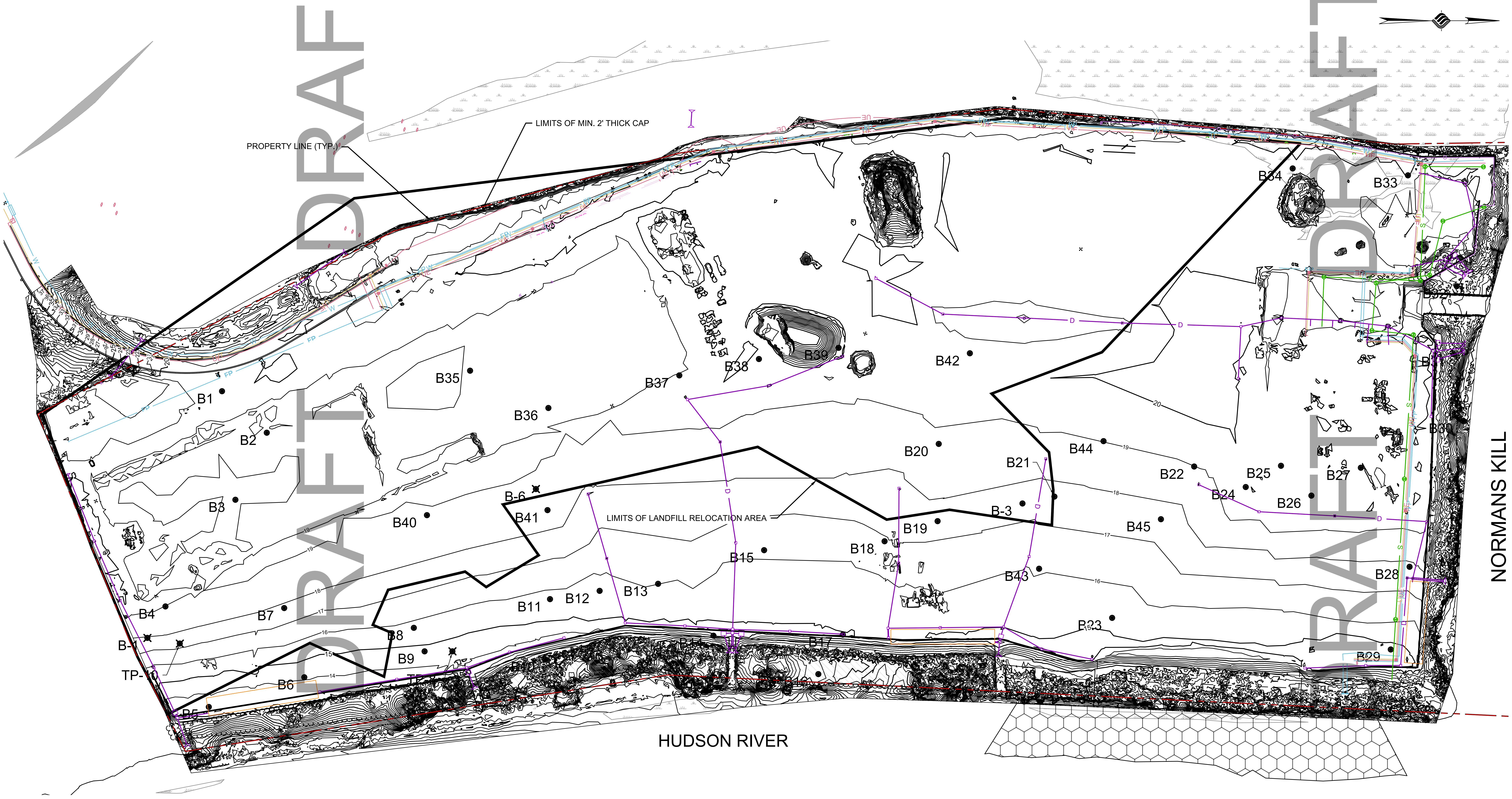
🟥 Approximate Aerial Extents of Coal Ash Material

AERIAL EXTENT OF LANDFILL MATERIAL - PRE-CONSTRUCTION

Beacon Island Parcel
Bethlehem, Albany County, New York

Drawn By:	Drawing:	Scale:	Project No.:	Date :
CJD	Fig. 2	As Noted	AT5596	October 2022

ATLANTIC TESTING LABORATORIES, Limited
Albany, NY Binghamton, NY Canton, NY Elmira, NY Poughkeepsie, NY
Plattsburgh, NY Rochester, NY Syracuse, NY Utica, NY Watertown, NY
WBE Certified Company
www.AtlanticTesting.com



AS-BUILT BASED ON AN AERIAL SURVEY FROM OCTOBER 15, 2024.

0 150 300 450 FT

SCALE

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
PERMIT FIGURE

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT EXPANSION SITE - SITE PREPARATION

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	1"=150'
DATE	OCTOBER 18, 2024
PROJECT	18641.00

CONCEPTUAL
FIGURE
NOT FOR
CONSTRUCTION

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
LANDFILL RECLAMATION
AS-BUILT PLAN

DRAWING NUMBER
FIGURE 4

OF

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES - SITE PREPARATION\LANDFILL REC - FIGURE 4 AS-BUILT.DWG

CURRENT GRAVEL CAP THICKNESS			
Number	Minimum Thickness	Maximum Thickness	Color
1	2.00	>2.00	🟩

COMPARISON OF THE BOTTOM OF GRAVEL SECTION TO THE DRONE SURFACE DATED OCTOBER 15, 2024.

0 150 300 450 FT
SCALE

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
PERMIT FIGURE

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT EXPANSION SITE - SITE PREPARATION

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	1"=150'
DATE	OCTOBER 18, 2024
PROJECT	18641.00

CONCEPTUAL
FIGURE
NOT FOR
CONSTRUCTION

DRAWING TITLE
LANDFILL RECLAMATION
SITE CAP
(GRAVEL THICKNESS)

DRAWING NUMBER
FIGURE 5

OF

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES - SITE PREPARATION\LANDFILL REC - FIGURE 5 CAP THICKNESS.DWG

APPENDIX A

6 NYCRR Part 360 Registration

NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION

Division of Materials Management, Region 4
1130 North Westcott Road, Schenectady, NY 12306-2014
P: (518) 357-2045 | F: (518) 357-2398
www.dec.ny.gov

October 28, 2022

Port of Albany
106 Smith Boulevard
Albany, NY 12202 United States

Re: Beacon Island - Bethlehem

Landfill reclamation - registration   Registration # 01L12211

Dear Port of Albany:

Enclosed is the validated copy of your 6 NYCRR Part 360 registration, which becomes effective on October 28, 2022 and expires on October 27, 2027.

This letter only acknowledges receipt of your registration form and does not, in any way, verify that the information which you provided on the form is true or correct.

You are reminded that 6NYCRR Part 360 contains various requirements that must be followed to warrant your facility's continued status as a registered facility. Enclosed is a list of general requirements for a registered facility described in Section 360.15.

This registration does not exempt or preclude you from complying with any other applicable federal, state, or local laws, rules or regulations. If you have any questions regarding this matter, please contact me at the above telephone number.

Sincerely,

Jonathan Whitcomb
Assistant Engineer

NEW YORK STATE OF OPPORTUNITY | Department of Environmental Conservation

DIVISION OF MATERIALS MANAGEMENT

REGISTRATION FOR A SOLID WASTE MANAGEMENT FACILITY

THIS IS NOT A UPA PERMIT

DATE ISSUED: 10/28/2022
DATE EXPIRES: 10/27/2027

1. FACILITY

Beacon Island
River Road
Bethlehem
Albany (*county*)

2. FACILITY OWNER

Port of Albany
106 Smith Boulevard
Albany
NY 12202

3. FACILITY OPERATOR

Port of Albany
106 Smith Boulevard
Albany
NY 12202

4. SITE OWNER

Port of Albany
106 Smith Boulevard
Albany
NY 12202

5. REGISTERED ACTIVITY

Types	Authorized Waste
Landfill reclamation - registration (01L12211) [363]	N/A - Landfill reclamation of existing coal ash

Maximum Throughput Limit: 0 cubic yards/day

Storage: N/A

6. OPERATIONS SCHEDULE - Normal schedule of operation
N/A

7. NAME(S) OF ALL MUNICIPALITIES SERVED - Albany(Co), Bethlehem(T)

N.Y.S.D.E.C.
REGION 4 HEADQUARTERS
1130 N. WESTCOTT ROAD
SCHENECTADY, NY 12306-2014

This registration does not exempt or preclude you from complying with any other applicable federal, state, or local laws, rules or regulations.

APPENDIX B

Inactive Solid Waste Management Facility or Activity Notification Form

Division of Materials Management
New York State Department of Environmental Conservation

INACTIVE SOLID WASTE MANAGEMENT FACILITY OR ACTIVITY NOTIFICATION FORM

FACILITY NAME: Beacon Island

FACILITY ADDRESS: River Road

FACILITY CITY: Bethlehem **STATE:** NY **ZIP CODE:** 12077

TYPE OF INACTIVE FACILITY OR ACTIVITY: (Check all applicable boxes)

- ☐ **Anaerobic Digestion – permit**
- ☐ **Anaerobic Digestion – registration**
- ☐ **C&D Processing – permit**
- ☐ **C&D Processing – registration**
- ☐ **Composting – Source Separated Organic Waste – permit**
- ☐ **Composting – Source Separated Organic Waste – registration**
- ☐ **Composting – Yard Waste – permit**
- ☐ **Composting – Yard Waste – registration**
- ☐ **Composting/other Processing – Biosolids/other**
- ☐ **Household Hazardous Waste**
- ☐ **Land Application – Biosolids/Septage/other – permit**
- ☐ **Land Application – Nonrecognizable Food Processing Waste – registration**
- ☐ **Land Application – Septage - registration**
- ☐ **Landfill – Construction & Demolition Debris**
- ☐ **Landfill – Industrial/Commercial**
- ☐ **Landfill – Land Clearing Debris**
- ☐ **Landfill – Long Island**
- ☐ **Landfill – Municipal Solid Waste**
- ☐ **Mobile Vehicle Crushers**
- ☐ **Municipal Waste Combustor**
- ☐ **Recyclable Handling & Recovery**
- ☐ **Regulated Medical Waste – Radiopharmacy**
- ☐ **Regulated Medical Waste – Onsite Treatment**
- ☐ **Regulated Medical Waste – Commercial Treatment**
- ☐ **Regulated Medical Waste – Transfer Station**
- ☐ **Storage – Biosolids/Septage – permit**
- ☐ **Storage – Nonrecognizable Food Processing Waste**
- ☐ **Storage – Septage – registration**
- ☐ **Transfer Station – permit**
- ☐ **Transfer Station – registration**
- ☐ **Vehicle Dismantling Facility**
- ☐ **Waste Tire Storage – Dealer**
- ☐ **Waste Tire Storage – New Product Manufacturing**
- ☐ **Waste Tire Storage – Onsite Energy Recovery**
- ☐ **Waste Tire Storage – permitted**
- ☐ **Waste Tire Storage – Retreader**
- ☒ **Other** Landfill Reclamation

DEC ACTIVITY CODE(S) OR REGISTRATION NUMBER(S): 01L12211

FACILITY COUNTY: Albany

NYSDEC REGION #: 4

This document certifies that the type of facility or activity identified above is no longer operational. The owner/ operator relinquishes their NYSDEC permit/registration and retains no other permit, registrations, or licenses related to the identified activity. It is recognized that in order to resume operation, a new permit application or registration form must be submitted to the Department for processing and approval. This notification does not excuse the facility from any closure, post-closure, or other requirements identified in 6 NYCRR Part 360.

I hereby affirm under penalty of perjury that information provided on this form was prepared by me or under my supervision and direction and is true to the best of my knowledge and belief, and that I have the authority to sign this form pursuant to 6 NYCRR Part 360. I am aware that any false statement made herein is punishable as a Class A misdemeanor pursuant to Section 210.45 of the Penal Law.

Name (Print or Type) ____________
Title (Print or Type) ____________
(_____) ____________ Phone Number

Address ____________
City ____________
State and Zip Code ____________

Signature ____________
Date ____________

APPENDIX C

Site Inspection Form

Site Cap/Site Cover Inspection

Facility: Beacon Island
River Road, Bethlehem, Albany County, New York

Date of Inspection:		Type of Inspection:	☐ Annual ☐ Severe Weather Event ☐ Significant Site Operations
Weather Conditions:			

Site Cap/Site Cover Conditions

Evidence of significant differential settlement (i.e., depressions, water ponding, sloughing)?	
Evidence of erosion?	
Vegetation established and maintained to minimize erosion?	
Vegetative layers in good condition?	
Evidence of woody vegetation?	
Evidence of damage due to burrowing animals?	
Evidence of damage or disturbance by people or equipment?	
Evidence of excavation and material replacement?	
Evidence of spills?	

Stormwater/Leachate Components

Stormwater components functional?	
Drainage features free of obstruction?	
Evidence of sediment buildup in drainage components?	
Evidence of leachate breakouts or seeps?	

Records Review	
Records of repairs or modifications to site cap since previous inspection?	
Records of activities related to Soil Management Plan since previous inspection?	
Records of groundwater monitoring since previous inspection?	
Records of stormwater inspections or maintenance since previous inspection?	
Other records?	

Notes

Geotechnical Engineering Report

Proposed Marmen Manufacturing Facility
Port of Albany, New York

February 4, 2022
Terracon Project No. JB215020

Prepared for:
McFarland-Johnson, Inc.
Saratoga Springs, New York

Prepared by:
Terracon Consultants - NY, Inc.
Albany, New York

February 4, 2022

McFarland-Johnson, Inc.
66 Railroad Place – Suite 402
Saratoga Springs, NY 12866

Attn: Mr. Steven Boisvert, P.E.
p: (518) 580-9380
e: sboisvert@mjinc.com

Re: Geotechnical Engineering Report
Proposed Marmen Manufacturing Facility
Port of Albany, New York
Terracon Project No. JB215020

Dear Mr. Boisvert:

We have completed the Geotechnical Engineering services for the referenced project. This study was performed in general accordance with Terracon proposal no. PJB215020 and the agreement for subconsultant professional services between McFarland-Johnson and Terracon entered into on or about June 1, 2021. This report presents the findings of the subsurface exploration and provides geotechnical recommendations concerning earthwork and the design and construction of foundations, floor slabs and pavements for the project.

Terracon submitted a draft geotechnical report for this project in October 2021, and we understand the design team has completed their review of the draft report. This final report has been prepared cognizant of comments made through the review and evaluation process and has been updated and/or revised accordingly.

We appreciate the opportunity to be of service to you. If you have any questions concerning this report or if we may be of further service, please contact us at your convenience.

Sincerely,
Terracon Consultants-NY, Inc.

John S. Hutchison, P.E.
Senior Engineer

Joseph Robichaud, Jr., P.E.
Principal / Office Manager

Fred Dente, P.E.
Independent Consultant

REPORT TOPICS

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Note: This report was originally delivered in a web-based format. **Orange Bold** text in the report indicates a referenced section heading. The PDF version also includes hyperlinks which direct the reader to that section and clicking on the ***GeoReport*** logo will bring you back to this page. For more interactive features, please view your project online at client.terracon.com.

ATTACHMENTS

EXPLORATION AND TESTING PROCEDURES
SITE LOCATION AND EXPLORATION PLANS
EXPLORATION RESULTS
SUPPORTING INFORMATION

Note: Refer to each individual Attachment for a listing of contents

Geotechnical Engineering Report

Proposed Marmen Manufacturing Facility
Port of Albany, New York

Terracon Project No. JB215020
February 4, 2022

INTRODUCTION

This report presents the results of our subsurface exploration and geotechnical engineering services performed for the proposed Marmen manufacturing facility on Beacon Island at the Port of Albany, New York. The purpose of these services is to provide information and geotechnical engineering recommendations relative to:

- Subsurface soil conditions
- Groundwater conditions
- Site preparation and earthwork
- Seismic site classification
- Slope stability
- Foundation design and construction
- Floor slab design and construction
- Pavement design and construction
- Retaining wall design and construction
- Frost considerations

The geotechnical engineering scope of services for this project included the advancement of 18 conventional test borings to depths ranging from 30.1 to 165.0 feet below existing site grades, completion of 12 test pits to depths between 11.5 and 16 feet, site reconnaissance by a geotechnical engineer, laboratory testing of selected soil samples, and preparation of this summary report.

Previous subsurface and/or geotechnical investigations have been completed by Dente/Terracon and others on the Beacon Island site. These include:

- Environmental Subsurface Investigation and Soil Sampling – ATL, October 2020
- Subsurface Exploration Data Report (for Wharf) – CME Associates, October 2020
- Subsurface Investigation (at Bridge Site) – ATL, May 2020
- Supplemental Geotechnical Report – Dente/Terracon, July 2017
- Preliminary Geotechnical Evaluation – CME Associates, April 2017
- Phase II Environmental Site Assessment – Bergmann Associates, April 2017

Information from these previous studies has been considered in the preparation of this report and is included herein where referenced and as applicable.

Note that an additional six boreholes were included in a contingency work scope which was ultimately not carried out, as the information from the base scope boreholes coupled with that from the previous investigations at the site was ultimately judged sufficient for the purposes of this study.

Maps indicating the site and test boring locations are included as the attached **Site Location** and **Exploration Plan**, respectively.

SITE CONDITIONS

Existing conditions at the site are summarized in the following table:

Item	Description
Parcel Information	The project site is located in the town of Bethlehem, New York along the west side of the Hudson River, south of the currently developed portion of the Port of Albany and the point at which the Normanskill Creek empties into the river. The site is about 80 acres in size, with geographic coordinates at the approximate center of the parcel at 42.6038° N, 73.7656° W.
Existing Improvements	None, other than an abandoned railroad spur.
Current Ground Cover	Woods and heavy vegetation currently comprise the ground cover across much of the site, although some trails and traveled ways have been established in places. A clearing exists at the south end of the site.
Existing Topography	Topographic mapping provided for our use indicates that existing landside grades currently range between elevations of about 7 and 21 feet, and slope down accordingly along the banks of the tidally influenced river and creek where mean high water level is reportedly elevation 3.8 feet.
Geology	NYS geologic mapping indicates alluvial deposits in the site locale. Previous subsurface investigations in the area indicate the site is mantled with fill materials and river sediments, followed in sequence with depth by alluvial deposits, glaciolacustrine silt and clay, glacial till and ultimately shale bedrock.

The site is situated in an area once occupied by Beacon Island and a portion of Cabbage Island in the Hudson River, along with side channels of the river that separated the islands from both the mainland and from one another. Review of available historical topographic and aerial imaging reveals that previously submerged portions of the site have been filled over the last 100 years or so, in effect joining the site with the mainland.

As has been described in the previously referenced reports, much of this filling occurred through the placement of waste coal ash from the power generating station just south of the site. The plant was coal fired upon its construction in the early 1950s until about 1970, when its boilers were converted to use fuel oil and later natural gas. Waste coal ash during the plant's coal burning years was disposed of on the project site, primarily on the site's west side and at its south end. The method of placement of the coal ash is unknown with certainty, but is believed to have been transported in bulk and pushed/tracked into place as opposed to hydraulically placed.

PROJECT DESCRIPTION

General

As we understand it, the project entails construction of a new industrial facility where off-shore wind turbine supports will be manufactured. In general, this will involve the fabrication of large cylindrical tower sections and transition pieces from flat steel stock. Raw material will arrive at the existing Port of Albany north of the site and will be transported to the site via a new bridge which is to be built across the Normanskill Creek (we have addressed the bridge in a separate geotechnical report). Finished product will be shipped out from a wharf to be constructed near the site's northeast corner (note that the wharf is being designed by others and is not addressed herein).

The facility will be comprised of four separate buildings (Buildings A thru D), along with a gravel surfaced yard area for the storage/staging of finished tower sections and transition pieces prior to shipment from the wharf. The function and relative size of each proposed building are outlined as follows:

- Building A – Plate Preparation and Welding (291,617 sq.ft.)
- Building B – Welding Finishing (89,074 sq.ft.)
- Building C – Blast-Metallization-Paint (142,371 sq.ft.)
- Building D – Internal Assembly/Finishing (67,217 sq.ft.)

Plans call for these to be single-story, high-bay, slab-on-grade buildings with pre-fabricated metal superstructures. No below grade levels are planned, although one or more service pits up to 8 feet in depth will be included in Buildings A and C. The buildings and some areas about their exterior will include rails embedded in the slabs to facilitate production flow and material transport with tower rotators and transfer cars on the rails. The buildings will also feature overhead cranes for picking and moving materials and equipment about their interiors.

In the gravel surfaced yard area, the tower sections will be staged/stored horizontally, and the transition pieces will be staged/stored vertically. The fabricated product will be moved about using large reach stackers and self-propelled modular transporters (SPMTs) as described below. Curbs will in general define the limits of the yard area, beyond which grades will slope down to the river or creek.

Anticipated Loads

Generally speaking, the products to be manufactured at the facility, the materials from which they will be fabricated, and the equipment required to move these items about are all rather large and heavy. Marmen has furnished a load case document outlining a number of anticipated loading conditions associated with the anticipated material handling and plant operations. These include:

Load Case	Description
A	Tower rotator on rails
B	Transfer car on rails
C	Steel shells on slab
D	Metallization transfer car
E	Plate trailer, 130,000 lb. tandem axle
F	36,000 lb. capacity forklift
G	70,000 lb. capacity forklift
H	HLM 3500 reach stacker (loaded)
J	HLM 3500 reach stacker (unladen)
K	Tower section and transition piece storage

As we understand it, load cases A thru D in the table above represent those which will act on rails embedded in the building or exterior slabs, or those which will be imparted on the slabs themselves, whereas load cases E thru J may act on either the slabs or on the gravel surfaced yard area. Load case K represents storage of the manufactured product which will take place only in the yard area east of the buildings.

For the purposes of this evaluation, we understand that combined live and dead loads within the building and exterior slab areas will not exceed 600 pounds per square foot (psf) when aggregated across a given building (or slab) footprint. Individual building column loads, when coupled with crane loads, are not expected to exceed 256 kips at Buildings B thru D. At Building A this load combination is anticipated to be upwards of 899 kips, or in the most extreme case 1365 kips assuming maximum snow, crane and operational loads all coinciding which, in the event this were to occur, would be transient. The design team has informed us that there are no substantial reciprocating loads.

Among the transport vehicles which will traverse the gravel surfaced yard, it appears the loaded reach stacker represents the most severe case. The reach stacker laden front axle design load is 449 metric tons (495 tons imperial) which will ride on five large tires inflated to 8.0 bar (117.6 psi) each. Total area under the front axle is about 156 sq.ft., resulting in an overall unit ground pressure of about 6,300 psf beneath the axle.

Although not listed among the load cases, self-propelled modular transporters (SPMTs) will also traverse the yard. Each SPMT has a design laden gross weight of 240 metric tons (265 tons imperial) which will ride on 16 polyfilled tires. Total area under the carriage is about 146 sq.ft., resulting in an overall unit ground pressure of about 3,600 psf beneath the carriage. It is understood that both the reach stackers and SPMTs will be restricted from areas west of Buildings A, B and C, and north of Buildings C and D.

We understand that fabricated tower sections will be upwards of 10 meters (32.8 feet) in diameter, 50 meters (164 feet) in length and will weigh up to 800 metric tons (1,760,000 pounds), while the transition pieces will be upwards of 10 meters (32.8 feet) in diameter, 35 meters (115 feet) in length and will weigh up to 800 metric tons (1,760,000 pounds).

As detailed in load case K, plans call for the tower sections to be staged horizontally on moveable storage fixtures, one on each end. Each fixture is to have two bearing plates which will bear on the gravel yard surface, each plate 20 sq.ft. in plan area, this resulting in a unit contact pressure upwards of 22,000 psf as currently planned.

The transition pieces are to be staged in a vertical position, on modular jersey barrier-like units 1.25 meters (4.1 feet) wide at their base and 10 to 14 meters (32.8 to 45.9 feet) in length. Each transition piece is to be supported on three units, with resulting contact pressures at the base of the units bearing on the gravel yard surface between 3,300 psf and 4,700 psf.

Tolerable Settlements

The Marmen load case document outlines tolerances for relative rail displacements and accommodating these will largely be a function of slab stiffness, as we understand it.

The document lists maximum allowable settlement at exterior man door and garage door slabs as 1 inch relative to the building, and maximum allowable settlement at interior and exterior slabs with rails as ½ inch relative to the rails and/or building.

While we have not been provided with allowable settlement for the buildings as a whole, it is our understanding that steel framed, metal clad structures of this type are relatively settlement tolerant, and displacements of two to three inches can usually be accommodated without causing a structural concern.

In the yard area, we understand the end user acknowledges rutting, aggregate kick-out and/or settlement of the aggregate surface will occur with use over time, and that they will re-dress and re-level the yard area surface as needed. It is further understood that settlement beneath the tower section storage fixtures need only be limited such that the tower sections remain off the ground, while allowable differential settlement beneath the transition piece modular units is reportedly 3 inches.

APPENDIX B-3E

RFP # 2025-12

BEACON ISLAND PHASE 3
Packaged Wastewater Treatment Plant
and
Fire Pump House and Marine Inlet

REFERENCE DOCUMENTS (5 of 5)

Reference Documents

Beacon Island Phase 3 Program

- NYSDEC Article 11 and 15 Permits. Dated 11/10/22 - Page 1
- State Pollutant Discharge Elimination System (SPDES) Permit. Dated 10/1/23 - Page 21
- Stormwater Pollution Protection Plan (SWPPP). Dated 6/20/22 - Page 59
- Soil Management Plan. Dated 10/23/22 - Page 564
- Landfill Closure Certification Report. Dated 10/21/24 - Page 634
- Geotechnical Engineering Report. Dated 2/2/2023 - Page 659
- Army Corps of Engineers Permit. Date 4/10/23 - Page 779
- Community Air Monitoring Plan (CAMP). Dated 10/23/22 – Page 806

Proposed Grades

Finish floor elevation at each of the proposed buildings is 21.0 feet, which in general is several feet or more above existing site grades within the proposed building footprints. The approximate difference in elevation between existing site grades and proposed finished floor level at each building is summarized as follows:

Building	Approx. Existing Grade Elev. (ft)	Finished Floor Elev. (ft)	Difference Between Exist. Grade and Finished Floor (ft)
A	13 to 19	21.0	2 to 8 overall (but generally in the range of 6 to 8)
B	11 to 17	21.0	4 to 10 overall (but generally in the range of 8 to 10)
C	7 to 13	21.0	8 to 14 overall (but generally in the range of 10 to 12)
D	7 to 19	21.0	2 to 14 overall (but generally in the range of 2 to 4)

From the buildings and progressing eastward across the yard area, proposed grades slope gently toward the river at an inclination of about 3 percent or flatter, to elevations between about 13 and 16 feet. Both cuts and fills will be required in the yard area to establish finish grades, which are as much as 6 feet lower than existing grade in places, and in general up to about 8 feet higher than existing grade. New fill approaching 14 feet in thickness will be required in a limited area about Building D.

As previously noted, curbs will in general define the limits of the yard area, beyond which grades will slope down to the river (or creek as applicable), at inclinations typically between 1V:3H and 1V:4H. Additionally, a retaining wall is planned on the west side of Building C. The wall will be approximately 780 feet in total length, with retained height upwards of about 13 feet.

It is also our understanding that disturbance to the existing shoreline(s) is to be minimized so as to preserve existing trees and whatever visual screening from the waterways they provide.

Retaining Walls

Plans call for a retaining wall on the west side of Building C. The wall will be approximately 780 feet in total length, with retained height upwards of about 13 feet. As currently envisioned this will be a mechanically stabilized earth (MSE) type wall.

Exclusions

Finally, we note that incoming raw materials will initially be received at another site, this located at 700 Smith Boulevard in the currently developed portion of the Port. Plans at that location call for a 20,000 sq.ft. receiving and pre-assembly building (Building E), along with temporary storage of steel plates, flanges and miscellaneous items in an accompanying yard area. We have addressed Building E and the proposed bridge at the north end of Beacon Island in separate reports issued in January 2022.

This report does not address the proposed access road linking the subject site to River Road/NYS Route 144 or the proposed automobile parking areas west of the buildings at the subject site. We are currently awaiting authorization from National Grid to complete test borings in their right-of-way as a basis for evaluating the potential impacts of these features from a geotechnical standpoint and providing earthwork recommendations as appropriate.

If any of the above information is incorrect, please let us know so we can review the conclusions and recommendations provided in this report for applicability to the actual design and update the report as appropriate.

As the design of the project progresses and site grading plans and structural loads are fully developed, we should be retained to assess such additional information relative to the recommendations contained herein.

SUBSURFACE CHARACTERIZATION

We have developed a general characterization of the subsurface conditions based upon our review of the subsurface exploration results (from this and previous studies), geologic setting and our understanding of the project. This characterization, termed GeoModel, forms the basis of our geotechnical analysis and evaluation of site preparation and foundation options. Conditions encountered at each exploration point are indicated on the individual subsurface logs. The logs can be found in the **Exploration Results** and the GeoModel in the **Figures** sections of this report.

Subsurface Profile

The following model layers were identified within the subsurface profile. For a more detailed view of the model layers with depths at each boring location, refer to the GeoModel.

Model Layer	Layer Name	General Description
1	**Fill**	In general coal ash on the west side and south end of the site. Elsewhere sand, silt, gravel and/or clay in varying proportion, along with occasional organics and/or foreign material such as cinders, slag, brick, metal, wood.

2	**Alluvium**	Sand with lesser amounts of gravel, frequently intermixed or interbedded with silts and/or clays. Relatively minor amounts of organics common.
3	**Silt and Clay**	Glaciolacustrine silt and clay deposit.
4	**Glacial Till**	Fine sand and silt with embedded coarser sands, gravel, rock fragments. Some cobbles and boulders. Sometimes clayey.
5	**Bedrock**	Shale bedrock. Upper few feet relatively weathered.

Surface Materials and Fill Soils

Although generally somewhat brushy and/or wooded, topsoil was generally scarce in the coal ash disposal areas. Elsewhere, topsoil or forest mat was present at the ground surface at thicknesses between about 0.3 and 1.0 feet as indicated on the test pit logs. We note the indicated topsoil thicknesses should be regarded as a rough approximation only and should not be relied upon for construction quantity estimates; contractors are advised to make their own estimates or determination of topsoil thickness and quality for bidding purposes.

Beneath whatever surface organic materials were present, fill and/or suspected fill soils were found at most locations, extending to depths between about 3 to as much as 29 feet below existing grade. Coal ash was the most prevalent fill material as outlined below. Otherwise, the fills generally consisted of sand, silt, gravel and/or clay in varying proportion, along with occasional organics and/or foreign material such as cinders, slag, brick, metal and wood. Some of these materials likely represent river sediments, reworked native soils or dredge spoil. The relative density of the non-coal ash fill as indicated by measured SPT N-values was most often in the loose to medium dense range.

As has been described in the previously referenced reports, much of the filling on the site has occurred through the bulk placement of waste coal ash from the south adjoining power generating station. The plant was coal fired upon its construction in the early 1950s until about 1970, when its boilers were converted to use other fuels. Waste coal ash during the plant's coal burning years was disposed of on the project site, primarily along the site's west side and at its south end. The method of placement of the coal ash is unknown but is believed to have been transported in bulk and pushed/tracked into place as opposed to hydraulically placed. Relative density of the coal ash indicated by measured SPT N-values was typically very loose, and it was noted that some vibration of the ground was evident underfoot as a large tracked excavator traversed the ground surface in the coal ash area while moving from location to location in the course of excavating the test pits.

Laboratory testing of coal ash samples recovered from the site indicates it is comprised primarily of silt (66 to 76 percent by weight) and fine sand (19 to 27 percent) sized particles and classifies among the ML group using the Unified Soil Classification System (USCS). Coarser sand and clay size particles are present in trace amounts. Maximum dry density of the coal ash as determined by ASTM D1557 (modified Proctor) was between 61.8 and 64.2 pounds per cubic foot (pcf) with optimum moisture content between 38.2 and 42.1 percent. These results are in keeping with what would be expected based on published accounts concerning the engineering properties of coal ash. Relatively minor amounts of organics were commonly noted in the ash fill as well, but overall the material was found to be rather consistent in composition.

It should be noted here that beneficial reuse of coal ash as a building material is not uncommon in the construction industry. In addition to its use as an additive in concrete, coal ash is generally regarded as suitable for construction of engineered structural fills for building sites, foundations and embankments, among other applications. Its usefulness as such is outlined in ASTM E2277, which cites low unit weight and relatively high shear strength, along with ease of handling and compaction as positive attributes of coal ash.

That said, the uncontrolled manner in which the material was placed is a concern as it relates to site development, and what follows herein should be viewed in this context. We regard the other miscellaneous fills and river sediments similarly (in the absence of gross debris, organics, or whatever otherwise unsuitable materials may be found). And despite the overall potential usefulness of coal ash as a fill material, the Ductile Iron Pipe Research Association (DIPRA) considers coal ash a known corrosive environment. Accordingly, the ash should be considered potentially aggressive to ductile iron piping systems and possibly other buried metallic pipes/elements placed within it.

Finally, while not found to be prevalent across the site, it should be understood that localized pockets of coarse, unsuitable debris may be present in places, as evidenced by buried railroad ties identified by Bergmann in the course of their 2017 study. The railroad ties were found at test pit TP-8 (located along the access road in southeast portion of site) between the depths of 8 and 12 feet below grade. Also note that fill materials and native soils were found to be similar in composition in places, rendering distinction between them difficult; the depth of fill as indicated on the logs should be considered approximate.

Alluvial Soils

Native soils beneath the existing fill materials were found to consist of alluvium, typically composed of sands with lesser amounts of gravel, frequently intermixed or interbedded with silt and/or clay. Relatively minor amounts of organics were commonly noted in these soils also. The alluvial soils extended to depths of about 25 to 55 feet (or as little as 20 feet at B-21-11) and exhibited a typically loose relative density. In the instances where the recovered soils were primarily fine-grained, their relative consistency was most often very soft.

Silt and Clay

Underlying the alluvium was a lacustrine silt and clay deposit which extended to depths of about 40 to 155 feet, generally increasing in depth to the east and more markedly to the south across the site. The silts and clays in this deposit were characteristically gray in color and very soft in relative consistency. Layers consisting primarily of silt were occasionally found therein. An exception to this is noted at borehole B-21-11, where no lacustrine soils were found between the alluvium and an unusually shallow glacial till deposit at a depth of 20 feet.

Laboratory testing performed on selected soil samples indicates that the gray silt and clay soils at this site are typically low to medium plasticity silts/clays categorized as CL or CL-ML in the USCS. A tabular summary of the most recent laboratory test results on these cohesive soils is provided below.

Boring/Test Pit ID	Depth (ft)	Natural Water Content (%)	Liquid Limit	Plasticity Index
B-21-7	60-62	26.4	NP	NP
B-21-17	40-42	30.5	31	12
B-21-18	35-37	35.5	33	11
B-21-20	40-42	33.2	31	11
B-21-23	110-112	20.6	23	6
TP-21-3	6-6.5	19.6	33	14
TP-21-7	3-3.5	18.9	33	12

As indicated in the table above, measured liquid limits ranged from 23 to 33 percent, and corresponding plasticity indices ranged from 6 to 14 percent. The natural moisture content of these soils ranged from 20.6 to 35.5 percent and was typically nearer the liquid limit in the deeper deposits. Laboratory testing results on the silt and clay deposit from previous studies have been similar. UU triaxial shear testing from previous studies also indicates its undrained shear strength is between about 580 and 640 psf.

Previous consolidation testing on the silt and clay deposit at the Beacon Island site and our experience with these Glacial Lake Albany lacustrine soils in the region indicate these deposits have been preconsolidated; that is, they have been subjected to stresses greater than current overburden pressures and have consolidated under these excess pressures. The preconsolidation is believed to be the result of a combination of stresses induced through desiccation, or drying, caused by the regional lowering of the water table during the geologic past and by loading from overburden soils which existed previously in the area but have since been eroded.

The available information indicates a net preconsolidation pressure of 4,000 psf or greater in the upper silt and clay; the net preconsolidation pressure and over-consolidation ratio (OCR) typically diminish with increasing depth. Previous cone penetrometer testing performed across the Beacon Island site indicates the OCR ranges from upwards of about 6 in the upper overburden soils to about 1.2 or less at depths greater than 100 feet. Undrained shear strengths of 500 to 750 psf are typical for the gray Glacial Lake Albany silts and clays in the region, this consistent with the results of UU triaxial testing previously completed at the site as noted above.

Glacial Till

Glacial till soils were found beneath the lacustrine silts and clays at most locations, although no till was encountered atop the underlying bedrock at boreholes B-21-10 and B-21-15. The till typically consisted of fine sand and silt (occasionally clayey) with embedded coarser sands, gravel and rock fragments, and was generally between about 3 and 12 feet in thickness (or as much as 22 feet thick at borehole B-21-17). Its relative density was most often in the dense to very dense range.

Cobbles and boulders are common in glacial till soils in the region and were frequently encountered in the till at this site as well. Note that the split spoon sampler employed in the SPT testing has an inside diameter of 1.375 inches which thereby limits recovery of coarser material and the extent to which coarser materials are represented in laboratory gradation testing. We also note that granular seams or layers within the till soils and at the till/bedrock interface may be more permeable than the surrounding soils and rock and may be under a slight artesian pressure.

Bedrock

Bedrock was encountered at depths between 45 and 159 feet below the existing ground surface, generally increasing in depth to the east and more markedly to the south across the site. This correlates with a bedrock surface elevation in the range of about –34 to –143 feet (below MSL). Note that rock may also have been encountered (or nearly so) upon refusal of the drill tooling at a depth of 30.1 feet (approx. elevation –19 feet) in borehole B-21-11, although this was not confirmed through rock coring at this location.

The upper few feet of rock were typically relatively weathered. Confirmatory rock core sampling of the less weathered underlying rock in general revealed weak shale with very close to moderate joint, fracture and/or bedding spacing at a relatively high angle. Bands or layers of medium strong sandstone or graywacke were occasionally encountered, as were occasional siltstone seams and quartz veins. Rock quality designation (RQD) ranged from 8 (very poor) to 58 (fair) and averaged about 38 percent overall.

For information purposes, the Geologic Map of New York (New York State Education Department, 1970) maps bedrock underlying the project area as Normanskill shale with minor constituents of mudstone and sandstone, along with shale and graywacke of the Austin Glen Formation.

Groundwater Conditions

Based on the recovery of wet soil samples and groundwater level measurements from this and previous investigations, groundwater in general appears to about 3 to 14 feet below the existing ground surface, this corresponding to groundwater elevations in the range of approximately 3 to 14 feet.

Mean high water in the Hudson River/Normanskill Creek is at an elevation of about 4 feet, and groundwater is in general expected at or near this level. A number of observation wells from previous investigations were observed on the site, and water level readings taken in these wells during this investigation tend to support this conclusion. Note however that these waters are tidal, normally within a range of about four to five feet, and tides are therefore expected to routinely affect water levels in and around the site. Information provided for our use indicates that extreme floodwaters may rise to about elevation 18 feet or more.

Additionally, as evidenced by some of the shallower observed water levels, locally perched or trapped groundwater may be present at times within the upper soils, particularly during seasonally wet periods and following heavy or extended periods of precipitation.

Groundwater elevations at the site should be expected to vary with seasonal fluctuations in precipitation and runoff, and with rising and falling water levels in the Hudson River. Tidal changes in the Hudson River are also expected to influence groundwater levels within a few hundred feet of shore to some degree daily. Additionally, grade adjustments on and around the site, surrounding drainage improvements and/or periodic flooding may also affect the water table.

GEOTECHNICAL OVERVIEW

General Discussion

In our opinion, the investigation completed at the project site revealed subsurface conditions that, with the exception of the coal ash fill, are typical along the Hudson River in the Albany area. The conditions are also generally consistent with those revealed through previous investigations at the site. The upper soils are composed of coal ash, miscellaneous fill and river sediments which are underlain by, in sequence with depth, alluvium, soft silt and clay, glacial till and ultimately shale bedrock. Groundwater is expected at or near the level of the river, or roughly 3 to 14 feet below existing site grades.

From a geotechnical standpoint, the site presents some challenges in the context of the proposed construction and planned heavy industrial loading. There are a number of factors which will impact on site development including:

- The bulk uncontrolled coal ash fill, along with other miscellaneous fills and river sediments
- Extensive cut and fill requirements
- Soft clays at depth which are subject to time-dependent consolidation settlement
- Weak subgrades relative to vehicular and material loading in yard area

Some key points for each of these factors are discussed in the following paragraphs, together with our recommended development approach.

It should be understood that the performance of the planned buildings and site features will ultimately be dependent upon successful implementation of the earthworks recommended herein. Retaining Terracon for construction period geotechnical observation, testing and consulting services will maintain continuity between the design and construction phases which can minimize risks and provide cost saving benefits to the Owner.

In general, the footprints of Buildings A, B and C are situated over the coal ash fill in their entirety, while miscellaneous fills consisting of sand, silt and clay with lesser amounts of foreign matter are present in the area of Building D. The uncontrolled coal ash fill, together with the other miscellaneous fills and river sediments, are not considered suitable for direct support of conventional shallow spread foundations and slab-on-grade construction. These materials offer marginal or unreliable bearing capacity and are subject to excessive post-construction settlement in the absence of some means to improve them.

To this end, we have evaluated a number of ground improvement methods in terms of their potential to enhance the bearing capacity and settlement characteristics of the existing fills and native deposits in-place, considering likely cost, impact to schedule and so on. These include deep dynamic compaction (DDC), rammed aggregate piers and soil mixing, along with full or partial undercuts and replacement. Each of these options was ultimately dismissed, either on the basis of technical feasibility or perceived benefit relative to time and expense. Additionally, note that none of these options would relieve the necessity to preload the building pads and allow time sufficient for consolidation settlement of the deep soft clays to occur, as outlined subsequently herein.

Taking into account that several feet of new fill is required to raise site grades beneath the buildings, and to the extent the proposed buildings and rail embedded slabs are not highly sensitive to settlement, consideration may be given to their support on unit mat type foundations, provided the mats are made sufficiently stiff to resist discrete concentrated loads beneath columns, rails, etc. and distribute these over broader areas of the mat. While all fills required to raise site grades should consist of suitable soils, we recommend the mats rest on no less than three feet of imported select structural fill to ensure the quality, uniformity and integrity of materials directly beneath the foundations.

The use of mat foundations will require preloading the building pads and exterior rail areas with the subgrade fill required to establish proposed grades, together with a surcharge approximating the average building live and dead loads the foundation subgrades will support. Doing so as a means of improvement will allow the underlying fills, river sediments and deep clays to consolidate under the weight of these loads and limit post-construction settlement. Plans should include a sufficient waiting period for the time-dependent settlement to occur, estimated at upwards of three to four months. To the extent possible, whatever filling is required in the yard area should also occur early in the construction schedule so as to limit post-construction settlements there.

It should be understood the mat foundation option is offered as a relatively cost-effective and expedient means of developing the site considering the rather poor soil conditions and proposed usage. Assuming the recommendations herein are adhered to, we expect that post-construction settlements will remain within tolerable levels and overall performance of the foundations and buildings will be satisfactory. That said, a good deal of uncertainty remains concerning what is or may be buried in the bulk uncontrolled fills across the site, and the owner and/or end user must be willing to accept some accompanying risk of excessive settlement in exchange for the benefit to cost and schedule represented by the mat option. If this uncertainty cannot be accepted, the buildings and slabs should be supported on end bearing steel piles driven to refusal on bedrock.

Similarly, to the extent existing fills are left in place beneath new pavements in the storage/staging yard area, the owner and/or end user must accept some degree of risk that excessive long-term settlements may occur. As previously indicated, buried railroad ties were disclosed in a test pit during a previous investigation, and this test pit was located in the currently proposed yard area. Heavy proof rolling of exposed subgrades as described herein will help to identify unsuitable subgrades and mitigate, but not eliminate, the risk of long-term settlement. An exceptionally heavy reinforced aggregate pavement section has been developed in consideration of the appreciable reach stacker, SPMT and material storage area loads, together with the marginal subgrade conditions which now exist.

Selective reuse of suitable onsite cut materials will be possible beneath building pads and yard areas, with some limitations as discussed in the **Earthwork** section herein. Whatever environmental considerations are involved with the handling and/or reuse of coal ash and/or other materials on the site are beyond the scope of this report and have been addressed in the Soil Management Plan by ATL (October 2020). Additionally, as previously indicated, the Ductile Iron Pipe Research Association (DIPRA) considers coal ash a known corrosive environment, and the ash should therefore be considered potentially aggressive to ductile iron piping systems and possibly other buried metallic pipes/elements placed within it.

Finally, limited vibration resulting from heavy equipment tracking across the ground surface was felt underfoot in the coal ash areas during the course of the investigation. It is possible a tendency for this to occur in association with heavy or reciprocating equipment will remain post-construction, in spite of the additional filling required to establish proposed grades. If the potential for such nuisance vibrations is perceived as a problem, this should be further studied by the end user and design team.

The following sections of this report provide more detailed recommendations to assist in planning for the geotechnical aspects of the project. We should be provided with the opportunity to review plans and specifications prior to their release for bidding to confirm that our recommendations were properly understood and implemented, and to allow us to refine our recommendations, if warranted, based upon the final design. The **General Comments** section provides an understanding of the report limitations.

SEISMIC CONSIDERATIONS

The seismic design requirements for buildings and other structures are based on Seismic Design Category. Assignment of seismic Site Class is required to determine the Seismic Design Category for a structure. The Site Class is based on the upper 100 feet of the site profile defined by a weighted average value of either shear wave velocity, standard penetration resistance or undrained shear strength pursuant to Section 20.4 of ASCE 7 and the International Building Code (IBC).

Seismic Site Classification

In our estimation, assignment of seismic Site Class D (stiff soil profile) for the project is justifiable. This determination is made based upon the results of shear wave velocity testing in seismic cone penetrometer tests previously completed at the site. Additional cone tests or geophysical testing may be performed to confirm this determination if desired.

Liquefaction

An evaluation of the potential for soil liquefaction to occur was made using the computer software program Liquefy Pro by CivilTech Corporation. An earthquake magnitude of 6.0 was assumed, and a peak ground acceleration (PGA) of 0.09g for the project area was used, this representing a two percent probability of exceedance in 50 years (2,500 year return period, as obtained from USGS earthquake hazards mapping). Based on these parameters and site specific conditions determined through the subsurface investigation, the calculated factor of safety against liquefaction is greater than 1.2. As such, liquefaction potential at the project site is considered low. However, seismically induced ground surface settlements may occur over the general area, with those at the project site estimated to not exceed 0.5 inch.

EARTHWORK

Earthwork is anticipated to include clearing and grubbing, stabilization of subgrade surfaces as necessary, bulk cuts and fills, preloading/surcharging the buildings pads, excavation for foundation construction and associated backfill. The following sections provide recommendations for use in the preparation of specifications for the work. Recommendations include critical quality criteria as necessary to render the site in the state considered suitable in our geotechnical engineering evaluation for new foundations and aggregate-surfaced pavement sections.

Construction site safety is the sole responsibility of the contractor, who controls the means, methods, and sequencing of construction operations. Under no circumstances shall the information provided herein be interpreted to mean Terracon is assuming responsibility for construction site safety or the contractor's activities; such responsibility is neither implied nor shall it be inferred.

Site Preparation

Site preparation should begin with stripping of existing topsoil and surficial organic matter as applicable from the new building and yard areas. Any remains of former structures or obviously unsuitable materials that may be found should also be removed.

Prior to placing fills to raise grades and/or after cuts are made to the plan subgrade elevations, the exposed grades should be heavily and thoroughly proof-rolled using a steel drum roller with a static weight of at least 10 tons. The roller should operate in its vibratory mode, unless requested otherwise by the Geotechnical Engineer observing the work, and travel at a speed not exceeding three feet per second (two miles per hour). The roller should complete at least eight passes over all subgrade surfaces (four each in opposing directions). The method of proof-rolling may be modified by the Geotechnical Engineer based upon the conditions revealed at the time of construction.

Soft areas identified by the proof-rolling should be investigated to determine the cause and stabilized accordingly. These investigations may include the excavation of test pits. If existing fills are found and determined by to be unsuitable by the Geotechnical Engineer, they should be removed and replaced as deemed necessary.

Settlement and Preloading

Plans indicate about 2 to 14 feet of new fills are required to raise site grades in the building areas, this representing a net increase in load intensity of roughly 200 to 1800 psf on the underlying subgrades. Added to this will be the building and operational loads which we understand will be no greater than 600 psf when aggregated across a given building (or slab) footprint. In our estimation, new loads of this magnitude will result in stresses at depth which approach but do not exceed preconsolidation pressures in the deep clay deposit, limiting settlements in the clay deposit to those in the recompression range.

Settlements will occur throughout the existing fills and overburden soils in response these loads. In general, the degree of settlement is expected to vary with the height of fill required to establish proposed grades, but we estimate that maximum settlements will be between roughly 4 and 6 inches beneath the building pads. As these estimated settlements are beyond that which are considered typical and tolerable, a preloading and settlement monitoring plan targeted at limiting post-construction settlements should be implemented.

Development of a detailed preloading and settlement monitoring program is beyond the scope of this report. However, the basic elements of preloading include placement of new fill material to proposed grade levels, together with a surcharge fill which approximates (or exceeds, within limits) anticipated overall post construction loading. Instrumentation is installed to track the settlement that occurs over time. The plan should be implemented early in the construction schedule and sufficient time allowed such that these settlements are essentially complete prior to building construction and final grading.

In doing so, it is important the preload/surcharge load intensity matches or exceeds total post-construction grading, building and operational loads without exceeding preconsolidation pressures in the clay deposit. We expect this can be accomplished by placing a surcharge fill 6 to 7 feet in height across the building pads once the site is filled to the proposed finish floor elevation of 21.0 feet (i.e., top of surcharge elevation 27 to 28 feet). The surcharge should extend to this height, but no higher; if the preconsolidation pressure in the clays is exceeded (either through surcharging or operationally post-construction) both the magnitude of overall settlement and the time required for consolidation to occur will be greater than that estimated herein.

For preliminary planning purposes, we recommend that the full height of the temporary surcharge extend at least 10 feet outside the planned building footprints; the embankment side slopes of the temporary surcharge should be inclined no steeper than 1V:2H.

Material composition and compaction of fills placed to nominal finish floor elevation should be as described elsewhere herein. The temporary surcharge fill above finished floor level may consist of whatever material is most expedient, and may be simply tracked into place provided its in-place density is 100 pounds per cubic foot (pcf) or greater.

The required waiting period for settlement to occur will depend on the consolidation rate of the soils but we estimate the process will be substantially complete within a period between say 6 weeks and 3 to 4 months once the full height of fill and surcharge is in place. This should be understood and accommodated in developing the project schedule. Settlement in the fills and upper soils is expected to occur relatively quickly and in a semi-elastic manner as new loads are applied, whereas recompression settlement of the deep clays is expected to occur more slowly over the course of weeks and months.

Instrumentation in the form of conventional settlement plates and settlement systems with pressure transducers should be provided as part of the preloading and settlement monitoring program to allow the rate and total amount of settlement that occurs to be measured. Other instruments such as piezometers and inclinometers may be included in the preloading program as determined appropriate during its design.

For preliminary planning purposes, it should be assumed that a combination of at least 12 settlement plate and pressure transducer type settlement systems will be required across the building pads, their locations to be selected by this Geotechnical Engineer. The preloading and settlement monitoring program should be reviewed with the contractor and the settlement plates installed prior to any fill placement (but after the site has been stripped and proof-rolled).

Immediately upon installation of each settlement system, the top of plate elevation and any readout device panels should be determined and recorded as the starting grade or initial reference point, along with the elevation at the top of the first extension pipe for conventional systems. Following this, approximately 12 inches of fill should be placed and compacted over the plate to properly seat and secure the platform, and the instruments resurveyed. The instruments and panels should be clearly marked and/or protected as necessary to prevent any disturbance or damage during construction activities.

When adding any subsequent extensions, the top of pipe elevation of the existing extension should first be obtained and recorded, and the top of pipe elevation of the new extension should be recorded immediately after being installed. Elevation data should be recorded and maintained such that the actual plate elevation can be referenced and determined at all times. Elevations should be obtained at each device at least twice weekly as the fill is being placed, and on a weekly basis thereafter during the hold period.

All survey monitoring should be performed under the supervision of a professional land surveyor, with elevations obtained to the nearest 0.01 foot and referenced to a consistent offsite benchmark(s) that is not susceptible to movement or damage over the monitoring period. Additionally, the elevation of the subgrade fill immediately adjacent to the instrument should also be obtained to the nearest 0.1 foot with each set of measurements.

The settlement system elevation should be determined for each measurement interval based on the survey data. The elevation of the subgrade fill at each monitoring interval should also be collected. Terracon should prepare a plot of relative movement (i.e., settlement) of the plate/system vs. time on an ongoing basis in order to allow interim evaluation of settlement conditions.

Careful monitoring of the instruments and whatever data is collected over the preload period will be necessary to determine the point at which recompression/consolidation settlement has essentially ended and building construction can begin. There is uncertainty in predicting both the magnitude of anticipated settlement and the time required for recompression settlement to occur, and this should be understood by all parties, thus the range in time planned for the holding period should be flexible. The preload and settlement monitoring program should be designed and monitored by this Geotechnical Engineer, who will determine the required duration and make interim evaluations of the results obtained therefrom.

Bulk Cut and Fill Considerations

As a considerable amount of cut and fill will be required to establish proposed grades, economic site development will likely be dependent on the reuse of cut soils as new subgrade fill to raise site grades as necessary. Accordingly, the challenges and limitations associated with their reuse should be understood.

The onsite soils, in some cases, contain appreciable quantities of fine-grained silt and/or clay and will therefore require control of their as-compacted moisture content within narrow limits to achieve requisite in-place density as the material is placed. It may be necessary to either dry the soil in windrows or add water prior to placement and compaction depending on the prevailing weather conditions at the time of construction or the in-situ moisture content of the soils as they are excavated. Should site development proceed during seasonally wet or cold periods, it will likely be difficult to adequately dry the siltier cut soils and it may be necessary to stabilize these soils with lime, fly ash or kiln dust, or to use an imported granular fill.

Topsoil, vegetation and other surface materials should be stripped from all cut/fill areas prior to earth moving operations. The subgrade fill should be firm and stable after it is placed and compacted, and should not “pump”, “weave” or otherwise exhibit instability during construction; soils should be undercut and replaced where unsatisfactory. The fill subgrades should also be properly graded, drained, sealed and/or protected from moisture and frost as necessary. Placement of fill over wet, soft, snow covered, or frozen subgrades should not be permitted. All bulk fill placement and compaction should be monitored and tested by a representative of the Geotechnical Engineer on a full-time basis.

Where new fills are required to raise site grades, some difficulty may be experienced in achieving proper compaction of the fill soils considering the existing unimproved subgrades. This may be of particular difficulty in lower, wetter portions of the site, or where the filling is attempted with cut soils of lesser quality. It may therefore be necessary to begin the new fills using better quality imported granular material for the initial one or two lifts. Consideration may also be given to placing an initial layer of oversize stone (e.g., surge stone or shot rock, with a maximum 8 inch particle size) to displace excessively loose or wet soils and establish a firm base from which to continue. Other methods of subgrade improvement which may be considered include the use of reinforcement with dry granular material and geogrids or soil modification with admixtures as noted above.

Based on the findings of the subsurface investigation, bulk cuts across the site are not expected to encounter a generalized groundwater condition. However, perched groundwater may be intercepted in places, possibly necessitating the construction of fabric lined and stone filled drainage trenches to relieve, collect and dispose of such waters.

Fill Material Types

As indicated above, it may be assumed that excavated onsite soils will in general be suitable for reuse in fill areas once cleansed of any oversized particles, unsuitable debris or organics, subject to the approval of the Geotechnical Engineer and based upon the conditions encountered at the time of construction. Cut soils essentially free of organics, debris or particles >6 inches in size may be considered suitable fill and placed in common fill areas throughout the site, but no closer than three feet from the bottom of any mat foundation. Excessively silty or clayey materials should not be used as a source of fill within yard areas, though may be considered for placement under mat foundation areas if spread in thin (say less than 8 inch) lifts. Unsuitable materials should be wasted offsite or in landscaped areas.

Material imported for general use should consist of well-graded sand or sand and gravel which meets the requirements stipulated for Select Granular Fill in section 733-11 of the NYSDOT Standard Specifications for Construction and Materials.

We recommend that mat foundations be supported on no less than three feet of imported select structural fill to ensure the quality, uniformity and integrity of materials directly beneath the buildings and exterior rails. Designated select structural fill should consist of an imported processed sand and gravel or crusher-run stone which meets the requirements stipulated for Type 2 or 4 Subbase material in section 304 of the NYSDOT Standard Specifications.

Fill Compaction Requirements

Fills beneath the building pads and pavements should be placed in uniform loose layers no more than about one-foot thick where heavy vibratory compaction equipment is used. Thinner lifts should be used as necessary where hand operated equipment is required for compaction. Each lift should be compacted to no less than 95 percent of its maximum dry density as determined by the Modified Proctor Compaction Test – ASTM D1557, and moisture content of the material being placed should be maintained within +/- 3 percent of its optimum moisture content. In landscape areas, the compaction requirement may be relaxed to 90 percent of maximum dry density.

Grading and Drainage

All grades should provide effective drainage away from the buildings during and after construction, with such drainage maintained throughout the life of the structures. Water retained next to buildings can result in soil movements greater than those outlined in this report, which may in turn lead to unsatisfactory differential floor slab and/or foundation displacements, cracked slabs and walls, or roof leaks.

Temporary Excavations

Excavations must be performed in accordance with OSHA 29 CFR, Part 1926, Subpart P and its appendices, along with any state and local codes, as applicable. The contractor should be aware that slope height, slope inclination, and excavation depth should in no instance exceed OSHA regulations. Flatter slopes than those stipulated by the regulations or temporary shoring may be required depending upon the soil/groundwater conditions encountered and other external factors. OSHA regulations are strictly enforced and if they are not followed, the owner, contractor, and/or earthwork and utility subcontractor could be liable and subject to substantial penalties.

Construction Observation and Testing

The earthwork efforts should be monitored under the direction of this Geotechnical Engineer. Monitoring should include documentation of adequate removal of topsoil and unsuitable fills, proof-rolling, and evaluation of foundation and yard area subgrades. If unanticipated conditions are encountered, this Geotechnical Engineer should prescribe mitigation options. Each lift of new compacted fill should be tested, evaluated, and reworked, as necessary, until approved by this Geotechnical Engineer prior to placement of additional lifts.

Foundation bearing grades and subgrades for floor slabs, pavements and concrete pads should also be evaluated under the direction of this Geotechnical Engineer. If unanticipated conditions are encountered, this Geotechnical Engineer should prescribe mitigation options.

It should be understood that subsurface conditions will be more fully known when the site is excavated. The continuation of this Geotechnical Engineer's services into the construction phase of the project and their continuous observations during earthwork and foundation construction will allow for validation of the subsurface conditions assumed to exist for this study and in the development of the design recommendations in this report, along with assessing any variations, providing interim recommendations as necessary and reviewing any associated design changes.

MAT FOUNDATIONS

Foundation Design Parameters

Over the course of this study, we were furnished with load distribution diagrams quantifying contact pressure beneath the mat foundations at selected column locations considering both building and operational loads. These diagrams indicate that load intensity may range upwards of about 2,500 to 3,500 psf over limited areas no greater than about 10 x 20 feet with the mat configured as currently planned. The diagrams further indicate that load intensity dissipates from the loaded areas such that contact pressure at the limits of a mat area measuring about 40 x 80 feet in plan dimension does not exceed about 1,500 psf. As previously noted, we understand that gross loading on the mats aggregated across the total floor area does not exceed 600 psf.

In view of the above, we expect the limiting pre-consolidation pressure within the deep lacustrine soils will not be exceeded and thus settlements will be controlled by recompression. Under these parameters, we estimate that post-construction mat settlements across the site will not exceed 1 to 2 inches. As mat design progresses, and other load cases are developed, they should be provided to us for review to determine whether these other loadings cause imposed stresses to exceed the pre-consolidation stress within the deep lacustrine soils.

The mat foundations should be constructed on a minimum three feet of select structural fill, over subgrades which have been prepared, preloaded and surcharged as described herein. Provided this is so, an effective modulus of subgrade reaction of 50 pounds per cubic inch (pci or psi/in) may be assumed at the top of the select structural fill layer.

Differential settlement across the mats will depend, in part, on their rigidity. We caution that differential settlements may occur due to non-uniform loading conditions both during and after the completion of construction. The mats must be designed, as needed, to accommodate the varying loading conditions and settlements. Preferably, construction should proceed such that differential loading is not created across the mats. When available, we should review the construction sequence and actual load distributions expected across the mats to refine the settlement estimates and evaluate differential settlement concerns.

Utilities, where they connect with the buildings, should be designed to accommodate the expected settlements. Within the buildings, the utilities should be placed within chaseways built into the mats for access. The utilities should not be planned or constructed either within or below the mats.

Frost protection at the perimeter of buildings or in unheated portions thereof should be provided by seating foundations four feet or greater below surrounding grades, or through the use of an appropriate frost protected shallow foundation (FPSF) detail.

Mat Foundation Construction Considerations

The foundations should be seated directly on at least three feet of imported select structural fill, which is itself placed over subgrades prepared as described herein. All final bearing grades should be firm, stable, and free of loose soil, mud, water and frost. This Geotechnical Engineer should approve the condition of the foundation bearing grades immediately prior to placement of reinforcing steel and concrete.

SERVICE PITS

As previously indicated, one or more service pits up to 8 feet in depth (this corresponding with approximately elevation 13 feet) will be included in Buildings A and C. With floodwaters expected to rise upwards of elevation 18 feet, elevated groundwater may subject the pits to uplift pressures (buoyancy). Some means should therefore be incorporated to resist uplift, whether this be through self-weight of the pits, base extensions or some other method. Adequate waterproofing measures should also be provided.

Otherwise, the pits should be equipped with an open sump and pump system, with the pumps designed to dewater a specified volume that would be dependent upon the flood elevation, soil medium surrounding the pits, and the actual plan dimensions and depths of the pits.

Note that the pit walls should be designed to resist lateral earth pressures as outlined below.

RETAINING WALLS

The parameters given below are provided to analyze internal and external stability of the wall system and should be suitable for preliminary design purposes. We note however that the MSE retaining wall planned west of Building C will apparently be situated on the loose coal ash fills and will therefore be subject to settlement concerns similar to the buildings. While we expect the wall foundation subgrades can be improved through preloading as described elsewhere herein, it should be understood that the full height of the preload must in this case extend laterally to at least 5 feet beyond the planned wall face, with the preload embankment side slope temporarily extending beyond the wall. The preload materials would then need to be removed from the retaining wall area and the wall system and its reinforcing constructed following the preload program. If this is not feasible or possible, consideration should be given to a different type of wall system more tolerant to settlement that can be built in conjunction with the site fills (as noted below).

The wall reinforcement system should also be considered in conjunction with overall site design. Based on the anticipated coal ash subgrades upon which the wall will be situated, we expect that satisfying global stability concerns will ultimately be a controlling factor in design. Reinforcement geogrid lengths upwards of 20 to 30 feet or more may be necessary depending on the wall system chosen, and the sequencing of geogrid installation with fills required to raise site grades should be coordinated as appropriate. In our estimation, a Geosynthetic Reinforced Soil System (GRSS) type wall is better suited to the expected site conditions as compared with the MSE type wall currently under consideration. GRSS walls are more tolerant to settlement and thus could be built in conjunction with the fills to raise site grades. Wall design would be a subsequent service that we should provide.

All earth-retaining walls should be designed to resist the lateral pressures generated by earth backfill and any temporary or permanent surcharge loads. The following design parameters are provided to assist in calculating lateral earth pressures and analyze wall stability as applicable:

- Soil angle of internal friction - 30 degrees
- Coefficient of At-Rest earth pressure (k_o) - 0.50
- Coefficient of Active earth pressure (k_a) - 0.33
- Coefficient of Passive earth pressure (k_p) - 3.00
- Total unit weight of compacted soil - 130 pcf

The recommended design parameters assume that the backfill consists of imported select granular or structural fill as outlined in the **Earthwork** section herein and that the backfill remains permanently well-drained. Water must not be allowed to collect against the wall unless the wall is designed to accommodate the added hydrostatic pressure. Use of excavated site soils for wall backfill should be avoided. The parameters are also based on idealized non-sloping conditions on each side of the wall and should be considered preliminary subject to review when grades are finalized. Where slopes are present either in front of or behind the walls, the coefficients of lateral earth pressure must be adjusted accordingly.

SHORELINE AND SLOPE STABILITY

An evaluation of global shoreline stability was made at several selected sections along the banks of the Hudson River and Normanskill in consideration of the proposed grading and loading conditions, including the heavy transport vehicles. A total of five sections were evaluated, three along the river and two along the creek.

In developing each section, existing and proposed topography was taken from the site plans furnished to us, and the subsurface profile was compiled from information as revealed by the test borings and test pits. A uniform surcharge load for the material staging and/or equipment loadings was assumed based on the loading information provided, and was applied on the inside of the curb line indicated on the plans.

The slope and foundation geometries were analyzed by inputting data from the inferred subsurface profiles into the global stability evaluation software, SLOPE/W by Geo-Slope International, Ltd. Typical engineering properties for the soils were selected based upon the laboratory testing completed for this and previous studies together with our local experience. Groundwater conditions were modeled two ways: one considering the nominal static conditions encountered during our subsurface investigations, and another emulating rapid drawdown conditions as may occur after a flood event.

Under these parameters, the factor of safety against global failure of the shoreline was generally determined to be satisfactory (1.3 or greater). Typical industry standard targets a minimum factor of safety of 1.3, or 1.5 for critical structures.

However, a vulnerability to rotational slope failure was identified where concentrated loads are applied in close proximity to descending slopes. We therefore recommend that a minimum distance of 25 feet be maintained between concentrated loads (staged materials, reach stackers and SPMTs, etc.) and the crest of descending slopes.

Additionally, it was found that slopes along the shoreline are in general marginally stable against shallow, surficial type failures in the event of rapid water level drawdown as may occur following a flooding event. If armoring of the shoreline slopes to enhance their surficial stability is not a regulatory preferable solution, the prompt repair of any shallow failures will be required should a triggering flood event occur. Failure to address these surficial sloughs could result in propagation of the failures, potentially impacting greater portions of the slope and eventually upland yard areas.

It should be understood that stability of the soil slope, approach embankment and foundation geometries were modeled under the conditions outlined herein. Changes in feature location, geometry or grading, along with erosion or natural events can impact global stability. We should be retained to perform additional analyses and consulting as the final plans are developed.

Finally, we note that in general, any permanent cuts or embankment fills along the waterways should be sloped no steeper than one vertical on three horizontal (1V:3H). Steeper slopes may be considered on a case-by-case basis. All slopes should be vegetated, armored with riprap or otherwise protected against erosion as appropriate.

YARD AREA PAVEMENTS

Our design parameters assume the existing fills will be left in place and stabilized as detailed in the **Earthwork** section of this report. The owner must accept some degree of risk for excessive pavement settlement or failure if the existing fills are left in place. As previously indicated, whatever filling is required in the yard area should occur early in the construction schedule so as to limit post-construction settlements.

Reach Stacker and SPMT Use

The gravel-surface pavement section presented below was developed in conjunction with Tensar, primarily in consideration of the outsize reach stacker and SPMT loads that will traverse the site. PCASE software and assumed parameters based on the findings of our investigation were used in its development. We understand the end user acknowledges some rutting, aggregate kick-out and/or settlement of the aggregate surface will occur over time, and that they will periodically re-dress and re-level the yard area surface as needed in the course of their operations.

Except where noted and as applicable, all materials should meet the requirements specified in the latest edition of the New York State Department of Transportation (NYSDOT) Standard Specifications for Construction and Materials.

Reinforced Aggregate-Surface Pavement Design

Layer	Description	NYSDOT Reference	Thickness (inches)
1	Surface Aggregate	Section 733-04, Type 2	12
2	Base Aggregate	Section 733-04, Type 2 (or AASHTO #57 blend)	18
3	Geogrid	Section 737-07 (Tensar NX850 or equal)	Single ply
4	Base Aggregate	Section 733-04, Type 2 (or AASHTO #57 blend)	18
5	Geogrid	Section 737-07 (Tensar NX850 or equal)	Single ply
6	Non-woven Separation/ Drainage Geotextile	Table 737-01C	Single ply

Construction of the yard area pavement section and the reinforced approach embankment section at the bridge should be coordinated to ensure proper overlap and to ensure that placed geogrids/geotextiles are not damaged in the course of utility installation. The geogrid should be installed per the manufacturer's specifications, with prescribed overlap at seams, unless detailed otherwise.

Tower Section and Transition Piece Storage

The pavement section listed above should be provided throughout the storage yard area and anywhere that reach stackers or SPMTs will move about.

As outlined previously herein, plans call for the tower sections to be stored/staged horizontally on moveable storage fixtures, one on each end. Each fixture is to have two bearing plates which will bear on the storage yard's gravel surface. With each plate 20 sq.ft. in plan area, this results in a unit contact pressure upwards of 22,000 psf as currently planned.

While it is understood that settlement beneath the tower section storage fixtures need only be limited such that the tower sections remain off the ground, such settlements should be maintained within practical limits to avoid excessive tensile stresses in the geogrid reinforcement, which may result in damage to or failure of the grid and pavement system. To this end, we recommend the bearing plates be proportioned such that their contact pressure is limited to about 10,000 psf or less when bearing on the gravel pavement surface.

It is understood the transition pieces are to be staged in a vertical position, on modular jersey barrier-like units approximately 4.1 feet wide at their base and 32.8 to 45.9 feet in length. Each transition piece is to be supported on three of these units, and based on the information provided, we estimate that contact pressures at the base of the units bearing on the yard's gravel surface will be between 3,300 psf and 4,700 psf. We expect that differential settlement beneath these units will be maintained within the reported tolerable limit of 3 inches provided that loads do not exceed those indicated and are applied uniformly as shown. Nevertheless, we recommend that settlement beneath the transition pieces stored vertically be carefully monitored upon initial loading due to the uncertainty associated with the underlying previously filled subgrades.

GENERAL COMMENTS

Our analysis and opinions are based upon our understanding of the project, the geotechnical conditions in the area, and the data obtained from our site exploration. Natural variations will occur between exploration point locations or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until during or after construction. Terracon should be retained as the Geotechnical Engineer, where noted in this report, to provide observation and testing services during pertinent construction phases. If variations appear, we can provide further evaluation and supplemental recommendations. If variations are noted in the absence of our observation and testing services on-site, we should be immediately notified so that we can provide evaluation and supplemental recommendations.

Our Scope of Services does not include either specifically or by implication any environmental or biological (e.g., mold, fungi, bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials or conditions. If the owner is concerned about the potential for such contamination or pollution, other studies should be undertaken.

Our services and any correspondence or collaboration through this system are intended for the sole benefit and exclusive use of our client for specific application to the project discussed and are accomplished in accordance with generally accepted geotechnical engineering practices with no third-party beneficiaries intended. Any third-party access to services or correspondence is solely for information purposes to support the services provided by Terracon to our client. Reliance upon the services and any work product is limited to our client and is not intended for third parties. Any use or reliance of the provided information by third parties is done solely at their own risk. No warranties, either express or implied, are intended or made.

Site characteristics as provided are for design purposes and not to estimate excavation cost. Any use of our report in that regard is done at the sole risk of the excavating cost estimator as there may be variations on the site that are not apparent in the data that could significantly impact excavation cost. Any parties charged with estimating excavation costs should seek their own site characterization for specific purposes to obtain the specific level of detail necessary for costing. Site safety, and cost estimating including, excavation support, and dewatering requirements and design are the responsibility of others. If changes in the nature, design, or location of the project are planned, our conclusions and recommendations shall not be considered valid unless we review the changes and either verify or modify our conclusions in writing.

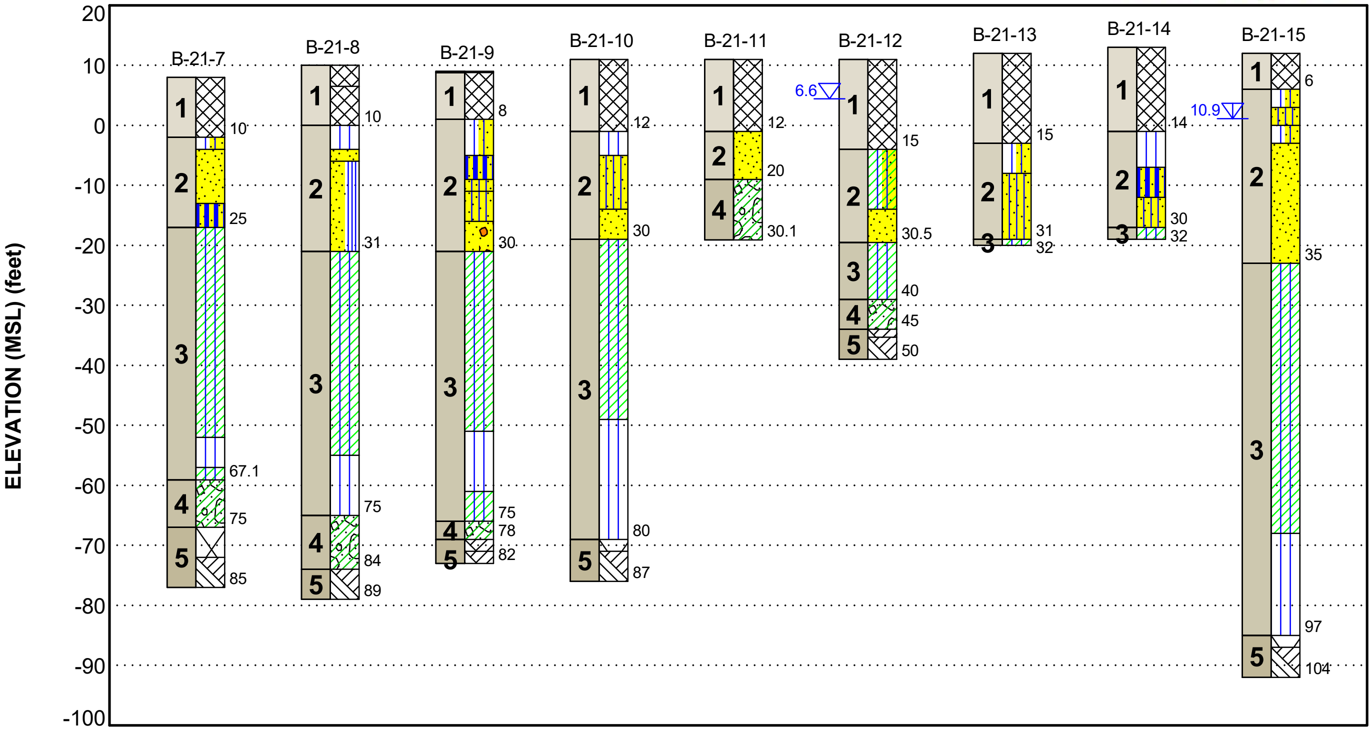
FIGURES

Contents:

GeoModel (4 pages)

GEOMODEL

Proposed Marmen Manufacturing Facility ■ Glenmont, NY
Terracon Project No. JB215020



This is not a cross section. This is intended to display the Geotechnical Model only. See individual logs for more detailed conditions.

Model Layer	Layer Name	General Description
1	Fill	In general coal ash on the west side and south end of the site. Elsewhere sand, silt, gravel and/or clay in varying proportion, along with occasional org. and/or foreign matter
2	Alluvium	Sand with lesser amounts of gravel, frequently intermixed or interbedded with silts and/or clays. Relatively minor amounts of organics common.
3	Silt and Clay	Glaciolacustrine silt and clay deposit.
4	Glacial Till	Fine sand and silt with embedded coarser sands, gravel, rock fragments. Some cobbles and boulders. Sometimes clayey.
5	Bedrock	Shale bedrock. Upper few feet relatively weathered.

LEGEND

⊠ Fill	▥ Sandy Silt	▨ Glacial Till	▥ Poorly-graded Sand with Silt	▩ Poorly-graded Sand with Gravel
▥ Silt with Sand	▨ Silty Clay	◸ Weathered Rock	▤ Topsoil	▨ Silty Clay with Sand
▩ Poorly-graded Sand	▥ Silt	⊠ Bedrock	▥ Silty Sand	

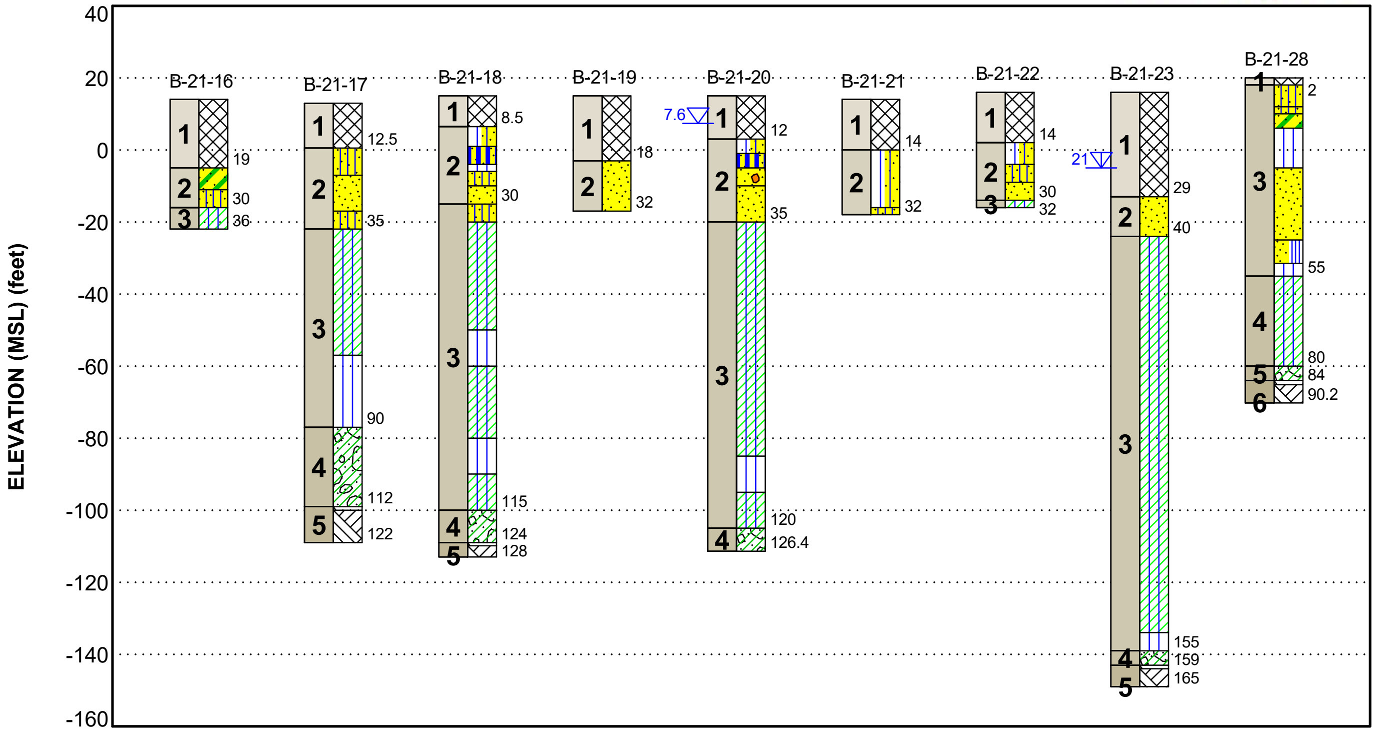
▽ First Water Observation
▽ Second Water Observation

NOTES:
Layering shown on this figure has been developed by the geotechnical engineer for purposes of modeling the subsurface conditions as required for the subsequent geotechnical engineering for this project. Numbers adjacent to soil column indicate depth below ground surface.

Groundwater levels are temporal. The levels shown are representative of the date and time of our exploration. Significant changes are possible over time. Water levels shown are as measured during and/or after drilling. In some cases, boring advancement methods mask the presence/absence of groundwater. See individual logs for details.

GEOMODEL

Proposed Marmen Manufacturing Facility ■ Glenmont, NY
Terracon Project No. JB215020



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3	Silt and Clay	Glaciolacustrine silt and clay deposit.
4	Glacial Till	Fine sand and silt with embedded coarser sands, gravel, rock fragments. Some cobbles and boulders. Sometimes clayey.
5	Bedrock	Shale bedrock. Upper few feet relatively weathered.

LEGEND

- ▨ Fill
- ▨ Clayey Sand
- ▨ Silty Sand
- ▨ Silty Clay
- ▨ Poorly-graded Sand
- ▨ Silt
- ▨ Glacial Till
- ▨ Weathered Rock
- ▨ Bedrock
- ▨ Silt with Sand
- ▨ Sandy Silt
- ▨ Poorly-graded Sand with Gravel
- ▨ Poorly-graded Sand with Silt

▽ First Water Observation
▽ Second Water Observation

Groundwater levels are temporal. The levels shown are representative of the date and time of our exploration. Significant changes are possible over time. Water levels shown are as measured during and/or after drilling. In some cases, boring advancement methods mask the presence/absence of groundwater. See individual logs for details.

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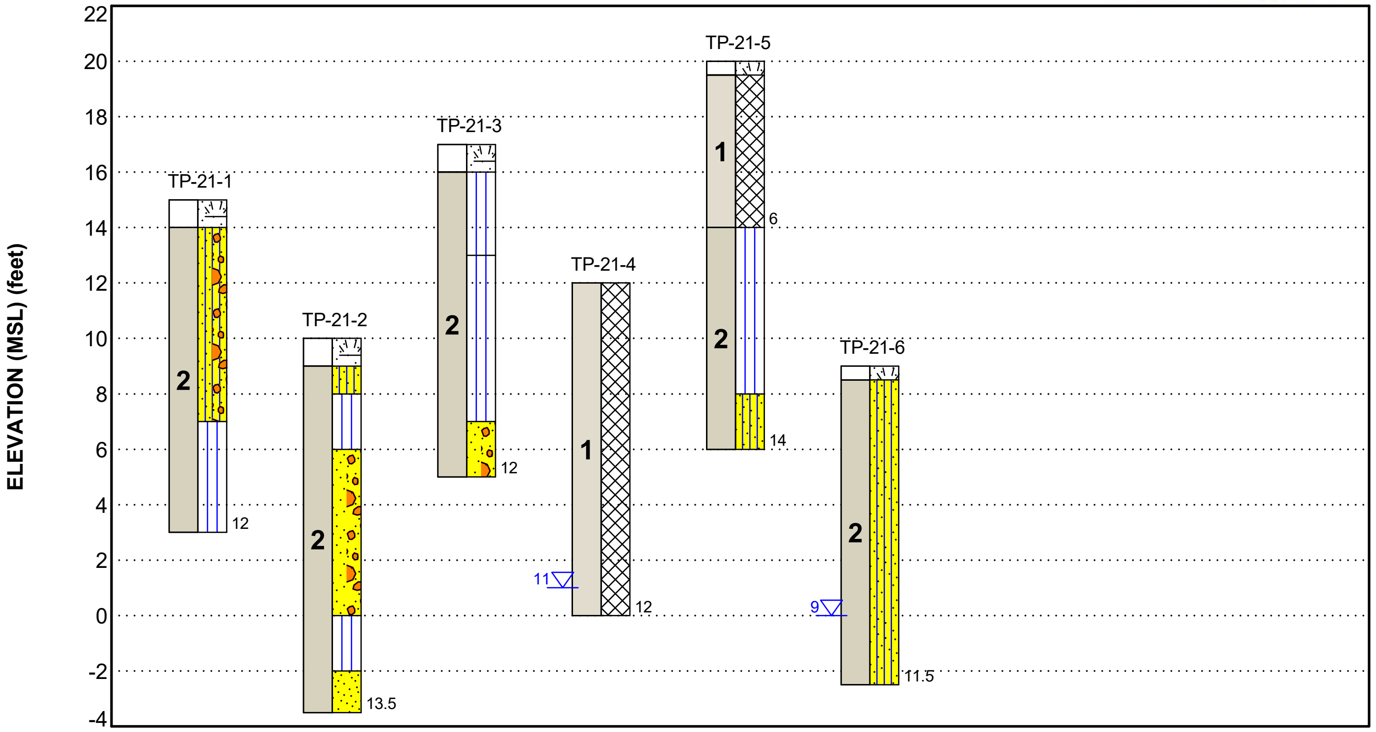
Proposed Marmen Manufacturing Facility ■ Glenmont, NY
Terracon Project No. JB215020



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4	Glacial Till	Fine sand and silt with embedded coarser sands, gravel, rock fragments. Some cobbles and boulders. Sometimes clayey.
5	Bedrock	Shale bedrock. Upper few feet relatively weathered.

LEGEND

▨ Topsoil
▨ Silty Sand with Gravel
▥ Silt
▥ Silty Sand
▨ Poorly-graded Sand with Gravel
▨ Poorly-graded Sand
▩ Fill

▽ First Water Observation
▽ Second Water Observation

Groundwater levels are temporal. The levels shown are representative of the date and time of our exploration. Significant changes are possible over time. Water levels shown are as measured during and/or after drilling. In some cases, boring advancement methods mask the presence/absence of groundwater. See individual logs for details.

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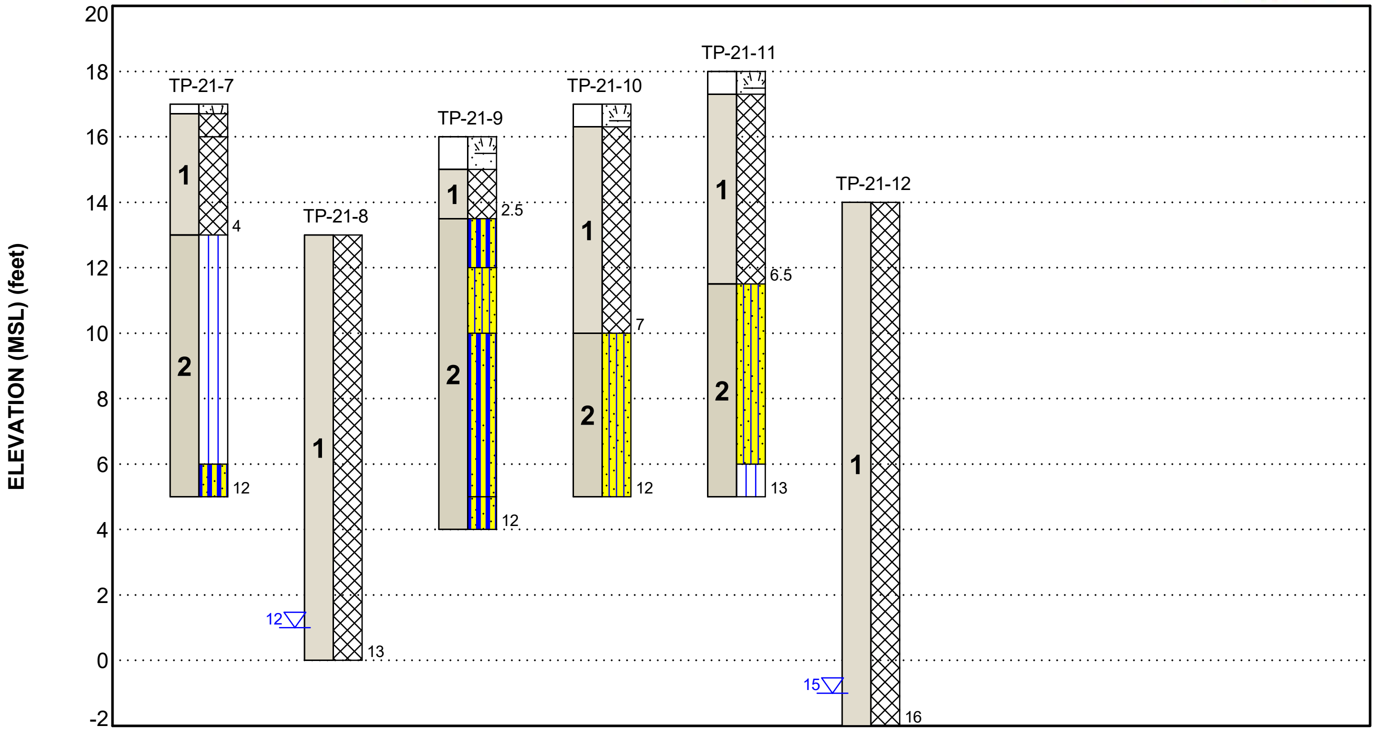
GEOMODEL

Proposed Marmen Manufacturing Facility ■ Glenmont, NY
Terracon Project No. JB215020



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Model Layer	Layer Name	General Description
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5	Bedrock	Shale bedrock. Upper few feet relatively weathered.

LEGEND

- ▤ Topsoil
- ⊠ Fill
- ▥ Silt
- ▥ Sandy Silt
- ▥ Silty Sand

▽ First Water Observation
▽ Second Water Observation

Groundwater levels are temporal. The levels shown are representative of the date and time of our exploration. Significant changes are possible over time.
Water levels shown are as measured during and/or after drilling. In some cases, boring advancement methods mask the presence/absence of groundwater. See individual logs for details.

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Numbers adjacent to soil column indicate depth below ground surface.

ATTACHMENTS

EXPLORATION AND TESTING PROCEDURES

Field Exploration

Boring Nos.	Boring Depth (feet)	Location
B-21-7 thru B-21-23, B-21-28	30.1 to 165.0	Proposed building footprints

Test Pit Nos.	Test Pit Depth (feet)	Location
TP-1 thru TP-12	11.5 to 16	Proposed building footprints and yard area

Test Location Layout and Elevations: The test boring and test pit locations were selected on the basis of the preliminary plant layout provided to us and were established in the field by Terracon using a hand-held GPS unit, taped measurements and/or visual reference from existing site features. The boreholes and test pits were located as planned, within the limitations of access, existing structures and/or utilities.

Ground surface elevation at each borehole/test pit location was estimated based upon our interpolation between topographic contours shown on the site plans provided to us. If more precise locations and/or elevations are desired, the as-completed test locations should be surveyed.

Subsurface Exploration Procedures: The test borings were made using a standard rotary drill rig equipped with hollow-stem augers, flush-joint casing and rock core tooling. As the borehole was advanced, the soils were generally sampled at intervals of five feet or less in accordance with the Standard Method for Penetration Test and Split-Barrel Sampling of Soils, ASTM D1586. In the split-barrel sampling procedure, a standard 2-inch outer diameter split-barrel sampling spoon is driven into the ground by a 140-pound automatic hammer falling 30-inches. The number of blows required to advance the sampling spoon the middle 12-inches of a normal 24-inch penetration is recorded as the Standard Penetration Test (SPT) resistance value. The SPT resistance values, also referred to as N-values, are indicated on the subsurface logs at the corresponding test depths.

A total of three undisturbed Shelby tube samples were taken (or attempted) in the silt and clay (or otherwise soft subgrade soils) as indicated on the boring logs.

Upon meeting refusal, the refusal material was typically cored to allow its characterization. The coring was completed in general accordance with ASTM D2113 – Standard Practice for Rock Core Drilling and Sampling of Rock for Site Investigation using an NQ-size double tube core barrel.

The boreholes were backfilled with auger cuttings and/or sand upon their completion.

Our exploration team prepared field boring logs as part of the drilling operations. These field logs included visual classifications of the materials encountered during drilling and our interpretation of the subsurface conditions between samples. The sampling depths, penetration distances, and other sampling information were recorded on the field boring logs.

The soil and rock core samples were placed in appropriate containers and taken to our soils laboratory for visual classification by a geologist or geotechnical engineer. The soils were described based on the material's color, texture and plasticity in general accord with the Unified Soil Classification System (USCS) as summarized herein. Rock classification was conducted using locally accepted practices for engineering purposes; petrographic analysis may reveal other rock types. Final individual boring logs were prepared, and they represent the Geotechnical Engineer's interpretation of the field logs and include modifications as appropriate based on observations and/or testing of the samples in our laboratory.

The test pits were excavated using a track excavator and observed by a geotechnical engineer from our office. The soils at the test pit locations were classified as the excavations were made and were logged as described above. Upon the completion of each test pit, the excavation was methodically backfilled in lifts, with each lift tamped with the excavator bucket.

The subsurface logs for the test borings and test pits are presented herein, along with a summary sheet and key which explains the terms and symbols used in their preparation.

Laboratory Testing

Selected recovered samples from the test borings were submitted for laboratory testing as part of the subsurface investigation, to confirm the visual classifications and to provide quantitative index properties for use in the geotechnical evaluation. This testing was performed in general accordance with the following standard methods:

- ASTM D2216 - Standard Test Methods for Laboratory Determination of Water (Moisture) Content of Soil - and Rock by Mass (35 samples tested)
- ASTM D422 - Standard Test Method for Particle-Size Analysis of Soils (w/o hydrometer) (16 samples tested)
- ASTM D422 - Standard Test Method for Particle-Size Analysis of Soils (w/ hydrometer) (8 samples tested)
- ASTM D4318 - Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils (7 samples tested)
- ASTM D2974 - Standard Test Methods for Determining the Water (Moisture) Content, Ash Content, and Organic Material of Peat and Other Organic Soils (4 samples tested)

- ASTM D1557 - Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (4 samples tested)

SITE LOCATION AND EXPLORATION PLANS

Contents:

Site Location Plan
Exploration Plan

Note: All attachments are one page unless noted above

SITE LOCATION

Proposed Marmen Manufacturing Facility ■ Glenmont, NY
February 2022 ■ Terracon Project No. JB215020

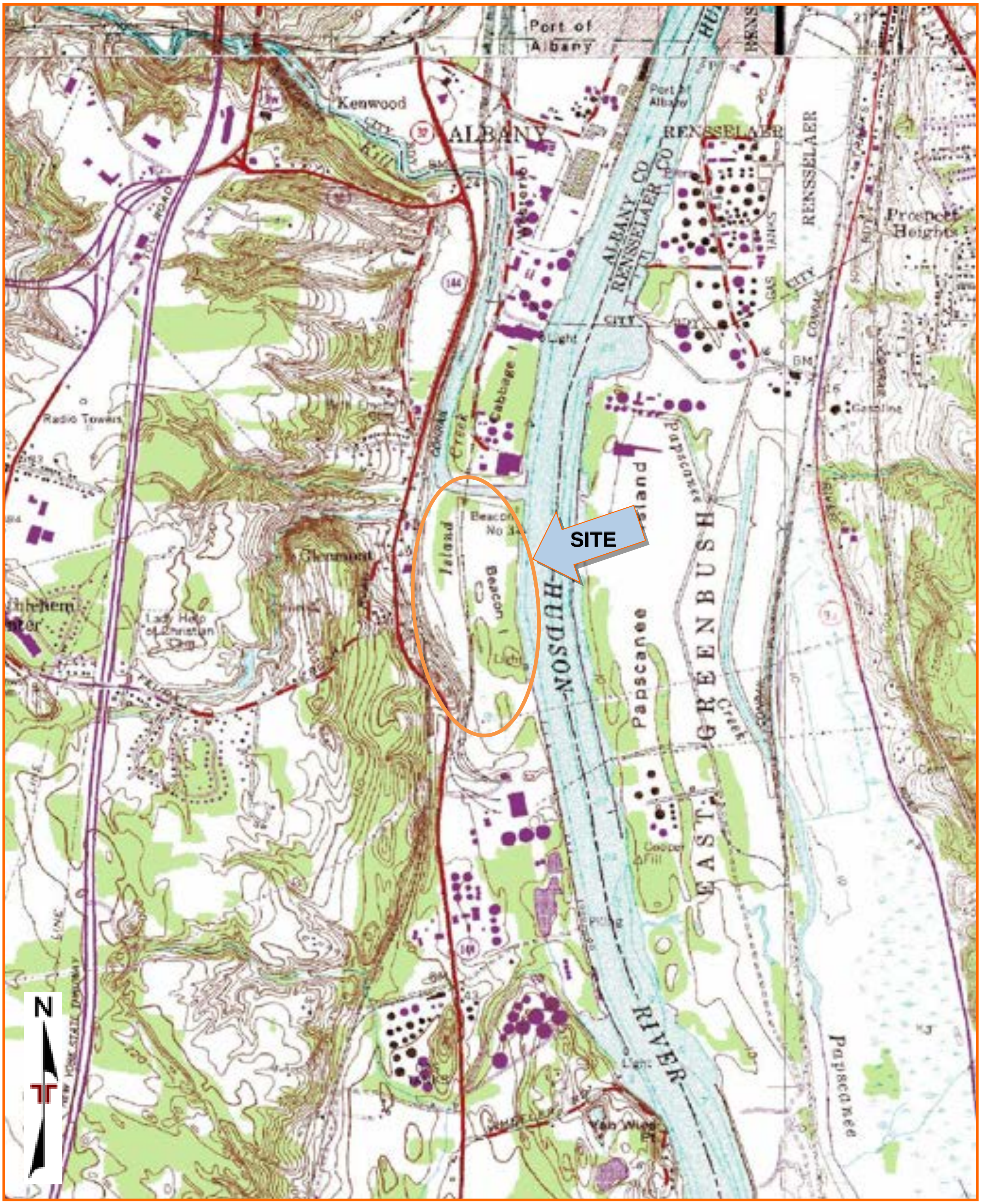
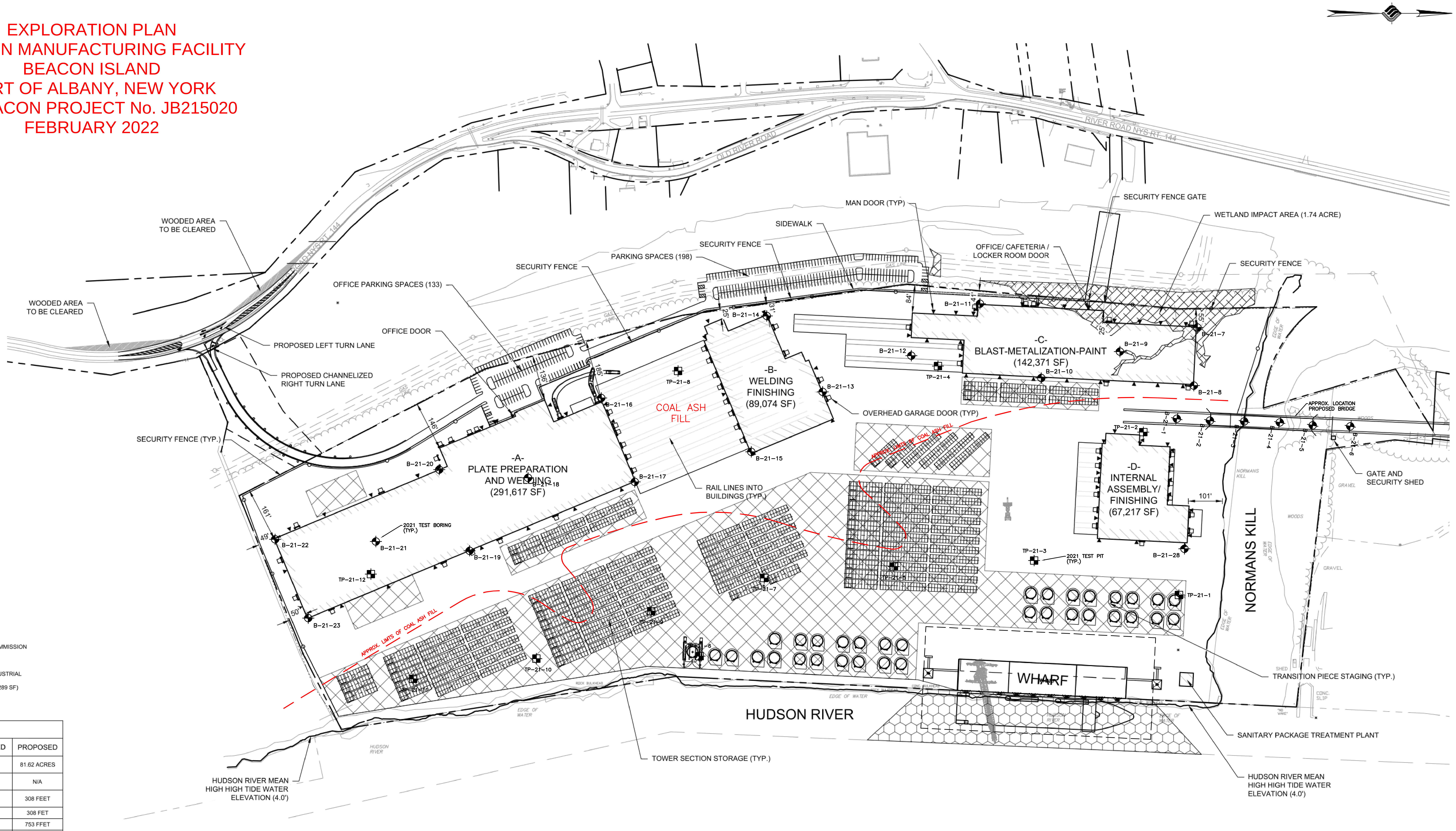


DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS
NOT INTENDED FOR CONSTRUCTION PURPOSES

TOPOGRAPHIC MAP IMAGE COURTESY OF THE U.S. GEOLOGICAL SURVEY
QUADRANGLES INCLUDE: ALBANY, NY (1/1/1994), TROY SOUTH, NY (1/1/1980),
DELMAR, NY (1/1/1980) and EAST GREENBUSH, NY (1/1/1980).

EXPLORATION PLAN
MARMEN MANUFACTURING FACILITY
BEACON ISLAND
PORT OF ALBANY, NEW YORK
TERRACON PROJECT No. JB215020
FEBRUARY 2022



PROJECT DATA

1. APPLICANT / LAND OWNER:
   ALBANY PORT DISTRICT COMMISSION
   106 SMITH BOULEVARDS
   ALBANY, NEW YORK 12205
2. EXISTING ZONING: HEAVY INDUSTRIAL
3. LOT AREA: 81.62 ACRES (3,555,289 SF)
4. BUILDING HEIGHT: 85 FT

SITE DATA

FEATURE	REQUIRED	PROPOSED
MINIMUM LOT SIZE, NONRESIDENTIAL	5 ACRES	81.62 ACRES
MINIMUM FRONT YARD, FROM RIGHT-OF-WAY	100 FEET	N/A
MINIMUM FRONT YARD, FROM CENTER LINE	125 FEET	308 FEET
MINIMUM SIDE YARD	25 FEET	308 FET
MINIMUM REAR YARD	50 FEET	753 FFET
MINIMUM HIGHWAY FRONTAGE	150 FEET	2140 FEET
MAXIMUM HEIGHT	LESSER OF 4 STORIES OR 60 FEET	85 FEET*
MINIMUM LOT DEPTH	200 FEET	2850 FEET
MINIMUM LOT WIDTH	150 FEET	757 FEET
MAXIMUM LOT COVERAGE	30%	16.6%

* WILL REQUEST A VARIANCE

ZONING:

EXISTING: ±81.62 ACRES HEAVY INDUSTRIAL

PROPOSED: ±81.62 ACRES HEAVY INDUSTRIAL

TAX ACCOUNT NUMBERS: 98.00-2-10.23
98.01-2-1

PARKING:

1 SPACE FOR EACH 2 EMPLOYEES ON MAXIMUM WORKING SHIFT.

TOTAL EMPLOYEES = 350

REQUIRED:
175 TOTAL SPACES REQUIRED

PROVIDED:
CAR PARKING 331 SPACES PROVIDED

331 TOTAL SPACES PROVIDED

ADA SPACES REQUIRED:
PER 2010 ADA STANDARDS FOR ACCESSIBLE DESIGN

REQUIRED (331 SPACE LOT): 9 SPACES
PROVIDED (331 SPACE LOT): 9 SPACES

C:\USERS\USMEARING\ACCDOCS\MCFARLAND JOHNSON INC\PORT OF ALBANY EXPANSION\PROJECT FILES\06-CADD\CIVIL\SHEET FILES\18641.00-GEN-LAYO.DWG

LEGEND

PROPERTY LINE	— - - — - - —	PROPOSED BUILDING FOOTPRINT	▨
BUILDING SETBACK	— — — —	MATERIAL STORAGE AREAS	▨
EXISTING RAIL LINE	++++++++++	WETLAND IMPACT AREAS	▨
EXISTING TREE LINE	⌒⌒⌒⌒	WHARF DREDGING AREA	⬡
DITCH CENTERLINE	——		
ROADSIDE SWALE	— · —		
OVERHEAD DOORS	◘		
MAN DOORS	◂		

0 150 300 450 FT
SCALE

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
CONCEPT PLANS

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION

DRAWN JES
DESIGNED AJF
CHECKED AJF
SCALE 1"=40'
DATE MAY 2021
PROJECT 18641.00

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
GENERAL LAYOUT PLAN

DRAWING NUMBER
SP-00
---- OF 7

EXPLORATION RESULTS

Contents:

Test Boring and Test Pit Logs (47 pages)
Laboratory Test Results (26 pages)

Note: All attachments are one page unless noted above

BORING LOG NO. B-21-7

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6070° Longitude: -73.7673°

Approximate Surface Elev.: 8 (Ft.) +/-

MODEL LAYER	DEPTH		ELEVATION (Ft.)
1	10.0	**FILL - COAL ASH** , dark gray, moist to wet, very loose	-2+/-
2	12.0	**SILT WITH SAND (ML)**, seams of organics and fine to medium sand, gray to brown, wet, very soft	-4+/-
	21.0	**POORLY GRADED SAND (SP)**, occasional gray silty sand seams, fine to coarse grained, brown, wet, very loose to loose **Grades with trace gravel**	-13+/-
	25.0	**SANDY SILT (ML)**, with bands of clay, gray, wet, soft	-17+/-
3		**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft to soft **Grades to varved silt and clay from about 30-35'**	

DEPTH (Ft.)	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	☒	18	1-1-1-1 N=2		
	☒	22	1-1/12"-1 N=1		
5	☒	21	WH/24"		
	☒	24	WH/24"		99.4
	☒	24	WH/24"		
10	☒	22	WH/24"		
	☒	22	WH/12"-2-4 N=2		
15	☒	19	2-3-2-3 N=5		
20	☒	19	WH/12"-3-2 N=3		
25	☒	22	WH/12"-4-3 N=4		
30	☒	24	WH/24"		
35	☒	22	WH/24"		
40	☒	24	WH/24"		
45	☒	24	WH/24"		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 80', NQ core barrel to 85'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer
WR = Weight of Rods

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 06-28-2021

Boring Completed: 06-28-2021

Drill Rig: Diedrich D-50

Driller: S. Morey

Project No.: JB215020

BORING LOG NO. B-21-7

Page 2 of 2

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6070° Longitude: -73.7673°

Approximate Surface Elev.: 8 (Ft.) +/-

MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)
3		**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft to soft *(continued)*	
	60.0	**SILT (ML)**, trace clay, gray, wet, soft	-52+/-
	65.0	**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft	-57+/-
4	67.1	**SILTY GRAVEL (GM)**, frequent cobbles and boulders, gray, wet, dense, (GLACIAL TILL) **Difficult rollerbit advancement noted at about 67.1'**	-59+/-
5	75.0	**WEATHERED SHALE**	-67+/-
	80.0	**SHALE**, gray, moderately weathered with occasional 1-2" thick completely weathered bands, very close to close fracture spacing with high angle joints and bedding, very poor RQD **Graywacke layer from about 83-83.5'** **Frequent siltstone lenses from 83.5-85'**	-72+/-
	85.0	***Boring Terminated at 85 Feet***	-77+/-

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
50		☒	24	WR/24"		
55		☒	22	WR/18"-WH		
60		☒	6	3-2-2-2 N=4		26.4
65		☒	24	WR/12"-WH/12"		
70		☒	5	47-17-16-17 N=33		
75		☒	1	50/2"		
80		■	37	REC=62% RQD=8%		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 80', NQ core barrel to 85'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 06-28-2021	Boring Completed: 06-28-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-8

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6070° Longitude: -73.7667°

Approximate Surface Elev.: 10 (Ft.) +/-

MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)
1		**FILL - SILT WITH SAND (ML)**, occasional clay seams, trace roots and organic seams, gray, moist, medium stiff	
	3.5		6.5+/-
		FILL - COAL ASH , fine to medium grained, dark gray, moist to wet, very loose to loose	
	10.0		0+/-
2		**SILT (ML)**, little organics, gray, wet, very soft **Grades to pieces of wood, gray to brown at 10.5'** **Grades to occasional fine to medium sand and clay seams**	
	14.0		-4+/-
		POORLY GRADED SAND (SP), trace silt, fine to medium grained, brown, wet, very loose	
	16.0		-6+/-
		POORLY GRADED SAND WITH SILT (SP-SM), fine to medium grained, brown, wet, loose	
	31.0	**Grades to trace gravel**	-21+/-
3		**VARVED SILT AND CLAY (CL-ML)**, gray, wet, very soft to stiff	

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	20	2-4-4-4 N=8		22.8
		☒	19	4-4-5-6 N=9		
5		☒	17	2-2-1-1 N=3		
		☒	18	WH/12"-2-2 N=2		
		☒	24	WH/24"		
10		☒	24	WH/24"	13.5	53.4
		☒	24	WH/24"		
15		☒	18	WH/12"-2-2 N=2		
		☒	24	2-2-2-2 N=4		
20		☒	24	WH-2-2-2 N=4		
25		☒	22	2-3-4-5 N=7		26.8
30		☒	20	2-3-6-4 N=9		
35		☒	24	WH/24"		
40		☒	24	WH/24"		
45		☒	24	WH/24"		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 84', NQ core barrel to 89'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer
WR = Weight of Rods

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-07-2021	Boring Completed: 07-09-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-8

Page 2 of 2

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6070° Longitude: -73.7667°

Approximate Surface Elev.: 10 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	DESCRIPTION	ELEVATION (Ft.)
3	▨		**VARVED SILT AND CLAY (CL-ML)**, gray, wet, very soft to stiff *(continued)* **Grades to banded silt and clay**	
		65.0		-55+/-
3			**SILT (ML)**, occasional clay bands and fine sand partings, gray, wet, medium stiff to stiff	
		75.0		-65+/-
4			**CLAYEY GRAVEL WITH SAND (GC)**, gray, wet, medium dense to dense, (GLACIAL TILL)	
		84.0		-74+/-
5			**SHALE**, gray, occasional quartz veins, slightly weathered, weak, close to moderate fracture spacing with high angle joints and fractures, poor RQD	
		89.0		-79+/-
			Boring Terminated at 89 Feet	

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
50		☒	24	WH/24"		
55		☒	24	WR/12"-WH/12"		
60		☒	24	WR/24"		
65		☒	24	WR/12"-5-5 N=5		
70		☒	7	5-5-7-7 N=12		
75		☒	22	WH-7-16-17 N=23		
80		☒	19	6-11-22-26 N=33		
85		▮	58	REC=97% RQD= 47%		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 84', NQ core barrel to 89'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-07-2021	Boring Completed: 07-09-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-9

PROJECT: Proposed Marmen Manufacturing Facility

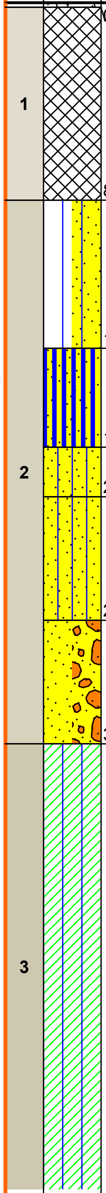
CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6064° Longitude: -73.7671°

Approximate Surface Elev.: 9 (Ft.) +/-



MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)
1	0.2	**TOPSOIL**	9+/-
	8.0	**FILL - COAL ASH**, dark gray, moist to wet, very loose **Little organics noted**	1+/-
2	14.0	**SILT WITH SAND (ML)**, little organics, gray to brown, wet, very soft **Fine to medium grained sand lenses from about 10 to 14'**	-5+/-
	18.0	**SANDY SILT (ML)**, with clay, trace organics, gray to brown, wet, very soft **Grades to trace gravel**	-9+/-
	20.0	**SILTY SAND (SM)**, trace gravel, fine to medium grained, gray to brown, wet, very loose	-11+/-
	25.0	**SILTY SAND (SM)**, with clay partings (approx. 1/8" thick), fine grained, brown, wet, very loose	-16+/-
	30.0	**POORLY GRADED SAND WITH GRAVEL (SP)**, trace silt, fine to coarse grained, brown, wet, loose	-21+/-
3		**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft	

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	4	WH/24"		
		☒	22	WH/24"		
5		☒	1	WH/24"		
		☒	19	WH/24"		
		☒	24	WH/24"		
10		☒	22	WH/24"		
		☒	24	WH/24"		
15		☒	19	WH/24"		
		☒	24	WH/24"		
		☒	18	WH/18"-2		
20		☒	21	2-1-1/12" N=1		
25		☒	21	4-5-4-5 N=9		
30		☒	24	WH/24"		
35		☒	24	WH/24"		
40		☒	22	WH/24"		
45		☒	24	WH/24"		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 78', NQ core barrel to 82'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer
WR = Weight of Rods

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-29-2021	Boring Completed: 07-29-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-9

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6064° Longitude: -73.7671°

Approximate Surface Elev.: 9 (Ft.) +/-

MODEL LAYER	DEPTH		ELEVATION (Ft.)
3		**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft *(continued)*	
	60.0	**SILT (ML)**, occasional clay bands, gray, wet, very soft to medium stiff	-51+/-
	70.0	**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft	-61+/-
4	75.0	**CLAYEY GRAVEL WITH SAND (GC)**, occasional cobbles and boulders, gray, wet, very dense, (GLACIAL TILL)	-66+/-
5	78.0	**SANDSTONE**, gray, slightly weathered, medium strong, moderate fracture spacing with high angle joints and fractures, fair RQD	-69+/-
	80.0	**SHALE**, with quartz veins, gray, slightly weathered, weak rock, very close to close fracture spacing with high angle joints and fractures, fair RQD	-71+/-
	82.0	***Boring Terminated at 82 Feet***	-73+/-

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
50		☒	24	WR/12"-WH/12"		
55		☒	24	WR/18"-WH		
60		☒	24	6-4-3-1 N=7		
65		☒	24	WH/12"-1-1 N=1		
70		☒	24	WH/24"		
75		☒	4	50/4"		
80		▮	42	REC=88% RQD=58%		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 78', NQ core barrel to 82'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-29-2021	Boring Completed: 07-29-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-10

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6057° Longitude: -73.7668°

Approximate Surface Elev.: 11 (Ft.) +/-

MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)
1		**FILL - COAL ASH** , dark gray, moist to wet, very loose **Trace rootlets noted** **Little organics noted**	
	12.0		-1+/-
2		**SILT (ML)**, little organics, occasional sand lenses, brown, wet, very soft **Aquatic shells encountered at about 13'**	
	16.0		-5+/-
		SILTY SAND (SM), trace organics, fine to medium grained, gray to brown, wet, loose	
	25.0		-14+/-
		POORLY GRADED SAND (SP), trace gravel, fine to medium grained, brown, loose	
	30.0		-19+/-
3		**VARVED SILT AND CLAY (CL-ML)**, gray, wet, very soft	

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	18	1-1-2-1 N=3		
		☒	12	1/12"-1/12" N=1		
5		☒	5	WH/24"		31.7
		☒	2	WH/24"		
		☒	22	WH/24"		
10		☒	24	WH/24"	8.2	46.7
		☒	24	WH/24"		
15		☒	18	WH/18"-1		
		☒	24	1-2-2-3 N=4		34.9
20		☒	21	2-2-2-2 N=4		
25		☒	18	3-4-4-3 N=8		
30		☒	24	WH/24"		
35						
40		☒	24	WH/24"		
45						

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 82', NQ core barrel to 87'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer
WR = Weight of Rods

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-25-2021	Boring Completed: 08-25-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-10

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6057° Longitude: -73.7668°

Approximate Surface Elev.: 11 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
3		**VARVED SILT AND CLAY (CL-ML)**, gray, wet, very soft *(continued)* **Grades to banded silt and clay**		50		☒	24	WH/24"		
		60.0	-49+/-	60		☒	24	WH/24"		
		SILT (ML), occasional clay bands, gray, wet, very soft to medium stiff		70		☒	24	3-2-3-5 N=5		
		80.0	-69+/-							
5		**PROBABLE WEATHERED ROCK**		80		☒	2	50/2"		
		82.0	-71+/-							
		SHALE, frequent siltstone lenses and occasional quartz veins, slightly weathered (highly weathered 82 to 82.4' and 85.2 to 85.8'), weak rock, close to moderate fracture spacing with high angle joints, fair RQD		85		▮	60	REC=100% RQD=56%		
		87.0	-76+/-							
		Boring Terminated at 87 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 82', NQ core barrel to 87'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-25-2021	Boring Completed: 08-25-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/11/21

BORING LOG NO. B-21-11

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6052° Longitude: -73.7676°

Approximate Surface Elev.: 11 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)
1	▨		**FILL - COAL ASH**, dark gray, moist to wet, very loose **Little organics noted**	
		12.0		-1+/-
2	▨		**POORLY GRADED SAND (SP)**, trace silt seams and trace organics, fine to medium grained, gray to brown, wet, very loose	
		20.0		-9+/-
4	▨		**SILTY SAND WITH GRAVEL (SM)**, occasional cobbles and boulders, gray, wet, very dense, (GLACIAL TILL)	
		30.1		-19+/-
			Sampler Refusal at 30.1 Feet	

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	1	WH/24"		
		☒	1	WH/24"		
5		☒	22	WH/24"		
		☒	24	WH/24"		
		☒	24	WH/24"		
10		☒	22	WH/24"		
		☒	21	WH/12"-2-2 N=2		
15		☒	17	WH-2-1-1 N=3		
		☒	19	2-2-1-2 N=3		
20		☒	7	29-50/2"		
25			1	50/1"		
30			1	50/1"		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 30'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 09-01-2021	Boring Completed: 09-01-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-12

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6047° Longitude: -73.7670°

Approximate Surface Elev.: 11 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	DESCRIPTION	ELEVATION (Ft.)
1	▩	15.0	**FILL - COAL ASH** , dark gray, moist to wet, very loose **Trace rootlets noted** **Little organics noted**	-4+/-
2	▨	25.0	**SILTY CLAY WITH SAND (CL-ML)**, trace to little organics, gray to brown, wet, very soft	-14+/-
2	▒	30.5	**POORLY GRADED SAND (SP)**, trace silt and gravel, fine to coarse grained, gray to brown, wet, loose	-19.5+/-
3	▨	40.0	**BANDED SILT AND CLAY (CL-ML)**, gray, wet, soft	-29+/-
4	▨	45.0	**CLAYEY SAND WITH GRAVEL (SC)**, frequent cobbles and boulders, gray, wet, very dense, (GLACIAL TILL)	-34+/-
5	▧	46.3	**SHALE**, with frequent siltstone lenses and quartz veins, gray to black, slightly weathered, weak rock, very close fracture spacing with high angle joints/fractures, poor RQD	-35.5+/-

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	18	1-1-2-1 N=3		
		☒	21	1-1-1-1 N=2		
5		☒	24	1-1/12"-1 N=1		
	▽	☒	12	1/24"		
		☒	19	WH/24"		
10		☒	19	WH/18"-2		
		☒	10	WH/24"		
15		☒	24	WH/24"		
20		☒	22	WH/24"		
25		☒	13	3-3-2-2 N=5		
30		☒	21	4-2-2-2 N=4		
35		☒	24	4-2-1-1 N=3		
40		☒	2	50/2"		
45		▮	50	REC=83%		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 45', NQ core barrel to 50'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer

WATER LEVEL OBSERVATIONS

▽ *6.6' after 8-10' sample*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-31-2021	Boring Completed: 08-31-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-12

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PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6047° Longitude: -73.7670°

Approximate Surface Elev.: 11 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
5	▨	**GRAYWACKE**, fine-grained, gray to black, slightly weathered, medium strong, moderate fracture spacing with high angle joints/fractures, poor RQD *(continued)* 50.0	-39+/-	50		▮		RQD=38%		
		Boring Terminated at 50 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 45', NQ core barrel to 50'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

▽ *6.6' after 8-10' sample*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-31-2021

Boring Completed: 08-31-2021

Drill Rig: Diedrich D-50

Driller: S. Morey

Project No.: JB215020

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/11/21

BORING LOG NO. B-21-13

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE:
River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6039° Longitude: -73.7666°

Approximate Surface Elev.: 12 (Ft.) +/-

MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)
1		**FILL - COAL ASH** , trace rootlets, dark gray, moist to wet, very loose **Little wood/organics nioted**	
	15.0		-3+/-
2		**SILT WITH SAND (ML)**, little organics, gray to brown, wet, very soft	
	20.0		-8+/-
		SILTY SAND (SM), trace organics, fine to medium grained, gray, wet, very loose to loose **Grades to trace gravel at about 25'**	
	31.0		-19+/-
3		**BANDED SILT AND CLAY (CL-ML)**, gray, wet, soft	
	32.0	***Boring Terminated at 32 Feet***	-20+/-

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
0-2		☒	4	WH-1-2-1 N=3		
2-4		☒	22	1-1-1-1 N=2		
4-6		☒	21	WH/24"		
6-8		☒	24	WH/24"		
8-10		☒	24	WH/24"		
10-12		☒	21	1-1/12"-1 N=1		
15-17		☒	19	WH/24"		
20-22		☒	18	1-1/12"-1 N=1		
25-27		☒	17	WH-3-2-2 N=5		
30-32		☒	21	4-3-2-2 N=5		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 30'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 09-07-2021	Boring Completed: 09-07-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-14

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6035° Longitude: -73.7675°

Approximate Surface Elev.: 13 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)
1	▦	14.0	**FILL - COAL ASH** , trace rootlets, dark gray, moist to wet, very loose	-1+/-
2	▥	20.0	**SILT (ML)**, with clay, trace sand, little organics, gray, wet, very soft	-7+/-
2	▥	25.0	**SANDY SILT (ML)**, trace organics, brown to gray, wet, very soft	-12+/-
2	▥	30.0	**SILTY SAND (SM)**, fine to medium grained, gray, wet, loose	-17+/-
3	▨	32.0	**BANDED SILT AND CLAY (CL-ML)**, gray, wet, soft	-19+/-
			Boring Terminated at 32 Feet	

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	1	WH/18"-1		
		☒	2	1-1-1-1 N=2		
5		☒	0	WH/18"-1		
		☒	17	WH/24"		
		☒	24	WH/24"		
10		☒	7	WH-1-1/12" N=1		
		☒	18	WH/24"		
15		☒	22	WH/24"		
		■				
20		☒	14	WH/18"-2		
25		☒	19	2-2-2-3 N=4		
30		☒	24	3-2-2-3 N=4		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 30'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 09-07-2021	Boring Completed: 09-07-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/11/21

BORING LOG NO. B-21-15

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6034° Longitude: -73.7660°

Approximate Surface Elev.: 12 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	▩		**FILL - COAL ASH** , dark gray, moist to wet, very loose				☒	10	WH/24"		
1	▩						☒	14	WH/24"		
	▩	6.0		6+/-	5		☒	13	WH-1-1/12" N=1		
	▨	9.0	**SILT WITH SAND (ML)**, some organics, gray to brown, moist, very soft to soft	3+/-			☒	22	1-2-2-2 N=4		
	▨		**SILTY SAND (SM)**, some roots and organics, gray to brown, wet, very loose		10		☒	24	WH/24"		
	▨	12.0		0+/-		▽	☒	18	WH/24"		
	▨	15.0	**SILT WITH SAND (ML)**, some organics, dark brown, moist, very soft	-3+/-			☒	22	WH/24"		
	▨		**POORLY GRADED SAND (SP)**, trace silt bands, fine to medium grained, brown, wet, loose to medium dense		15		☒	21	3-3-3-3 N=6		
2	▨		**Grades to grayish brown**		20		☒	18	2-3-2-2 N=5		
	▨		**Grades to trace gravel**		25		☒	19	4-4-6-8 N=10		
	▨	35.0		-23+/-	30		☒	19	4-3-4-6 N=7		
	▧		**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft		35		☒	24	WH/24"		
	▧				40		☒	24	WH/24"		
3	▧				45						

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 99', NQ core barrel to 104'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer
WR = Weight of Rods

WATER LEVEL OBSERVATIONS

▽ *10.9' after weekend with drillhead at 35'*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-18-2021	Boring Completed: 08-18-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-15

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6034° Longitude: -73.7660°

Approximate Surface Elev.: 12 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
3		**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft *(continued)*							
			50		☒	24	WR/12"-WH/12"		
			55						
		Grades to varved	60		☒	24	WR/24"		
			65						
		Grades to banded	70		☒	6	WR/24"		
			75						
		80.0 -68+/-							
		SILT (ML), trace rootlets, occasional clay bands, gray, wet, soft to stiff	80		☒	24	7-7-5-7 N=12		
			85						
			90		☒	24	WH/12"-1-1 N=1		
			95						

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 99', NQ core barrel to 104'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

▽ *10.9' after weekend with drillhead at 35'*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-18-2021	Boring Completed: 08-18-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-15

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan Latitude: 42.6034° Longitude: -73.7660° Approximate Surface Elev.: 12 (Ft.) +/- DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
3		97.0 / -85+/-							
5		**WEATHERED ROCK** 99.0 / -87+/-							
		SHALE and GRAYWACKE, alternating bands (approx. 6-12" thick) of shale and graywacke, gray, slightly weathered, weak to medium-strong, very close to moderate fracture spacing with high angle joints, poor RQD 104.0 / -92+/-	100			0 48	50/0" REC=80% RQD=47%		
		Boring Terminated at 104 Feet							

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 99', NQ core barrel to 104'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

▽ *10.9' after weekend with drillhead at 35'*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-18-2021	Boring Completed: 08-18-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-16

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6021° Longitude: -73.7666°

Approximate Surface Elev.: 14 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
1		**FILL - COAL ASH** , trace rootlets, dark gray, moist to wet, very loose			☒	19	1-1-2-1 N=3		
					☒	18	1-1-1-1 N=2		
			5		☒	22	1-1-1-1 N=2		
					☒	24	WH/12"-1-1 N=1		
					☒	24	WH/24"		
			10		☒	18	1-1/18"		
					☒	13	WH/24"		
		Little organics noted	15		☒	22	WH/24"		
		19.0 -5+/-							
2		**CLAYEY SAND (SC)**, fine to coarse grained sand with clay bands, brown, wet, very loose	20		☒	18	2-2-1-1 N=3		
		25.0 -11+/-							
		SILTY SAND (SM), trace gravel, fine to medium grained, gray, wet, loose	25		☒	19	2-3-3-4 N=6		
		30.0 -16+/-							
3		**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft	30		☒	24	WH/24"		
					■	0			
			35		■	15			
		36.0 -22+/-							
		Boring Terminated at 36 Feet							

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 32'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 09-08-2021	Boring Completed: 09-08-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-17

Page 1 of 3

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6024° Longitude: -73.7657°

Approximate Surface Elev.: 13 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
1			**FILL - COAL ASH** , dark gray, moist to wet, very loose to loose				☒	0	2-3-3-5 N=6		
							☒	10	2-1-2-2 N=3		
					5		☒	13	2-1-1-1 N=2		
							☒	21	WH/24"		
							☒	18	WR-WH/18"		
		12.5		0.5+/-	10		☒	22	WH/24"		34.6
2			**SILTY SAND (SM)**, little organics, gray to brown, wet, very loose to medium dense				☒	22	WH/24"		
			Grades to trace gravel		15		☒	24	WH/12"-4-2 N=4		48.3
							☒	24	4-4-6-6 N=10		
		20.0		-7+/-							
			POORLY GRADED SAND (SP), fine to medium grained, brown, wet, loose		20		☒	21	3-3-3-3 N=6		
			Grades to fine to coarse, trace gravel		25		☒	19	3-4-5-7 N=9		20.3
		30.0		-17+/-							
			SILTY SAND (SM), gray to brown, wet, loose		30		☒	19	4-4-4-4 N=8		
		35.0		-22+/-							
3			**VARVED SILT AND CLAY (CL-ML)**, gray, wet, very soft		35		☒	24	WH/24"		
					40		☒	24	WH/24"		30.5
					45						

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 112', NQ core barrel thru boulder seam 90 to 105'. NQ core barrel to 122'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer
WR = Weight of Rods

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-11-2021	Boring Completed: 08-11-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/11/21

BORING LOG NO. B-21-17

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6024° Longitude: -73.7657°

Approximate Surface Elev.: 13 (Ft.) +/-

MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)
3		**VARVED SILT AND CLAY (CL-ML)**, gray, wet, very soft *(continued)* **Grades to banded**	
3	70.0	**SILT (ML)**, occasional clay bands, gray, wet, very soft to soft	-57+/-
4	90.0	**CLAYEY SAND WITH GRAVEL (SC)**, occasional to frequent cobbles and boulders, gray **Hard sampler refusal at about 90', cored through frequent cobble and boulder seams in the glacial till from about 90 to 105'**	-77+/-

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
50		☒	24	WR/12"-WH/12"		
60		☒	24	WH/24"		
70		☒	24	WR/18"-WH		
80		☒	24	4-1-1-12 N=2		17.5
90			0	50/0"		
90-95			60	REC=100%		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 112', NQ core barrel thru boulder seam 90 to 105'. NQ core barrel to 122'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-11-2021	Boring Completed: 08-11-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-17

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE:
River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6024° Longitude: -73.7657°

Approximate Surface Elev.: 13 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
4		**CLAYEY SAND WITH GRAVEL (SC)**, occasional to frequent cobbles and boulders, gray *(continued)*	100			24	REC=40%		
			105			46	REC=76%		
		112.0 -99+/-	110						
5		113.0 **HIGHLY WEATHERED SHALE** -100+/-							
		SHALE, frequent siltstone lenses and graywacke bands, occasional quartz veins, gray, slightly to moderately weathered, weak rock to medium-strong, close to moderate fracture spacing with high angle joints, very poor to poor RQD **Core Run #1: Very Poor RQD**	115			30	REC=50% RQD=13%		
		Core Run #2: Poor RQD, highly weathered from 117 to 117.5', graywacke with quartz seams from 117.5 to 119.5' 122.0 -109+/-	120			42	REC=70% RQD=32%		
		Boring Terminated at 122 Feet							

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 112', NQ core barrel thru boulder seam 90 to 105'. NQ core barrel to 122'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-11-2021	Boring Completed: 08-11-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/11/21

BORING LOG NO. B-21-18

Page 1 of 3

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6015° Longitude: -73.7657°

Approximate Surface Elev.: 15 (Ft.) +/-

MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)
1		**FILL - COAL ASH**, dark gray, moist, very loose	
	8.5		6.5+/-
2		**SILT WITH SAND (ML)**, little organics, gray to brown, wet, very soft **Pieces of wood observed from 12-14'**	
	14.0		1+/-
		SANDY SILT (ML), little organics, occasional clay and sand lenses, brown, wet, very soft to soft	
	19.0		-4+/-
		SILT (ML), little organics, brown, wet, very soft	
	21.0		-6+/-
		SILTY SAND (SM), little organics, gray, wet, very loose	
	25.0		-10+/-
		POORLY GRADED SAND (SP), trace silt and gravel, fine to medium grained, gray, wet, loose	
	30.0		-15+/-
3		**SILTY SAND (SM)**, fine to medium grained, brown, wet, loose	
	35.0		-20+/-
		VARVED SILT AND CLAY (CL-ML), gray, wet, very soft	

SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
☒	12	1-1-1-1 N=2		
☒	2	WH/24"		
☒	1	WH/24"		
☒	2	WH/24"		
☒	22	WH/24"		
☒	22	WH/24"		44.8
☒	24	WH/24"	11.2	59.1
☒	6	1/12"-1/12" N=1		
☒	24	WH/12"-3-3 N=3		
☒	24	WH/24"		
☒	17	3-3-4-3 N=7		
☒	21	3-3-4-4 N=7		
☒	24	WR-WH/18"		35.5
☒	24	WR/18"-WH		

DEPTH (Ft.): 5, 10, 15, 20, 25, 30, 35, 40, 45

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 123', NQ core barrel to 128'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer
WR = Weight of Rods

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-04-2021	Boring Completed: 08-04-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/11/21

BORING LOG NO. B-21-18

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6015° Longitude: -73.7657°

Approximate Surface Elev.: 15 (Ft.) +/-

MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
3		**VARVED SILT AND CLAY (CL-ML)**, gray, wet, very soft *(continued)*		50						
		Grades to banded		55		☒	24	WR/18"-WH		89.9
				60						
	65.0	**SILT (ML)**, gray, wet, very soft	-50+/-	65		☒	24	WR/18"-WH		
				70						
	75.0	**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft	-60+/-	75		☒	22	WR-WH/18"		
				80						
				85		☒	24	WH/24"		
				90						
	95.0	**SILT (ML)**, occasional clay bands, gray, wet, medium stiff	-80+/-	95		☒		7-3-5-8		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 123', NQ core barrel to 128'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-04-2021	Boring Completed: 08-04-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-18

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6015° Longitude: -73.7657°

Approximate Surface Elev.: 15 (Ft.) +/-

MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		SILT (ML), occasional clay bands, gray, wet, medium stiff *(continued)*					24	N=8		
				100						
3	105.0	**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft - seam primarily silt noted	-90+/-	105			24	WR/24"		20.1
				110						
4	115.0	**CLAYEY SAND WITH GRAVEL (SC)**, occasional cobbles and boulders, gray, moist, very dense	-100+/-	115			7	28-50/5"		
				120						
5	124.0	**WEATHERED SHALE**	-109+/-							
	124.9	**GRAYWACKE**, occasional shale lenses, gray, slightly weathered, weak to medium strong, very close to moderate fracture spacing with high angle joints, poor RQD	-110+/-	125			48	REC=80% RQD=28%		
	128.0		-113+/-							
		Boring Terminated at 128 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 123', NQ core barrel to 128'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 08-04-2021	Boring Completed: 08-04-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-19

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6010° Longitude: -73.7649°

Approximate Surface Elev.: 15 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	ELEVATION (Ft.)
1	▨	**FILL - COAL ASH**, dark gray, moist to wet, very loose **Little organics noted** **Poor recovery 15-17', grades to occasional clayey seams** 18.0	-3+/-
2	░	**POORLY GRADED SAND (SP)**, trace silt and gravel, fine to medium grained, gray to brown, wet, loose **Grades to trace organics** 32.0	-17+/-

Boring Terminated at 32 Feet

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	12	WH/12"-1/12"		
		☒	14	WH/12"-1/12"		
5		☒	13	WH-1-1-1 N=2		
		☒	14	1-1-1-1 N=2		
		☒	14	1/12"-1/12" N=1		
10		☒	9	1-1/18"		
		☒	10	WH-1/12"-1 N=1		
15		☒	1	WH/24"		
20		☒	12	3-2-2-2 N=4		
25		☒	12	2-2-4-4 N=6		
30		☒	12	3-3-4-3 N=7		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 30'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 09-03-2021

Boring Completed: 09-03-2021

Drill Rig: Diedrich D-50

Driller: S. Morey

Project No.: JB215020

BORING LOG NO. B-21-20

PROJECT: Proposed Marmen Manufacturing Facility

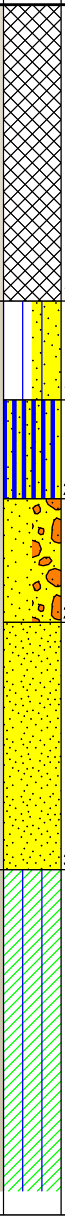
CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6008° Longitude: -73.7658°

Approximate Surface Elev.: 15 (Ft.) +/-



MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)
1	12.0	**FILL - COAL ASH** , dark gray, moist to wet, very loose **Pieces of wood, trace organics from about 4 to 7'**	3+/-
2	16.0	**SILT WITH SAND (ML)**, little organics, gray to brown, wet, very soft to soft	-1+/-
2	20.0	**SANDY SILT (ML)**, trace organics and roots, fine grained, brown, wet, very soft	-5+/-
2	25.0	**POORLY GRADED SAND WITH GRAVEL (SP)**, trace silt, fine to medium grained, brown, wet, loose	-10+/-
2	35.0	**POORLY GRADED SAND (SP)**, trace silt, fine to medium grained, brown, wet, loose to medium dense **Grades to fine to coarse grained, trace gravel**	-20+/-
3		**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft **Fine sand seam, trace roots at about 41'**	

WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	☒	4	1-1/12"-1 N=1		
	☒	18	1/12"-1/12" N=1		
	☒	7	WH/12"-1-2 N=1		
▽	☒	24	WH/24"		106.4
	☒	0	WH/24"		
	☒	22	WH/24"		
	☒	22	WH/24"		39.5
	☒	24	WH/12"-3-2 N=3		
	☒	24	WH/24"	3.7	44.7
	☒	21	3-4-3-4 N=7		
	☒	21	WH-3-4-4 N=7		
	☒	19	3-5-5-3 N=10		
	☒	24	WH/24"		
	☒	24	WH/24"		33.2

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 120', NQ core barrel thru boulders to 123', roller bit to 125'.

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer
WR = Weight of Rods

WATER LEVEL OBSERVATIONS

▽ *7.6' after 6-8' sample*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-20-2021	Boring Completed: 07-20-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-20

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6008° Longitude: -73.7658°

Approximate Surface Elev.: 15 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
3		**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft *(continued)*							
		Grades to varved silt and clay	50		☒	24	WR/12"-WH/12"		
			55						
			60		☒	24	WR/24"		
			65						
			70		☒	24	WR/18"-WH		
			75						
		Grades to banded silt and clay	80		☒	24	WR/12"-WH/12"		31.1
			85						
		Grades to varved silt and clay	90		☒	24	WR/18"-WH		
			95						

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 120', NQ core barrel thru boulders to 123', roller bit to 125'.

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

▽ *7.6' after 6-8' sample*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-20-2021	Boring Completed: 07-20-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-20

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6008° Longitude: -73.7658°

Approximate Surface Elev.: 15 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
3	▨	**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft *(continued)* 100.0	-85+/-							
		SILT (ML), occasional clay bands, gray, wet, very soft		100		☒	24	WH/24"		
				105						
		110.0	-95+/-							
	▨	**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft		110		☒	5	WR/24"		
				115						
		120.0	-105+/-							
4		**CLAYEY GRAVEL WITH SAND (SC)**, occasional to frequent cobbles and boulders, gray, moist to wet, very dense, (GLACIAL TILL) **Cored through frequent cobbles and boulders from 120 to 123'**		120		☒ ▮	12 17	50/5"		
				125		☒	12	73-31-50/5"		
		126.4	-111.5+/-							
		Sampler Refusal at 126.4 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 120', NQ core barrel thru boulders to 123', roller bit to 125'.

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

▽ *7.6' after 6-8' sample*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-20-2021	Boring Completed: 07-20-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-21

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6002° Longitude: -73.7650°

Approximate Surface Elev.: 14 (Ft.) +/-

MODEL LAYER	DEPTH		ELEVATION (Ft.)
1		**FILL - COAL ASH**, dark gray, moist to wet, very loose **Frequent roots from about 4 to 6', occasional to trace roots 6 to 8'**	
	14.0		0+/-
2		**SILT WITH SAND (ML)**, trace clay seams, occasional to trace organics, brown to black, wet, very soft **Little organics from 14-18'** **Few 0.5" thick organic bands noted**	
	30.0		-16+/-
		SILTY SAND (SM), trace organics, gray to brown, wet, very loose	
	32.0		-18+/-
		Boring Terminated at 32 Feet	

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	14	WH/24"		
		☒	15	WH/24"		
5		☒	7	1/12"-1/12" N=1		
		☒	24	1-1-1-1 N=2		
		☒	24	2-1-2-1 N=3		
10		☒	12	1-1/12"-1 N=1		
		☒	14	WH/24"		
15		☒	24	WH/24"		
		☒	24	WH/24"		
20		☒	18	WH/24"		
25		☒	24	WH/24"		
30		☒	22	WH/24"		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 30'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 09-02-2021	Boring Completed: 09-02-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-22

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.5994° Longitude: -73.7650°

Approximate Surface Elev.: 16 (Ft.) +/-

MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)
1	14.0	**FILL - COAL ASH**, dark gray, moist, very loose to loose **Becomes wet** **Little organics noted** **Reed-like stalk noted**	2+/-
2	20.0	**SILT WITH SAND (ML)**, some organics and rootlets/roots, brown to gray, wet, very soft **Trace aquatic shell fragments from 16 to 18'**	-4+/-
2	25.0	**SILTY SAND (SM)**, occasional organic silty seams, fine to medium grained, gray, wet, loose	-9+/-
2	30.0	**POORLY GRADED SAND (SP)**, fine to medium grained, brown, wet, medium dense	-14+/-
3	32.0	**VARVED SILT AND CLAY (CL-ML)**, gray grading to brown, wet, stiff	-16+/-
		Boring Terminated at 32 Feet	

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		☒	12	WH/24"		
		☒	18	WH-2-2-3 N=4		
5		☒	14	2-2-2-3 N=4		
		☒	24	2-2-2-2 N=4		
		☒	18	1-1-1-1 N=2		
10		☒	6	1-1-1/12" N=1		
		☒	10	WH/18"-1		
15		☒	10	WH/24"		
		☒	24	WH/24"		
20		☒	14	WH-5-4-6 N=9		
25		☒	12	2-2-11-14 N=13		
30		☒	24	4-6-5-7 N=11		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 30'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 09-03-2021	Boring Completed: 09-03-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-23

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.5997° Longitude: -73.7641°

Approximate Surface Elev.: 16 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
1		**FILL - COAL ASH**, trace roots, dark gray, moist, loose to medium dense			☒	1	4-4-2-1 N=6		
					☒	12	2-2-2-2 N=4		
			5		☒	12	1-2-2-3 N=4		
					☒	19	6-5-6-5 N=11		
		Becomes wet			☒	21	WH/24"		
		Trace organics noted	10		☒	24	WH/24"		
					☒	22	WH/24"		89.2
			15		☒	22	WH/24"		
					☒	19	WH/24"		
			20	▽	☒	24	WR/24"		
		Trace pieces of wood	25		☒	24	WH/18"-1		
		29.0 -13+/-							
2		**POORLY GRADED SAND (SP)**, fine to medium grained, brown, wet, loose to medium dense	30		☒	21	3-5-5-4 N=10		22.1
		Piece of wood encountered at about 32'							
			35		☒	19	3-3-3-4 N=6		
		40.0 -24+/-							
3		**VARVED SILT AND CLAY (CL-ML)**, gray, wet, very soft	40		☒	24	WR/12"-WH/12"		
			45		☒	24	WR-WH/18"		30.0

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 160', NQ core barrel to 165'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer
WR = Weight of Rods

WATER LEVEL OBSERVATIONS

▽ *21' after overnight with drillhead at ~90'*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-10-2021	Boring Completed: 07-13-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-23

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.5997° Longitude: -73.7641°

Approximate Surface Elev.: 16 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
3	▨	**VARVED SILT AND CLAY (CL-ML)**, gray, wet, very soft *(continued)*							
		Grades to banded silt and clay	50		☒	24	WH/24"		
		Grades to varved silt and clay	55		☒	24	WR/12"-WH/12"		
		Grades to banded silt and clay	60		☒	24	WR-WH/18"		
			65		☒	24	WR-WH/18"		
			70		☒	24	WR/12"-WH/12"		
			75						
			80		☒	24	WR/12"-WH/12"		
			85						
			90		☒	24	WR-WH/18"		
			95						

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 160', NQ core barrel to 165'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

∇ *21' after overnight with drillhead at ~90'*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-10-2021	Boring Completed: 07-13-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-23

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.5997° Longitude: -73.7641°

Approximate Surface Elev.: 16 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		VARVED SILT AND CLAY (CL-ML), gray, wet, very soft *(continued)*							
		Grades to varved silt and clay	100		☒	24	WR/18"-WH		
			105						
		Grades to occasional fine sand partings, medium-stiff at 110-112' sample	110		☒	24	5-3-3-7 N=6		20.6
			115						
3		**Grades to banded silt and clay**	120		☒	24	WR/24"		
			125						
		Grades to varved silt and clay	130		☒	24	WH/24"		
			135						
			140		☒	24	WR/24"		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 160', NQ core barrel to 165'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

▽ *21' after overnight with drillhead at ~90'*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-10-2021	Boring Completed: 07-13-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-23

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.5997° Longitude: -73.7641°

Approximate Surface Elev.: 16 (Ft.) +/-

MODEL LAYER	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
3		**VARVED SILT AND CLAY (CL-ML)**, gray, wet, very soft *(continued)*		145						
	150.0		-134+/-	150						
		SILT (ML), with occasional clay seams, gray, wet, very soft					24	WR/24"		
	155.0		-139+/-	155						
4		**SILTY SAND (SM)**, trace gravel, fine grained, gray, wet, very dense, (GLACIAL TILL)					15	40-50/5"		
	159.0		-143+/-							
	160.0	**WEATHERED SHALE**	-144+/-	160			0	50/0"		
5		**SHALE**, with quartz veins, slightly weathered, weak rock, very close to close fracture spacing with high angle fractures, poor RQD					58	REC=96% RQD=45%		
	165.0	**Sandstone and siltstone lenses from 164 to 165'**	-149+/-	165						
		Boring Terminated at 165 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 160', NQ core barrel to 165'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

∇ *21' after overnight with drillhead at ~90'*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-10-2021	Boring Completed: 07-13-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-28

Page 1 of 2

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6069° Longitude: -73.7649°

Approximate Surface Elev.: 20 (Ft.) +/-

MODEL LAYER	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
1	2.0	**POSSIBLE FILL - SILT WITH SAND (ML)**, some mottling, trace rootlets and gravel, occasional clayey seams, brown, moist, stiff	18+/-			☒	12	2-4-5-7 N=9		
3	8.0	**SILTY SAND (SM)**, trace rootlets and gravel, brown, moist, medium dense **Grades to very moist**	12+/-			☒	14	5-6-7-10 N=13		
				5		☒	19	21-10-9-9 N=19		
						☒	19	6-5-5-7 N=10		
	10.0	**SILTY SAND (SM)**, occasional fine to medium grained sand seams, trace organics, gray, wet, very loose	10+/-			☒	21	WH/18"-1		
	14.0	**CLAYEY SAND (SC)**, with wet, gray clay seams, fine to coarse grained, gray, wet, medium dense	6+/-	10		☒	22	8-10-11-12 N=21		
						☒	24	8-8-10-10 N=18		
	25.0	**SILT (ML)**, trace organics and clay, gray to brown, wet, very soft **Brown clay seams from 16-25'**	-5+/-	15		☒	24	WH/24"		
						☒	24	WH/24"		
				20		☒	24	WH/24"		
	45.0	**POORLY GRADED SAND (SP)**, trace silt, fine to medium grained, brown, wet, loose to medium dense	-25+/-	25		☒	21	3-2-4-3 N=6		
		Grades to fine to coarse sand		30		☒	19	7-5-4-5 N=9		
				35		☒	19	4-4-6-6 N=10		
		Grades to gray, trace gravel		40		☒	24	4-6-5-5 N=11		
		POORLY GRADED SAND WITH SILT (SP-SM), trace gravel, fine to medium grained, gray, wet, medium dense		45		☒	18	6-7-7-4 N=14		

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 85.2', NQ core barrel to 90.2'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

Logged by: JCH
WH = Weight of Hammer
WR = Weight of Rods

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-01-2021	Boring Completed: 07-01-2021
Drill Rig: Diedrich D-50	Driller: S. Morey
Project No.: JB215020	

BORING LOG NO. B-21-28

Page 2 of 2

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6069° Longitude: -73.7649°

Approximate Surface Elev.: 20 (Ft.) +/-

MODEL LAYER	DEPTH		ELEVATION (Ft.)
3		**POORLY GRADED SAND WITH SILT (SP-SM)**, trace gravel, fine to medium grained, gray, wet, medium dense *(continued)*	
3	51.5	**SILT (ML)**, occasional fine to medium sand seams, trace gravel, gray, wet, stiff	-31.5+/-
4	55.0	**BANDED SILT AND CLAY (CL-ML)**, gray, wet, very soft **Trace organics noted 75-77'**	-35+/-
5	80.0	**SILTY GRAVEL WITH SAND (GM)**, gray, wet, dense, (GLACIAL TILL)	-60+/-
6	84.0	**WEATHERED SHALE**	-64+/-
6	85.2	**SHALE**, gray, occasional quartz veins, slightly weathered, weak rock, close fracture spacing with high angle joints and bedding, poor RQD **Frequent siltstone seams from about 87-89'**	-65+/-
	90.2	***Boring Terminated at 90.2 Feet***	-70+/-

DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
50		☒	19	7-7-7-3 N=14		
55		☒	24	WR-WH/18"		
60		☒	24	WR-WH/18"		
65		☒	24	WR/18"-WH		
70		☒	24	2-1/12"-1 N=1		
75		☒	24	WR/18"-WH		
80		☒	22	21-16-14-24 N=30		
85			1	50/2"		
		▮	56	REC=96% RQD=45%		
90						

Stratification lines are approximate. In-situ, the transition may be gradual.

Hammer Type: Automatic

Advancement Method:
Tricone rollerbit to 85.2', NQ core barrel to 90.2'

Abandonment Method:
Boring backfilled with soil cuttings upon completion.

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

WATER LEVEL OBSERVATIONS

Groundwater measurements not obtained as water was used for borehole advancement

Terracon
30 Corporate Cir Ste 201
Albany, NY

Boring Started: 07-01-2021

Boring Completed: 07-01-2021

Drill Rig: Diedrich D-50

Driller: S. Morey

Project No.: JB215020

TEST PIT LOG NO. TP-21-1

Page 1 of 1

PROJECT: **Proposed Marmen Manufacturing Facility**

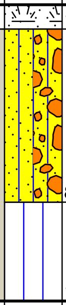
CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6070° Longitude: -73.7643°

Approximate Surface Elev.: 15 (Ft.) +/-



MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	1.0	**TOPSOIL**, dark brown, approx. 1' topsoil at ground surface	14+/-							
2	8.0	**SILTY SAND WITH GRAVEL (SM)**, brown, moist	7+/-	5		⛏				
	12.0	**CLAYEY SILT (ML)**, gray, moist, (operator notes greater excavation resistance)	3+/-	10		⛏				

Test Pit Terminated at 12 Feet

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

- logged by JSH

WATER LEVEL OBSERVATIONS

No measurable groundwater in test pit upon completion of excavation

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-16-2021	Test Pit Completed: 09-16-2021
Excavator: Kobelco SK270SR	Operator: Peter K. Frueh Excavating
Project No.: JB215020	

TEST PIT LOG NO. TP-21-2

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

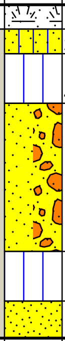
CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6062° Longitude: -73.7662°

Approximate Surface Elev.: 10 (Ft.) +/-



MODEL LAYER	DEPTH	DESCRIPTION	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	1.0	**TOPSOIL**, dark brown, approx. 1' topsoil w/ roots at ground surface	9+/-							
2	2.0	**SILTY SAND (SM)**, brown, moist	8+/-							
	4.0	**CLAYEY SILT (ML)**, mottled, gray, moist	6+/-							
	10.0	**POORLY GRADED SAND WITH GRAVEL (SP)**, trace silt, brown, moist, occasional clay nodules noted	0+/-	5		●				
	12.0	**SILT (SM)**, with organics, gray, wet, some roots, water seeps in from this layer, hole caves below this depth	-2+/-	10						
	13.5	trace silt, fine to coarse grained, brown, very moist	-3.5+/-							

Test Pit Terminated at 13.5 Feet

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

- logged by JSH

WATER LEVEL OBSERVATIONS

No measurable groundwater in test pit upon completion of excavation

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-17-2021	Test Pit Completed: 09-17-2021
Excavator: Kobelco SK270SR	Operator: Peter K. Frueh Excavating
Project No.: JB215020	

TEST PIT LOG NO. TP-21-3

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6057° Longitude: -73.7644°

Approximate Surface Elev.: 17 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		1.0	**TOPSOIL**, dark brown, approx. 1' topsoil at ground surface	16+/-							
2		4.0	**CLAYEY SILT (ML)**, with sand, mottled, brown, moist - water seeps in at brown/gray interface	13+/-							
		10.0	**CLAYEY SILT (ML)**, with sand, trace gravel, organics, gray, moist, (operator notes greater excavation resistance) - grades lean clay	7+/-	5						19.6
		12.0	**POORLY GRADED SAND WITH GRAVEL (SP)**, trace silt, fine to medium grained, brown, very moist, occasional clay nodules noted up to 3-4 inches in size	5+/-	10						
			Test Pit Terminated at 12 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

- logged by JSH

WATER LEVEL OBSERVATIONS

No measurable groundwater in test pit upon completion of excavation

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-16-2021	Test Pit Completed: 09-16-2021
Excavator: Kobelco SK270SR	Operator: Peter K. Frueh Excavat ng
Project No.: JB215020	

TEST PIT LOG NO. TP-21-4

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6049° Longitude: -73.7674°

Approximate Surface Elev.: 12 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
1	▨	12.0	**<u>FILL - COAL ASH</u>**, dark gray, very moist, nil topsoil at ground surface, some roots in upper 1' - hole caves below 3' - becomes wet - hole caves excessively below 10', ash becomes saturated w/ pudding-like consistency	0+/-	5 10	▽	●				67.3
			Test Pit Terminated at 12 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

- logged by JSH
- ground in this area shakes underfoot when tracked over by excavator

WATER LEVEL OBSERVATIONS

▽ *At completion of test pit*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-17-2021

Test Pit Completed: 09-17-2021

Excavator: Kobelco SK270SR

Operator: Peter K. Frueh Excavating

Project No.: JB215020

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/11/21

TEST PIT LOG NO. TP-21-5

Page 1 of 1

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6045° Longitude: -73.7646°

Approximate Surface Elev.: 20 (Ft.) +/-

MODEL LAYER	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
	0.5	**TOPSOIL**, dark brown, approx. 6" topsoil at ground surface	19.5+/-							
1	6.0	**FILL - SILTY SAND WITH GRAVEL (SM)**, little organics, gray, moist, (fuel oil odor noted) - grades poorly graded sand with gravel	14+/-	5		⛏				
2	12.0	**CLAYEY SILT (ML)**, with sand, trace gravel, organics, gray, moist, (operator notes greater excavation resistance)	8+/-	10						
	14.0	**SILTY SAND (SM)**, gray-brown, very moist	6+/-							

Test Pit Terminated at 14 Feet

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

- logged by JSH

WATER LEVEL OBSERVATIONS

No measurable groundwater in test pit upon completion of excavation

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-16-2021	Test Pit Completed: 09-16-2021
Excavator: Kobelco SK270SR	Operator: Peter K. Frueh Excavating
Project No.: JB215020	

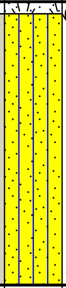
TEST PIT LOG NO. TP-21-6

Page 1 of 1

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan Latitude: 42.6040° Longitude: -73.7636° Approximate Surface Elev.: 9 (Ft.) +/- DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		0.5 **TOPSOIL**, dark brown, approx. 6" topsoil at ground surface 8.5+/-							
2		**SILTY SAND (SM)**, brown, moist, some roots in upper 3'	5						
		- grades poorly grades sand with silt (SP-SM)			●				
		- becomes wet, caves excessively below this depth		▽					
		- some rootlets, little woody organics noted 6' - 7'							
		11.5 -2.5+/-	10						
		Test Pit Terminated at 11.5 Feet							

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:
- logged by JSH

WATER LEVEL OBSERVATIONS

▽ *At completion of test pit*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-16-2021	Test Pit Completed: 09-16-2021
Excavator: Kobelco SK270SR	Operator: Peter K. Frueh Excavating
Project No.: JB215020	

TEST PIT LOG NO. TP-21-7

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6035° Longitude: -73.7648°

Approximate Surface Elev.: 17 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH	DESCRIPTION	ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
1		0.3	**TOPSOIL**, dark brown, approx. 3" topsoil at ground surface	16.5+/-							
		1.0	**FILL - COAL ASH**	16+/-							
		4.0	**FILL - LEAN CLAY**, with rootlets, blocky texture, gray, moist	13+/-				✓			18.9
2		11.0	**CLAYEY SILT (ML)**, with sand, trace gravel, organics, gray, moist	6+/-	5 10						
		12.0	**SANDY SILT (ML)**, brown, moist	5+/-							
			Test Pit Terminated at 12 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

- logged by JSH

WATER LEVEL OBSERVATIONS

No measurable groundwater in test pit upon completion of excavation

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-16-2021	Test Pit Completed: 09-16-2021
Excavator: Kobelco SK270SR	Operator: Peter K. Frueh Excavating
Project No.: JB215020	

TEST PIT LOG NO. TP-21-8

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan Latitude: 42.6028° Longitude: -73.7669° Approximate Surface Elev.: 13 (Ft.) +/- DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
1	▨	**<u>FILL - COAL ASH</u>**, dark gray, very moist, nil topsoil at ground surface, some roots in upper 1' - hole caves below 2' - becomes wet - hole caves excessively below 11', ash becomes saturated w/ pudding-like consistency 13.0 / 0+/-	5 10	▽					63.9
		Test Pit Terminated at 13 Feet							

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

- logged by JSH
- ground in this area shakes underfoot when tracked over by excavator

WATER LEVEL OBSERVATIONS

▽ *At completion of test pit*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-17-2021	Test Pit Completed: 09-17-2021
Excavator: Kobelco SK270SR	Operator: Peter K. Frueh Excavating
Project No.: JB215020	

TEST PIT LOG NO. TP-21-9

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6025° Longitude: -73.7642°

Approximate Surface Elev.: 16 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		1.0	**TOPSOIL**, dark brown, approx. 1' topsoil at ground surface	15+/-							
1		2.5	**FILL - SILTY SAND WITH GRAVEL** , brown-gray, moist, piece concrete noted	13.5+/-							
2		4.0	**SANDY SILT (ML)**, with clay, mottled, gray, moist	12+/-							
		6.0	**SILTY SAND (SM)**, gray, moist	10+/-	5						
		11.0	**SANDY SILT (ML)**, with clay, rootlets, brown-gray, moist - operator notes easier excavtion effort below 8'	5+/-	10						
		12.0	**SANDY SILT (ML)**, trace organics, brown, very moist	4+/-							

Test Pit Terminated at 12 Feet

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:
- logged by JSH

WATER LEVEL OBSERVATIONS

No measurable groundwater in test pit upon completion of excavation

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-16-2021

Test Pit Completed: 09-16-2021

Excavator: Kobelco SK270SR

Operator: Peter K. Frueh Excavating

Project No.: JB215020

THIS BORING LOG IS NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GEO SMART LOG-NO WELL JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/11/21

TEST PIT LOG NO. TP-21-10

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6016° Longitude: -73.7636°

Approximate Surface Elev.: 17 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		0.7	**TOPSOIL**, dark brown, approx. 8" topsoil at ground surface	16.5+/-							
1		7.0	**FILL - SILTY SAND WITH GRAVEL** , brown-gray, moist, occasional angular cobbles, clayey lumps, little brick, slag, wood noted	10+/-	5						
2		12.0	**SILTY SAND (SM)**, brown, moist	5+/-	10						
			Test Pit Terminated at 12 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

- logged by JSH

WATER LEVEL OBSERVATIONS

No measurable groundwater in test pit upon completion of excavation

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-16-2021	Test Pit Completed: 09-16-2021
Excavator: Kobelco SK270SR	Operator: Peter K. Frueh Excavating
Project No.: JB215020	

TEST PIT LOG NO. TP-21-11

PROJECT: **Proposed Marmen Manufacturing Facility**

CLIENT: **McFarland Johnson**
Saratoga Springs, NY

SITE: **River Road**
Glenmont, NY

LOCATION See Exploration Plan

Latitude: 42.6005° Longitude: -73.7634°

Approximate Surface Elev.: 18 (Ft.) +/-

MODEL LAYER	GRAPHIC LOG	DEPTH		ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
		0.7	**TOPSOIL**, dark brown, approx. 8" topsoil at ground surface	17.5+/-							
1	▨		**FILL - SILTY SAND WITH GRAVEL** , brown, moist, trace plastic, metal, cinders - becomes gray, w/ little wood, stalky organics, cobbles		5		●				
		6.5		11.5+/-							
2			**SILTY SAND (SM)**, trace organics, gray, moist, occasional clayey lumps noted		10						
		12.0		6+/-							
		13.0	**SILT (ML)**, with sand, bown-gray, moist	5+/-							
			Test Pit Terminated at 13 Feet								

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

- logged by JSH

WATER LEVEL OBSERVATIONS

No measurable groundwater in test pit upon completion of excavation

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-16-2021	Test Pit Completed: 09-16-2021
Excavator: Kobelco SK270SR	Operator: Peter K. Frueh Excavating
Project No.: JB215020	

TEST PIT LOG NO. TP-21-12

Page 1 of 1

PROJECT: Proposed Marmen Manufacturing Facility

CLIENT: McFarland Johnson
Saratoga Springs, NY

SITE: River Road
Glenmont, NY

MODEL LAYER	GRAPHIC LOG	LOCATION See Exploration Plan Latitude: 42.5999° Longitude: -73.7648° Approximate Surface Elev.: 14 (Ft.) +/- DEPTH / ELEVATION (Ft.)	DEPTH (Ft.)	WATER LEVEL OBSERVATIONS	SAMPLE TYPE	RECOVERY (In.)	FIELD TEST RESULTS	ORGANIC CONTENT (%)	WATER CONTENT (%)
1		**FILL - COAL ASH** , dark gray, very moist, nil topsoil at ground surface, some reedy vegetation in upper few feet							
			5						70.8
		- becomes wet, hole caves below 10'	10						
		- some reedy vegetation, swampy odor noted - ash becomes saturated w/ pudding-like consistency							
		16.0 -2+/-	15	▽					
		Test Pit Terminated at 16 Feet							

Stratification lines are approximate. In-situ, the transition may be gradual.

Advancement Method:
3' wide excavator bucket

Abandonment Method:
Test pit backfilled in lifts, tamped with excavator bucket

See Exploration and Testing Procedures for a description of field and laboratory procedures used and additional data (If any).

See Supporting Information for explanation of symbols and abbreviations.

Elevation interpolated from topographic site plan.

Notes:

- logged by JSH

WATER LEVEL OBSERVATIONS

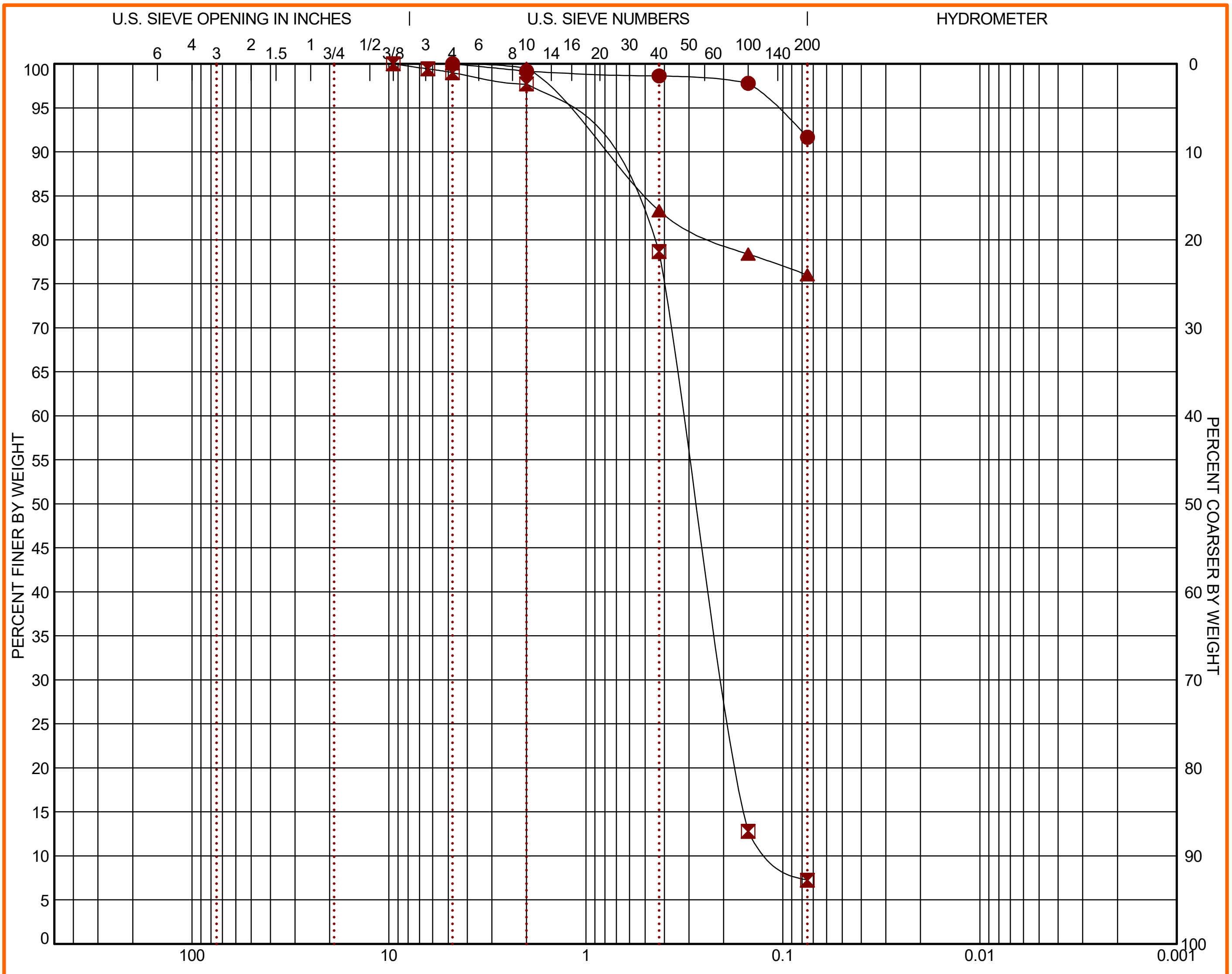
▽ *At completion of test pit (and rising)*

Terracon
30 Corporate Cir Ste 201
Albany, NY

Test Pit Started: 09-17-2021	Test Pit Completed: 09-17-2021
Excavator: Kobelco SK270SR	Operator: Peter K. Frueh Excavating
Project No.: JB215020	

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136



COBBLES	GRAVEL		SAND			SILT OR CLAY
	coarse	fine	coarse	medium	fine	

	BORING ID	DEPTH	% COBBLES	% GRAVEL	% SAND	% SILT	% FINES	% CLAY	USCS
●	B-21-7	6 - 8	0.0	0.0	8.4		91.6		ML
☒	B-21-8	25 - 27	0.0	1.0	91.7		7.3		SP-SM
▲	B-21-10	4 - 6	0.0	0.0	24.0		76.0		ML

GRAIN SIZE			
	●	☒	▲
D_{60}		0.316	
D_{30}		0.197	
D_{10}		0.106	

COEFFICIENTS			
	●	☒	▲
C_C		1.16	
C_U		2.99	

●		☒		▲	
Sieve	% Finer	Sieve	% Finer	Sieve	% Finer
#4	100.0	3/8"	100.0	#4	100.0
#10	99.15	#4	99.43	#10	99.51
#40	98.63	#10	98.98	#40	83.3
#100	97.78	#40	97.7	#100	78.37
#200	91.65	#100	78.68	#200	76.01
		#200	12.78		
			7.25		

SOIL DESCRIPTION	
●	SILT (ML)
☒	POORLY GRADED SAND with SILT (SP-SM)
▲	SILT with SAND (ML)

REMARKS	
●	
☒	
▲	

PROJECT: Proposed Marmen Manufacturing Facility

PROJECT NUMBER: JB215020

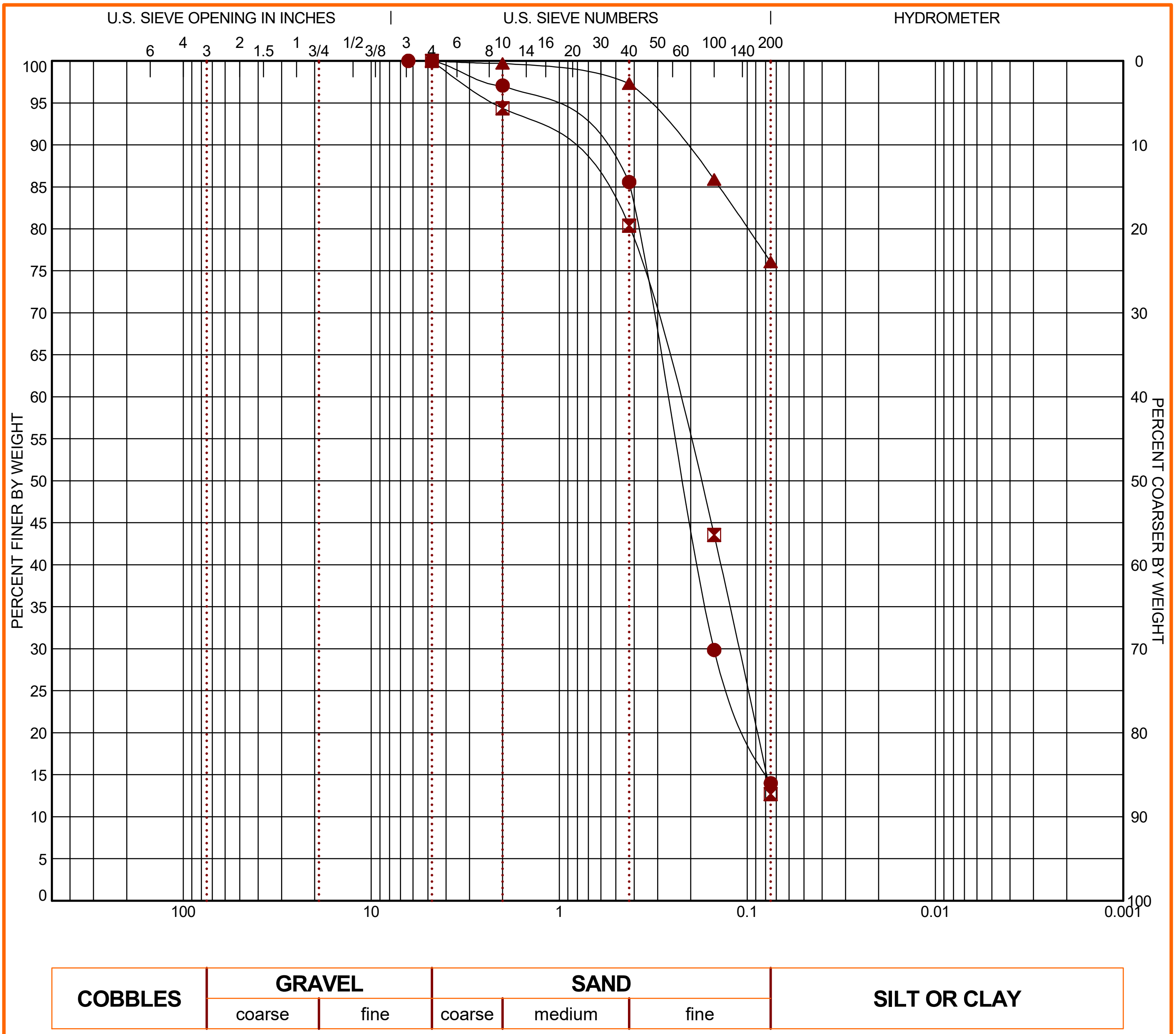
SITE: River Road
Glenmont, NY

Terracon
30 Corporate Cir Ste 201
Albany, NY

CLIENT: McFarland Johnson
Saratoga Springs, NY

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136



	BORING ID	DEPTH	% COBBLES	% GRAVEL	% SAND	% SILT	% FINES	% CLAY	USCS
●	**B-21-10**	16 - 18	0.0	0.0	86.0		14.0		SM
☒	**B-21-17**	14 - 16	0.0	0.0	87.3		12.7		SM
▲	**B-21-20**	6 - 8	0.0	0.0	23.9		76.1		ML

GRAIN SIZE			
	●	☒	▲
D_{60}	0.264	0.239	
D_{30}	0.15	0.111	
D_{10}			

COEFFICIENTS			
	●	☒	▲
C_C			
C_U			

●		☒		▲	
Sieve	% Finer	Sieve	% Finer	Sieve	% Finer
#4 #10 #40 #100 #200	100.0 99.99 97.06 85.57 29.86 13.99	#4 #10 #40 #100 #200	100.0 94.35 80.38 43.57 12.69	#4 #10 #40 #100 #200	100.0 99.7 97.29 85.87 76.05

SOIL DESCRIPTION	
●	SILTY SAND (SM)
☒	SILTY SAND (SM)
▲	SILT with SAND (ML)

REMARKS	
●	
☒	
▲	

PROJECT: Proposed Marmen Manufacturing
Facility

SITE: River Road
Glenmont, NY

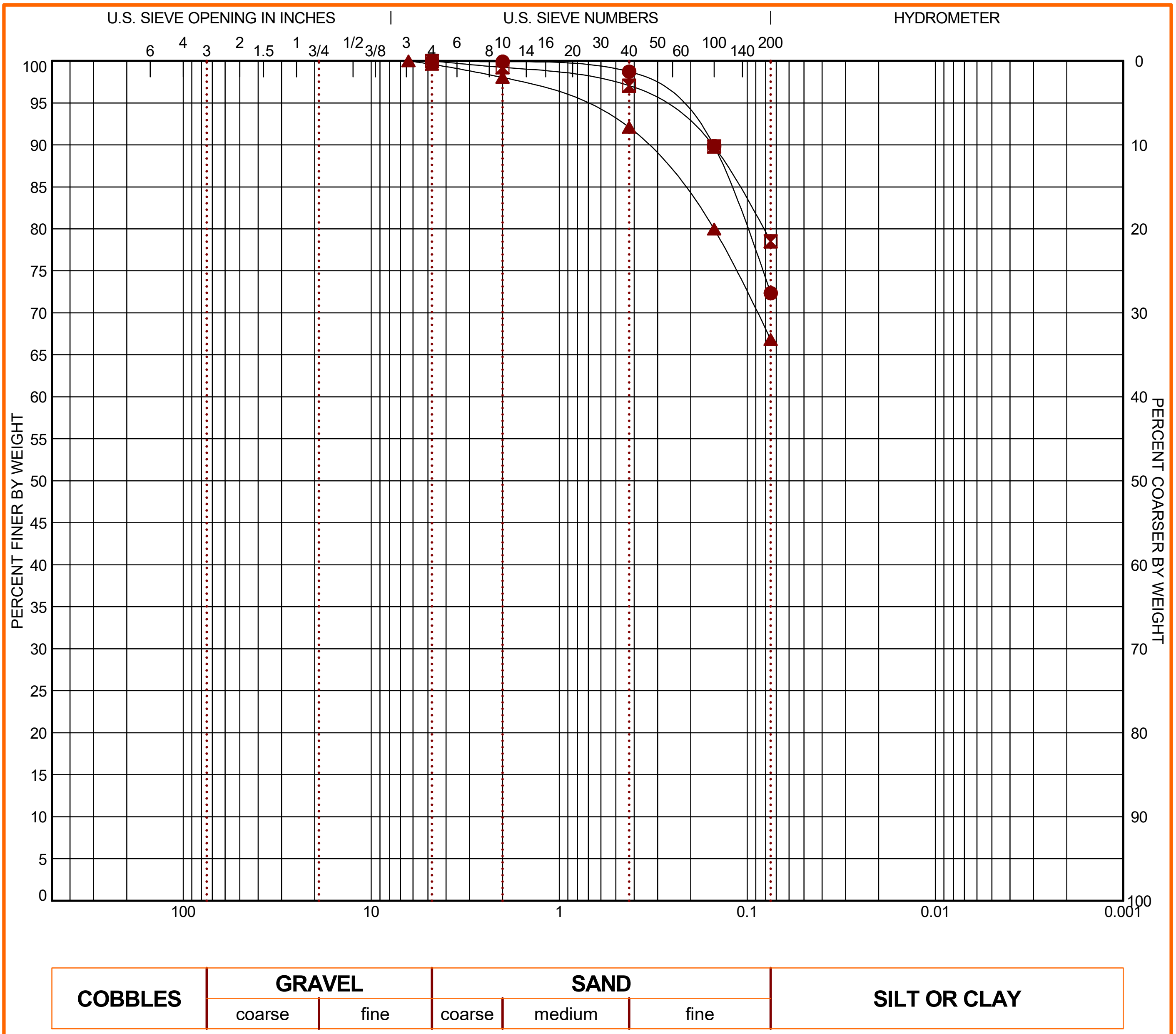
Terracon
30 Corporate Cir Ste 201
Albany, NY

PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136



	BORING ID	DEPTH	% COBBLES	% GRAVEL	% SAND	% SILT	% FINES	% CLAY	USCS
●	S-1	1 - 1.1	0.0	0.0	27.7		72.3		ML
⊠	S-2	1 - 3	0.0	0.0	21.5		78.5		ML
▲	S-3	1 - 3	0.0	0.4	32.8		66.8		ML

GRAIN SIZE

	●	⊠	▲
D_{60}			
D_{30}			
D_{10}			

COEFFICIENTS

	●	⊠	▲
C_C			
C_U			

●		⊠		▲	
Sieve	% Finer	Sieve	% Finer	Sieve	% Finer
#4	100.0	#4	100.0	#4	100.0
#10	99.9	#10	99.23	#10	99.59
#40	98.72	#40	97.06	#40	98.02
#100	89.86	#100	89.8	#100	92.1
#200	72.34	#200	78.52	#200	79.97
					66.83

SOIL DESCRIPTION

●	SILT with SAND (ML)
⊠	SILT with SAND (ML)
▲	SANDY SILT (ML)

(coal ash)

REMARKS

●	
⊠	
▲	

PROJECT: Proposed Marmen Manufacturing
Facility

SITE: River Road
Glenmont, NY

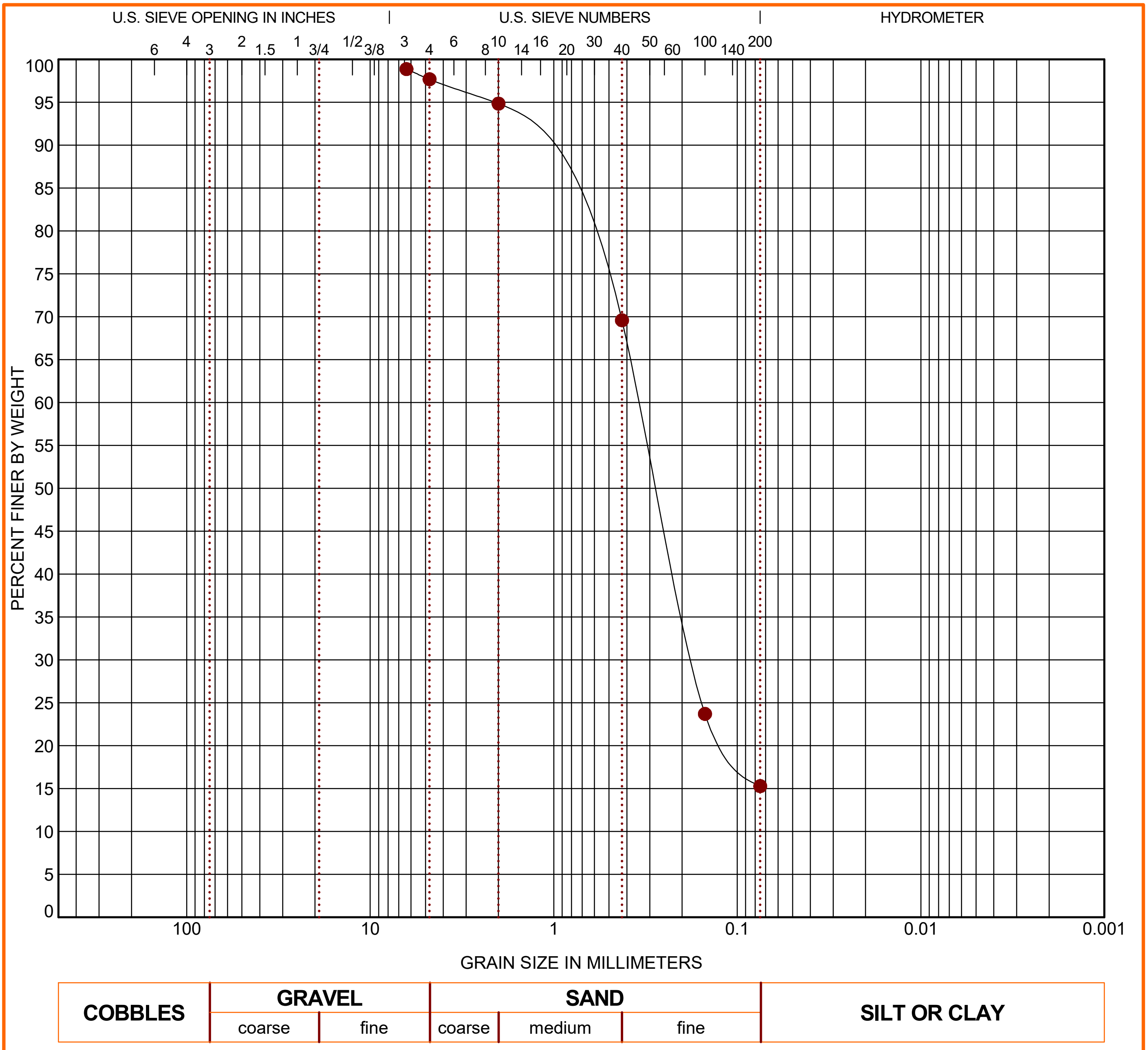
Terracon
30 Corporate Cir Ste 201
Albany, NY

PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136



Boring ID	Depth	USCS Classification	WC (%)	LL	PL	PI	Cc	Cu
● TP-21-1	4 - 4.5	SILTY SAND (SM)		NP	NP	NP		

Boring ID	Depth	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Silt	%Fines	%Clay
● TP-21-1	4 - 4.5	6.35	0.342	0.173			1.2	82.4		15.3	

PROJECT: Proposed Marmen Manufacturing
Facility

SITE: River Road
Glenmont, NY

Terracon
30 Corporate Cir Ste 201
Albany, NY

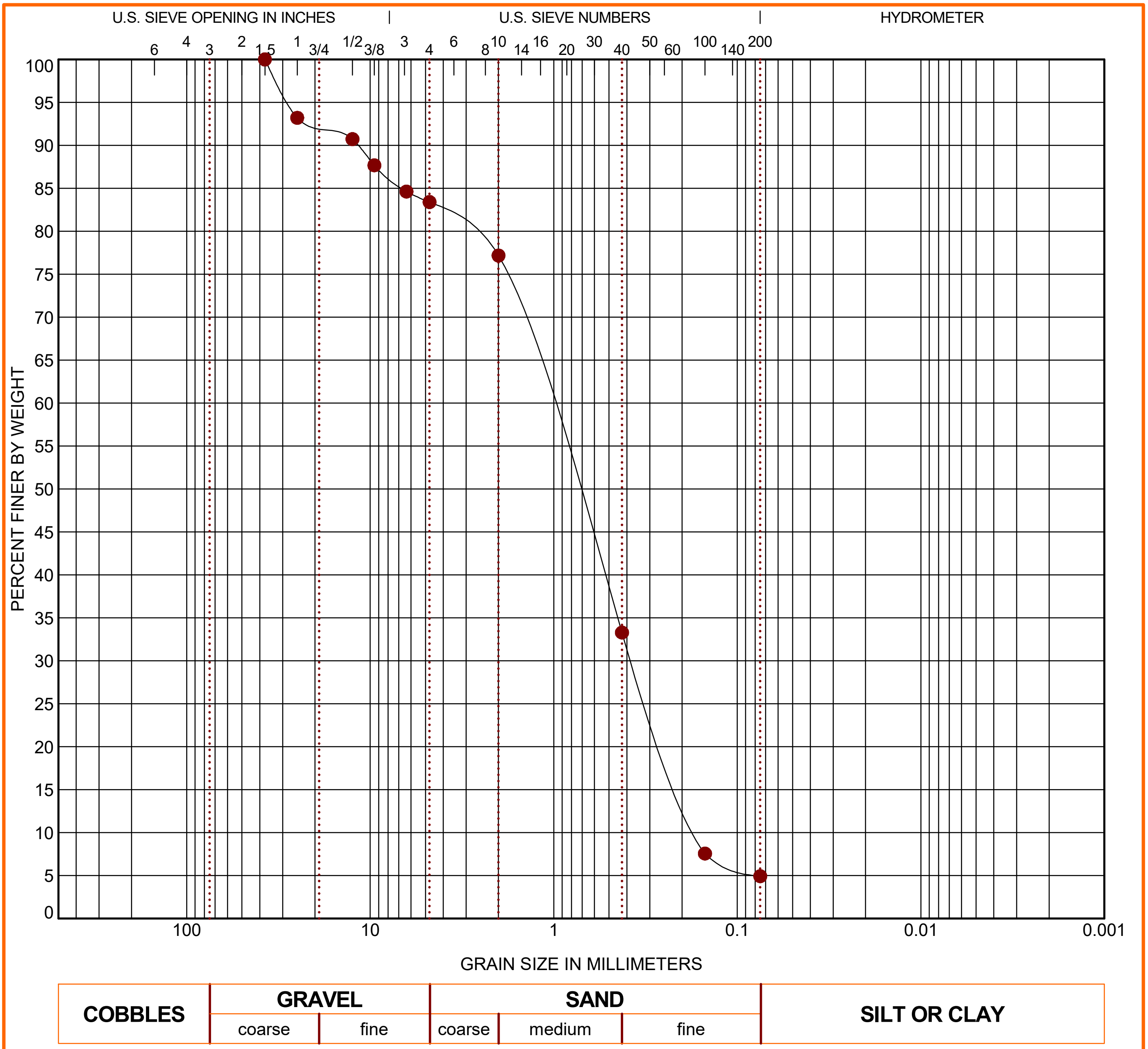
PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: USCS-2 JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/4/21

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136



Boring ID		Depth	USCS Classification	WC (%)	LL	PL	PI	Cc	Cu
●	TP-21-2	5 - 5.5	POORLY GRADED SAND with GRAVEL (SP)		NP	NP	NP	0.77	6.59

Boring ID		Depth	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Silt	%Fines	%Clay
●	TP-21-2	5 - 5.5	37.5	1.091	0.372	0.166	0.0	16.6	78.5		4.9	

PROJECT: Proposed Marmen Manufacturing Facility

SITE: River Road
Glenmont, NY

Terracon
30 Corporate Cir Ste 201
Albany, NY

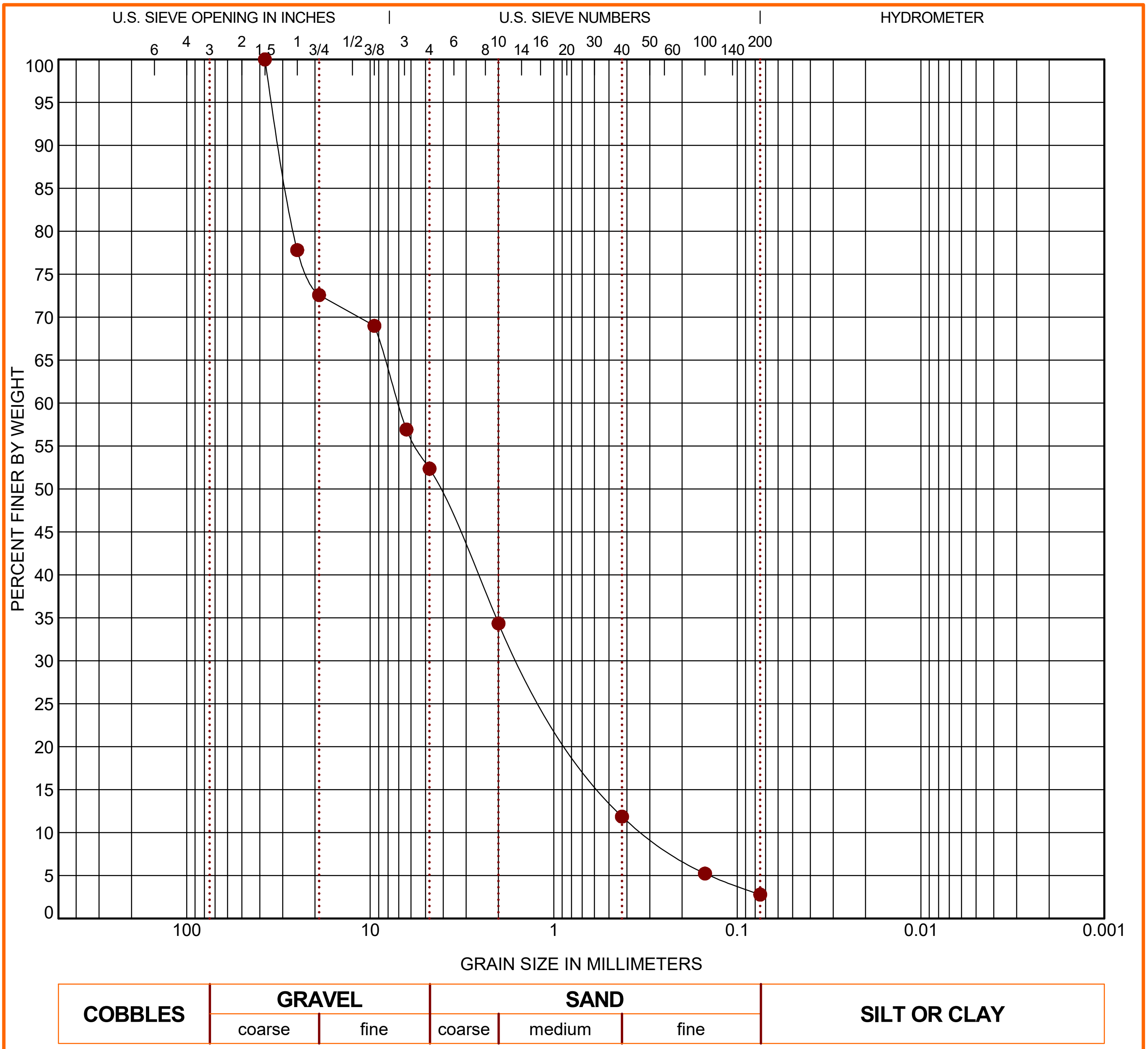
PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: USCS-2 JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/4/21

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136



Boring ID	Depth	USCS Classification	WC (%)	LL	PL	PI	Cc	Cu
● TP-21-5	4 - 4.5	POORLY GRADED SAND with GRAVEL (SP)		NP	NP	NP	0.98	22.17

Boring ID	Depth	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Silt	%Fines	%Clay
● TP-21-5	4 - 4.5	37.5	7.038	1.483	0.317	0.0	47.6	49.6		2.8	

PROJECT: Proposed Marmen Manufacturing
Facility

SITE: River Road
Glenmont, NY

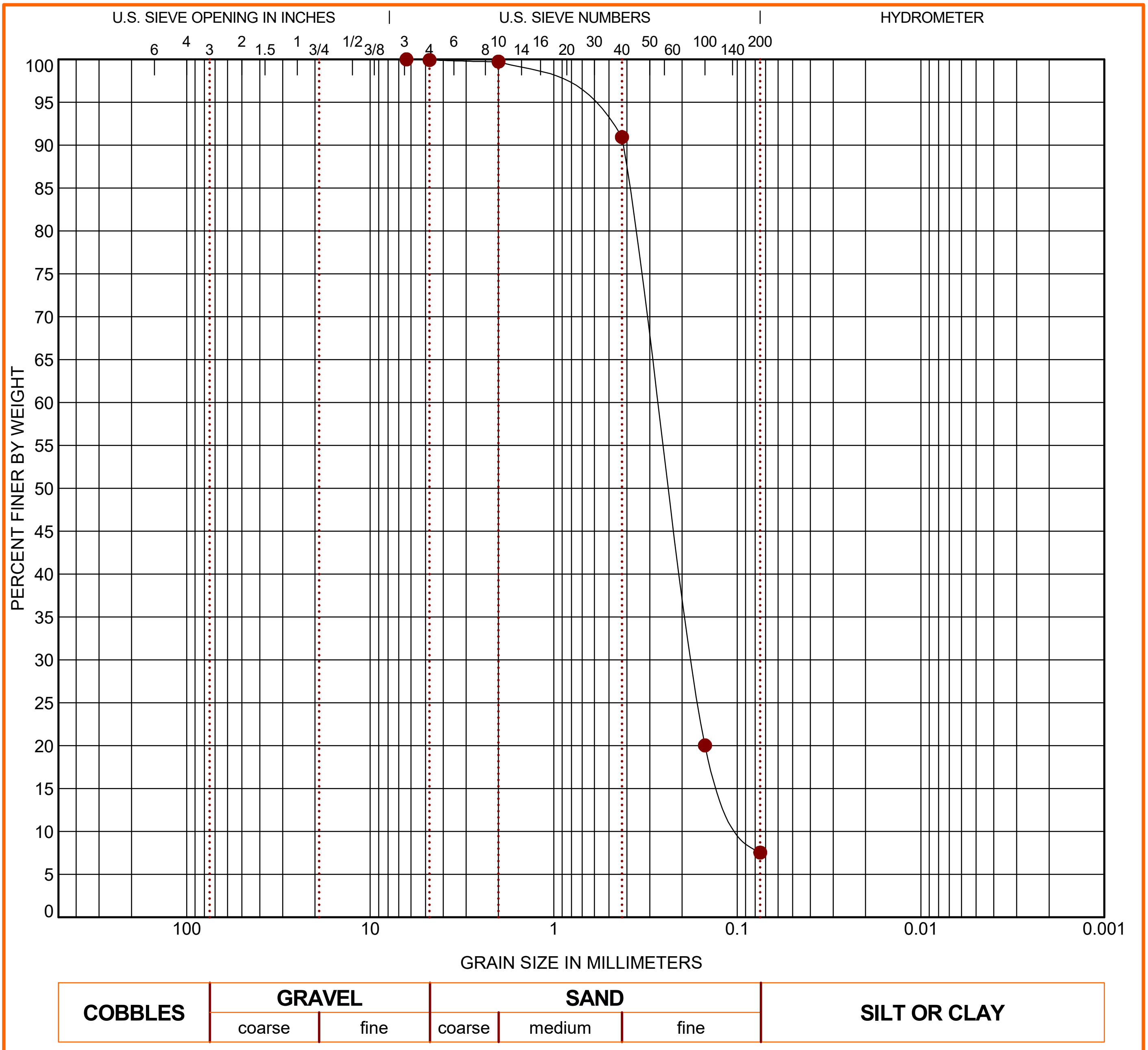
Terracon
30 Corporate Cir Ste 201
Albany, NY

PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136



	Boring ID	Depth	USCS Classification	WC (%)	LL	PL	PI	Cc	Cu
●	TP-21-6	6 - 6.5	POORLY GRADED SAND with SILT (SP-SM)		NP	NP	NP	1.30	3.14

	Boring ID	Depth	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Silt	%Fines	%Clay
●	TP-21-6	6 - 6.5	6.35	0.27	0.174	0.086	0.0	0.1	92.4		7.5	

PROJECT: Proposed Marmen Manufacturing Facility

SITE: River Road
Glenmont, NY

Terracon
30 Corporate Cir Ste 201
Albany, NY

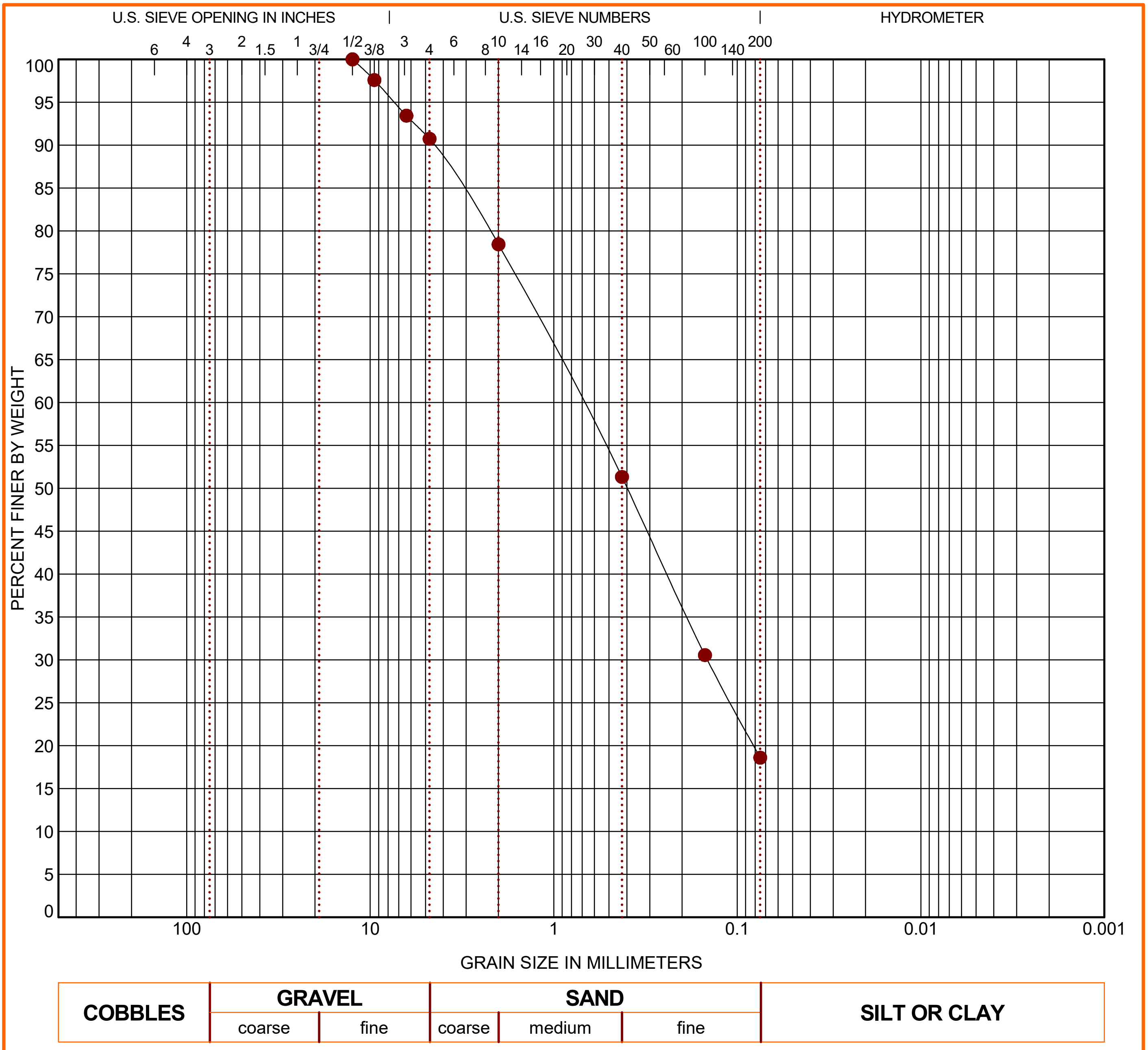
PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: USCS-2 JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/4/21

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136



Boring ID	Depth	USCS Classification	WC (%)	LL	PL	PI	Cc	Cu
● TP-21-9	5 - 5.5	SILTY SAND (SM)		NP	NP	NP		

Boring ID	Depth	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Silt	%Fines	%Clay
● TP-21-9	5 - 5.5	12.5	0.698	0.145		0.0	9.3	72.1		18.6	

PROJECT: Proposed Marmen Manufacturing
Facility

SITE: River Road
Glenmont, NY

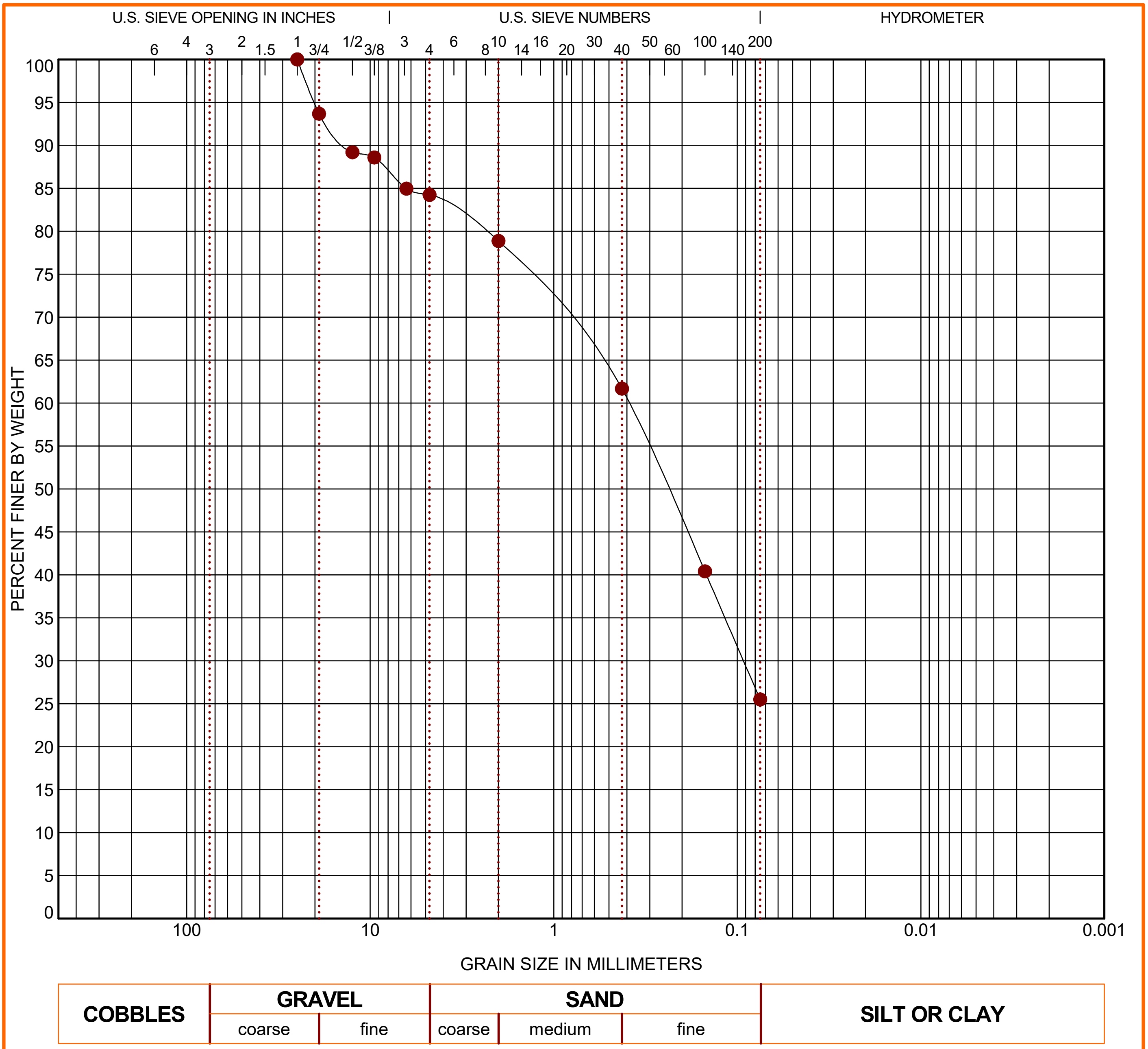
Terracon
30 Corporate Cir Ste 201
Albany, NY

PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136



Boring ID	Depth	USCS Classification	WC (%)	LL	PL	PI	Cc	Cu
● TP-21-10	4 - 4.5	SILTY SAND with GRAVEL (SM)		NP	NP	NP		

Boring ID	Depth	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Silt	%Fines	%Clay
● TP-21-10	4 - 4.5	25	0.392	0.092		0.0	15.8	58.7		25.5	

PROJECT: Proposed Marmen Manufacturing
Facility

SITE: River Road
Glenmont, NY

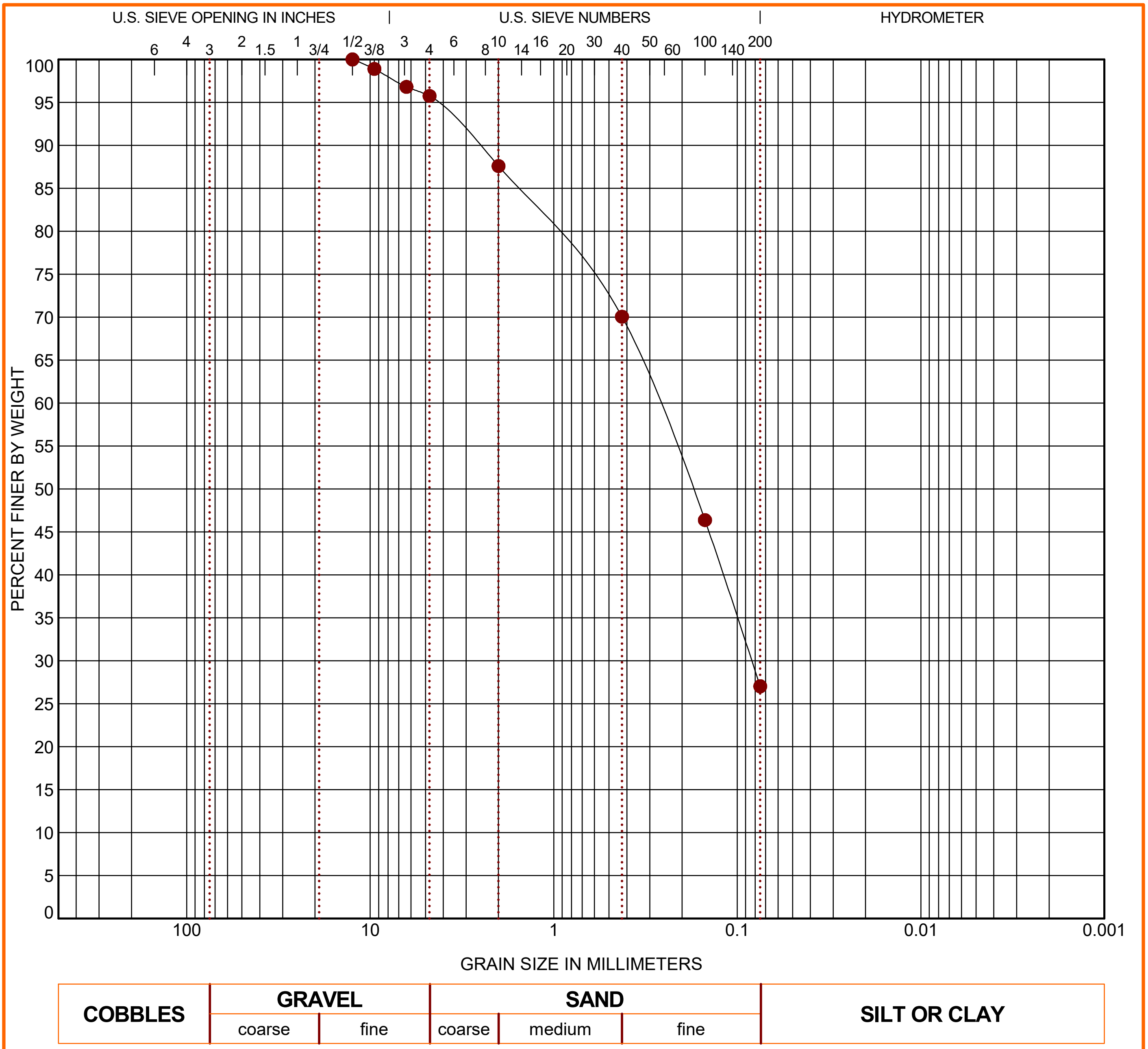
Terracon
30 Corporate Cir Ste 201
Albany, NY

PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

GRAIN SIZE DISTRIBUTION

ASTM D422 / ASTM C136



Boring ID	Depth	USCS Classification	WC (%)	LL	PL	PI	Cc	Cu
● TP-21-11	4 - 4.5	SILTY SAND (SM)		NP	NP	NP		

Boring ID	Depth	D_{100}	D_{60}	D_{30}	D_{10}	%Cobbles	%Gravel	%Sand	%Silt	%Fines	%Clay
● TP-21-11	4 - 4.5	12.5	0.273	0.083		0.0	4.3	68.7		27.0	

PROJECT: Proposed Marmen Manufacturing
Facility

SITE: River Road
Glenmont, NY

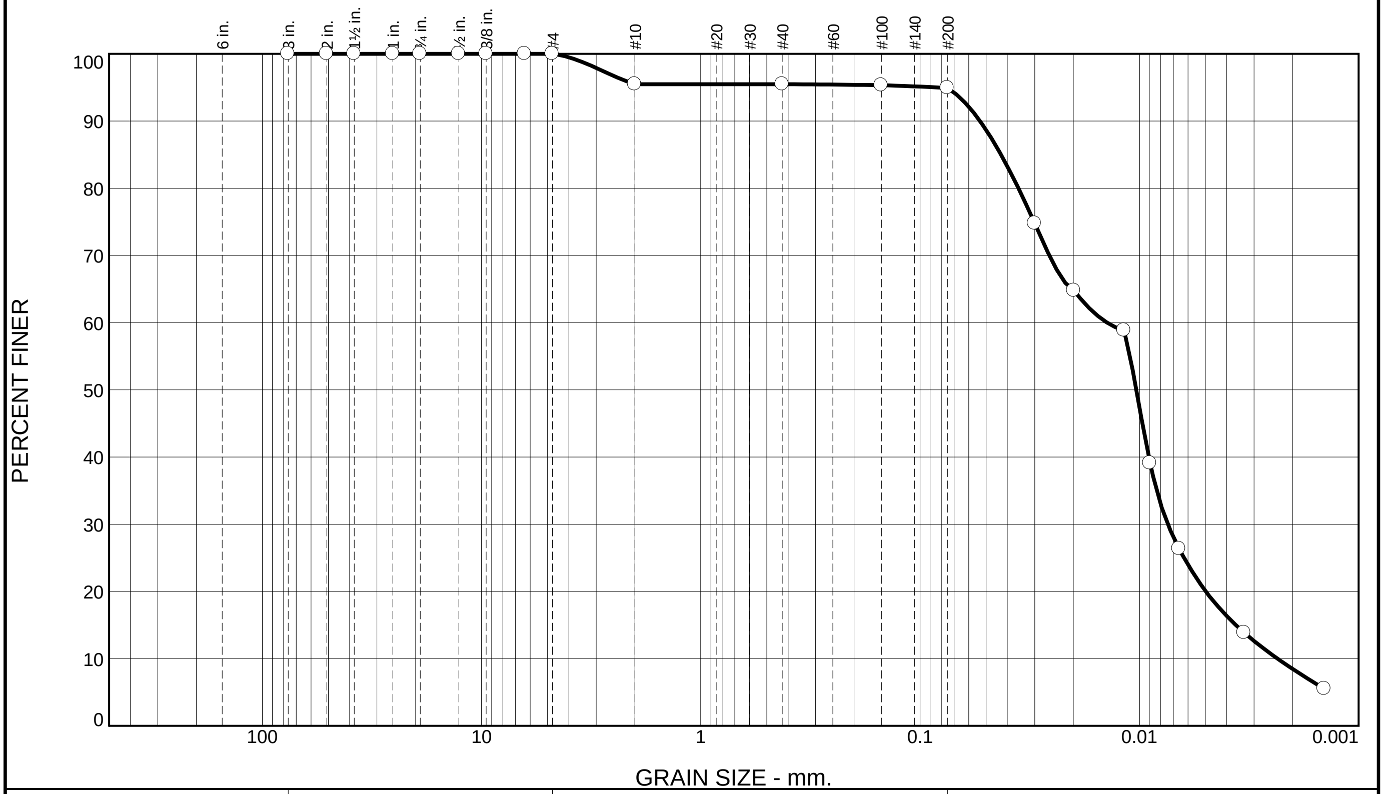
30 Corporate Cir Ste 201
Albany, NY

PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. GRAIN SIZE: USCS-2 JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/4/21

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	4.5	0.0	0.6	74.8	20.1

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100.0		
2"	100.0		
1.5"	100.0		
1"	100.0		
0.75"	100.0		
0.5"	100.0		
0.375"	100.0		
0.25"	100.0		
#4	100.0		
#10	95.5		
#40	95.5		
#100	95.3		
#200	94.9		

* (no specification provided)

Material Description

SILT

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D_{90}= 0.0533 D_{85}= 0.0428 D_{60}= 0.0140

D_{50}= 0.0103 D_{30}= 0.0074 D_{15}= 0.0036

D_{10}= 0.0023 C_u= 5.99 C_c= 1.67

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Per ASTM D422

Source of Sample: B-21-17 80'-82'

Date: 9-9-21

Terracon Consultants-NY, Inc.

Albany, NY

Client: McFarland Johnson

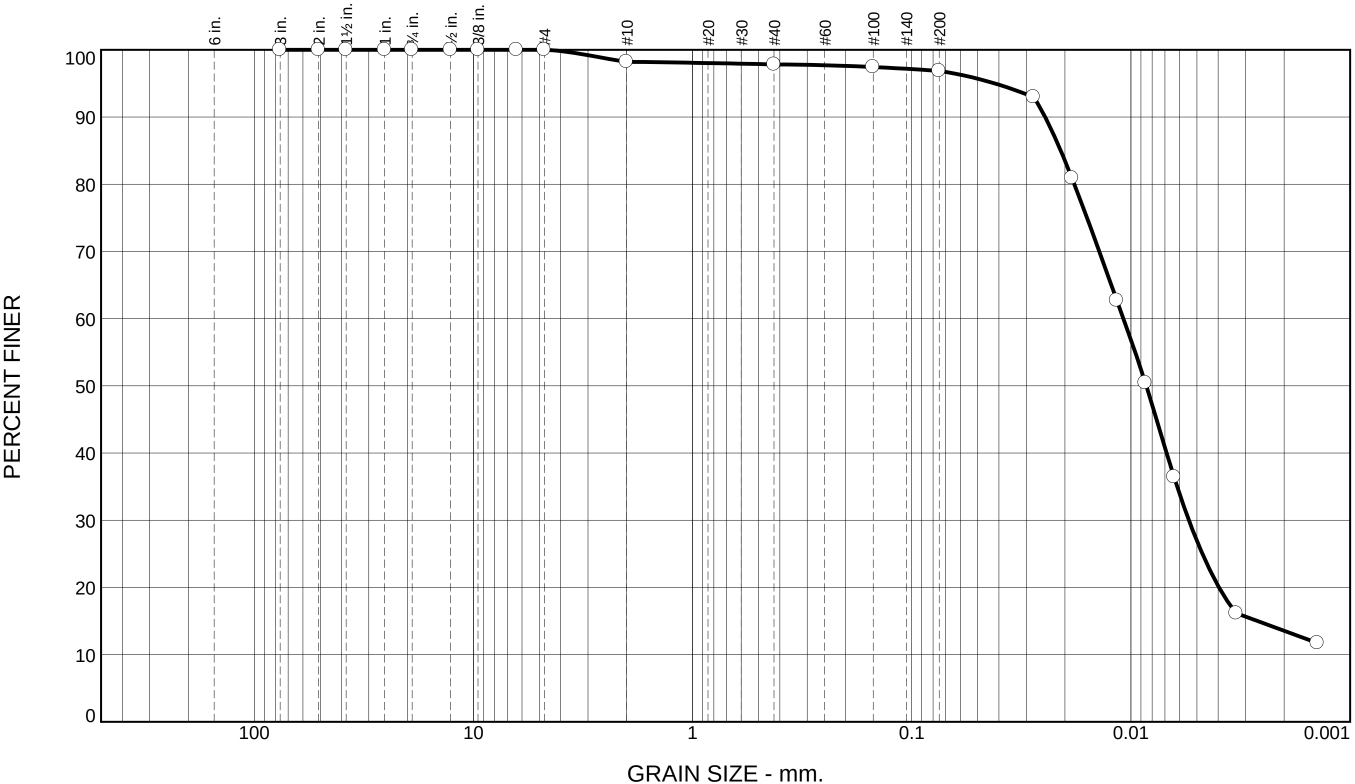
Project: Proposed Marmen Manufacturing Facility
Albany, NY

Project No: JB215020

Figure B-21-17 80'-82'

Tested By: AB **Checked By:** JH

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	1.8	0.4	1.0	69.9	26.9

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100.0		
2"	100.0		
1.5"	100.0		
1"	100.0		
0.75"	100.0		
0.5"	100.0		
0.375"	100.0		
0.25"	100.0		
#4	100.0		
#10	98.2		
#40	97.8		
#100	97.4		
#200	96.8		

* (no specification provided)

Material Description

SILT

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D_{90}= 0.0246 D_{85}= 0.0209 D_{60}= 0.0108
D_{50}= 0.0085 D_{30}= 0.0054 D_{15}= 0.0026
D_{10}= C_u= C_c=

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Per ASTM D422

Source of Sample: B-21-18 105'-107'

Date: 9-9-21

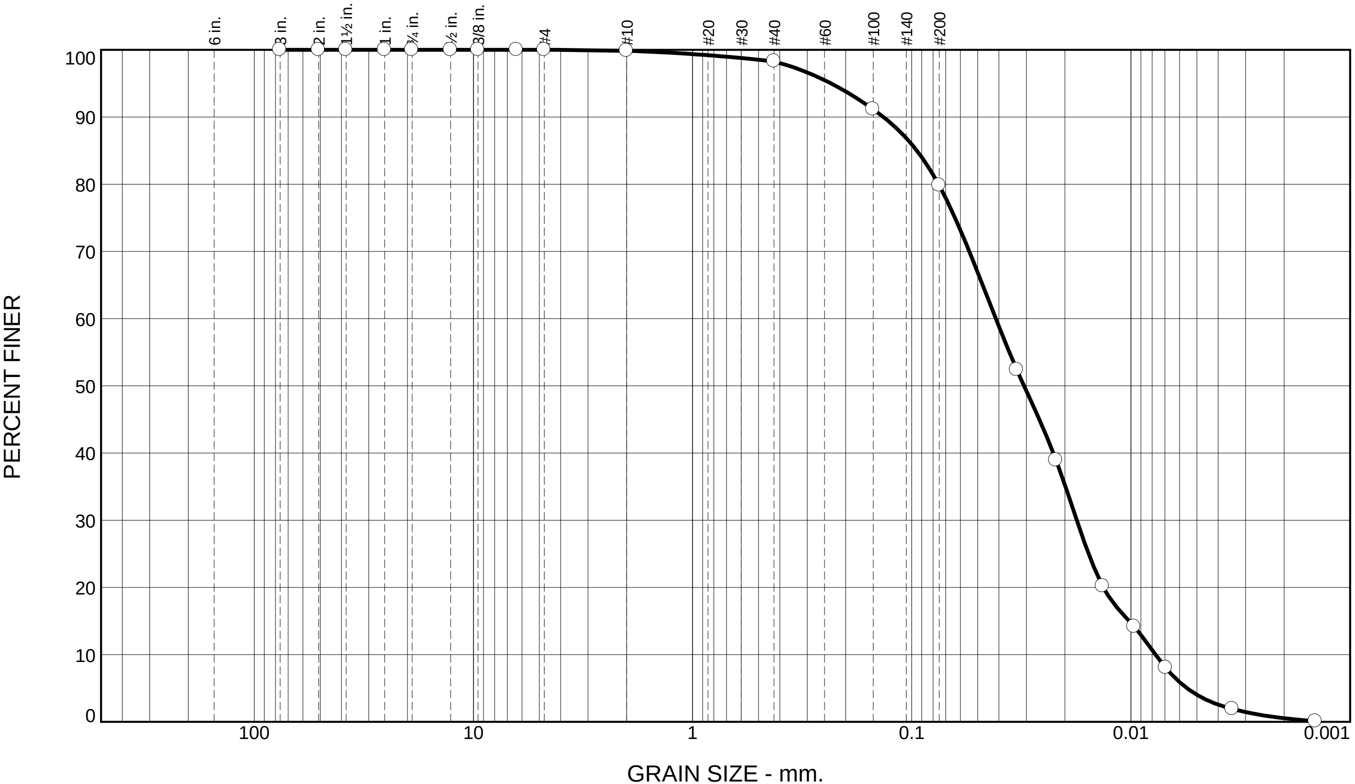
Terracon Consultants-NY, Inc.

Albany, NY

Client: McFarland Johnson
Project: Proposed Marmen Manufacturing Facility
Albany, NY
Project No: JB215020 **Figure** B-21-18 105'-107'

Tested By: AB **Checked By:** JH

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.1	1.6	18.5	75.8	4.0

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100.0		
2"	100.0		
1.5"	100.0		
1"	100.0		
0.75"	100.0		
0.5"	100.0		
0.375"	100.0		
0.25"	100.0		
#4	100.0		
#10	99.9		
#40	98.3		
#100	91.1		
#200	79.8		

* (no specification provided)

Material Description

SILT

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D_{90}= 0.1348 D_{85}= 0.0948 D_{60}= 0.0414
D_{50}= 0.0308 D_{30}= 0.0177 D_{15}= 0.0102
D_{10}= 0.0077 C_u= 5.35 C_c= 0.98

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Per ASTM D422

Source of Sample: B-21-23 12'-14'

Date: 9-9-21

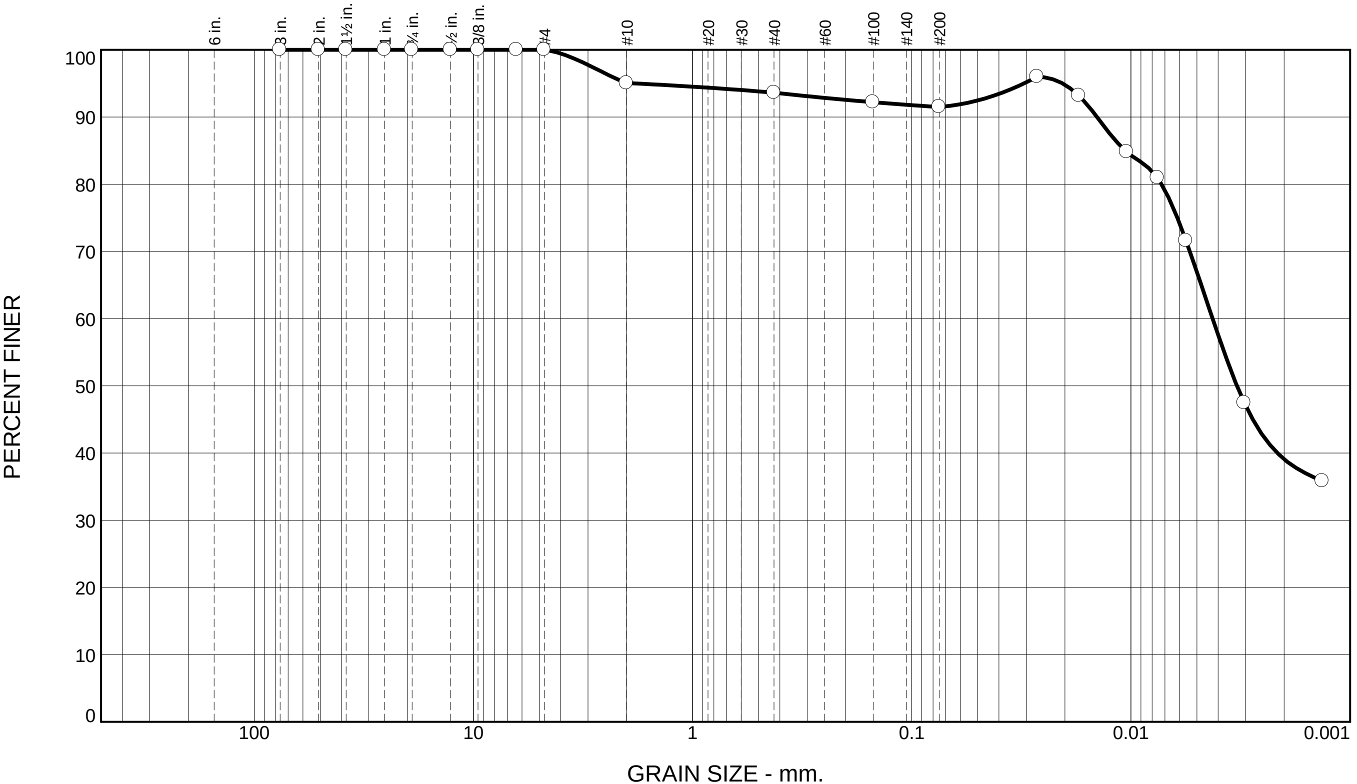
Terracon Consultants-NY, Inc.

Albany, NY

Client: McFarland Johnson
Project: Proposed Marmen Manufacturing Facility
Albany, NY
Project No: JB215020 **Figure** B-21-23 12'-14'

Tested By: AB **Checked By:** JH

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	5.0	1.4	2.1	24.6	66.9

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100.0		
2"	100.0		
1.5"	100.0		
1"	100.0		
0.75"	100.0		
0.5"	100.0		
0.375"	100.0		
0.25"	100.0		
#4	100.0		
#10	95.0		
#40	93.6		
#100	92.2		
#200	91.5		

* (no specification provided)

Material Description

Lean Clay

Atterberg Limits

PL= 19 LL= 33 PI= 14

Coefficients

D_{90}= 0.0143 D_{85}= 0.0106 D_{60}= 0.0042
D_{50}= 0.0033 D_{30}= D_{15}=
D_{10}= C_u= C_c=

Classification

USCS= CL AASHTO= A-6(12)

Remarks

Per ASTM D422

Source of Sample: TP-21-3 6'-6.5'

Date: 9-29-21

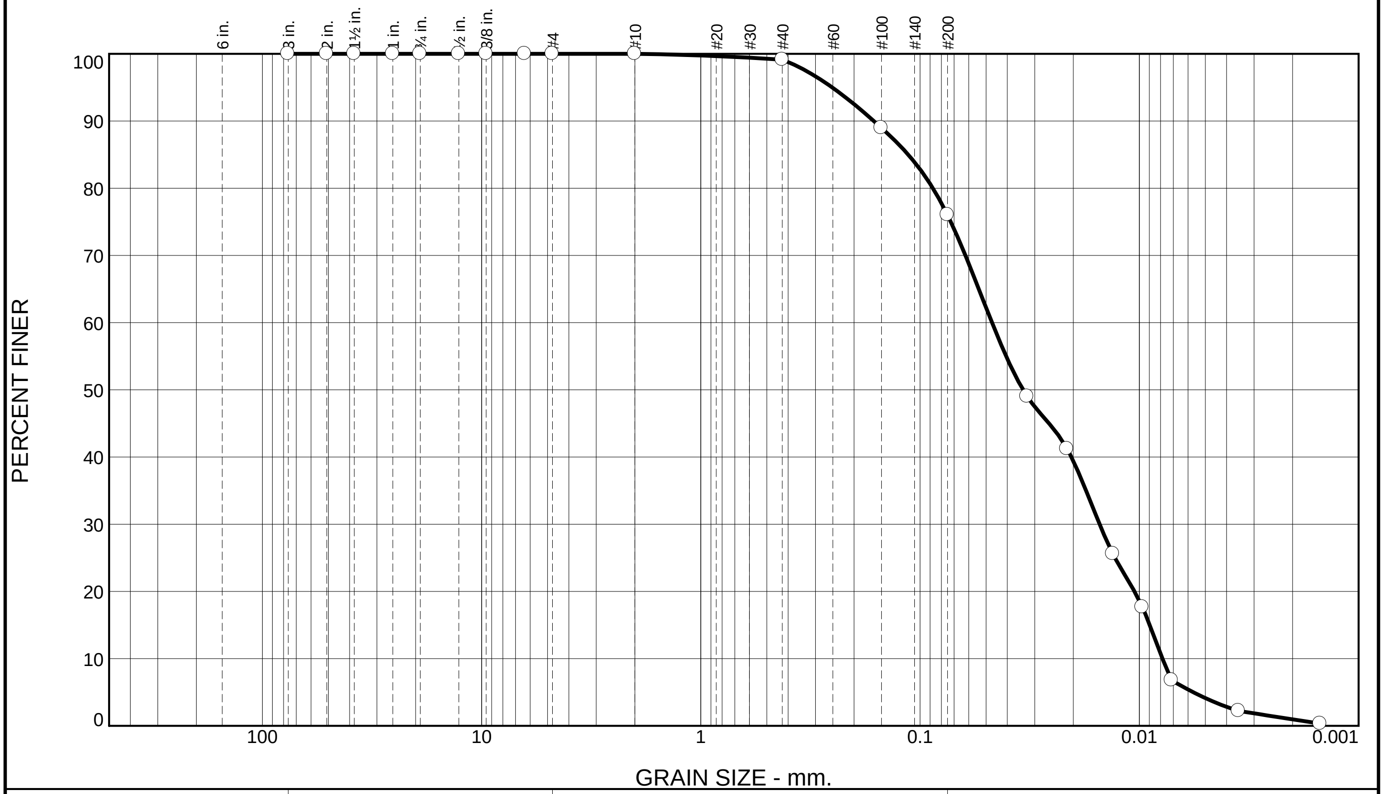
Terracon Consultants-NY, Inc.

Albany, NY

Client: McFarland Johnson
Project: Proposed Marmen Manufacturing Facility
Albany, NY
Project No: JB215020 **Figure** TP-21-3 6'-6.5'

Tested By: AB **Checked By:** JH

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.9	23.1	71.9	4.1

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100.0		
2"	100.0		
1.5"	100.0		
1"	100.0		
0.75"	100.0		
0.5"	100.0		
0.375"	100.0		
0.25"	100.0		
#4	100.0		
#10	100.0		
#40	99.1		
#100	89.0		
#200	76.0		

* (no specification provided)

Material Description

Silt with sand

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D_{90}= 0.1626 D_{85}= 0.1134 D_{60}= 0.0470
D_{50}= 0.0339 D_{30}= 0.0152 D_{15}= 0.0090
D_{10}= 0.0078 C_u= 5.99 C_c= 0.63

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Per ASTM D422

Source of Sample: TP-21-4 4'-4.5'

Date: 9-29-21

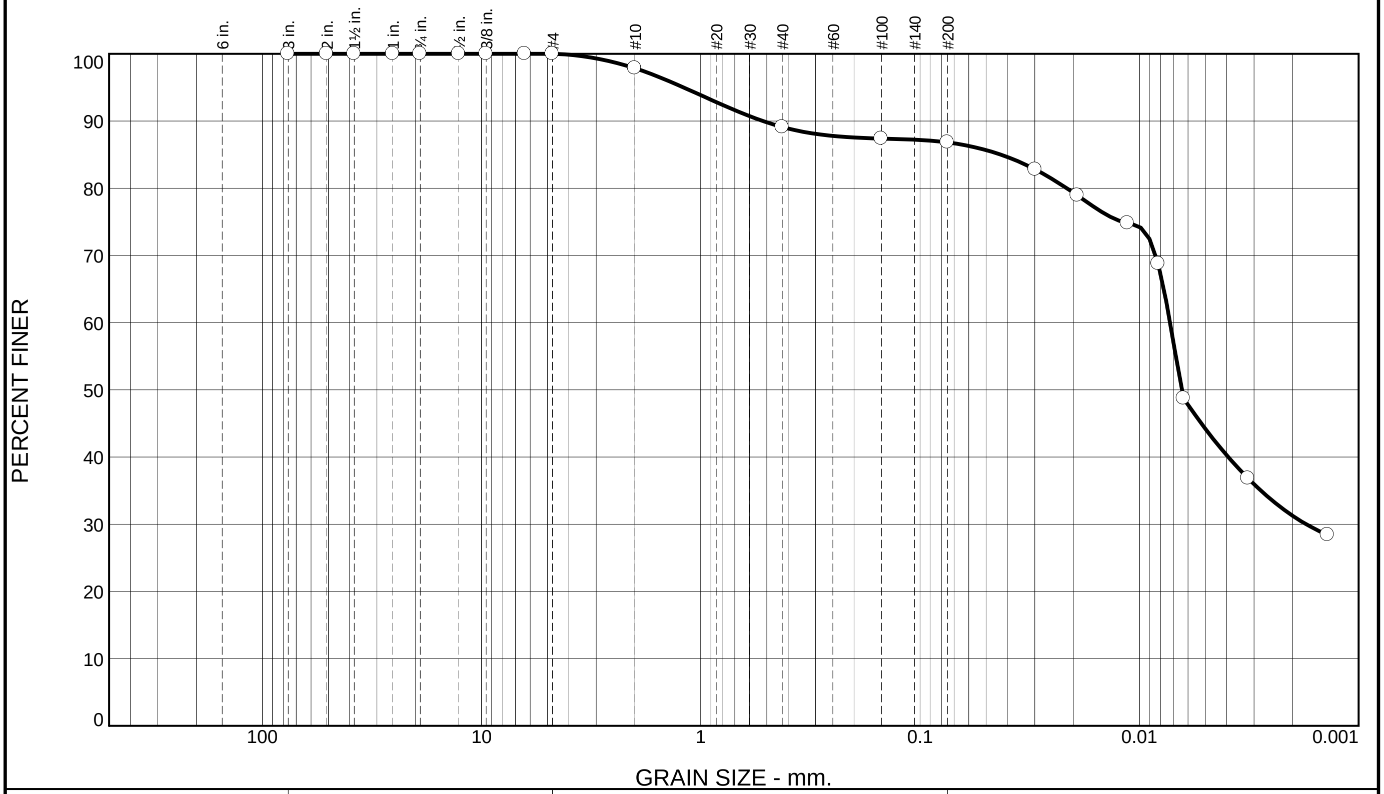
Terracon Consultants-NY, Inc.

Albany, NY

Client: McFarland Johnson
Project: Proposed Marmen Manufacturing Facility
Albany, NY
Project No: JB215020 **Figure** TP-21-4 4'-4.5'

Tested By: AB **Checked By:** JH

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	2.1	8.8	2.3	42.6	44.2

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100.0		
2"	100.0		
1.5"	100.0		
1"	100.0		
0.75"	100.0		
0.5"	100.0		
0.375"	100.0		
0.25"	100.0		
#4	100.0		
#10	97.9		
#40	89.1		
#100	87.4		
#200	86.8		

* (no specification provided)

Material Description

Lean Clay

Atterberg Limits

PL= 21 LL= 33 PI= 12

Coefficients

D_{90}= 0.5219	D_{85}= 0.0429	D_{60}= 0.0072
D_{50}= 0.0064	D_{30}= 0.0017	D_{15}=
D_{10}=	C_u=	C_c=

Classification

USCS= CL AASHTO= A-6(10)

Remarks

Per ASTM D422

Source of Sample: TP-21-7 3'-3.5'

Date: 9-29-21

Terracon Consultants-NY, Inc.

Albany, NY

Client: McFarland Johnson

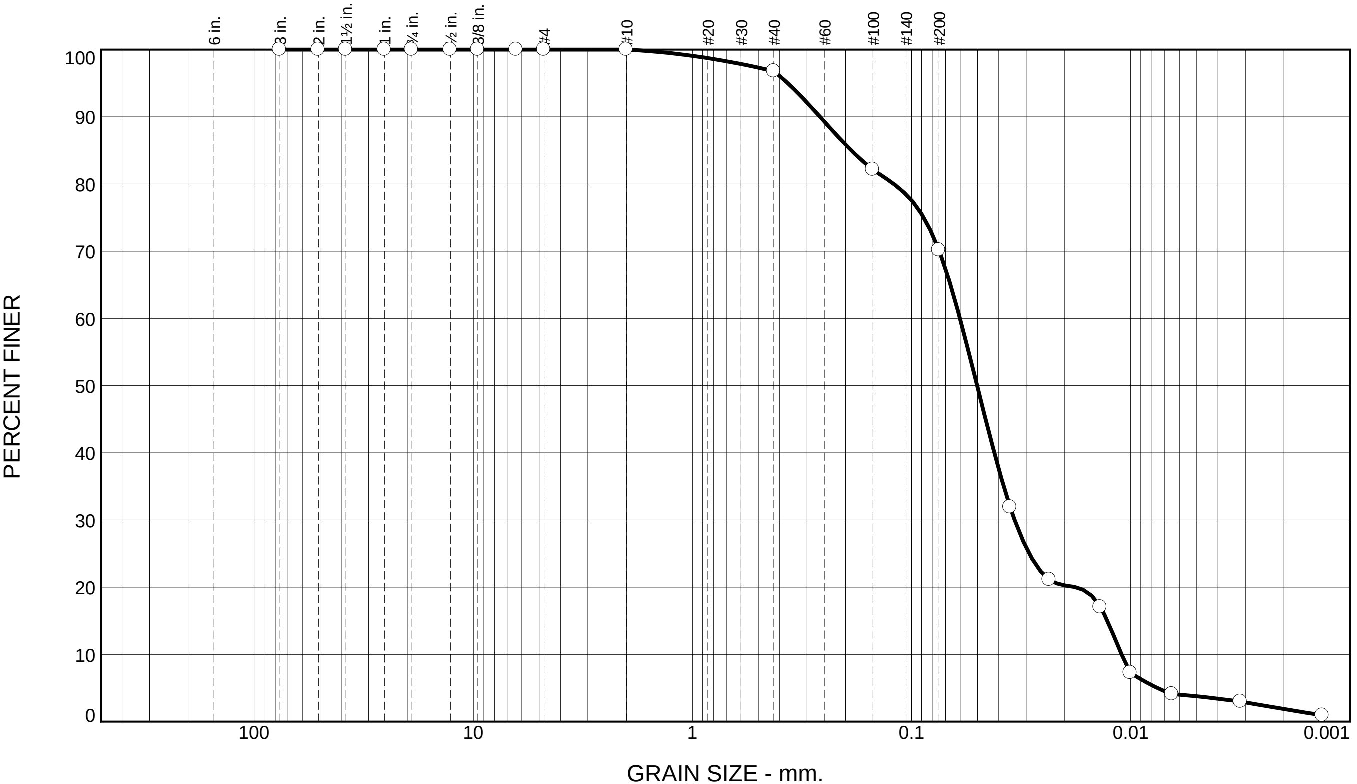
Project: Proposed Marmen Manufacturing Facility
Albany, NY

Project No: JB215020

Figure TP-21-7 3'-3.5'

Tested By: AB **Checked By:** JH

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	3.2	26.6	66.4	3.8

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100.0		
2"	100.0		
1.5"	100.0		
1"	100.0		
0.75"	100.0		
0.5"	100.0		
0.375"	100.0		
0.25"	100.0		
#4	100.0		
#10	100.0		
#40	96.8		
#100	82.2		
#200	70.2		

* (no specification provided)

Material Description

Silt with sand

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D_{90}= 0.2617 D_{85}= 0.1883 D_{60}= 0.0601
D_{50}= 0.0502 D_{30}= 0.0338 D_{15}= 0.0128
D_{10}= 0.0110 C_u= 5.46 C_c= 1.73

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Per ASTM D422

Source of Sample: TP-21-8 3'-3.5'

Date: 9-29-21

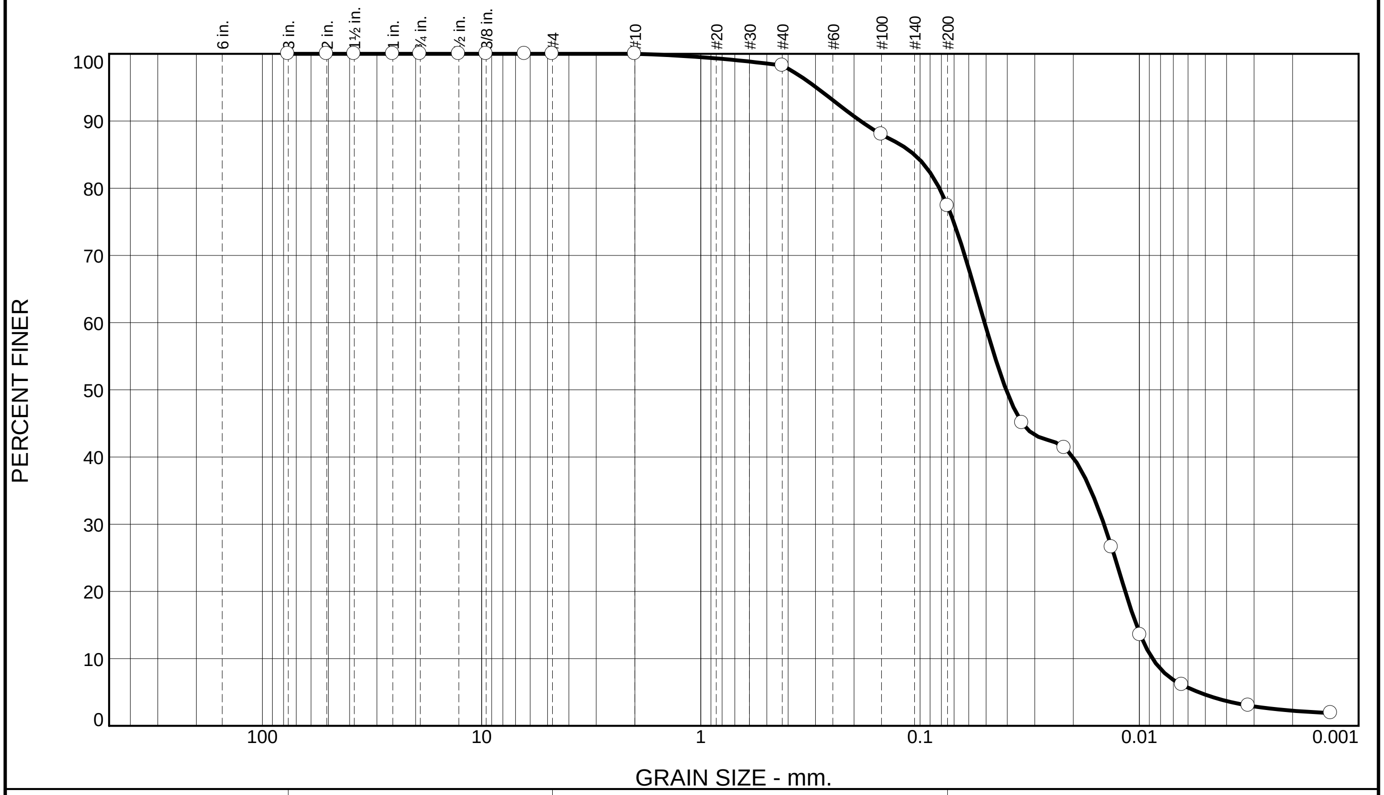
Terracon Consultants-NY, Inc.

Albany, NY

Client: McFarland Johnson
Project: Proposed Marmen Manufacturing Facility
Albany, NY
Project No: JB215020 **Figure** TP-21-8 3'-3.5'

Tested By: AB **Checked By:** JH

Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	1.7	20.9	72.8	4.6

SIEVE SIZE	PERCENT FINER	SPEC.* PERCENT	PASS? (X=NO)
3"	100.0		
2"	100.0		
1.5"	100.0		
1"	100.0		
0.75"	100.0		
0.5"	100.0		
0.375"	100.0		
0.25"	100.0		
#4	100.0		
#10	100.0		
#40	98.3		
#100	88.0		
#200	77.4		

* (no specification provided)

Material Description

Silt with sand

Atterberg Limits

PL= NP LL= NP PI= NP

Coefficients

D_{90}= 0.1870 D_{85}= 0.1062 D_{60}= 0.0508
D_{50}= 0.0405 D_{30}= 0.0145 D_{15}= 0.0103
D_{10}= 0.0087 C_u= 5.83 C_c= 0.47

Classification

USCS= ML AASHTO= A-4(0)

Remarks

Per ASTM D422

Source of Sample: TP-21-12 5'-5.5'

Date: 9-29-21

Terracon Consultants-NY, Inc.

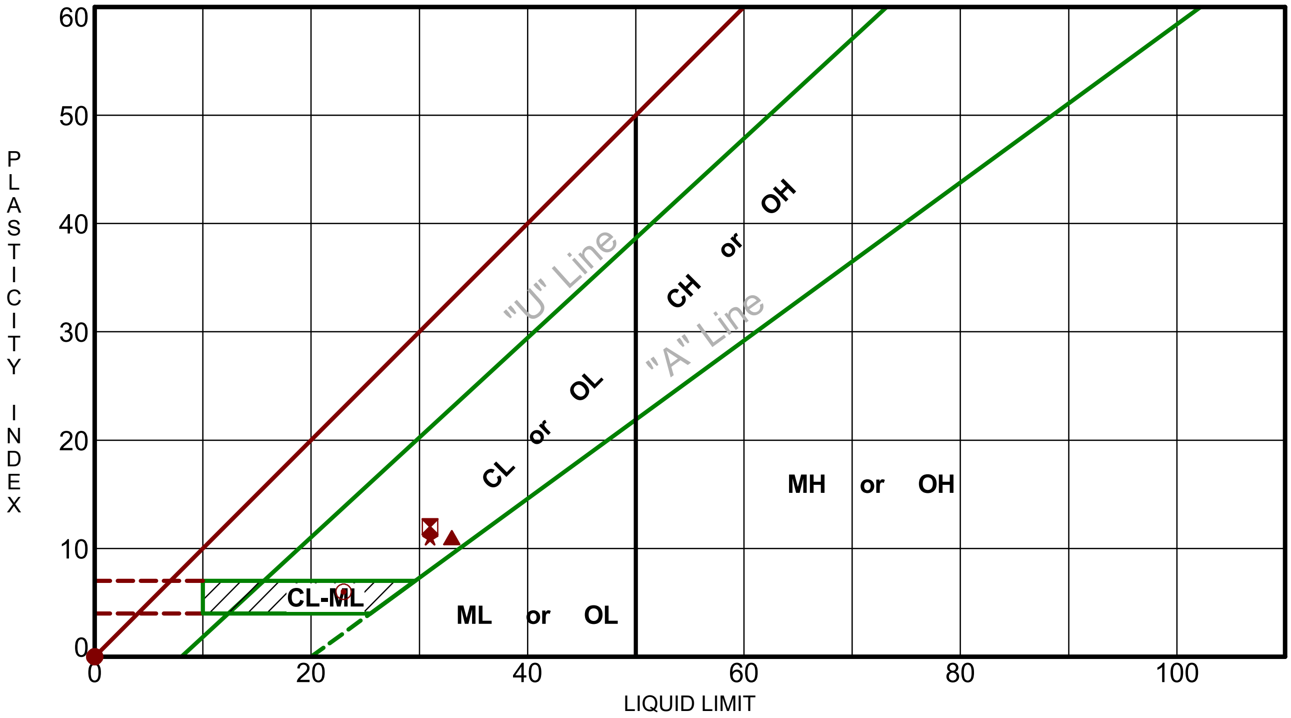
Albany, NY

Client: McFarland Johnson
Project: Proposed Marmen Manufacturing Facility
Albany, NY
Project No: JB215020 **Figure** TP-21-12 5'-5.5'

Tested By: AB **Checked By:** JH

ATTERBERG LIMITS RESULTS

ASTM D4318



Boring ID		Depth	LL	PL	PI	Fines	USCS	Description
●	B-21-7	60 - 62	NP	NP	NP			
⧗	B-21-17	40 - 42	31	19	12		CL	Lean Clay
▲	B-21-18	35 - 37	33	22	11		CL	Lean Clay
★	B-21-20	40 - 42	31	20	11		CL	Lean Clay
⊙	B-21-23	110 - 112	23	17	6		CL-ML	Silty clay

PROJECT: Proposed Marmen Manufacturing Facility

SITE: River Road
Glenmont, NY

30 Corporate Cir Ste 201
Albany, NY

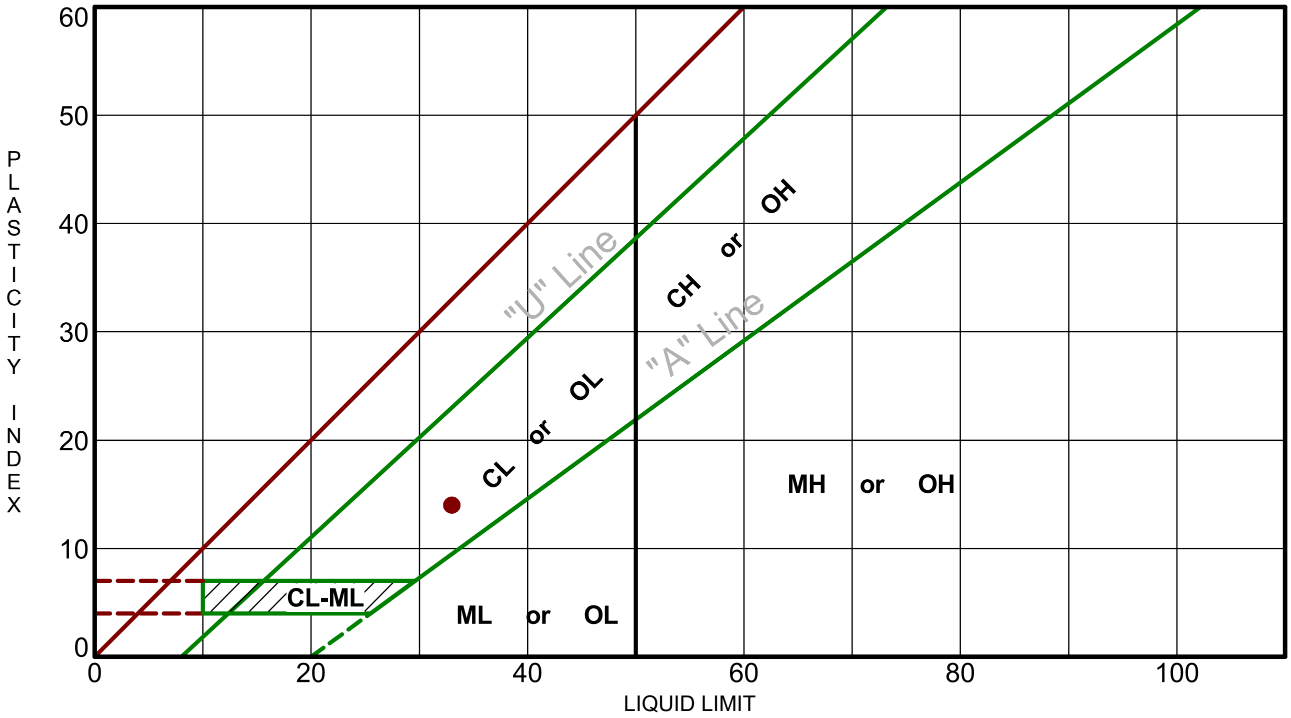
PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. ATTERBERG LIMITS JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 9/16/21

ATTERBERG LIMITS RESULTS

ASTM D4318



Boring ID		Depth	LL	PL	PI	Fines	USCS	Description
●	TP-21-3	6 - 6.5	33	19	14		CL	Lean Clay

PROJECT: Proposed Marmen Manufacturing Facility

SITE: River Road
Glenmont, NY

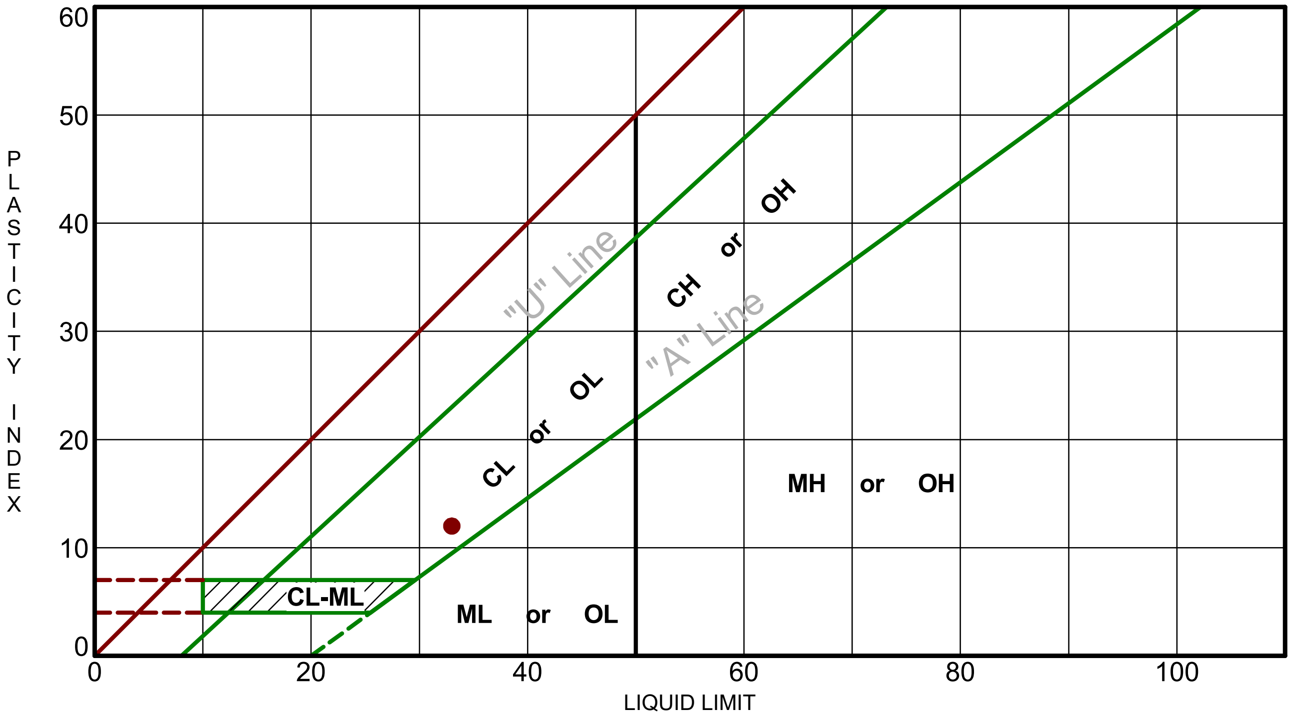
Terracon
30 Corporate Cir Ste 201
Albany, NY

PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

ATTERBERG LIMITS RESULTS

ASTM D4318



Boring ID	Depth	LL	PL	PI	Fines	USCS	Description
● TP-21-7	3 - 3.5	33	21	12		CL	Lean Clay

PROJECT: Proposed Marmen Manufacturing Facility

PROJECT NUMBER: JB215020

SITE: River Road
Glenmont, NY

Terracon
30 Corporate Cir Ste 201
Albany, NY

CLIENT: McFarland Johnson
Saratoga Springs, NY

Summary of Laboratory Results

Sheet 1 of 1

BORING ID	Depth (Ft.)	Water Content (%)	Organic Content (%)
B-21-7	6-8	99.4	
B-21-7	60-62	26.4	
B-21-8	0-2	22.8	
B-21-8	6-8		13.5
B-21-8	10-12	53.4	
B-21-8	25-27	26.8	
B-21-10	4-6	31.7	
B-21-10	10-12	46.7	8.2
B-21-10	16-18	34.9	
B-21-17	10-12	34.6	
B-21-17	14-16	48.3	
B-21-17	25-27	20.3	
B-21-17	40-42	30.5	
B-21-17	80-82	17.5	
B-21-18	10-12	44.8	
B-21-18	12-14	59.1	11.2
B-21-18	35-37	35.5	
B-21-18	55-57	89.9	
B-21-18	105-107	20.1	
B-21-20	6-8	106.4	
B-21-20	12-13.4	39.5	
B-21-20	16-18	44.7	3.7
B-21-20	40-42	33.2	
B-21-20	80-82	31.1	
B-21-23	12-14	89.2	
B-21-23	30-32	22.1	
B-21-23	45-47	30.0	
B-21-23	110-112	20.6	
S-1	1-1.1	41.1	
S-2	1-3	54.9	
S-3	1-3	67.7	
TP-21-3	6-6.5	19.6	
TP-21-4	4-4.5	67.3	
TP-21-7	3-3.5	18.9	
TP-21-8	3-3.5	63.9	
TP-21-12	5-5.5	70.8	

PROJECT: Proposed Marmen Manufacturing Facility

SITE: River Road
Glenmont, NY

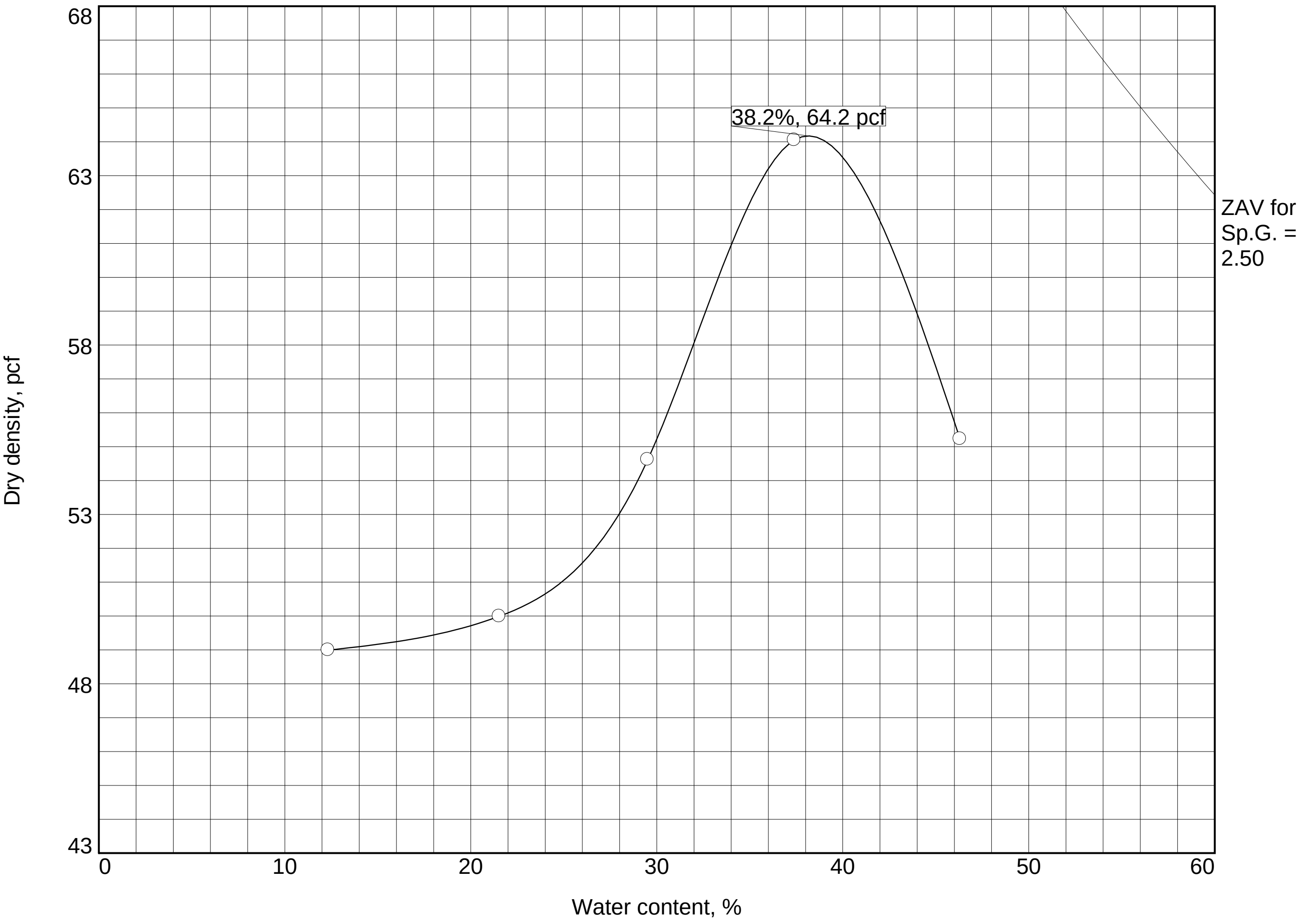
Terracon
30 Corporate Cir Ste 201
Albany, NY

PROJECT NUMBER: JB215020

CLIENT: McFarland Johnson
Saratoga Springs, NY

LABORATORY TESTS ARE NOT VALID IF SEPARATED FROM ORIGINAL REPORT. SMART LAB SUMMARY-PORTRAIT JB215020 PORT OF ALBANY MA.GPJ TERRACON_DATATEMPLATE.GDT 10/4/21

COMPACTION TEST REPORT



Test specification: ASTM D 1557-00 Method A Modified

Elev/ Depth	Classification		Nat. Moist.	Sp.G.	LL	PI	% > #4	% < No.200
	USCS	AASHTO						
1'-3'	ML			2.5				

TEST RESULTS	MATERIAL DESCRIPTION
Maximum dry density = 64.2 pcf Optimum moisture = 38.2 %	SILT with SAND (Coal ash)

Project No. JB215020 **Client:** McFarland Johnson

Project: Proposed Marmen Manufacturing Facility

Albany, NY

○**Source of Sample:** S-2 1'-3'

Terracon Consultants-NY, Inc.

Albany, NY

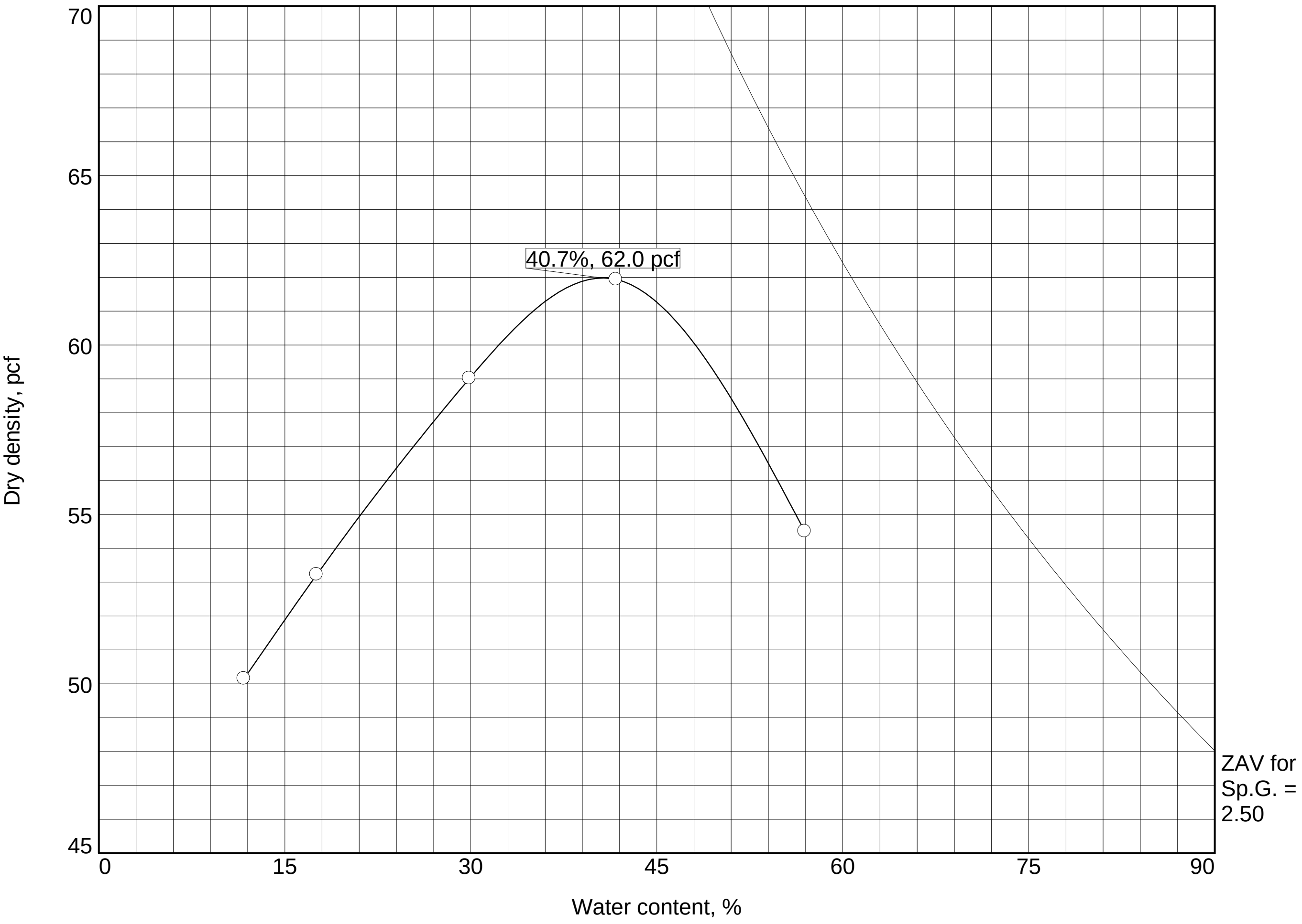
Remarks:

Per ASTM D1557

Figure S-2 1'-3'

Tested By: AB **Checked By:** JH

COMPACTION TEST REPORT



Test specification: ASTM D 1557-00 Method A Modified

Elev/ Depth	Classification		Nat. Moist.	Sp.G.	LL	PI	% > #4	% < No.200
	USCS	AASHTO						
	ML	A-4(0)		2.5	NP	NP	0.0	76.0

TEST RESULTS	MATERIAL DESCRIPTION
Maximum dry density = 62.0 pcf Optimum moisture = 40.7 %	Silt with sand (Coal ash)

Project No. JB215020 **Client:** McFarland Johnson

Project: Proposed Marmen Manufacturing Facility

Albany, NY

○**Source of Sample:** TP-21-4 4'-4.5'

Terracon Consultants-NY, Inc.

Albany, NY

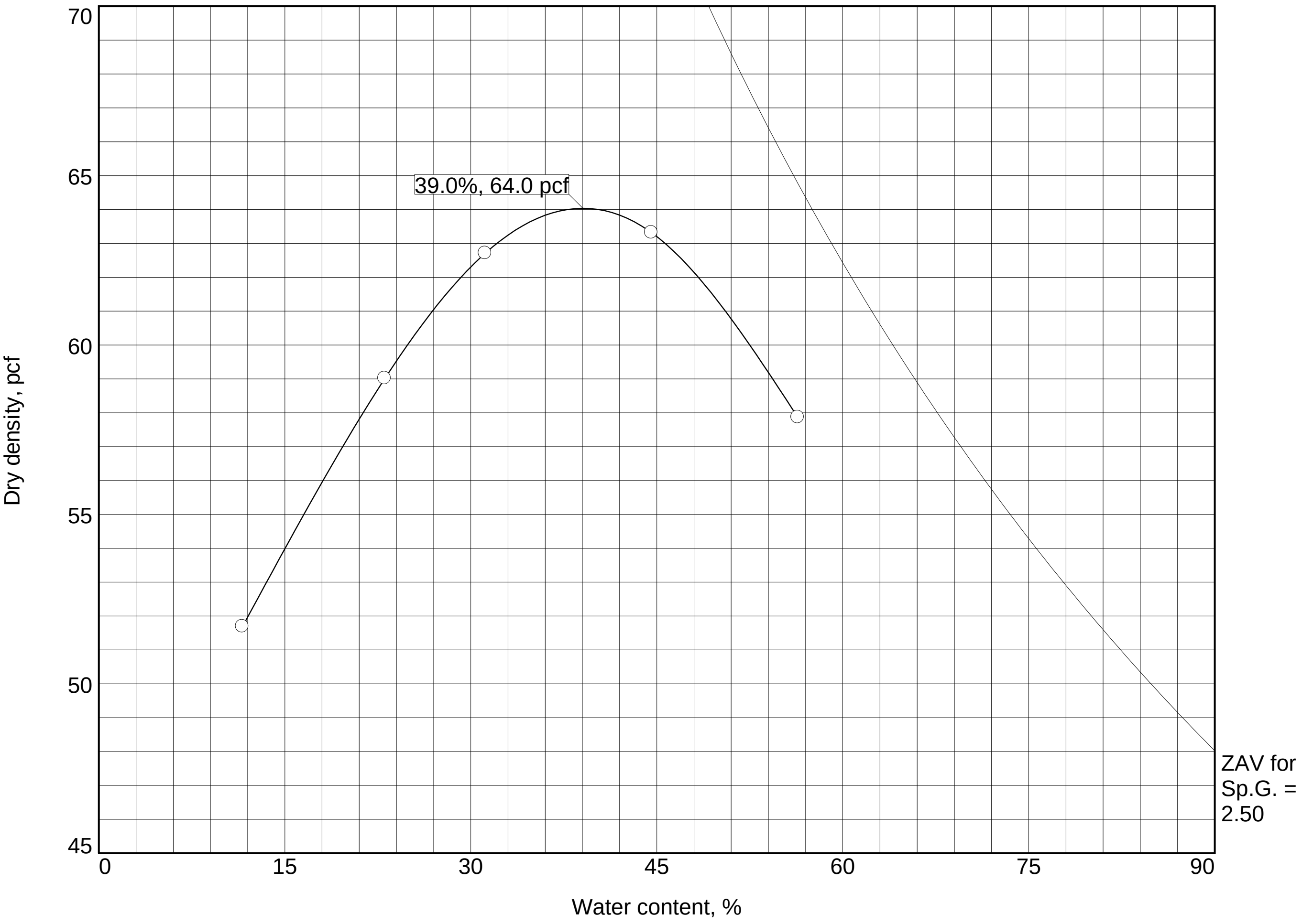
Remarks:

Per ASTM D1557

Figure TP-21-4 4'-4.5'

Tested By: AB **Checked By:** JH

COMPACTION TEST REPORT



Test specification: ASTM D 1557-00 Method A Modified

Elev/ Depth	Classification		Nat. Moist.	Sp.G.	LL	PI	% > #4	% < No.200
	USCS	AASHTO						
	ML	A-4(0)		2.5	NP	NP	0.0	70.2

TEST RESULTS	MATERIAL DESCRIPTION
Maximum dry density = 64.0 pcf Optimum moisture = 39.0 %	Silt with sand (Coal ash)

Project No. JB215020 **Client:** McFarland Johnson
Project: Proposed Marmen Manufacturing Facility
Albany, NY
○**Source of Sample:** TP-21-8 3'-3.5'

Terracon Consultants-NY, Inc.

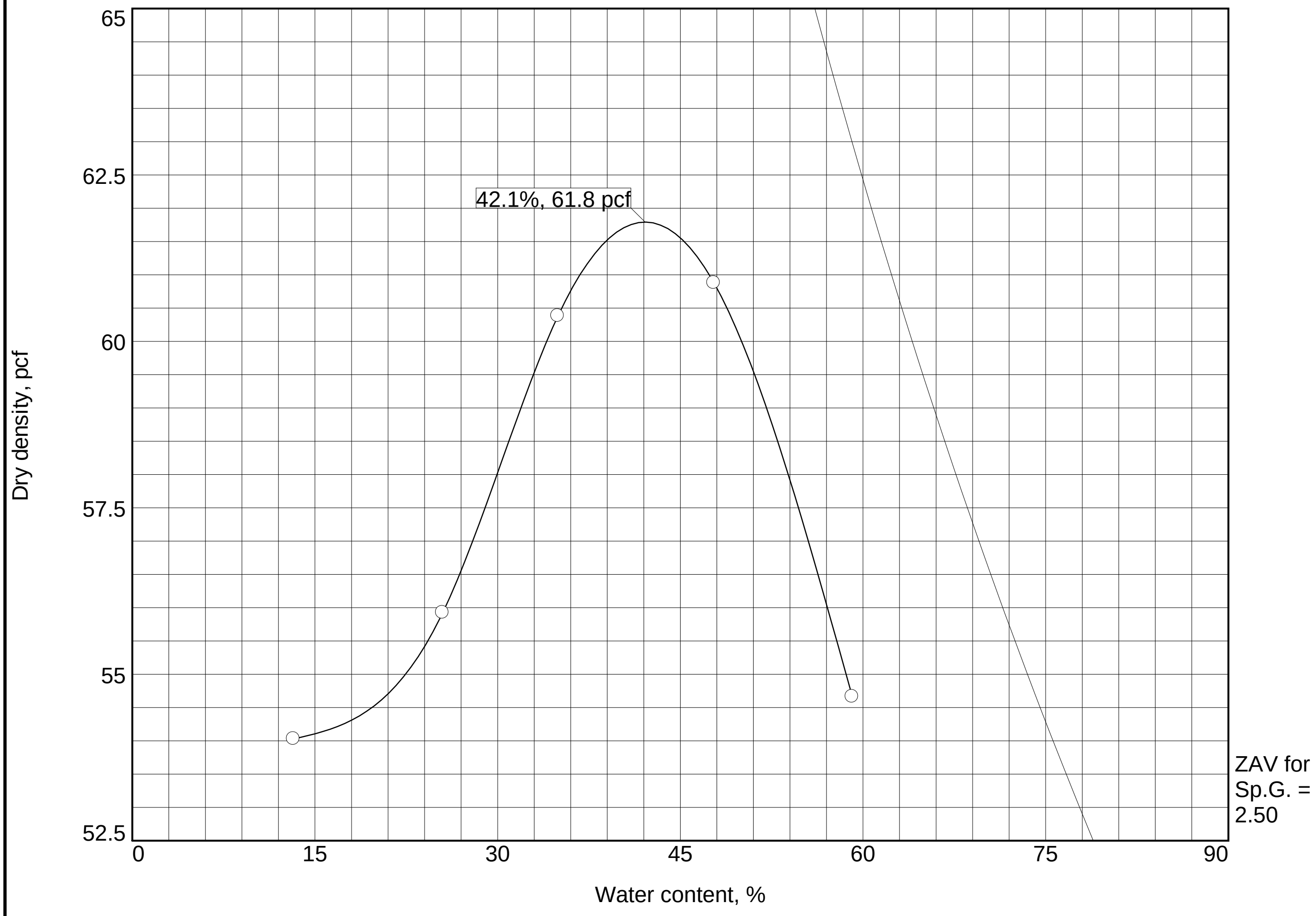
Albany, NY

Remarks:
Per ASTM D1557

Figure TP-21-8 3'-3.5'

Tested By: AB **Checked By:** JH

COMPACTION TEST REPORT



Test specification: ASTM D 1557-00 Method A Modified

Elev/ Depth	Classification		Nat. Moist.	Sp.G.	LL	PI	% > #4	% < No.200
	USCS	AASHTO						
	ML	A-4(0)		2.5	NP	NP	0.0	77.4

TEST RESULTS	MATERIAL DESCRIPTION
Maximum dry density = 61.8 pcf Optimum moisture = 42.1 %	Silt with sand (Coal ash)

Project No. JB215020 **Client:** McFarland Johnson
Project: Proposed Marmen Manufacturing Facility
Albany, NY
○**Source of Sample:** TP-21-12 5'-5.5'

Remarks:
Per ASTM D1557

Terracon Consultants-NY, Inc.

Albany, NY

Figure TP-21-12 5'-5.5'

Tested By: AB **Checked By:** JH

SUPPORTING INFORMATION

Contents:

General Notes
Unified Soil Classification System
Description of Rock Properties

Note: All attachments are one page unless noted above

GENERAL NOTES

DESCRIPTION OF SYMBOLS AND ABBREVIATIONS

Proposed Marmen Manufacturing Facility ■ Glenmont, NY
Terracon Project No. JB215020

SAMPLING

▯	Rock Core	✋	Grab Sample
■	Shelby Tube	☒	Split Spoon

WATER LEVEL

▽	**Water Initially Encountered**
▽	**Water Level After a Specified Period of Time**
▼	**Water Level After a Specified Period of Time**
▨	**Cave In Encountered**

Water levels indicated on the soil boring logs are the levels measured in the borehole at the times indicated. Groundwater level variations will occur over time. In low permeability soils, accurate determination of groundwater levels is not possible with short term water level observations.

FIELD TESTS

N	**Standard Penetration Test Resistance (Blows/Ft.)**
(HP)	**Hand Penetrometer**
(T)	**Torvane**
(DCP)	**Dynamic Cone Penetrometer**
UC	**Unconfined Compressive Strength**
(PID)	**Photo-Ionization Detector**
(OVA)	**Organic Vapor Analyzer**

DESCRIPTIVE SOIL CLASSIFICATION

Soil classification as noted on the soil boring logs is based Unified Soil Classification System. Where sufficient laboratory data exist to classify the soils consistent with ASTM D2487 "Classification of Soils for Engineering Purposes" this procedure is used. ASTM D2488 "Description and Identification of Soils (Visual-Manual Procedure)" is also used to classify the soils, particularly where insufficient laboratory data exist to classify the soils in accordance with ASTM D2487. In addition to USCS classification, coarse grained soils are classified on the basis of their in-place relative density, and fine-grained soils are classified on the basis of their consistency. See "Strength Terms" table below for details. The ASTM standards noted above are for reference to methodology in general. In some cases, variations to methods are applied as a result of local practice or professional judgment.

LOCATION AND ELEVATION NOTES

Exploration point locations as shown on the Exploration Plan and as noted on the soil boring logs in the form of Latitude and Longitude are approximate. See Exploration and Testing Procedures in the report for the methods used to locate the exploration points for this project. Surface elevation data annotated with +/- indicates that no actual topographical survey was conducted to confirm the surface elevation. Instead, the surface elevation was approximately determined from topographic maps of the area.

STRENGTH TERMS

RELATIVE DENSITY OF COARSE-GRAINED SOILS (More than 50% retained on No. 200 sieve.) Density determined by Standard Penetration Resistance		**CONSISTENCY OF FINE-GRAINED SOILS** (50% or more passing the No. 200 sieve.) Consistency determined by laboratory shear strength testing, field visual-manual procedures or standard penetration resistance		
Descriptive Term (Density)	**Standard Penetration or N-Value Blows/Ft.**	**Descriptive Term (Consistency)**	**Unconfined Compressive Strength Qu, (tsf)**	**Standard Penetration or N-Value Blows/Ft.**
Very Loose	0 - 3	Very Soft	less than 0.25	0 - 1
Loose	4 - 9	Soft	0.25 to 0.50	2 - 4
Medium Dense	10 - 29	Medium Stiff	0.50 to 1.00	4 - 8
Dense	30 - 50	Stiff	1.00 to 2.00	8 - 15
Very Dense	> 50	Very Stiff	2.00 to 4.00	15 - 30
		Hard	> 4.00	> 30

RELEVANCE OF SOIL BORING LOG

The soil boring logs contained within this document are intended for application to the project as described in this document. Use of these soil boring logs for any other purpose may not be appropriate.

UNIFIED SOIL CLASSIFICATION SYSTEM

Criteria for Assigning Group Symbols and Group Names Using Laboratory Tests [A]					Soil Classification	
					Group Symbol	**Group Name** [B]
Coarse-Grained Soils: More than 50% retained on No. 200 sieve	**Gravels:** More than 50% of coarse fraction retained on No. 4 sieve	**Clean Gravels:** Less than 5% fines [C]	$Cu \geq 4$ and $1 \leq Cc \leq 3$ [E]		GW	Well-graded gravel [F]
			$Cu < 4$ and/or [Cc<1 or Cc>3.0] [E]		GP	Poorly graded gravel [F]
		Gravels with Fines: More than 12% fines [C]	Fines classify as ML or MH		GM	Silty gravel [F, G, H]
			Fines classify as CL or CH		GC	Clayey gravel [F, G, H]
	Sands: 50% or more of coarse fraction passes No. 4 sieve	**Clean Sands:** Less than 5% fines [D]	$Cu \geq 6$ and $1 \leq Cc \leq 3$ [E]		SW	Well-graded sand [I]
			$Cu < 6$ and/or [Cc<1 or Cc>3.0] [E]		SP	Poorly graded sand [I]
		Sands with Fines: More than 12% fines [D]	Fines classify as ML or MH		SM	Silty sand [G, H, I]
			Fines classify as CL or CH		SC	Clayey sand [G, H, I]
Fine-Grained Soils: 50% or more passes the No. 200 sieve	**Silts and Clays:** Liquid limit less than 50	**Inorganic:**	PI > 7 and plots on or above “A” line [J]		CL	Lean clay [K, L, M]
			PI < 4 or plots below “A” line [J]		ML	Silt [K, L, M]
		Organic:	Liquid limit - oven dried	< 0.75	OL	Organic clay [K, L, M, N]
			Liquid limit - not dried			Organic silt [K, L, M, O]
	Silts and Clays: Liquid limit 50 or more	**Inorganic:**	PI plots on or above “A” line		CH	Fat clay [K, L, M]
			PI plots below “A” line		MH	Elastic Silt [K, L, M]
		Organic:	Liquid limit - oven dried	< 0.75	OH	Organic clay [K, L, M, P]
			Liquid limit - not dried			Organic silt [K, L, M, Q]
Highly organic soils:	Primarily organic matter, dark in color, and organic odor				PT	Peat

[A] Based on the material passing the 3-inch (75-mm) sieve.

[B] If field sample contained cobbles or boulders, or both, add “with cobbles or boulders, or both” to group name.

[C] Gravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.

[D] Sands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with silt, SW-SC well-graded sand with clay, SP-SM poorly graded sand with silt, SP-SC poorly graded sand with clay.

[E] $Cu = D_{60}/D_{10}$ $\quad Cc = \frac{(D_{30})^2}{D_{10} \times D_{60}}$

[F] If soil contains ≥ 15% sand, add “with sand” to group name.

[G] If fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.

[H] If fines are organic, add “with organic fines” to group name.

[I] If soil contains ≥ 15% gravel, add “with gravel” to group name.

[J] If Atterberg limits plot in shaded area, soil is a CL-ML, silty clay.

[K] If soil contains 15 to 29% plus No. 200, add “with sand” or “with gravel,” whichever is predominant.

[L] If soil contains ≥ 30% plus No. 200 predominantly sand, add “sandy” to group name.

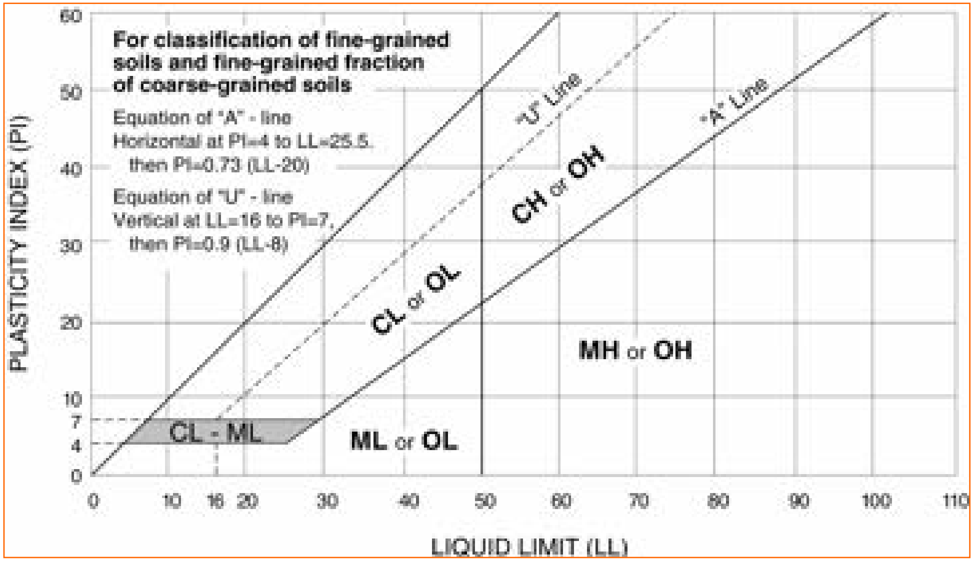
[M] If soil contains ≥ 30% plus No. 200, predominantly gravel, add “gravelly” to group name.

[N] PI ≥ 4 and plots on or above “A” line.

[O] PI < 4 or plots below “A” line.

[P] PI plots on or above “A” line.

[Q] PI plots below “A” line.



DESCRIPTION OF ROCK PROPERTIES

WEATHERING

Term	Description
Unweathered	No visible sign of rock material weathering, perhaps slight discoloration on major discontinuity surfaces.
Slightly weathered	Discoloration indicates weathering of rock material and discontinuity surfaces. All the rock material may be discolored by weathering and may be somewhat weaker externally than in its fresh condition.
Moderately weathered	Less than half of the rock material is decomposed and/or disintegrated to a soil. Fresh or discolored rock is present either as a continuous framework or as corestones.
Highly weathered	More than half of the rock material is decomposed and/or disintegrated to a soil. Fresh or discolored rock is present either as a discontinuous framework or as corestones.
Completely weathered	All rock material is decomposed and/or disintegrated to soil. The original mass structure is still largely intact.
Residual soil	All rock material is converted to soil. The mass structure and material fabric are destroyed. There is a large change in volume, but the soil has not been significantly transported.

STRENGTH OR HARDNESS

Description	Field Identification	Uniaxial Compressive Strength, psi (MPa)
Extremely weak	Indented by thumbnail	40-150 (0.3-1)
Very weak	Crumbles under firm blows with point of geological hammer, can be peeled by a pocket knife	150-700 (1-5)
Weak rock	Can be peeled by a pocket knife with difficulty, shallow indentations made by firm blow with point of geological hammer	700-4,000 (5-30)
Medium strong	Cannot be scraped or peeled with a pocket knife, specimen can be fractured with single firm blow of geological hammer	4,000-7,000 (30-50)
Strong rock	Specimen requires more than one blow of geological hammer to fracture it	7,000-15,000 (50-100)
Very strong	Specimen requires many blows of geological hammer to fracture it	15,000-36,000 (100-250)
Extremely strong	Specimen can only be chipped with geological hammer	>36,000 (>250)

DISCONTINUITY DESCRIPTION

Fracture Spacing (Joints, Faults, Other Fractures)		Bedding Spacing (May Include Foliation or Banding)	
Description	**Spacing**	**Description**	**Spacing**
Extremely close	< ¾ in (<19 mm)	**Laminated**	< ½ in (<12 mm)
Very close	¾ in – 2-1/2 in (19 - 60 mm)	**Very thin**	½ in – 2 in (12 – 50 mm)
Close	2-1/2 in – 8 in (60 – 200 mm)	**Thin**	2 in – 1 ft. (50 – 300 mm)
Moderate	8 in – 2 ft. (200 – 600 mm)	**Medium**	1 ft. – 3 ft. (300 – 900 mm)
Wide	2 ft. – 6 ft. (600 mm – 2.0 m)	**Thick**	3 ft. – 10 ft. (900 mm – 3 m)
Very Wide	6 ft. – 20 ft. (2.0 – 6 m)	**Massive**	> 10 ft. (3 m)

Discontinuity Orientation (Angle): Measure the angle of discontinuity relative to a plane perpendicular to the longitudinal axis of the core. (For most cases, the core axis is vertical; therefore, the plane perpendicular to the core axis is horizontal.) For example, a horizontal bedding plane would have a 0-degree angle.

ROCK QUALITY DESIGNATION (RQD) [1]

Description	RQD Value (%)
Very Poor	0 - 25
Poor	25 – 50
Fair	50 – 75
Good	75 – 90
Excellent	90 - 100

1. The combined length of all sound and intact core segments equal to or greater than 4 inches in length, expressed as a percentage of the total core run length.

Reference: U.S. Department of Transportation, Federal Highway Administration, Publication No FHWA-NHI-10-034, December 2009 Technical Manual for Design and Construction of Road Tunnels – Civil Elements

DEPARTMENT OF THE ARMY PERMIT

Permittee: Albany Port District Commission
Attention: Mr. Richard Hendrick, Chief Executive Officer
106 Smith Boulevard
Albany, New York 12202
(518) 463-2164

Permit Number: NAN-2021-00948-UDA

Date Issued: APR 10 2023

Issuing Office: U.S. Army Corps of Engineers, New York District

NOTE: The term "you" and its derivatives, as used in this permit, means the permittee or any future transferee. The term "this office" refers to the appropriate district or division office of the Corps of Engineers having jurisdiction over the permitted activity or the appropriate official of that office acting under the authority of the commanding officer.

You are authorized to perform work in accordance with the terms and conditions specified below.

Project Description: Dredging, with 10-years maintenance, of approximately 80,000 cubic yards (cy) of material via a barge-mounted crane with a closed clamshell bucket, from a 2.62-acre area of riverbed to a depth of 32 feet (ft) below the plane of the mean low water (MLW) with 2 ft over dredge to 34 ft. MLW. Double-walled silt curtains will be installed during the entire dredging operation. The dredged material will be placed into scows and will be allowed to settle in the scows before the water is decanted and discharged back into the Hudson River. Once the material has been decanted, the scows will be shipped via tug to the Clean Earth Clermont Dredged Processing Facility (Facility I.D. 215.734.1400), located at One Linden Avenue East, Jersey City, New Jersey for amendment and trucking to an upland disposal site.

A new wharf, 500 ft long by 93 ft wide, will be constructed along the west bank of the Hudson River. The wharf will consist of a heavy stone slope revetment, a high-modulus steel sheet pile cutoff wall, and a drilled shaft supported open wharf and relieving platform. Structures and fill installed below mean high high water elevation include 136, 48-inch diameter drilled shaft foundations with permanent steel casing, and approximately 1,162 CY of rip rap. Approximately 263 CY of the rip rap would be placed below mean high water elevation. The permanent steel casing for the drilled shaft foundations and the sheet pile wall components will be vibrated in, rather than utilizing an impact hammer. An impact hammer will only be used to seat the steel casing within the first few inches in the top of rock.

A new, 3-span bridge will be constructed over the Normans Kill channel. The bridge will be constructed on two (2) reinforced concrete drilled shafts. The construction of the northern shaft will require the discharge of fill material into 0.04-acre of wetland.

1145-2-303b (Hudson River/Albany Port District Commission/Dredging w/return flow, bulkhead installation, bridge construction, discharge of fill material)

Five new warehouse and manufacturing buildings totaling approximately 626,014 (SF) and associated attendant features including roadways, parking areas, utilities, an approximately 822 linear foot retaining wall, a wastewater treatment facility, stormwater management features and associated treated runoff outfalls to the Hudson River will be constructed. The construction of the warehouse and manufacturing buildings and attendant features will require the discharge of fill material into total of 0.81-acre of wetlands, comprised of approximately 0.30-acre of palustrine emergent wetland and 0.51-acre of palustrine forested wetland, respectively. In addition, a maximum of 0.33-acre of palustrine forested wetland will be permanently converted to an emergent cover type to facilitate the construction of the new buildings and attendant features. An 18-inch diameter water intake pipe and associated screening will be installed within the Hudson River.

To mitigate for unavoidable impacts to waters of the United States, the applicant shall purchase a total of 2.57 credits from the Middle Hudson Service Area of the Ducks Unlimited New York In-Lieu Fee Program, secure a restrictive covenant over 1,700 liner feet of on-site riparian buffer along the Hudson River, and implement a submerged aquatic vegetation transplantation and monitoring effort.

All work shall be performed in accordance with the attached drawings and Special Conditions (A) through (R) listed below, all which are hereby made a part of this permit.

Project Location: IN: Hudson River

AT: City of Albany and Town of Bethlehem, Albany County, New York

Permit Conditions:

General Conditions:

1. The time limit for completing the work authorized ends on April 10, 2033**. If you find that you need more time to complete the authorized activity, submit your request for a time extension to this office for consideration at least one month before the above date is reached.**

2. You must maintain the activity authorized by this permit in good condition and in conformance with the terms and conditions of this permit. You are not relieved of this requirement if you abandon the permitted activity, although you may make a good faith transfer to a third party in compliance with General Condition 4 below. Should you wish to cease to maintain the authorized activity or should you desire to abandon it without a good faith transfer, you must obtain a modification of this permit from this office, which may require restoration of the area.

3. If you discover any previously unknown historic or archeological remains while accomplishing the activity authorized by this permit, you must immediately notify this office of what you have

found. We will initiate the Federal and state coordination required to determine if the remains warrant a recovery effort or if the site is eligible for listing in the National Register of Historic Places.

4. If you sell the property associated with this permit, you must obtain the signature of the new owner in the space provided and forward a copy of the permit to this office to validate the transfer of this authorization.

5. If a conditioned water quality certification has been issued for your project, you must comply with the conditions specified in the certification as special conditions to this permit. For your convenience, a copy of the certification is attached if it contains such conditions.

6. You must allow representatives from this office to inspect the authorized activity at any time deemed necessary to ensure that it is being or has been accomplished in accordance with the terms and conditions of your permit.

Special Conditions:

(A) The permittee shall submit to this office the dates of commencement and completion of the herein authorized activities on the attached forms.

(B) The permittee, and its agents, shall conduct all dredging activities from September 1st through December 31st of any calendar year in order to minimize adverse aquatic impacts to diadromous fish spawning migrations and their peak biological activity, and impacts to Shortnose sturgeon (*Acipenser brevirostrum*) and Atlantic sturgeon (*A. oxyrinchus*).

(C) The permittee shall install a weighted double-walled silt curtain during dredging operations.

(D) The permittee shall maintain a copy of this permit on all vessels engaged in dredging and transporting dredged materials.

(E) The permittee shall undertake all dredging activities in such a manner as to avoid large refuse piles, ridges across the bed of the waterway or deep holes, which have a tendency to cause injury to navigable channels or the banks of the waterway.

(F) If any portion of the Federal Navigation Channel affected by contractor operations, then it must be restored to its project depth (32' MLLW), with the side slope (1H3V) also restored. In addition, the permittee shall perform hydrographic surveys of the federal navigation channel adjacent to the worksite upon completion of work. The survey shall be submitted to the Regulatory Branch Upstate Regulatory Field Office within 30 days of the completion of the activity authorized by this permit verification.

(G) The permittee shall, no less than fourteen (14) days prior to the commencement of any dredging, notify the United States Coast Guard office (USCG) of the commencement of any dredging and expected completion date, the hours of the day the work will be performed, the names of the vessels on-scene, the VHF radio channel(s) the vessels will monitor, and the project's 24/7 point(s) of contact. This information may be emailed to SECTORNYWWM@uscg.mil.

(H) No less than 14 days prior to the commencement of any in-water work, the permittee shall inform local waterway users of the commencement of the work, using the "Local Notice to Mariners." Information required to be provided in the Local Notice to Mariners can be found at http://www.navcen.uscg.gov. This information may be e-mailed to D01-SMB-LNM@uscg.mil.

(I) The permittee shall provide the National Oceanic and Atmospheric Administration National Ocean Service (NOAA-NOS) of the project's physical completion date and as-built specifications so that NOAA may initiate the appropriate navigation chart updates and corrections. This information must be submitted online at:

http://ocsdata.ncd.noaa.gov/idrs/discrepancy.aspx

(J) The permittee shall verify, in writing, compliance with all required notices to USCG and NOAA-NOS as called for above, within five (5) calendar days of each notice. Verifications shall include the permit number (NAN-2021-00948-UDA) and be delivered to the New York District by email at cenan.rfo@usace.army.mil (preferred) or to the following address:

DEPARTMENT OF THE ARMY
US Army Corps of Engineers
ATTN: CENAN-OP-RU
Upstate New York Section
1 Buffington St., Bldg. 10, 3rd Fl. North
Watervliet, New York 12189-4000

(K) The permittee understands and agrees that, if future operations by the United States require the removal, relocation, or other alteration, of the structure or work herein authorized, or if, in the opinion of the Secretary of the Army or his authorized representative, said structure or work shall cause unreasonable obstruction to the free navigation of the navigable waters, the permittee will be required, upon due notice from the Corps of Engineers, to remove, relocate, or alter the structural work or obstructions caused thereby, without expense to the United States. No claim shall be made against the United States on account of any such removal or alteration.

(L) Outdoor lighting shall be located or shielded so that it is not confused with any aids to navigation and does not interfere with navigation on the adjacent waterway.

(M) The permittee shall ensure The Federal Navigation Channel will remain clear of any barges, vessels, and/or floating equipment. Any equipment staged within the channel must be temporary, and their placement will be coordinated with the USACE Albany Field Office and USCG.

(N) Within sixty (60) days of the effective date of this permit, the permittee shall submit to the New York District, U.S Army Corps of Engineers and the National Oceanic and Atmospheric Administration National Marine Fisheries, a final version of the document entitled "Port of Albany Compensatory SAV Mitigation Plan - Draft", dated January 9, 2023, and prepared AKRF, Inc., for review and approval. The mitigation measures outline in the draft plan shall not be implemented until approved in writing by the U.S. Army Corps of Engineers. No dredging associated with the project shall be conducted until the mitigation plan has been approved in writing, and the 0.21-acre of SAV has been transplanted in accordance with the plan.

(O) The permittee shall accomplish compensatory mitigation through the purchase of 2.57 credits from the Middle Hudson Service Area of the Ducks Unlimited New York In-Lieu Fee Program (HUC 02020006 & ~35 square mile section of HUC 01100005). Within thirty (30) days of the effective date of this permit, the permittee shall supply this office with a copy of the signed Credit Sale Letter verifying acceptance of full payment by Ducks Unlimited New York In-Lieu Fee Program for the appropriate credits.

(P) The permittee shall secure restrictive covenants on the 2.87-acre riparian buffer to guarantee their preservation for wetland and wildlife resources. Within thirty (30) days from the effective date of this permit, the permittee shall accomplish the preservation of these areas by properly executing and filing the restrictive covenant, as described in the draft received by this office on July 5, 2022, with the Recorder of Deeds for Albany County and shall provide to this office a copy of the documents, as filed in County records, within thirty (30) days of its filing.

(Q) Within sixty (60) days of the effective date of this permit, the permittee shall submit the final dredge plan to this office for review and written approval.

(R) The permittee shall undertake the authorized filling activities in a manner aimed at reducing impacts upon the general environment. In addition, the permittee shall not stockpile fill or other materials in a manner conducive to erosion, or in areas likely to cause high turbidity runoff during storm events. All exposed soils shall be re-vegetated in a timely manner to further reduce potential effects. The permittee shall also fence off all wetlands and other sensitive ecological areas during construction periods to prevent equipment and personnel from entering these areas.

(S) The permittee shall ensure that all synthetic erosion control features (e.g., silt fencing, netting, mats), which are intended for temporary use during construction, are completely removed and properly disposed of after their initial purpose has been served. Only natural fiber materials, which will degrade after time, may be used as permanent measures, or if used temporarily, may be abandoned in place.
Further Information:

1. Congressional Authorities: You have been authorized to undertake the activity described above pursuant to:

(X) Section 10 of the Rivers and Harbors Act of 1899 (33 U.S. Code 403).

(X) Section 404 of the Clean Water Act (33 U.S. Code 1344).

( ) Section 103 of the Marine Protection, Research and Sanctuaries Act of 1972 (33 U.S.C. 1413).

2. Limits of this authorization:

a. This permit does not obviate the need to obtain other Federal, state, or local authorizations required by law.

b. This permit does not grant any property rights or exclusive privileges.

c. This permit does not authorize any injury to the property or rights of others.

d. This permit does not authorize interference with any existing or proposed Federal project.

3. Limits of Federal Liability: In issuing this permit, the Federal Government does not assume any liability for the following:

a. Damages to the permitted project or uses thereof as a result of other permitted or unpermitted activities or from natural causes.

b. Damages to the permitted project or uses thereof as a result of current or future activities undertaken by or on behalf of the United States in the public interest.

c. Damages to persons, property, or to other permitted or unpermitted activities or structures caused by the activity authorized by this permit.

d. Design or construction deficiencies associated with the permitted work.

e. Damage claims associated with any future modification, suspension, or revocation of this permit.

4. Reliance on Applicant's Data: The determination of this office that issuance of this permit is not contrary to the public interest was made in reliance on the information you provided.

5. Reevaluation of Permit Decision: This office may reevaluate its decision on this permit at any time the circumstances warrant. Circumstances that could require a reevaluation include, but are not limited to, the following:

a. You fail to comply with the terms and conditions of this permit.

b. The information provided by you in support of your permit application proves to have been false, incomplete, or inaccurate (See 4 above).

c. Significant new information surfaces which this office did not consider in reaching the original public interest decision.

Such a reevaluation may result in a determination that it is appropriate to use the suspension, modification, and revocation procedures contained in 33 CFR 325.7 or enforcement procedures such as those contained in 33 CFR 326.4 and 326.5. The referenced enforcement procedures provide for the issuance of an administrative order requiring you to comply with the terms and conditions of your permit and for the initiation of legal action where appropriate. You will be required to pay for any corrective measures ordered by this office, and if you fail to comply with such directive, this office may in certain situations (such as those specified in 33 CFR 209.170) accomplish the corrective measures by contract or otherwise and bill you for the cost.

6. Extensions: General Condition 1 establishes a time limit for the completion of the activity authorized by this permit. Unless there are circumstances requiring either a prompt completion of the authorized activity or a reevaluation of the public interest decision, the Corps will normally give favorable consideration to a request for an extension of this time limit.

PERMITTEE: Albany Port District Commission
PERMIT NO.: NAN-2021-00948-UDA

Your signature below, as permittee, indicates that you accept and agree to comply with the terms and conditions of this permit.

Richard J Hendrick
(PERMITTEE)

April 6, 2023
(DATE)

This permit becomes effective when the Federal official, designated to act for the Secretary of the Army, has signed below.

(DISTRICT ENGINEER)

APR 10 2023
(DATE)

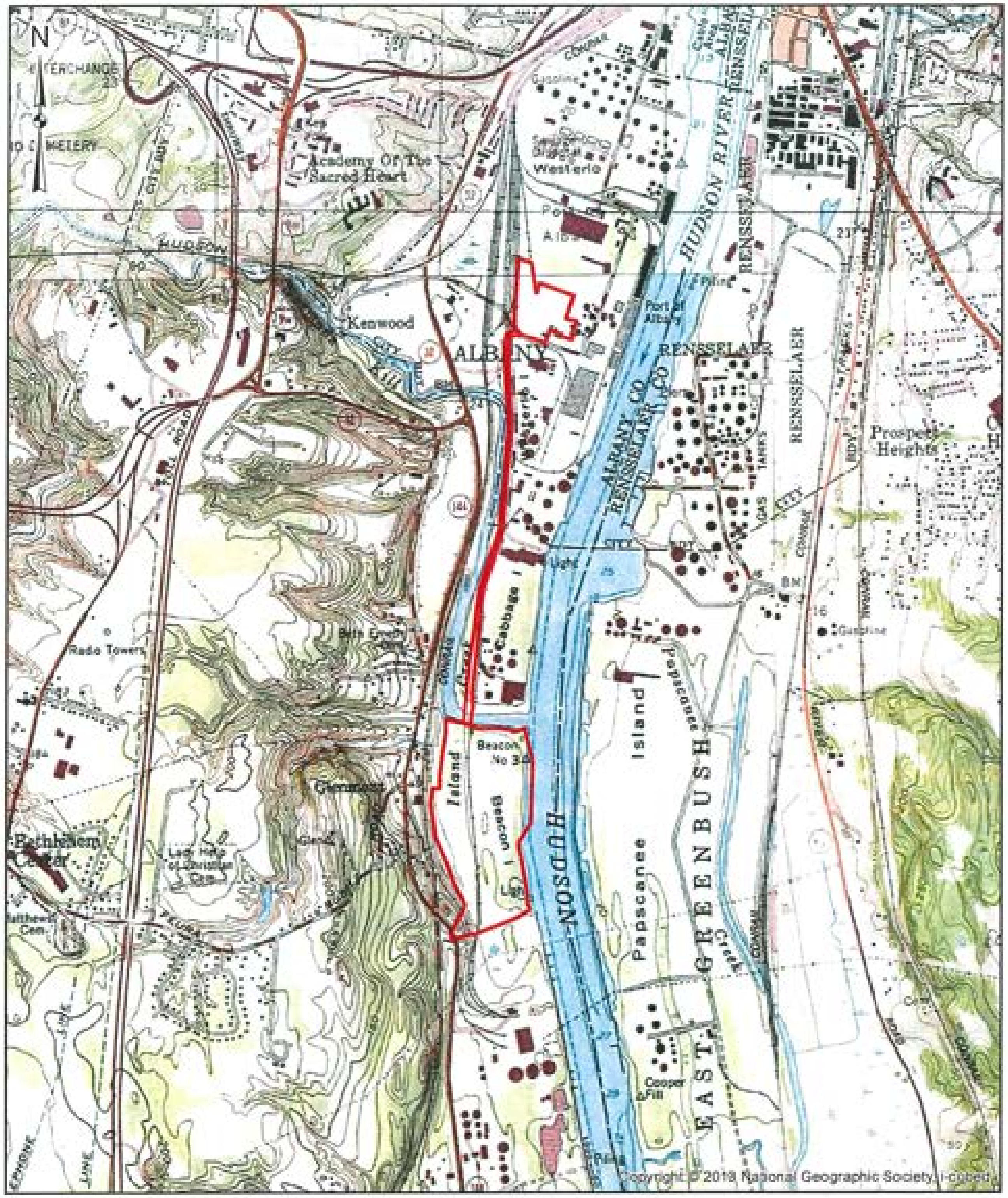
For and in behalf of
Matthew W. Luzzatto
Colonel, U.S. Army
Commander and District Engineer

When the structures or work authorized by this permit are still in existence at the time the property is transferred, the terms and conditions of this permit will continue to be binding on the new owner(s) of the property. To validate the transfer of this permit and the associated liabilities associated with compliance with its terms and conditions, have the transferee sign and date below. A copy of the permit signed by the transferee should be sent to this office.

(TRANSFEREE)

(DATE)

ENG FORM 1721, Nov 86 EDITION OF SEP 82 IS OBSOLETE. *(33 CFR 325 (Appendix A))*



Legend

🟥 Project Site

Notes:
1) Project boundary is approximate
2) All areas are approximate

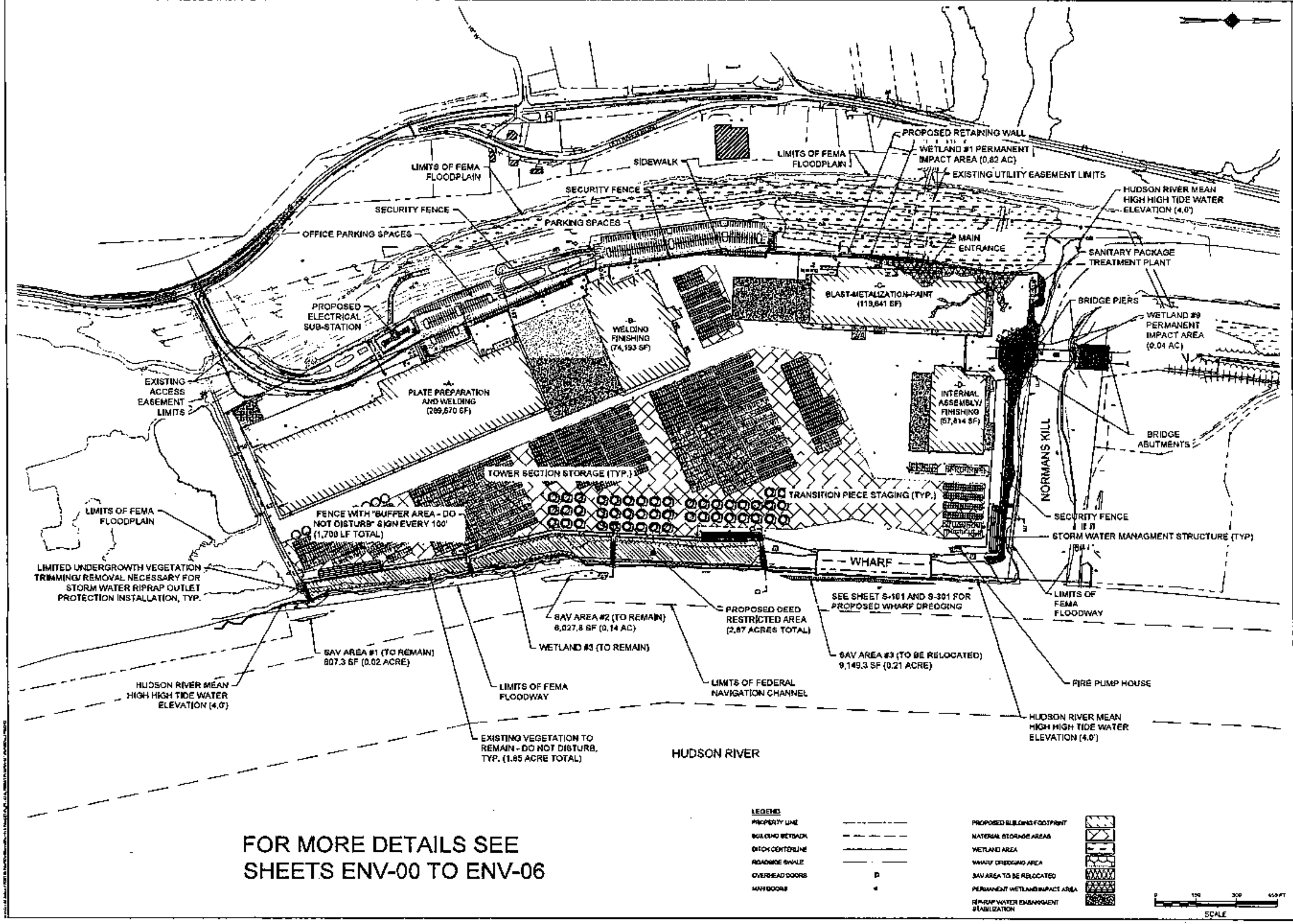
N:\15864.00 Albany Port Expansion\Draw\GIS\Supplemental Enviro 2021\Figure 2-1 - USGS Topo.mxd

PORT OF ALBANY DEVELOPMENT
TOWN OF BETHLEHEM, ALBANY COUNTY, NEW YORK

USGS TOPOGRAPHIC MAP

SCALE:	DATE:	FIGURE:
AS SHOWN	JULY 2021	2-1

McFarland Johnson



FOR MORE DETAILS SEE
SHEETS ENV-00 TO ENV-06

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-5383
SaratogaRCM@mjinc.com

PROJECT MILESTONE:
PERMIT SKETCHES

NO.	DATE	DESCRIPTION
1	9/06/22	GMP 100 SET
2	10/7/22	GMP 100 SET REVISIONS

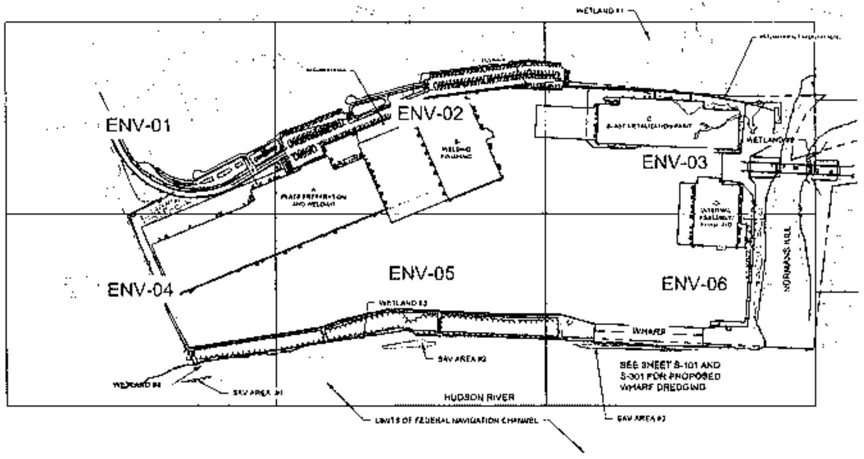
CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN: NSO
DESIGNED: NSO
CHECKED: AJF
SCALE: 1"=150'
DATE: MAY 2022
PROJECT: 18441.00

DRAWING TITLE
SITE PLAN OVERALL

DRAWING NUMBER
SP-01
1 of 14



GRADING NOTES:

1. REMOVE AND STOCKPILE TOPSOIL AS DIRECTED BY THE CONSTRUCTION MANAGER. REPLACE TOPSOIL TO A MINIMUM 4" DEPTH. ALL DISTURBED AREAS TO BE HYDROSEEDED AS DIRECTED BY THE CONSTRUCTION MANAGER.
2. CONTRACTOR SHALL BE RESPONSIBLE FOR THE MAINTENANCE AND REMOVAL OF TEMPORARY SEDIMENTATION CONTROLS, INCLUDING INLET PROTECTION AND SILT FENCE. EROSION CONTROL MEASURES SHALL NOT BE REMOVED BEFORE VEGETATION HAS OCCURRED COMPLETELY.
3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR RESTORATION OF TOPSOIL TO ALL DISTURBED AREAS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO MAINTAIN EROSION CONTROL MEASURES AT ALL TIMES.
4. EROSION CONTROL MEASURES WILL BE IMPLEMENTED IN ACCORDANCE WITH THE NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION, ALBANY COUNTY HEALTH DEPARTMENT, AND THE TOWN OF BETHLEHEM REQUIREMENTS.
5. ALL INLETS TO THE STORM SEWER SHALL HAVE STONE DROP INLET PROTECTION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING BEST MANAGEMENT PRACTICES (BMPS) UNTIL GROUND COVER IS ESTABLISHED.
6. SILT FENCE, JUTE MESH, AND/OR EROSION CONTROL BLANKETS WILL BE USED ON STEEP SLOPES AND WHEREVER NECESSARY TO CONTROL EROSION AND SILTATION OF EXISTING DRAINAGE SYSTEMS AS ORDERED BY THE ENGINEER OR SPECIFIED ON PLANS.
8. THE CONTRACTOR SHALL DESIGNATE A MEMBER OF HIS/HER FIRM TO BE RESPONSIBLE TO MONITOR EROSION CONTROL, EROSION CONTROL STRUCTURES, TREE PROTECTION AND PRESERVATION THROUGHOUT CONSTRUCTION.
9. ALL GRADING AND EARTHWORK SHALL BE IN CONFORMANCE WITH NEW YORK STATE STANDARD SPECIFICATIONS SECTION 203 - EXCAVATION AND EMBANKMENT, WHICH INCLUDES MAXIMUM EMBANKMENT LIFT THICKNESS ALLOWED BASED ON THE COMPACTION EQUIPMENT USED.
10. ALL PROPOSED ELEVATIONS SHOWN HEREON ARE FINISHED GRADE ELEVATION.
11. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING RIM ELEVATIONS IN RELATION TO PROPOSED GRADE PRIOR TO INSTALLATION.

STORM SEWER:

1. ALL HDPE PIPES SHALL FOLLOW NYSDOT SECTION 603-2 AND 706-12, BE SMOOTH INTERIOR.
2. PLACE RIP-RAP AROUND ALL END SECTIONS.
3. IN INSTANCES WHERE THE STORM SEWER CROSSES THE SANITARY SEWER A CRUSHED STONE ENCASEMENT SHALL BE PROVIDED AROUND THE SANITARY SEWER UP TO THE STORM SEWER COMPACT WITH APPROVED EQUIPMENT.
4. ALL CATCH BASINS AND STORM MANHOLES WITHIN PAVEMENT TO BE CONSTRUCTED TO WITHSTAND HS-20 LOADING.

LEGEND

PROPERTY LINE	— - - — - - —
BUILDING SETBACK	— — — — —
EXISTING RAIL LINE	++++++++++
EXISTING TREE LINE	
DITCH CENTERLINE	———————
ROADSIDE SWALE	——— ———
WETLAND AREA	☐

0 250 500 750 FT
SCALE

McFarland Johnson
60 RAILROAD PLACE
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SaratogaRCM@mjinc.com

PROJECT MILESTONE
PERMIT SKETCHES

NO.	DATE	DESCRIPTION

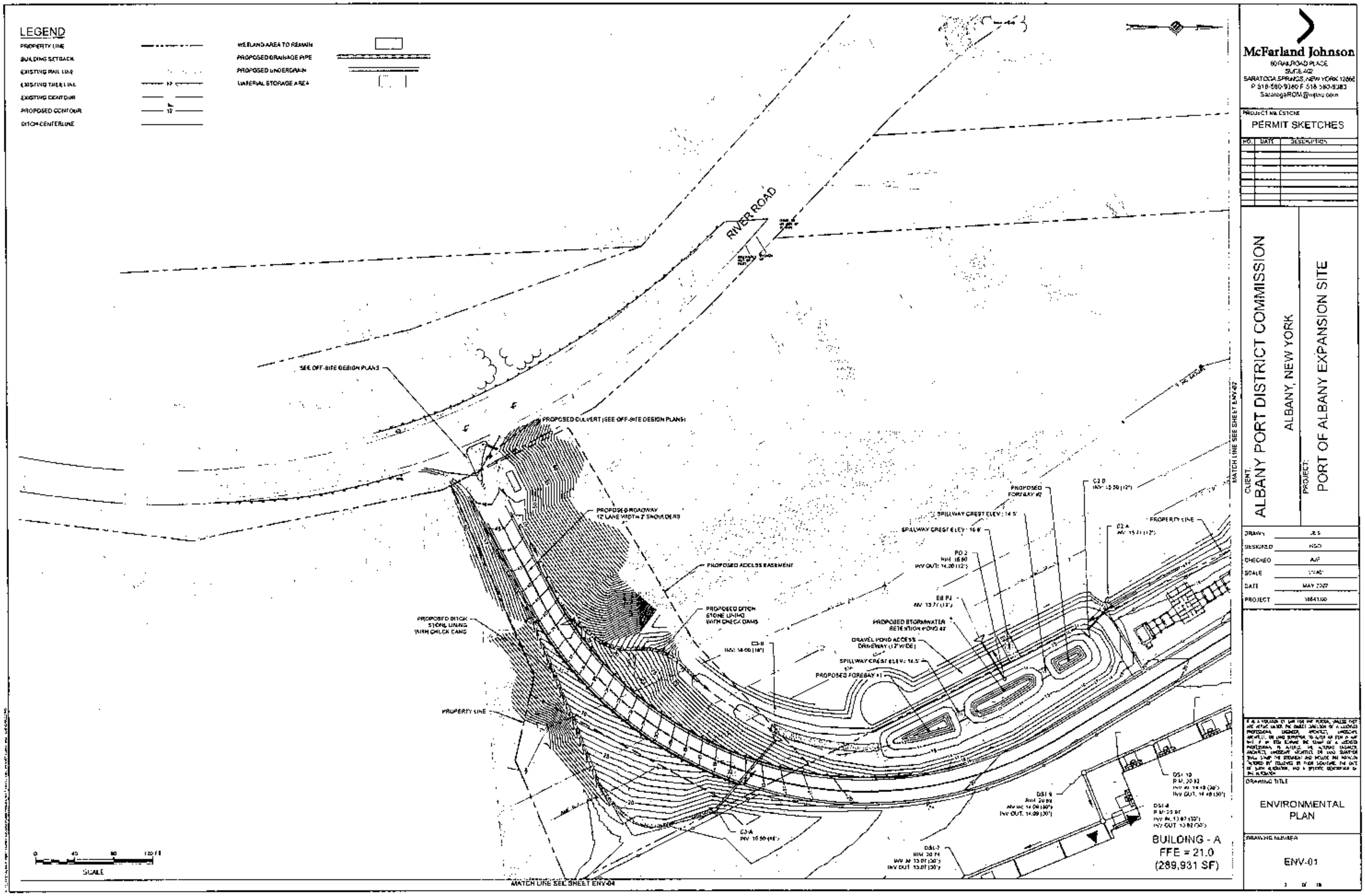
CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

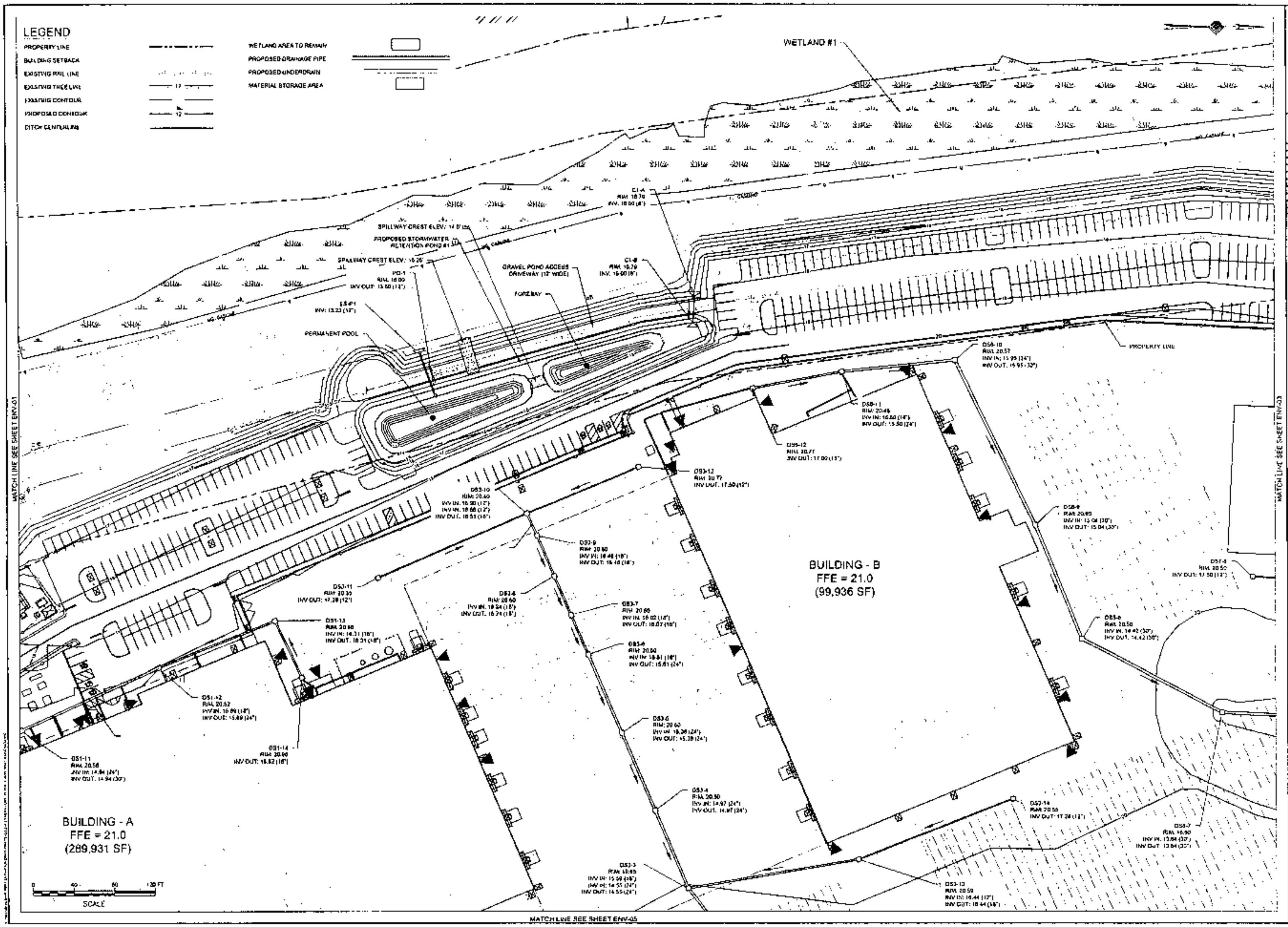
PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	1"=750'
DATE	MAY 2022
PROJECT	18641.00

DRAWING TITLE
ENVIRONMENTAL PLAN NOTES & INDEX

DRAWING NUMBER
ENV-00
1 OF 18


LEGEND
PROPERTY LINE
BUILDING SETBACK
EXISTING RAIL LINE
EXISTING TREE LINE
EXISTING CONTOUR
PROPOSED CONTOUR
12
DITCH CENTERLINE
WETLAND AREA TO REMAIN
PROPOSED DRAINAGE PIPE
PROPOSED UNDERDRAIN
MATERIAL STORAGE AREA
RIVER ROAD
SEE OFF-SITE DESIGN PLANS
PROPOSED CULVERT (SEE OFF-SITE DESIGN PLANS)
PROPOSED ROADWAY
12' LANE WIDTH 2' SHOULDERS
PROPOSED ACCESS EASEMENT
PROPOSED DITCH
STONE LINING
WITH CHECK DAMS
PROPOSED DITCH
STONE LINING
WITH CHECK DAMS
PROPERTY LINE
CB-B
INV: 14.00 (18")
SPILLWAY CREST ELEV: 14.5'
PROPOSED FOREBAY #1
GRAVEL POND ACCESS
DRIVEWAY (12' WIDE)
PROPOSED STORMWATER
RETENTION POND #2
EB PJ
INV: 13.77 (12")
PO 2
RIM: 16.80
INV OUT: 14.20 (12")
SPILLWAY CREST ELEV: 16.8'
SPILLWAY CREST ELEV: 14.5'
PROPOSED
FOREBAY #2
C2 D
INV: 15.50 (12")
C2 A
INV: 15.11 (12")
PROPERTY LINE
MATCH LINE SEE SHEET ENV-02
MATCH LINE SEE SHEET ENV-04
C3-A
INV: 16.50 (18")
DSL-7
RIM: 20.74
INV IN: 13.07 (30")
INV OUT: 13.07 (30")
DSI 9
RIM: 20.90
INV IN: 14.09 (30")
INV OUT: 14.09 (30")
DSI 10
RIM: 20.93
INV IN: 14.48 (30")
INV OUT: 14.48 (30")
DSI 8
RIM: 20.87
INV IN: 13.82 (30")
INV OUT: 13.82 (30")
BUILDING - A
FFE = 21.0
(289,931 SF)
0
40
80
120 FT
SCALE
McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P 518-580-9380 F 518-580-9383
SaratogaROM@mjinc.com
PROJECT MILESTONE
PERMIT SKETCHES
NO.
DATE
DESCRIPTION
CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK
PROJECT:
PORT OF ALBANY EXPANSION SITE
DRAWN
JES
DESIGNED
NSD
CHECKED
AJF
SCALE
1"=40'
DATE
MAY 2022
PROJECT
18641.00
DRAWING TITLE
ENVIRONMENTAL
PLAN
DRAWING NUMBER
ENV-01
3 OF 18



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SaratogaRCM@mjinc.com

PROJECT MILESTONE
PERMIT SKETCHES

NO	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

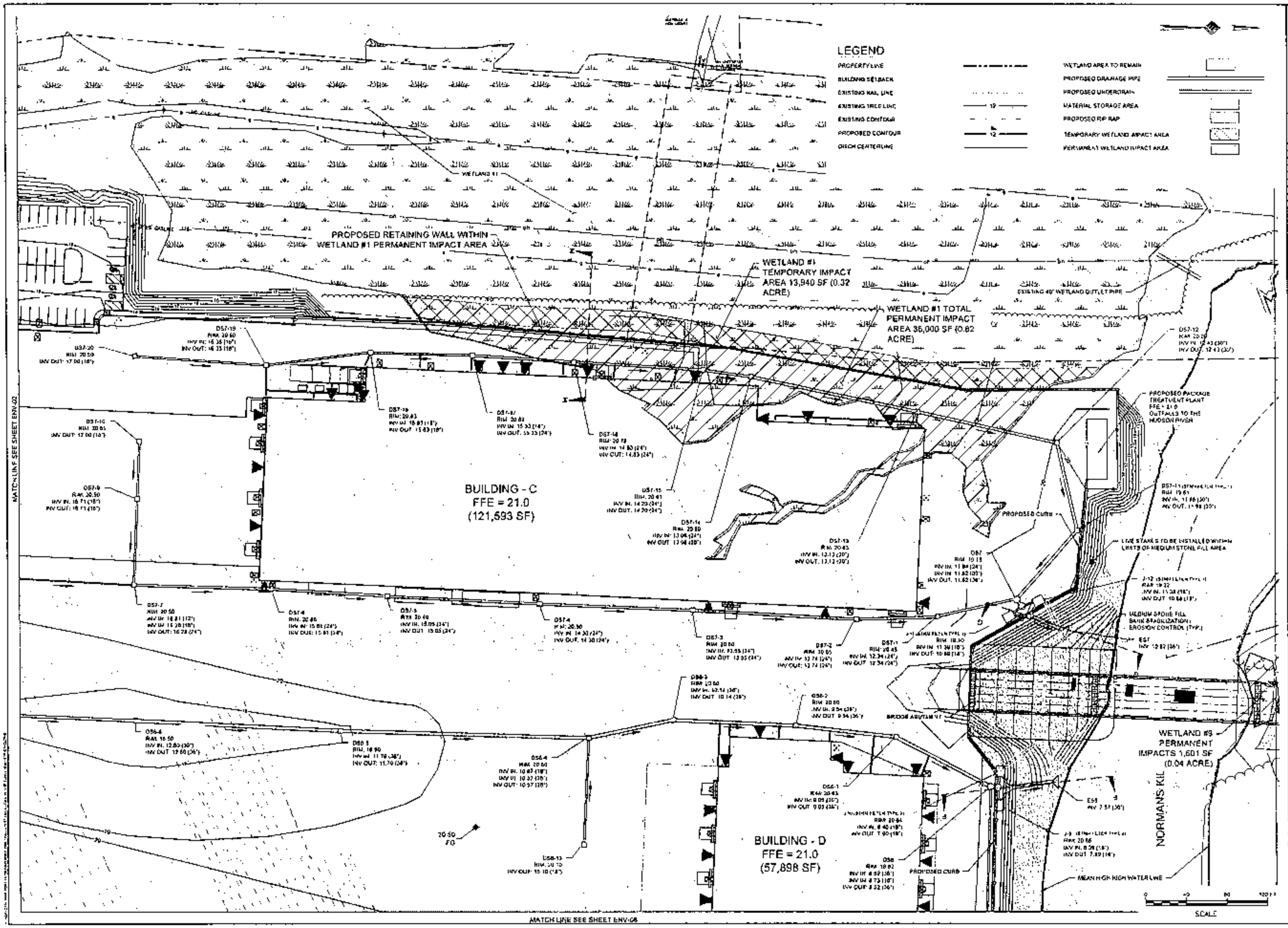
PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN: JLS
DESIGNED: NSG
CHECKED: AJF
SCALE: 1"=40'
DATE: MAY 2022
PROJECT: 18641.06

DRAWING TITLE
ENVIRONMENTAL PLAN

DRAWING NUMBER
ENV-02

4 OF 18



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SaratogaRGM@mjinc.com

PROJECT MILESTONE
PERMIT SKETCHES

NO	DATE	DESCRIPTION

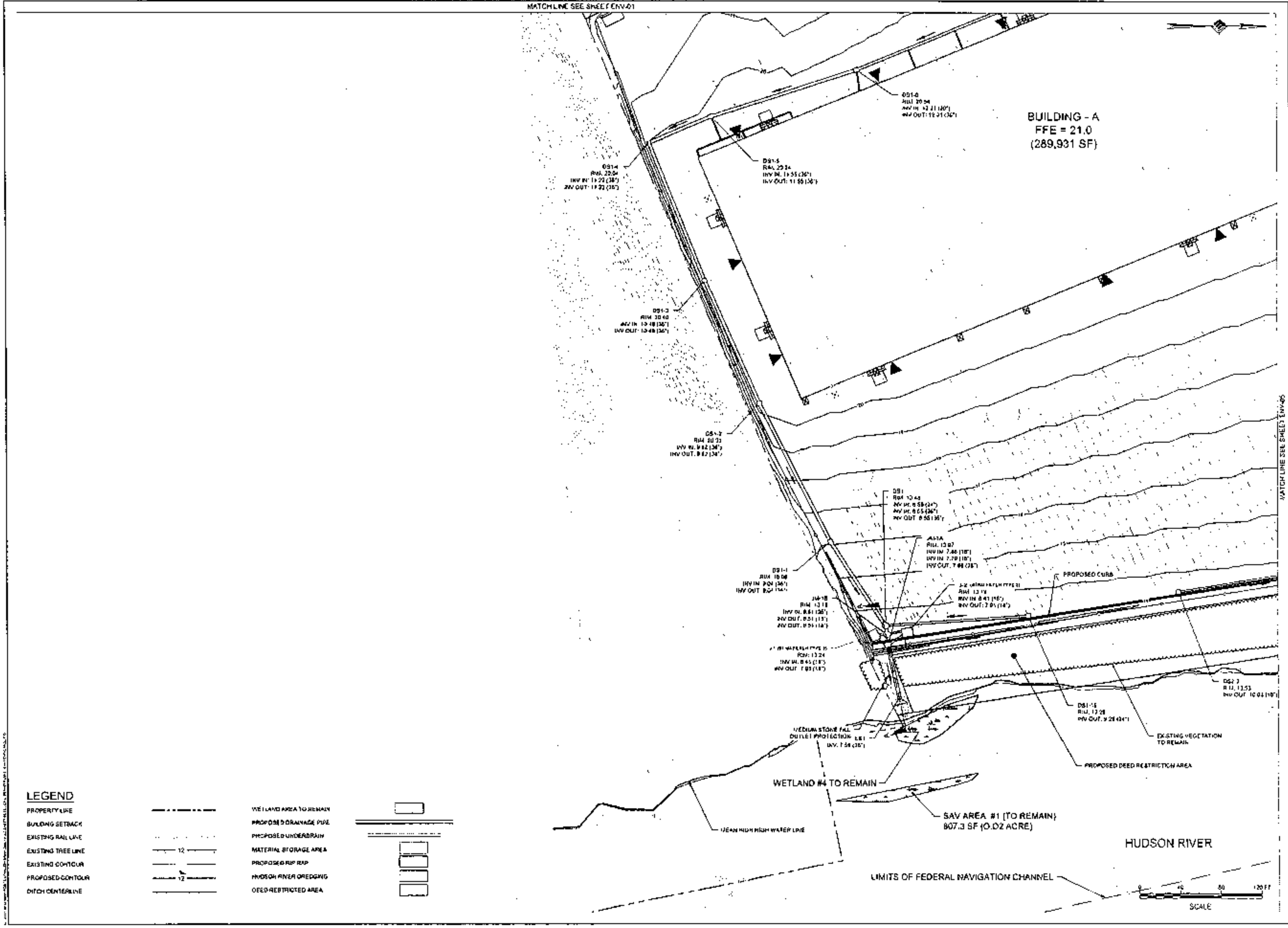
CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT
PORT OF ALBANY EXPANSION SITE

DRAWN	JLS
DESIGNED	HSO
CHECKED	AJF
SCALE	1"=40'
DATE	MAY 2022
PROJECT	19641.00

DRAWING TITLE
ENVIRONMENTAL PLAN

DRAWING NUMBER
ENV-03



McFarland Johnson
60 RAILROAD PLACE
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P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
PERMIT SKETCHES

NO.	DATE	DESCRIPTION

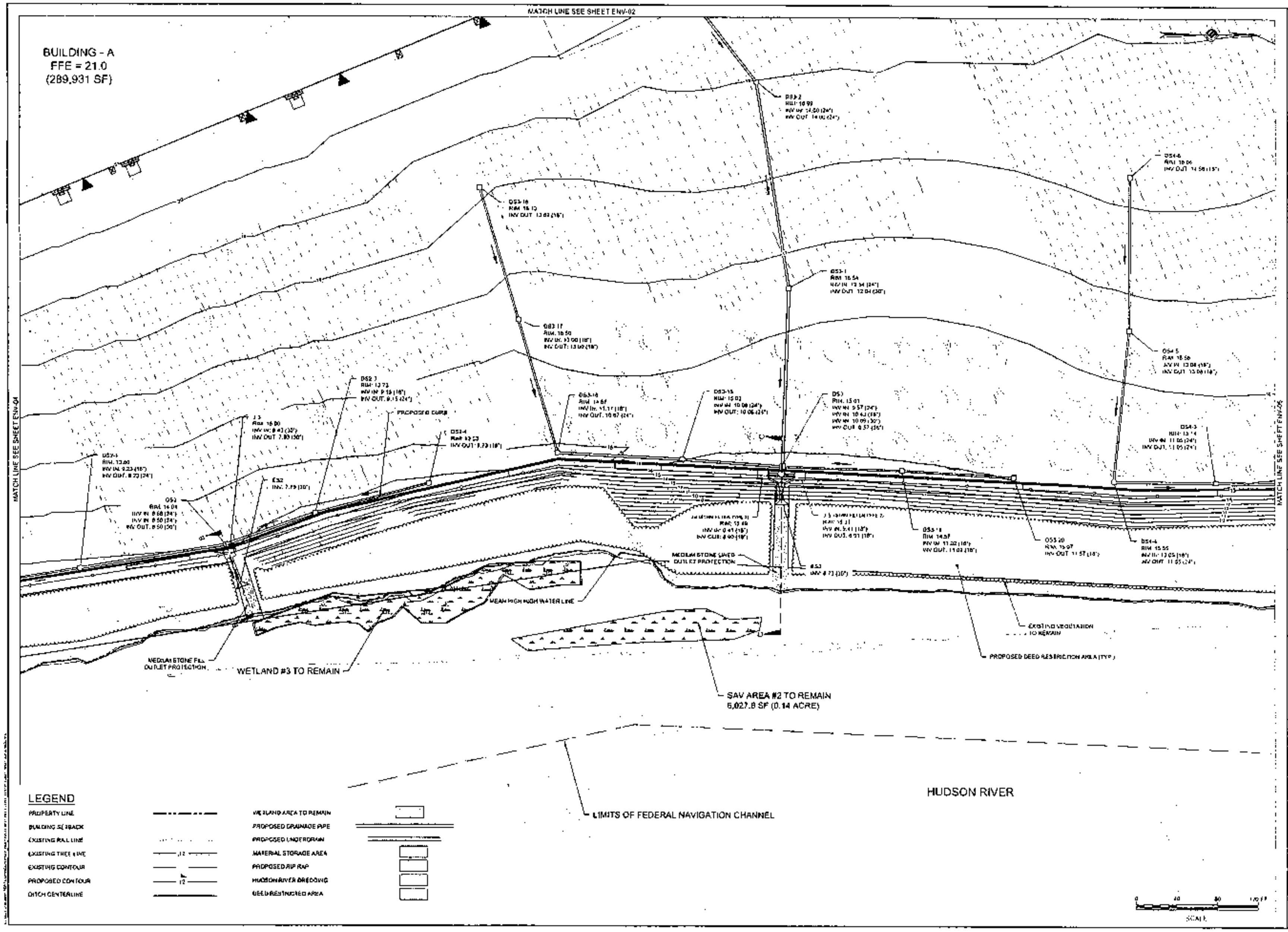
CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JES
DESIGNED	NSC
CHECKED	AJF
SCALE	1"=40'
DATE	MAY 2022
PROJECT	18645.02

DRAWING TITLE
ENVIRONMENTAL PLAN

DRAWING NUMBER
ENV-04



McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P 518-580-9380 F 518-580-5383
SaratogaHO@mjinc.com

PROJECT MILESTONE
FERMIT SKETCHES

NO.	DATE	DESCRIPTION

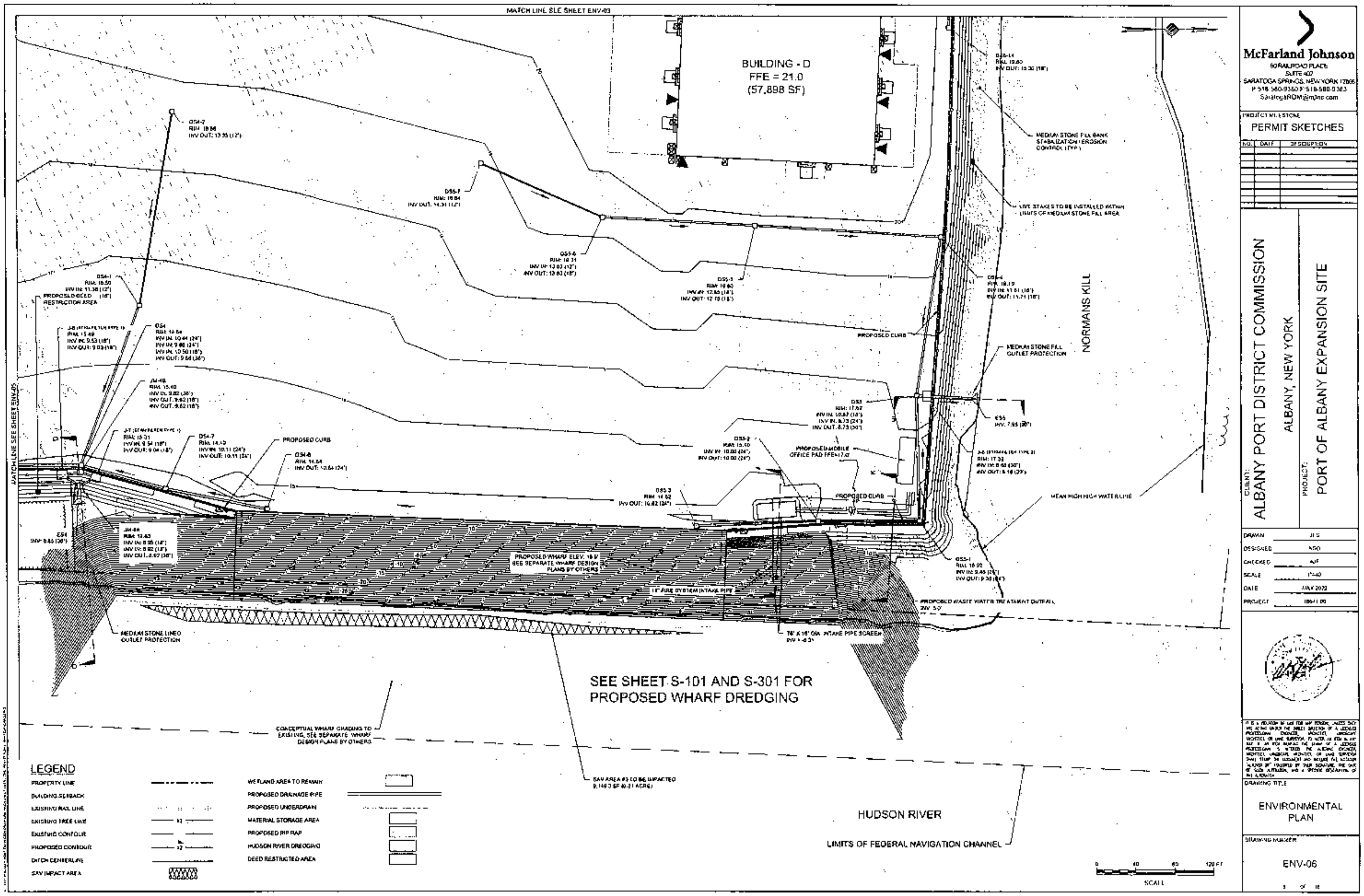
CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

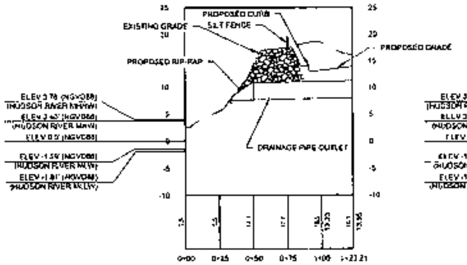
PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JES
DESIGNED	NSG
CHECKED	AJF
SCALE	1"=40'
DATE	MAY 2022
PROJECT	18845.00

DRAWING TITLE
ENVIRONMENTAL PLAN

DRAWING NUMBER
ENV-05
7 OF 16


MATCH LINE SEE SHEET ENV-03
MATCH LINE SEE SHEET ENV-05
BUILDING - D
FFE = 21.0
(57,898 SF)
NORMANS KILL
HUDSON RIVER
LIMITS OF FEDERAL NAVIGATION CHANNEL
SEE SHEET S-101 AND S-301 FOR
PROPOSED WHARF DREDGING
PROPOSED WHARF ELEV. 18.5'
SEE SEPARATE WHARF DESIGN
PLANS BY OTHERS
MEDIUM STONE FILL BANK
STABILIZATION / EROSION
CONTROL (TYP.)
LIVE STAKES TO BE INSTALLED WITHIN
LIMITS OF MEDIUM STONE FILL AREA
MEDIUM STONE FILL
OUTLET PROTECTION
MEAN HIGH HIGH WATER LINE
PROPOSED WASTE WATER TREATMENT OUTFALL
PROPOSED CURB
PROPOSED MOBILE
OFFICE PAD FFE=17.0
MEDIUM STONE LINED
OUTLET PROTECTION
PROPOSED DEED
RESTRICTION AREA
CONCEPTUAL WHARF GRADING TO
EXISTING, SEE SEPARATE WHARF
DESIGN PLANS BY OTHERS
SAV AREA #3 TO BE IMPACTED
FIRE SYSTEM INTAKE PIPE
DS4
DS4-1
DS4-2
DS4-7
DS4-8
DS5-1
DS5-2
DS5-3
DS5-4
DS5-5
DS5-6
DS5-7
DS5-14
ES4
ES5
JM-4A
JM-4B
LEGEND
PROPERTY LINE
BUILDING SETBACK
EXISTING RAIL LINE
EXISTING TREE LINE
EXISTING CONTOUR
PROPOSED CONTOUR
DITCH CENTERLINE
SAV IMPACT AREA
WETLAND AREA TO REMAIN
PROPOSED DRAINAGE PIPE
PROPOSED UNDERDRAIN
MATERIAL STORAGE AREA
PROPOSED RIP RAP
HUDSON RIVER DREDGING
DEED RESTRICTED AREA
0
40
80
120 FT
SCALE
McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P 518-580-9380 F 518-580-9383
SaratogaROM@mjinc.com
PROJECT MILESTONE
PERMIT SKETCHES
NO.
DATE
DESCRIPTION
CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK
PROJECT:
PORT OF ALBANY EXPANSION SITE
DRAWN
JLS
DESIGNED
NSO
CHECKED
AJF
SCALE
1"=40
DATE
MAY 2022
PROJECT
18641.00
DRAWING TITLE
ENVIRONMENTAL
PLAN
DRAWING NUMBER
ENV-06



SECTION A-A
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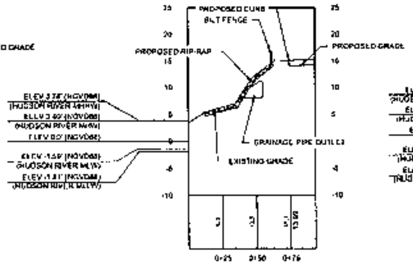
Vertical Scale: 1" = 10'



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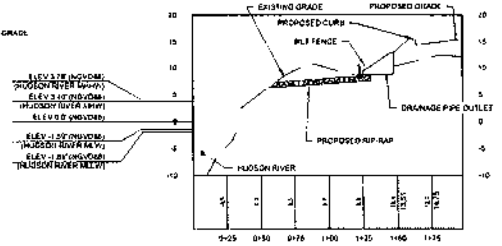

SECTION C-C
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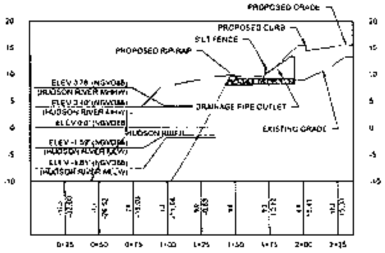
Vertical Scale: 1" = 10'



SECTION D-D
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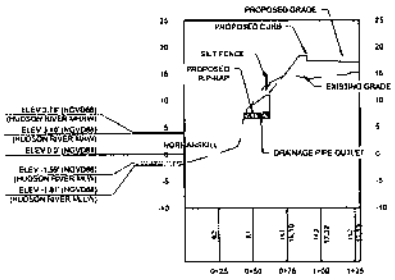
Vertical Scale: 1" = 10'



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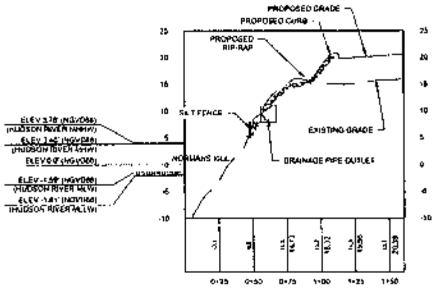

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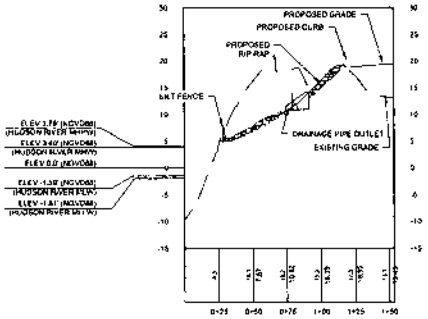
Vertical Scale: 1" = 10'



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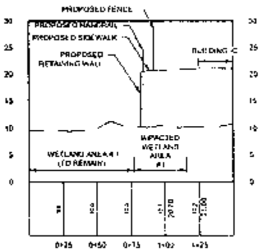
Horizontal Scale: 1" = 40'
Vertical Scale: 1" = 10'



SECTION H-H
Horizontal Scale: 1" = 40'
Vertical Scale: 1" = 10'

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P. 518-580-9380 F. 518-580-9383
SaratogaROW@mjinc.com

PROJECT MILESTONE
PERMIT SKETCHES

NO.	DATE	DESCRIPTION

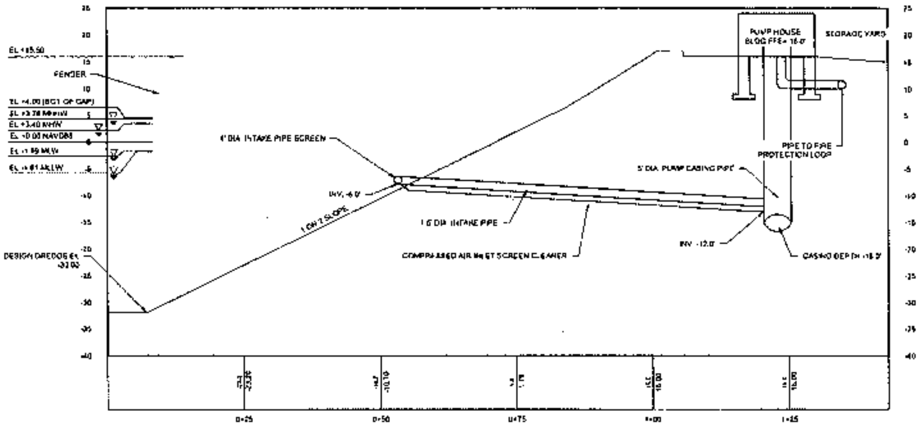
CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

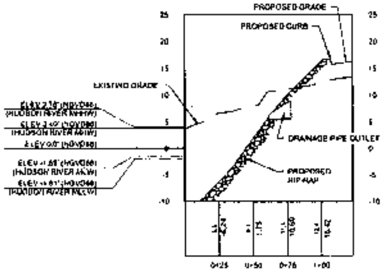
DRAWN	JLS
DESIGNED	KSO
CHECKED	AJF
SCALE	1"=40'
DATE	MAY 2022
PROJECT	18641.00

DRAWING TITLE
SECTION VIEWS

DRAWING NUMBER
XS-01



SECTION J-J
Horizontal Scale: 1" = 10'
Vertical Scale: 1" = 10'



WASTE WATER TREATMENT PLANT OUTFALL (K-K)
Horizontal Scale: 1" = 40'
Vertical Scale: 1" = 10'

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaRCM@mjinc.com

PROJECT MILESTONE
PERMIT SKETCHES

NO	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

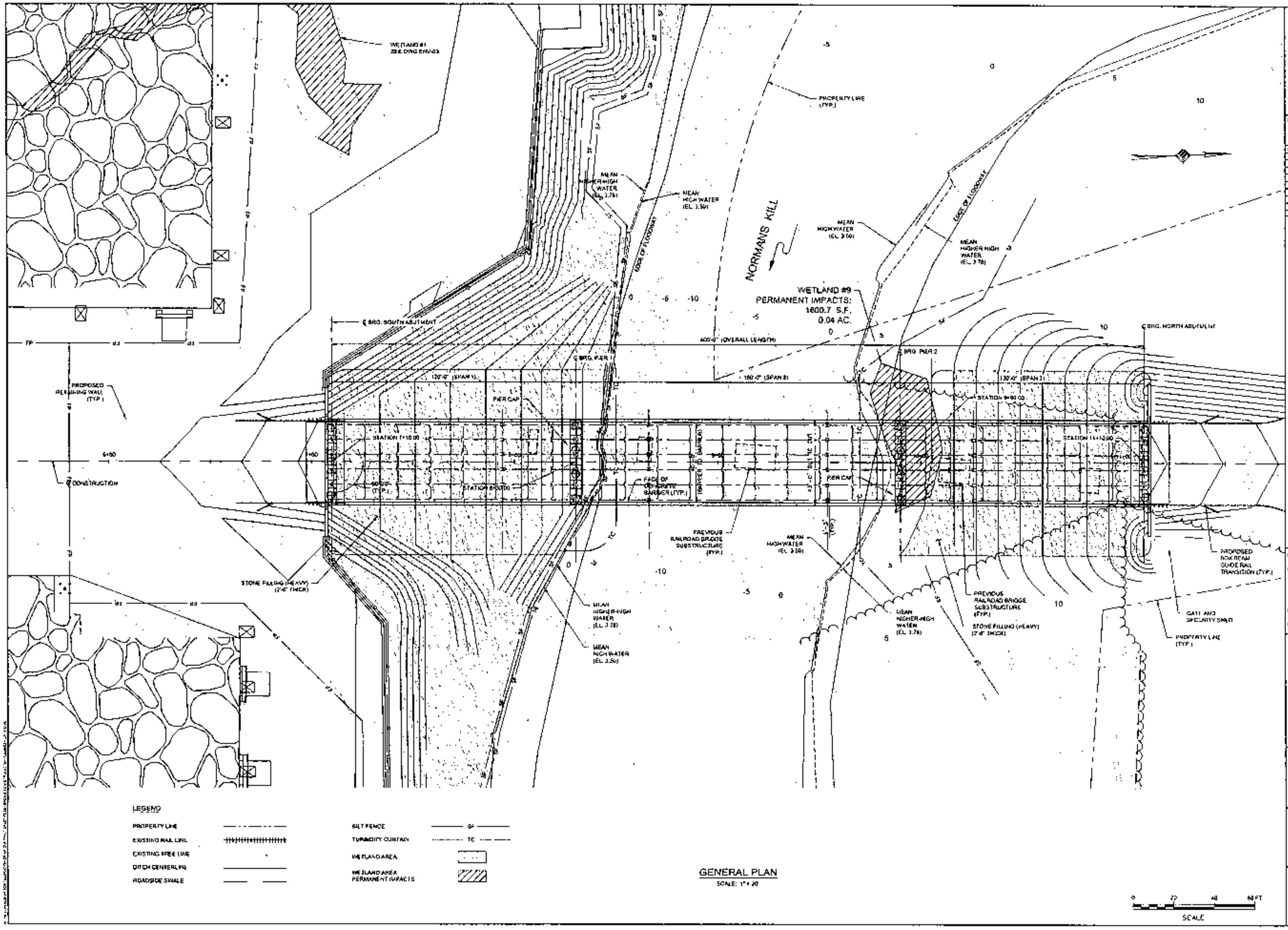
PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	KS
DESIGNED	ASD
CHECKED	AJ
SCALE	1" = 40'
DATE	MAY 2022
PROJECT	1664130

DRAWING TITLE
SECTION VIEWS

DRAWING NUMBER
XS-02

---- OF 18



McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P 518-580-9380 F 518-580 9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
PERMIT SKETCHES

NO.	DATE	DESCRIPTION

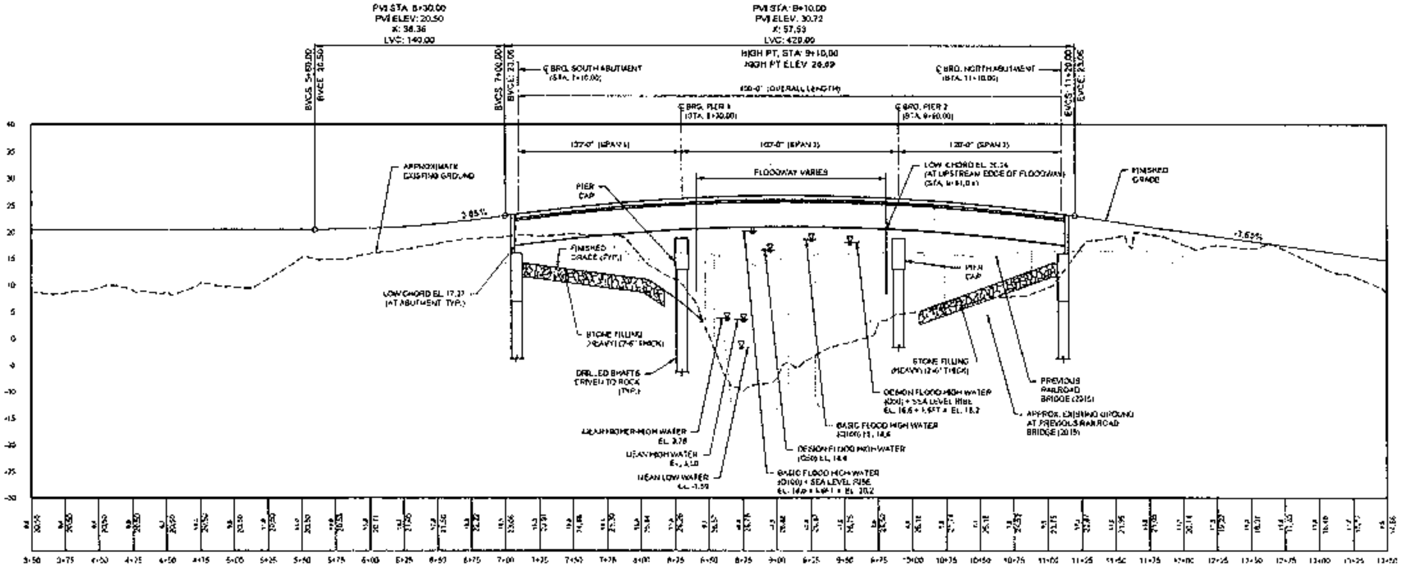
CLIENT
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT
PORT OF ALBANY SITE INFRASTRUCTURE
IMPROVEMENTS

DRAWN	SLM
DESIGNED	JLR
CHECKED	KJJ
SCALE	1" = 20'
DATE	MAY 2022
PROJECT	18481.00

DRAWING TITLE
BRIDGE
GENERAL PLAN

DRAWING NUMBER
ST-01



BRIDGE PROFILE

HORIZONTAL SCALE: 1" = 40'
VERTICAL SCALE: 1" = 10'

NOTE:

1. FINAL BRIDGE GRADING AND LIMITS AND TYPE OF SCOUR PROTECTION AT ABUTMENTS AND PIERS TO BE DEVELOPED DURING FINAL DESIGN

McFarland Johnson
651 HOLIDAY DRIVE
PLAZA 5 SUITE 400
PITTSBURGH, PENNSYLVANIA 15220
P. 412-205-1200
PittsburghROM@mjinc.com

PROJECT MILESTONE:
PERMIT SKETCHES

NO.	DATE	DESCRIPTION

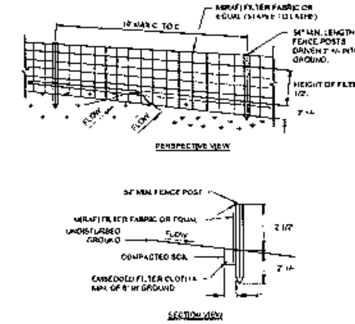
CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY SITE INFRASTRUCTURE IMPROVEMENTS

DRAWN	S.M.
DESIGNED	J.R.
CHECKED	R.L.
SCALE	Varies
DATE	JULY 2021
PROJECT	19461.00

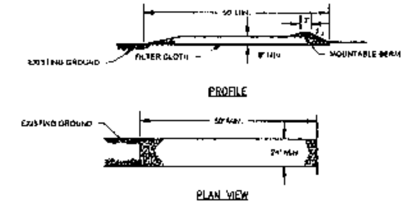
DRAWING TITLE:
BRIDGE PROFILE

DRAWING NUMBER:
ST-03



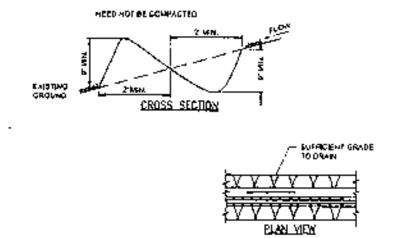
NOTES:
1. MIRAFI FILTER FABRIC TO BE SECURED TO FENCE POSTS WITH STAPLES. POSTS SHALL BE STEEL EITHER "T" OR "U" TYPE OR HARDWOOD.
2. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER THEY SHALL BE OVERLAPPED BY SIX INCHES AND FOLDED.
3. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN "BULGES" DEVELOP IN THE SILT FENCE.

SILT FENCE



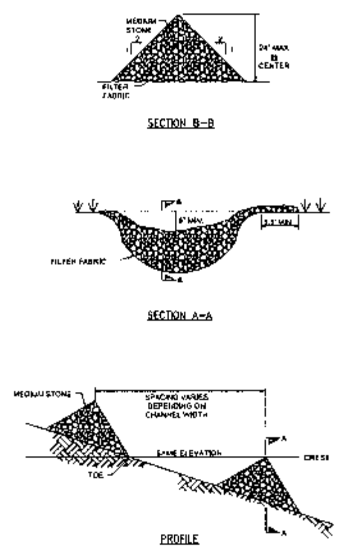
NOTES:
1. STONE SIZE - USE #3 CRUSHED STONE OR GRAVEL (PER NYSDOT SECTION 203).
2. LENGTH - NOT LESS THAN 50 FEET.
3. THICKNESS - NOT LESS THAN SIX (6) INCHES.
4. WIDTH - TWENTY-FOUR (24) FOOT IF SINGLE ENTRANCE TO SITE.
5. FILTER CLOTH - WILL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING OF STONE.
6. SURFACE WATER - ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED ACROSS THE ENTRANCE. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM WITH 5:1 SLOPES WILL BE PERMITTED.
7. MAINTENANCE - THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED IMMEDIATELY.
8. WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE AND WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE.
9. PERIODIC INSPECTION AND NEEDED MAINTENANCE SHALL BE PROVIDED AFTER EACH RAIN

STABILIZED CONSTRUCTION ENTRANCE



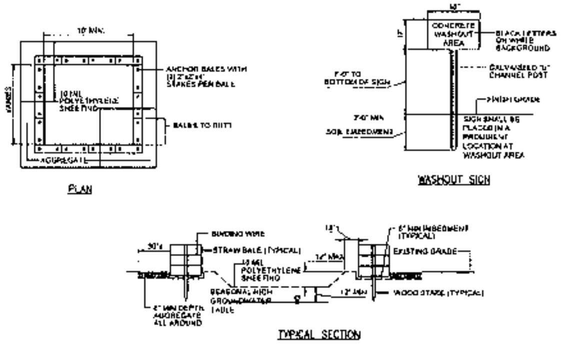
NOTES:
1. ALL PERIMETER DIKE/SWALE SHALL HAVE UNINTERRUPTED POSITIVE GRADE TO AN OUTLET
2. DIVERTED RUNOFF FROM A DISTURBED AREA SHALL BE CONVEYED TO A SEDIMENT TRAPPING DEVICE
3. DIVERTED RUNOFF FROM AN UNDISTURBED AREA SHALL OUTLET INTO AN UNDISTURBED STABILIZED AREA AT NON-EROSION VELOCITY.
4. THE SWALE SHALL BE EXCAVATED OR SHAPED TO LINE GRADE, AND CROSS SECTION AS REQUIRED TO MEET THE CRITERIA SPECIFIED IN THE STANDARD.
5. STABILIZATION OF THE AREA DISTURBED BY THE DIKE AND SWALE SHALL BE DONE IN ACCORDANCE WITH THE STANDARD AND SPECIFICATIONS FOR TEMPORARY SEEDING AND MULCHING, AND SHALL BE DONE WITHIN 10 DAYS.
6. PERIODIC INSPECTION AND REQUIRED MAINTENANCE MUST BE PROVIDED AFTER EACH RAIN EVENT.

PERIMETER DIKE



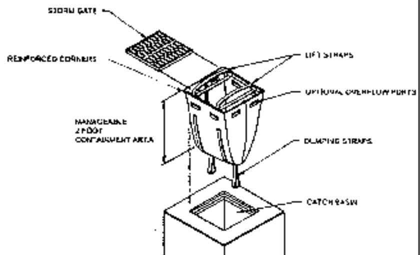
CONSTRUCTION SPECIFICATIONS
1. STONE WILL BE PLACED ON A FILTER FABRIC FOUNDATION TO THE LINES GRADES AND LOCATIONS SHOWN ON THE PLAN.
2. SET SPACING OF CHECK DAMS TO ASSURE THAT THE ELEVATIONS OF THE CREST OF THE DOWNSTREAM DAM IS AT THE SAME ELEVATION OF THE TOE OF THE UPSTREAM DAM.
3. EXTEND THE STONE A MINIMUM OF 1.5' BEYOND THE DITCH BANKS TO PREVENT CUTTING AROUND THE DAM.
4. PROTECT THE CHANNEL DOWNSTREAM OF THE LOWEST CHECK DAM FROM SCOUR AND EROSION WITH STONE OR LINER AS APPROPRIATE.
5. ENSURE THAT CHANNEL APPURTENANCES SUCH AS CULVERT ENTRANCES BELOW CHECK DAMS ARE NOT SUBJECT TO DAMAGE OR BLOCKAGE FROM DISPLACED STONES.

LIGHT STONE CHECK DAM



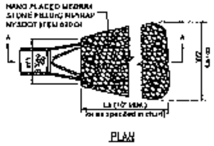
NOTES:
1. CONTAINMENT MUST BE STRUCTURALLY SOUND AND LEAK FREE AND CONTAIN ALL LIQUID WASTES.
2. CONTAINMENT DEVICES MUST BE OF SUFFICIENT QUANTITY OR VOLUME TO COMPLETELY CONTAIN THE LIQUID WASTES GENERATED.
3. WASHOUT MUST BE CLEANED OR NEW FACILITIES CONSTRUCTED AND READY TO USE ONCE WASHOUT IS 75% FULL.
4. WASHOUT AREA(S) SHALL BE INSTALLED IN A LOCATION EASILY ACCESSIBLE BY CONCRETE TRUCKS.
5. ONE OR MORE AREAS MAY BE INSTALLED ON THE CONSTRUCTION SITE AND MAY BE RELOCATED AS CONSTRUCTION PROGRESSES.
6. AT LEAST WEEKLY REMOVE ACCUMULATION OF SAND AND AGGREGATE AND DISPOSE OF PROPERLY.

CONCRETE WASHOUT



NOTES:
1. CONTENTS SHALL BE REMOVED PRIOR TO 60% CAPACITY & DISPOSED OF AS REQUIRED.

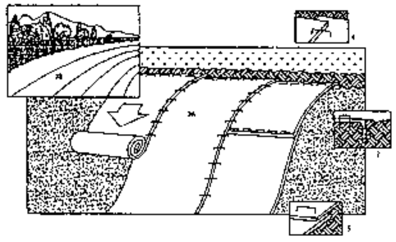
INLET PROTECTION



Do = Pipe Dia. (see plans)
La = Apron Length
W = Apron Width (centered on pipe)
d = Apron Depth

NOTES
1. d = 1.5 TIMES THE MAXIMUM STONE DIAMETER BUT NO LESS THAN 6".
2. INSTALL FILTER MIRAFI 600X OR APPROVED EQUAL FILTER FABRIC BETWEEN RIP-RAP AND SUBGRADE.

OUTLET PROTECTION - RIP RAP



INSTALLATION OF GEOSYNTHETICS EROSION CONTROL MATS
1. EROSION CONTROL MATS SHALL BE UTILIZED ON ALL SLOPES OF 3:1 OR GREATER REQUIRING PERMANENT STABILIZATION.
2. SLOPES SHALL BE PREPARED AND EROSION CONTROL MATS SHALL BE INSTALLED ANCHORED AND SOIL FILLED IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS AND INSTALLATION PROCEDURES.
3. SEEDING SHALL BE COMPLETED IN ACCORDANCE WITH THE PERMANENT SEEDING SPECIFICATIONS ABOVE PRIOR TO PLACEMENT OF THE EROSION CONTROL MAT. A SECOND APPLICATION OF PERMANENT SEEDING SHALL BE APPLIED AFTER THE MAT IS IN PLACE, PRIOR TO SOIL FILLING THE MAT.
4. CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTENANCE, RESEEDING AND REPAIR OF MATS AND SLOPES UNTIL PERMANENT STABILIZATION IS ACHIEVED.

NOTES
REFER TO GENERAL STAPLE PATTERN GUIDE FOR CORRECT STAPLE PATTERN RECOMMENDATIONS FOR SLOPE INSTALLATIONS.
1. PREPARE SOIL BEFORE INSTALLING BLANKETS, INCLUDING APPLICATION OF LIME, FERTILIZER AND SEED. NOTE: WHEN USING CELL-O-SEED DO NOT SEED PREPARED AREA, CELL-O-SEED MUST BE INSTALLED WITH PAPER SIDE DOWN.
2. BEGIN AT THE TOP OF THE SLOPE BY ANCHORING THE BLANKET IN 6" DEEP X 6" WIDE TRENCH, BACKFILL AND COMPACT THE TRENCH AFTER STAPLING.
3. ROLL THE BLANKETS (A) DOWN OR (B) HORIZONTALLY ACROSS THE SLOPE.
4. THE EDGES OF PARALLEL BLANKETS MUST BE STAPLED WITH APPROXIMATELY 2" OVERLAP.
5. WHEN BLANKETS MUST BE SPLICED DOWN THE SLOPE, PLACE BLANKETS END OVER END (SHINGLE STYLE) WITH APPROXIMATELY 4" OVERLAP, STAPLE THROUGH OVERLAPPED AREA, APPROXIMATELY 12" APART.
6. MANUFACTURER'S INSTALLATION INSTRUCTIONS SHALL SUPERCEDE THIS DETAIL.

EROSION CONTROL MAT INSTALLATION

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P 518-580-9380 F 518-580-9383
SaratogaRCM@mjinc.com

PROJECT MILESTONE
PERMIT SKETCHES

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

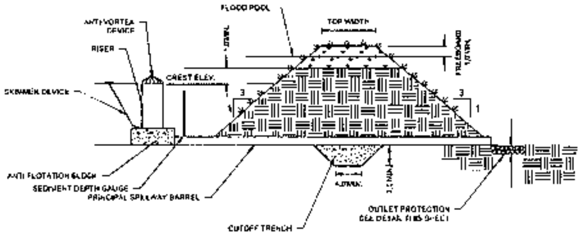
PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN JES
DESIGNED ASD
CHECKED AJW
SCALE 1"=42
DATE JULY 2021
PROJECT 18845.00

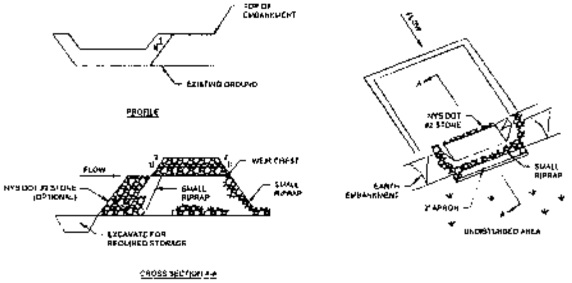
IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
EROSION AND SEDIMENT CONTROL DETAILS

DRAWING NUMBER
ESC-06
17 OF 18



SEDIMENT BASIN

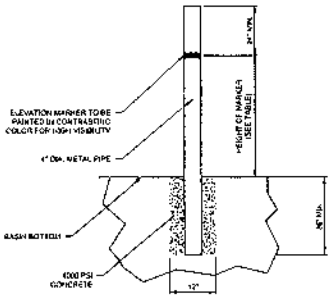


OPTION: A ONE FOOT LAYER OF NYS DOT #2 STONE MAY BE PLACED ON THE UPSTREAM SIDE OF THE RIPRAP IN PLACE OF THE EMBEDDED FILTER CLOTH.

NOTES:

1. AREA UNDER EMBANKMENT SHALL BE CLEARED, GRUBBED AND STRIPPED OF ANY VEGETATION AND ROOT MAT. THE POOL AREA SHALL BE CLEARED.
2. THE FILL MATERIAL FOR THE EMBANKMENT SHALL BE FREE OF ROOTS AND OTHER WOODY VEGETATION AS WELL AS OVER-SIZED STONES, ROCKS, ORGANIC MATERIAL OR OTHER OBJECTIONABLE MATERIAL. THE EMBANKMENT SHALL BE COMPACTED BY TRAVERSING WITH EQUIPMENT WHILE IT IS BEING CONSTRUCTED.
3. ALL CUT AND FILL SLOPES SHALL BE 2:1 OR FLATTER.
4. THE STONE USED IN THE OUTLET SHALL BE SMALL RIPRAP 4"-6" ALONG WITH A 1' THICKNESS OF 2" AGGREGATE PLACED ON THE UPGRADE SIDE ON THE SMALL RIPRAP OR EMBEDDED FILTER CLOTH IN THE RIPRAP.
5. SEDIMENT SHALL BE REMOVED AND TRAP RESTORED TO ITS ORIGINAL DIMENSIONS WHEN THE SEDIMENT HAS ACCUMULATED TO ½ THE DESIGN DEPTH OF THE TRAP. IT SHALL BE PLACED ON SITE AND STABILIZED.
6. THE STRUCTURE SHALL BE INSPECTED AFTER EACH RAIN AND REPAIRS MADE AS NEEDED.
7. CONSTRUCTION OPERATIONS SHALL BE CARRIED OUT IN SUCH A MANNER THAT EROSION AND SEDIMENT ARE CONTROLLED.
8. THE STRUCTURE SHALL BE REMOVED AND THE AREA STABILIZED WHEN THE DRAINAGE AREA HAS BEEN PROPERLY STABILIZED.
9. THE MAXIMUM DRAINAGE AREA IS 5 ACRES.

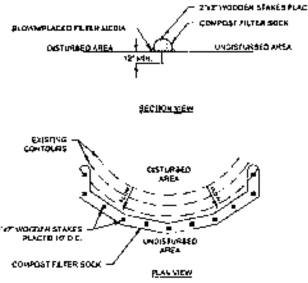
STONE OUTLET SEDIMENT TRAP



NOTES:

1. CONTRACTOR SHALL PLACE A 4" DIA. METAL PIPE AT A MINIMUM OF 36" BELOW THE BASIN BOTTOM ELEVATION ENCASED IN 12" OF 4000 PSI CONCRETE.
2. CONTRACTOR SHALL MARK THE CORRESPONDING ELEVATION FOR SEDIMENT STORAGE CLEANOUT ON THE PVC PIPE TO INDICATE WHEN MAINTENANCE IS REQUIRED.
3. SEDIMENT DEPTH GAUGE REQUIRED FOR SEDIMENT BASINS, FOREBAYS, AND INFILTRATION BASINS.

SEDIMENT DEPTH GAUGE



NOTES:

1. SOCK FABRIC SHALL MEET STANDARDS OF TABLE 5.1 OF NYS STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL. COMPOST SHALL MEET THE STANDARDS LISTED ON TABLE 5.2 OF NYS STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL.
2. COMPOST FILTER SOCK SHALL BE PLACED AT EXISTING LEVEL GRADE. BOTH ENDS OF THE SOCK SHALL BE EXTENDED AT LEAST 8 FEET UP SLOPE AT 45° TO THE MAIN SOCK ALIGNMENT. MAXIMUM SLOPE LENGTH ABOVE ANY SOCK SHALL NOT EXCEED THAT SHOWN ON FIGURE 5.3 OF NYS STANDARDS AND SPECIFICATIONS FOR EROSION AND SEDIMENT CONTROL. STAKES MAY BE INSTALLED IMMEDIATELY DOWNSLOPE OF THE SOCK IF SO SPECIFIED BY THE MANUFACTURER.
3. TRAFFIC SHALL NOT BE PERMITTED TO CROSS FILTER SOCKS.
4. ACCUMULATED SEDIMENT SHALL BE REMOVED WHEN IT REACHES HALF THE ABOVEGROUND HEIGHT OF THE SOCK AND DISPOSED IN THE MANNER DESCRIBED ELSEWHERE IN THE PLAN.
5. SOCKS SHALL BE INSPECTED WEEKLY AND AFTER EACH RUNOFF EVENT. DAMAGED SOCKS SHALL BE REPAIRED ACCORDING TO MANUFACTURER'S SPECIFICATIONS OR REPLACED WITHIN 24 HOURS OF INSPECTION.
6. BIODEGRADABLE FILTER SOCKS SHALL BE REPLACED AFTER 6 MONTHS, PHOTODEGRADABLE SOCKS AFTER 1 YEAR. POLYPROPYLENE SOCKS SHALL BE REPLACED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS.
7. UPON STABILIZATION OF THE AREA TRIBUTARY TO THE SOCKS, STAKES SHALL BE REMOVED. THE SOCK MAY BE LEFT IN PLACE AND VEGETATED OR REMOVED. IN THE LATTER CASE, THE MESH SHALL BE CUT OPEN AND THE MULCH SPREAD AS A SOIL SUPPLEMENT.

COMPOST FILTER SOCK

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P 518-580-9380 F 518-580-9383
SaratogaRCM@mjinc.com

PROJECT MILESTONE
PERMIT SKETCHES

NO.	DATE	DESCRIPTION

CLIENT
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

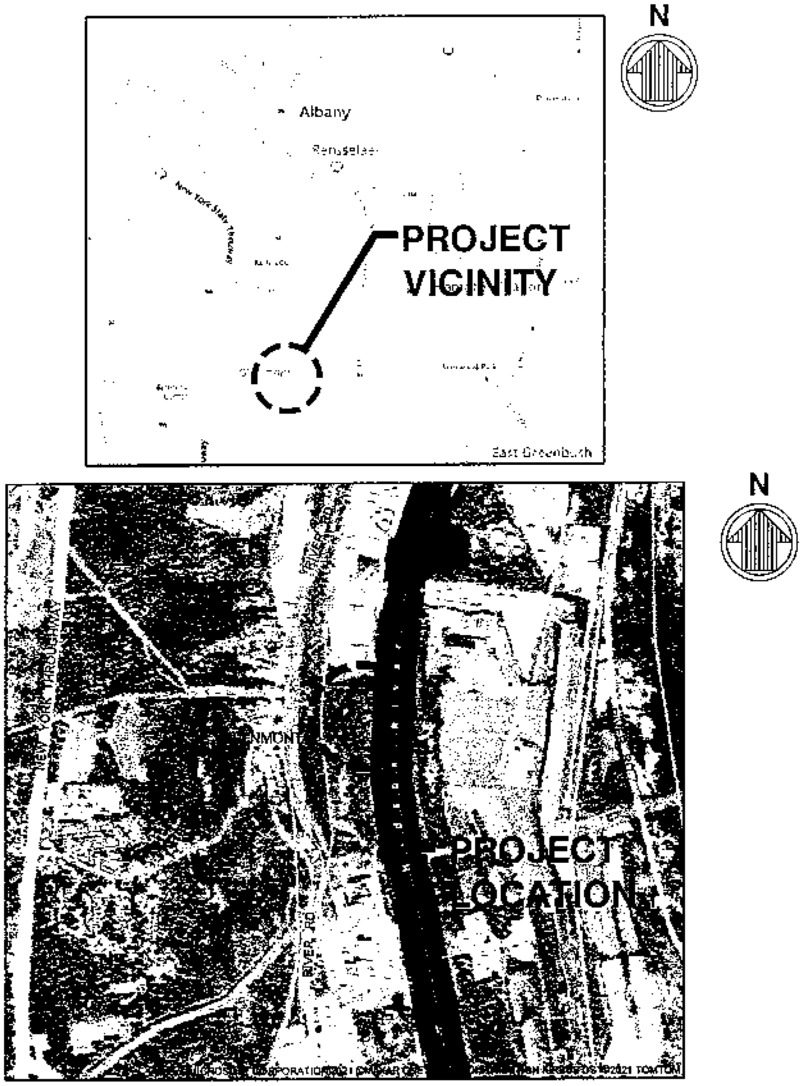
PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN	JLS
DESIGNED	MSO
CHECKED	AJF
SCALE	1"=40'
DATE	JULY 2021
PROJECT	18841.00

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
EROSION AND SEDIMENT CONTROL DETAILS

DRAWING NUMBER
ESC-07
16 OF 18



VICINITY AND LOCATION MAP

SCALE: N.T.S.

NOTES:

1. HORIZONTAL CONTROL REFERENCED TO NORTH AMERICAN DATUM OF 1983, STATE PLANE COORDINATE SYSTEM, NEW YORK, EAST ZONE, IN FEET.

2. WATER LEVEL DATUM IS BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929, AS FOLLOWS:

- MEAN HIGHER HIGH WATER LEVEL (MHHW) +4.56 FT (NGVD29)
- MEAN HIGH WATER LEVEL (MHW) = +4.18 FT (NGVD29)
- MEAN TIDE LEVEL (MTL) = +1.69 FT (NGVD29)
- MEAN LOW WATER LEVEL (MLW) = -0.80 FT (NGVD29)

PURPOSE: WHARF CONSTRUCTION
PERMIT SUBMITTAL-NOT TO BE USED
FOR CONSTRUCTION
DATUM: NGVD29

m&n engineering, p.c.

OWNER/APPLICANT:
ALBANY PORT DISTRICT COMMISSION
PORT OF ALBANY

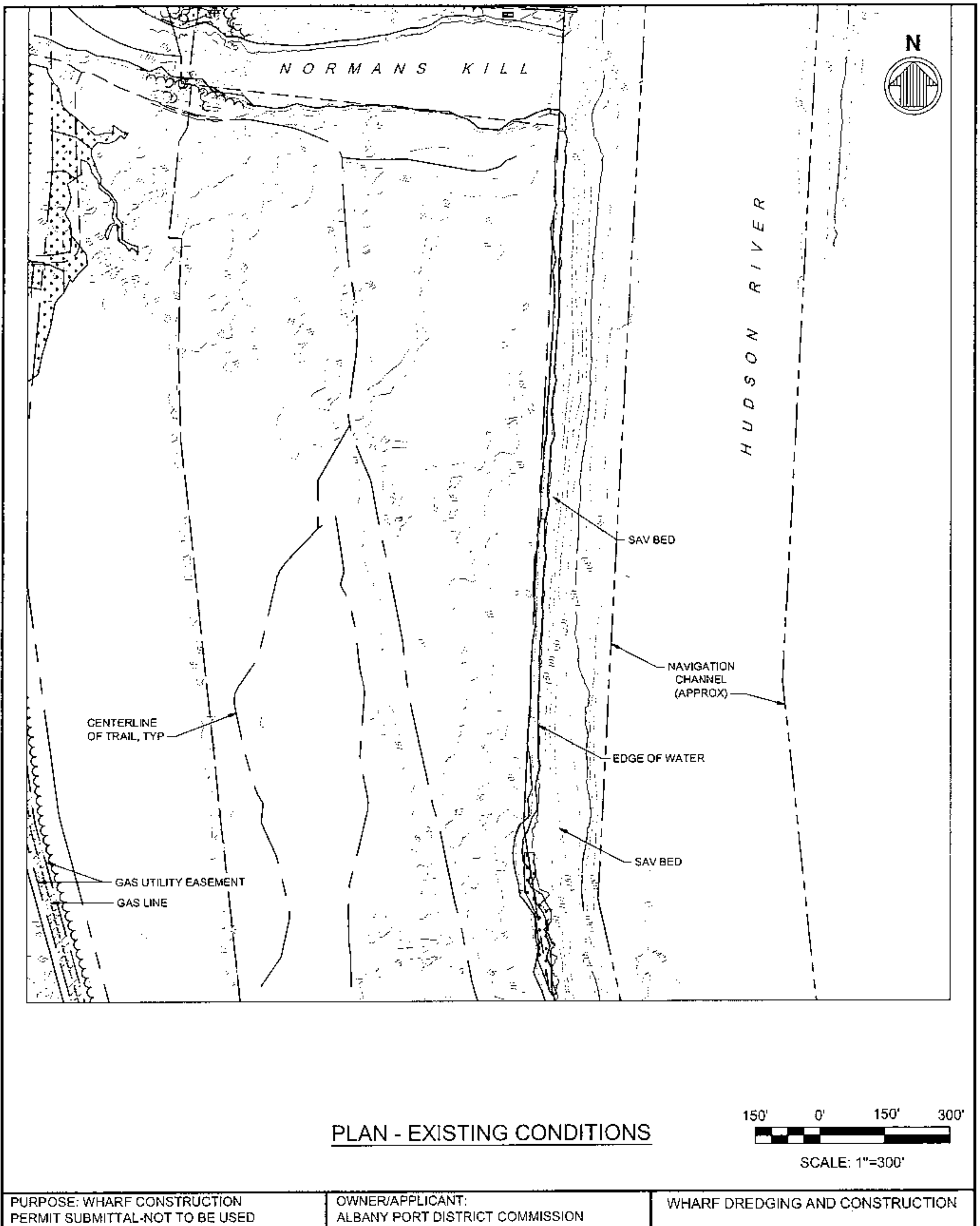
IN: HUDSON RIVER
NEAR: SOUTH OF ALBANY
LOCATION: PORT OF ALBANY
106 SMITH BOULEVARD
ALBANY, NEW YORK 12202

WHARF DREDGING AND CONSTRUCTION

VICINITY AND LOCATION

DATE: 2021-07-15

File: Q:\NY\10949-01\20 CADD_Active_Permits\1094901P-01 ; Plotted: 7/15/2021 8:44 AM by WILKINSON, MELISSA ; Saved: 7/14/2021 3:06 PM by MWILKINSON



PLAN - EXISTING CONDITIONS

PURPOSE: WHARF CONSTRUCTION
PERMIT SUBMITTAL-NOT TO BE USED
FOR CONSTRUCTION
DATUM: NGVD29

m&n engineering, p.c.

OWNER/APPLICANT:
ALBANY PORT DISTRICT COMMISSION
PORT OF ALBANY

IN:	HUDSON RIVER
NEAR:	SOUTH OF ALBANY
LOCATION:	PORT OF ALBANY 106 SMITH BOULEVARD ALBANY, NEW YORK 12202

WHARF DREDGING AND CONSTRUCTION

PLAN - EXISTING CONDITIONS

DATE: 2021-07-15

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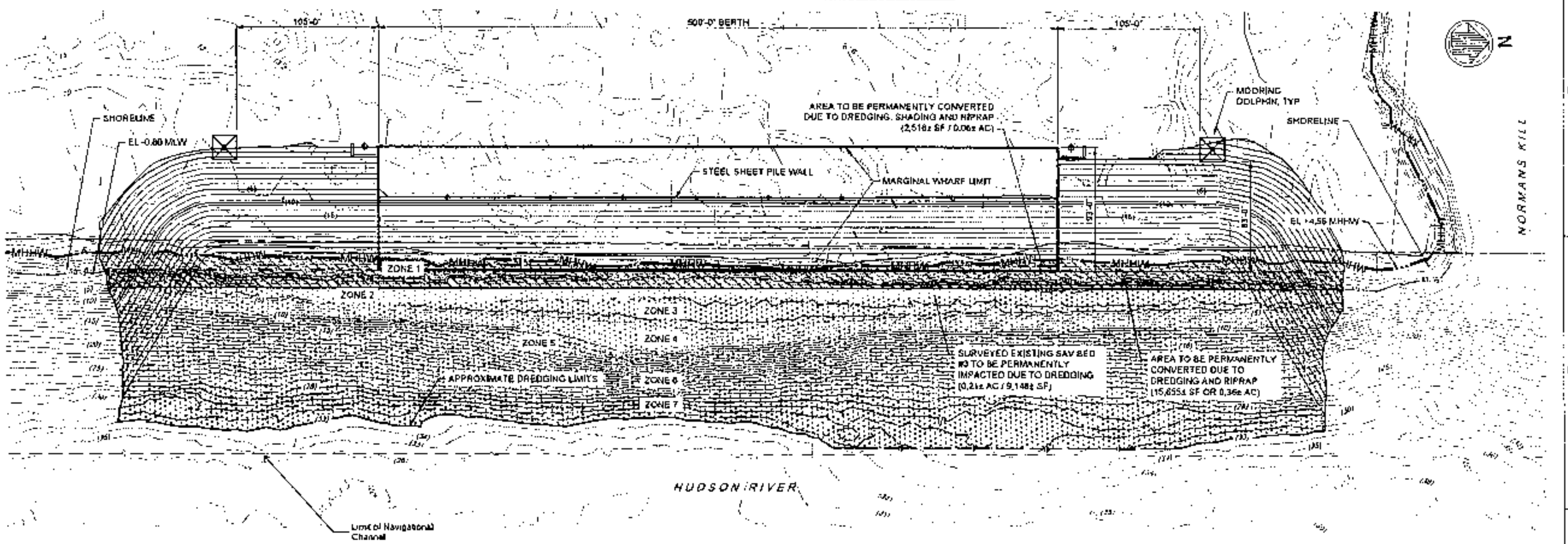
Proposed Wharf and Dredging - Impact Analysis



A1 / S-101 **PLAN - GENERAL ARRANGEMENT**
SCALE: 1" = 40'

ZONES		EXISTING ELEVATIONS (FEET)	HABITAT OF CONCERN	ZONES (ACRES)	MAIN IMPACTS / HABITAT CONVERSION (ACRES)		
					DREDGING	SHADING	RIPRAP
1	SEE LEGEND	MHHW TO 0	INTERTIDAL ZONE AND SHORE STRUCTURES	0.25	0.25	0.06	0.25
2	☐	-2 to -5	SAV BED #3 (SPARSE LOW DENSITY)	0.21	0.21		0.05
3	☐	0 TO -5	NATURAL RIVER BOTTOM (SILT CLAY, SAND AND SOME TRACE OF GRAVEL)	0.34	0.34		0.12
4	☐	-5 TO -10	NATURAL RIVER BOTTOM (SILT CLAY, SAND AND SOME TRACE OF GRAVEL)	0.24	0.24		
5	☐	-10 TO -15	NATURAL RIVER BOTTOM (SILT CLAY, SAND AND SOME TRACE OF GRAVEL)	0.24	0.24		
6	☐	-15 TO -28	NATURAL RIVER BOTTOM (SILT CLAY, SAND AND SOME TRACE OF GRAVEL)	0.78	0.79		
7	☐	-28 TO -33	NATURAL RIVER BOTTOM (SILT CLAY, SAND AND SOME TRACE OF GRAVEL)	0.56	0.55		
				TOTAL (ACRES):	2.82	0.06	0.42

LEGEND

—— (22) ——	EXISTING CONTOURS
—— (33) ——	PROPOSED CONTOURS
—— MHHW ——	EXISTING MEAN HIGHER HIGH WATER
☐	PROPOSED DREDGING
▨	DREDGING, SHADING AND RIPRAP IMPACTS
▨	DREDGING AND RIPRAP IMPACTS

NOTES

1. ZONE 1 INCLUDES SHADED AND RIPRAP AREAS.
2. ZONES 2-7 INCLUDES 2' ALLOWABLE OVERDREDGE.
3. ELEVATION DATUM: NAVD88.

40' 0 40' 80'
SCALE: 1"=40'

IT IS A VIOLATION OF THE NEW YORK STATE EDUCATION LAW FOR ANY PERSON, UNLESS HE IS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER AN ITEM ON THIS DOCUMENT IN ANY WAY. IF AN ITEM IS ALTERED, THE ALTERING ENGINEER SHALL AFFIX TO THE ITEM HIS SEAL AND THE NOTATION "ALTERED BY" FOLLOWED BY HIS SIGNATURE AND THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF SUCH ALTERATION.

MARINE FACILITIES ON THE PORT OF
ALBANY EXPANSION

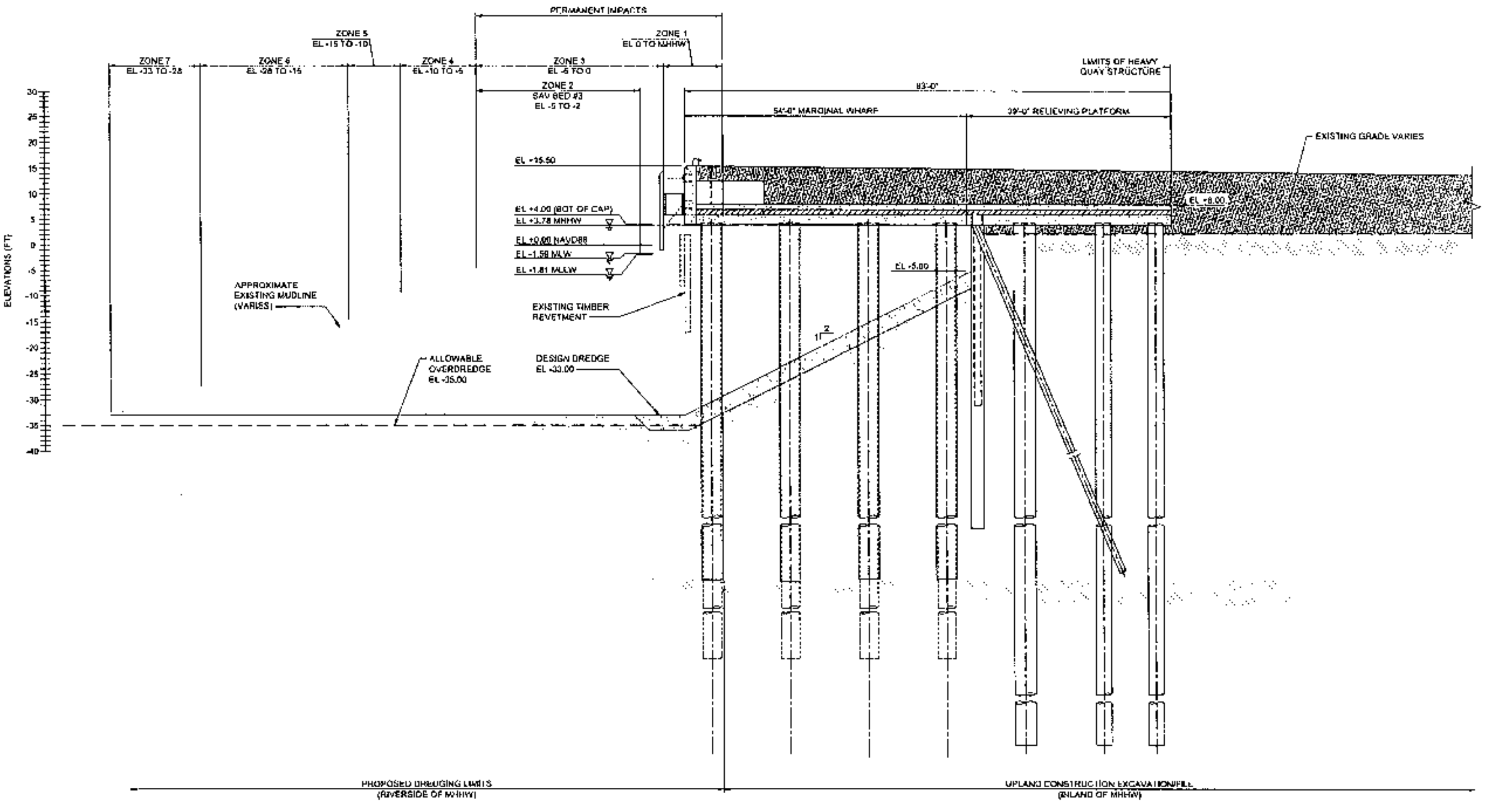
PLAN - GENERAL
ARRANGEMENT

man engineering, p.c.

ALBANY PORT
DISTRICT COMMISSION
ALBANY, NY 12202

Sheet
Reference No.
S-101

Proposed Wharf and Dredging - Impact Analysis



A1
S-501

SECTION - TYPICAL
SCALE: 1"=10'

NOTES:

1. FOR SHAFT AND ROCK SOCKET DETAILS, SEE SHEET S-501.
2. FOR BENT CAP DETAILS, SEE SHEETS S-506 THRU S-522.
3. FOR PC PANEL DETAIL, SEE SHEET S-524A.
4. ELEVATION DATUM: NAVD88.
5. SECTION TAKEN APPROXIMATE MID-POINT OF WHARF

10'-0" 0'-0" 10'-0" 20'-0"
SCALE: 1"=10'

IT IS A VIOLATION OF THE NEW YORK STATE EDUCATION LAW FOR ANY PERSON, UNLESS HE IS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, TO ALTER AN ITEM ON THIS DOCUMENT IN ANY WAY. IF AN ITEM IS ALTERED, THE ALTERING ENGINEER SHALL AFFIX TO THE ITEM HIS SEAL AND THE NOTATION "ALTERED BY" FOLLOWED BY HIS SIGNATURE AND THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF SUCH ALTERATION.

PORT OF ALBANY EXPANSION
HEAVY LIFT QUAY/WHARF

SECTION - TYPICAL

mån engineering, p.c.

PORT of ALBANY

ALBANY PORT
DISTRICT COMMISSION

Sheet
Reference No.

INDEX 2 OF 2

Port of Albany
Site Expansion,

Prepared for:

LaBella Associates
4 British American Blvd.
Latham, New York 12110

October 23, 2022

Revision 5

Approved by NYSDOH
October 28, 2022

Prepared by:
Watson & Associates, Occupational Hygiene and Safety, LLC
PO Box 31, Greenville, New York 12083
Project Number: 990214-002

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Appendices

Appendix A Proposed PM 10.0 Air Monitoring Locations

Appendix B
Appendix C Port of Albany Site Expansion Agency Contact List

Project Description

The Beacon Island site is located to the east of River Road (County Route 144) and along the west side of the Hudson River, in the Town of Bethlehem, Albany County, New York. The Beacon Island parcel consists of approximately eighty acres and is the site of a planned expansion for the Port of Albany. The site is to be developed for wind turbine manufacturing. Portions of the site were previously used as a fly ash landfill.

Scope

A Community Air Monitoring Plan (CAMP) is required to be implemented during excavation work for the Port of Albany Site Expansion. Various contractors will be performing ground intrusive activities to support the expansion infrastructure. This CAMP will apply **to all ground intrusive activities onsite**. The CAMP can be terminated once placement of two feet (2') of clean fill is completed in excavated areas.

This CAMP has been prepared in accordance with New York State Department of Environmental Conservation (NYSDEC) DER-10, TECHNICAL GUIDANCE FOR SITE INVESTIGATION AND REMEDIATION, dated May 2010 (DER-10).

DER-10 requires real-time monitoring for volatile organic compounds (VOCs) and/or particulates (i.e., dust) at the downwind perimeter of each designated work area when certain activities are in progress at contaminated sites. The CAMP is not intended for use in establishing action levels for worker respiratory protection. This will be the responsibility of each contractor. The intent of the CAMP is to provide a measure of protection for the downwind community, including residences and businesses and on-site workers not directly involved with the subject work activities. The action levels specified herein require air monitoring, corrective actions to abate emissions, and/or work shutdown. Additionally, the CAMP helps to confirm that work activities did not spread contamination off-site.

Please note that reliance on air monitoring will not preclude simple, common-sense measures to keep dust and odors at a minimum around the work areas.

Community Air Monitoring Plan

Fly ash is the primary contaminant of concern. Continuous real time monitoring will be required for all ground intrusive activities and handling of soils.

Activities that are anticipated or known to include the disturbance of fly ash include:

- Cut and fill for the foundations (please see building site map Aggregate Grading Plan, provided in Appendix B);
- Areas to be developed with asphalt/concrete surfaces;
- Areas to be developed with lawn/landscaping.

Per the Atlantic Soil Management Plan, “Within 14 days, and no less than 3 days, prior to commencing work activities…” the NYSDEC shall be notified of the planned work. This notification should be performed by the Owner and/or Contractor performing the site work. The Design Professional and Environmental Consultant must also be similarly notified.” A 14-day notification will be sufficient time to obtain and ship all required air monitoring equipment.

Particulate Monitoring – PM 10.0

Particulate concentrations will be monitored continuously during intrusive work at four (4) perimeter locations. Locations will be identified by a description of the location and compass heading. These locations will be fixed at the north, south, east, and west perimeter locations of the site. Air monitoring locations will be placed outside the active work boundaries. Proposed air monitoring locations are identified on the site map located in Appendix A. Location 1 will be to the North, Location 2 will be to the East, location 3 will be to the south and location 4 will be to the west.

Wind direction will be identified by use of a metrological station. Wind direction will be noted daily at the beginning of the shift and monitored every two hours. Wind direction will be reported by the direction from which it originates. For example, a north or northerly wind will indicate that the wind blows from the north to the south. It is anticipated that wind direction will shift and, at times, constantly. In the event of an alarm condition, the upwind monitor will be the monitor identified to be the unit upwind of the monitor with the current alarm condition. The appropriate response actions will then be implemented.

The particulate monitoring will be performed using real-time monitoring equipment capable of measuring particulate matter less than 10 micrometers in size (PM-10) and capable of integrating over a period of 15 minutes for comparison to the airborne particulate action level. Four (4) TSI DustTrak II, Model 8530 direct reading instruments with environmental enclosures will be utilized with PM 10.0 cyclones. These units will be required to have an extra battery to ensure that data collection can be obtained over the contractor's shift. The units will be placed prior to the excavation contractor performing **any intrusive work** and will be removed after work is completed each day. One additional PM 10.0 monitor will be available on-site in case of equipment failure.

Response Levels and Actions – PM 10.0

If the downwind PM-10 particulate level is 100 micrograms per cubic meter (μg/m^3) greater than background (upwind perimeter) for the 15-minute period or if airborne dust is observed leaving the work area, then dust suppression techniques must be employed. Work may continue with dust suppression techniques provided that downwind PM-10 particulate levels do not exceed 150 μg/m^3 above the upwind level and provided that no visible dust is migrating from the work area. The work area will be defined as the perimeter bounded by the dust monitors. If particulate levels are detected in excess of 150 μg/m^3, the upwind background level must be confirmed immediately. If the working site particulate measurement is greater than 100 μg/m^3 above the background level, additional dust suppression techniques must be implemented to reduce the generation of fugitive dust and corrective action taken to reduce the potential for contaminant migration. Corrective measures may include implementing additional dust suppression techniques. Should the action level of 150 μg/m^3 continue to be exceeded work must stop and DEC and DOH must be notified the same day. The notification shall include a description of the control measures implemented to prevent further exceedances.

If, after implementation of dust suppression techniques, downwind PM-10 particulate levels are greater than 150 µg/m^3 above the upwind level, work must be stopped, and a re-evaluation of activities initiated. Work can resume provided that dust suppression measures and other controls are successful in reducing the downwind PM-10 particulate concentration to within 150 µg/m^3 of the upwind level and in preventing visible dust migration.

Particulate Monitoring – PM 2.5

PM 2.5 particulate concentrations will be monitored continuously during intrusive work at the four (4) perimeter locations identified above. The PM 2.5 monitors will be collocated with the PM 10.0 monitors.

The particulate monitoring will be performed using real-time monitoring equipment capable of measuring particulate matter less than 2.5 micrometers in size (PM-2.5) and capable of integrating over a period of 15 minutes for comparison to the airborne particulate action level. The equipment must be equipped with an audible alarm to indicate exceedance of the action level. Two (2) TSI DustTrak II, Model 8530 direct reading instruments with environmental enclosures will be utilized with PM 2.5 cyclones. The units will be placed prior to the excavation contractor performing any intrusive work and will be removed after work is completed each day. One additional monitor will be available on-site in case of equipment failure.

Please note that Watson has verified that the rental agency has supplied "conductive tubing" with all Dust Trak II units.

Response Levels and Actions – PM 2.5

A PM -2.5 action level of 12.5 micrograms per cubic meter (µg/m^3) above the upwind perimeter PM -2.5 concentration on a fifteen minute average basis will trigger dust or smoke/exhaust control/s. A higher 15-minute average action level, such as the level of the daily National Ambient Air Quality Standard of 35 µg/m^3 could considered if an increase of 12.5 µg/m^3 is demonstrated to be incompatible with responsible construction activity. The combination of operation of off- and on-road diesel equipment and fine water misting for dust control in the work zone could and can produce elevated PM -2.5 readings.

Total Volatile Organics Air Monitoring

Total Volatile organic compounds (TVOCs) will be monitored with Rae Systems Photoionization detector with a 10.6 electron volt probe. Units will be collocated with the PM 10.0 Monitors at the site perimeter.

VOC Monitoring Response Levels, and Actions

Petroleum contamination has been identified on site. Perimeter VOC monitoring will be required to be performed as follows:

Upwind concentrations will be measured at the start of each workday as noted for the PM 10.0 monitors. Wind direction changes will be monitored as noted in the PM 10.0 section of this document.

The equipment should be calibrated prior to use per the manufacturer's directions.

The equipment will be set to calculate 15-minute running average concentrations, which will be compared to the levels specified below.

1. If the ambient air concentration of total organic vapors at the downwind perimeter of the work area or exclusion zone exceeds 5 parts per million (ppm) above background for the 15-minute average, work activities must be temporarily halted and monitoring continued. If the total organic vapor level readily decreases (per instantaneous readings) below 5 ppm over background, work activities can resume with continued monitoring.
2. If total organic vapor levels at the downwind perimeter of the work area or exclusion zone persist at levels in excess of 5 ppm over background but less than 25 ppm, work activities must be halted, the source of vapors identified, corrective actions

taken to abate emissions, and monitoring continued. After these steps, work activities can resume provided that the total organic vapor level 200 feet downwind of the exclusion zone or half the distance to the nearest potential receptor or residential/commercial structure, whichever is less - but in no case less than 20 feet, is below 5 ppm over background for the 15-minute average.

3. If the organic vapor level is above 25 ppm at the perimeter of the work area, activities must be shutdown.

4. All 15-minute readings must be recorded and be available for State (DEC and NYSDOH) personnel to review. Instantaneous readings, if any, used for decision purposes should also be recorded. Notifications of PID readings will be the same as for the dust monitoring.

Periodic Monitoring for Total VOCs

Periodic monitoring for VOCs will be required during non-intrusive activities such as the collection of soil and sediment samples or the collection of groundwater samples from existing monitoring wells. Periodic monitoring during sample collection will consist of taking a reading upon arrival at a sample location, monitoring while opening a well cap or overturning soil, monitoring during well baling/purging, and taking a reading prior to leaving a sample location.

Periodic monitoring will be performed during the initiation of all excavations to determine the likelihood of potential petroleum contamination. Visual clues, odor and PID readings will all be utilized to determine if the area is potentially contaminated. In the event of the discovery of a potential historical petroleum release, all work will cease, and LaBella Associates must be notified immediately.

LaBella Associates will maintain one (1) photoionizing detector (PID) with a minimum lamp energy of a minimum of 10.6 electron volts throughout the project in order to respond to any discovered petroleum contamination. This unit will be used as handheld survey instrument and will not be placed in the environmental enclosures and will be used for the periodic monitoring.

Documentation Requirements

The air monitoring technician will maintain an electronic daily log, documenting the location of each unit by serial number, and the upwind and downwind locations. At the end of each shift the technician will provide a summary report to LaBella Associates. The summary report will contain:

- any exceedances of action levels;
- any visual dust by location, date, and time;
- the name of the excavation contractor employee who was notified;
- the corrective actions taken by the excavation contractor;
- job or work task that generated the dust;
- location on sit; and
- 15-minute averages.

The report will also include overall average, maximum concentrations, time/date of maximum, total number of logged points, start time/date, total elapsed time (run duration), STEL concentration and time/date of occurrence. All daily logs and data will be stored by date and transmitted to LaBella Associates electronically via email.

There may be situations when dust is being generated and leaving the site and the monitoring equipment does not measure PM-10 at or above the action level. Since this situation has the potential to allow for the migration of contaminants off-site, it is unacceptable. The excavation contractor will be required to implement additional dust control measures if visual dust is observed.

Site Communications and Notification Requirements

The following site communications and notification requirements will be implemented on the project:

The air monitoring technician performing the CAMP monitoring shall notify the excavating contractor of a visual dust or alarm condition. This notification will be verbally or by cell phone.

The air monitoring technician shall notify the LaBella site contact via email with a daily summary as noted in the Document Requirements section.

The air monitoring technician will notify LaBella for all off-site dust excursions by phone immediately upon notification by the excavating contractor, review of an alarm condition, or visual observation.

The excavating contractor will notify the air monitoring technician of any dust excursion, whether the dust excursion was an offsite event or not, in the event that the air monitoring technician is not aware of the event. This notification will be by cell phone.

A weekly report will be generated that outlines work conducted, CAMP data, any exceedances, corrective actions and anticipated next steps in the event of any exceedances that were not able to be corrected. Additionally, if there are any exceedances that require work stoppage, DOH shall be notified and provided CAMP data for the entire workday in case there are inquiries from the public. The NYS DOH, NYS DEC, Town of Bethlehem, and Albany County Department of Health will be provided the weekly report. Agency contacts are provided in Appendix C.

The following table lists the site contact phone numbers.

Table 1: Site Contact Cell Phone Numbers

Company	Name	Cell Phone Number
Port of Albany	Roddy Yagan	518-463-8763
Labella Associates	Chris LaPointe	973-513-5759
WM Keller	Jameson Phillips	518-732-1066

Dust Control Measures

All excavating contractors must implement a dust control program for all intrusive activities to be performed. The NYS DEC notes that the following techniques have been shown to be effective for controlling the generation and migration of dust during construction activities:

- Applying water on haul roads;
- Wetting equipment and excavation faces;
- Spraying water on buckets during excavation and dumping;
- Hauling materials in properly tarped or watertight containers;
- Restricting vehicle speeds to 10 miles per hour (mph);
- Covering excavated areas and material after excavation activity ceases; and
- Reducing the excavation size and/or number of excavations.

NYS DEC's experience has shown that the chance of exceeding the 150 µg/m^3 action level is remote when the above measures have been utilized. When techniques involving water application are used, care must be taken not to use excess water, which can result in unacceptably wet conditions. Using atomizing sprays will prevent overly wet conditions, conserve water, and provide an effective means of suppressing dust.

The evaluation of weather conditions is necessary for proper dust control. When extreme weather conditions make dust control ineffective, work may need to be suspended.

Quality Assurance/Quality Control (QA/QC)

In order to ensure the validity of the fugitive dust measurements performed, the following QA/QC procedures will be followed:

Dust Trak II

All used batteries for the Dust Trak II units shall be charged every evening. Charged batteries shall be placed in the units for use each day.

Each DustTrak must be zeroed prior to use daily. Procedures are outlined on page 23 of the owner’s manual.

The maintenance of the DustTrak will follow the requirements outlined on pages 45 through 52 of the owner’s manual as required. These include:

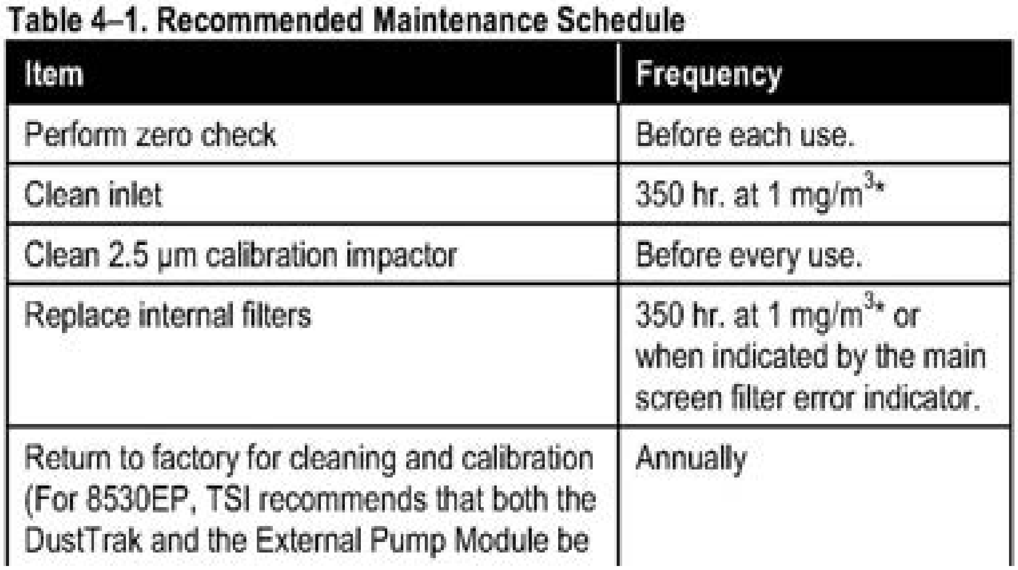


Table 4–1. Recommended Maintenance Schedule

Item	Frequency
Perform zero check	Before each use.
Clean inlet	350 hr. at 1 mg/m^{3*}
Clean 2.5 µm calibration impactor	Before every use.
Replace internal filters	350 hr. at 1 mg/m^{3*} or when indicated by the main screen filter error indicator.
Return to factory for cleaning and calibration (For 8530EP, TSI recommends that both the DustTrak and the External Pump Module be	Annually

RAE Systems PID

The PID shall be charged every evening.

Each PID shall be calibrated prior to use. Calibration and bump testing will be performed per the manufacturer’s directions.

Documentation Utilized

The following documents were utilized to develop this site-specific CAMP:

New York State Department of Environmental Conservation (NYSDEC) DER-10, TECHNICAL GUIDANCE FOR SITE INVESTIGATION AND REMEDIATION, dated May 2010;

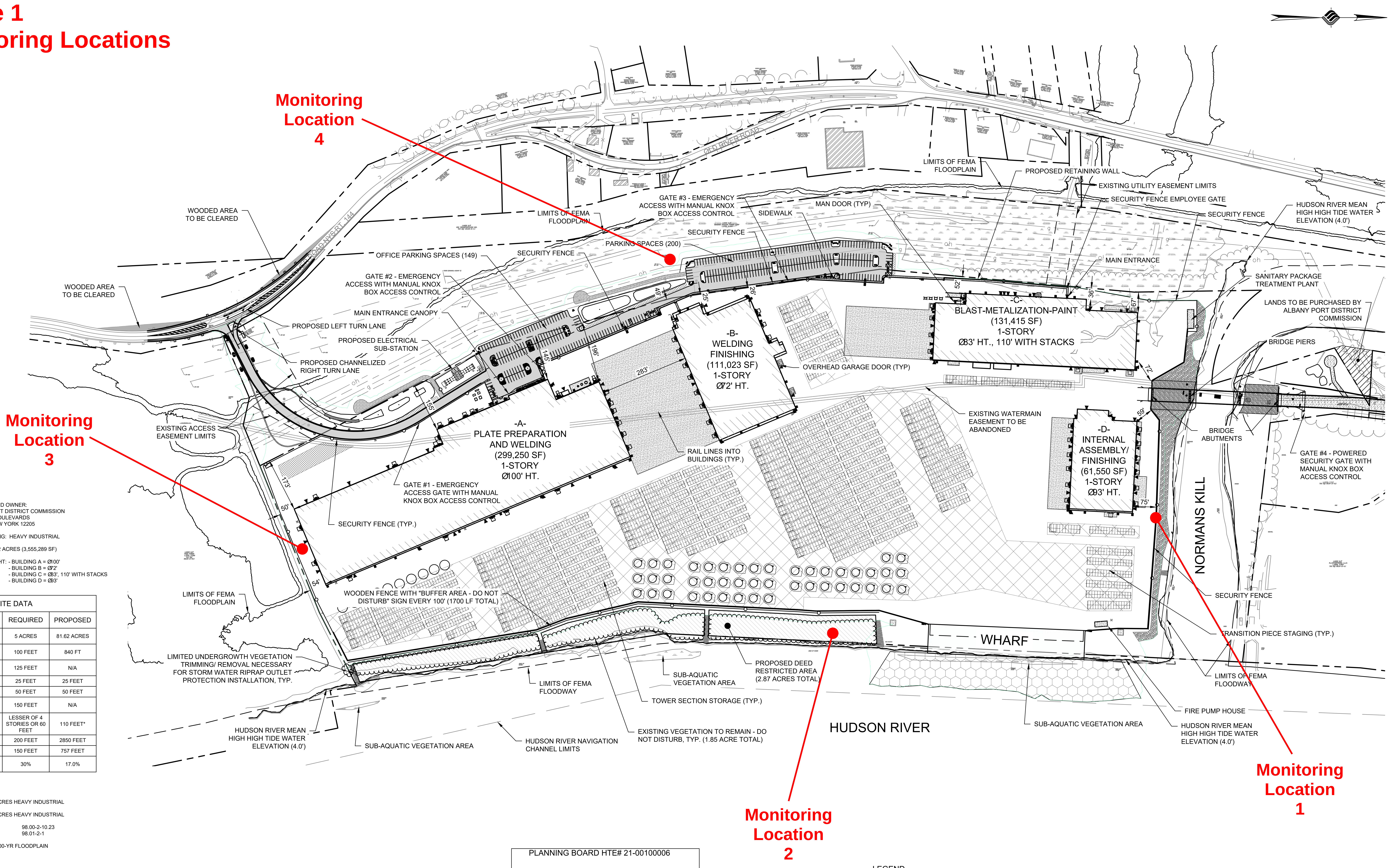
Atlantic Testing Laboratories Limited, SOIL MANAGEMENT PLAN, PORT OF ALBANY EXPANSION PROJECT, BEACON ISLAND PARCEL, BETHLEHEM, ALBANY COUNTY, NEW YORK, dated August 13, 2021; and

McFarland and Johnson, Aggregate Grading Plan, Drawing GR-02, Dated January 2022.

Appendix A

Figure 1 - Proposed Monitoring Locations
Figure 2 - Coal Ash Disturbance

Figure 1
Monitoring Locations



PROJECT DATA

1. APPLICANT / LAND OWNER:
   ALBANY PORT DISTRICT COMMISSION
   106 SMITH BOULEVARDS
   ALBANY, NEW YORK 12205
2. EXISTING ZONING: HEAVY INDUSTRIAL
3. LOT AREA: 81.62 ACRES (3,555,289 SF)
4. BUILDING HEIGHT:
   - BUILDING A = Ø100'
   - BUILDING B = Ø72'
   - BUILDING C = Ø83', 110' WITH STACKS
   - BUILDING D = Ø93'

SITE DATA		
FEATURE	REQUIRED	PROPOSED
MINIMUM LOT SIZE, NONRESIDENTIAL	5 ACRES	81.62 ACRES
MINIMUM FRONT YARD, FROM RIGHT-OF-WAY	100 FEET	840 FT
MINIMUM FRONT YARD, FROM CENTER LINE	125 FEET	N/A
MINIMUM SIDE YARD	25 FEET	25 FEET
MINIMUM REAR YARD	50 FEET	50 FEET
MINIMUM HIGHWAY FRONTAGE	150 FEET	N/A
MAXIMUM HEIGHT	LESSER OF 4 STORIES OR 60 FEET	110 FEET*
MINIMUM LOT DEPTH	200 FEET	2850 FEET
MINIMUM LOT WIDTH	150 FEET	757 FEET
MAXIMUM LOT COVERAGE	30%	17.0%

* WILL REQUEST A VARIANCE

ZONING:

EXISTING: Ø81.62 ACRES HEAVY INDUSTRIAL

PROPOSED: Ø81.62 ACRES HEAVY INDUSTRIAL

TAX ACCOUNT NUMBERS: 98.00-2-10.23
98.01-2-1

* ENTIRE SITE IS WITHIN 100-YR FLOODPLAIN

PARKING:

1 SPACE FOR EACH 2 EMPLOYEES ON MAXIMUM WORKING SHIFT.

TOTAL EMPLOYEES = 550

REQUIRED:
275 TOTAL SPACES REQUIRED

PROVIDED:
THE LARGEST SHIFT INCLUDES 180 EMPLOYEES WITH THE LARGEST SHIFT CHANGE INVOLVING 320 EMPLOYEES. INDIVIDUAL BUILDING PARKING DEMANDS FROM MARMEN WELCON HAVE BEEN PROVIDED BELOW:

BUILDING A = 168 SPACES
BUILDING B = 87 SPACES
BUILDING C & D = 100 SPACES TOTAL
TOTAL OPERATOR REQUESTED SPACES = 355

349 TOTAL SPACES PROVIDED

ADA SPACES REQUIRED:
PER 2010 ADA STANDARDS FOR ACCESSIBLE DESIGN

REQUIRED (349 SPACE LOT): 8 SPACES (6 STANDARD & 2 VAN)
FOR TOTAL PARKING 301-400 SPACES

PROVIDED (349 SPACE LOT): 9 SPACES (7 STANDARD & 2 VAN)

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES\18641.00-GEN-LAYO.DWG

PLANNING BOARD HTE# 21-00100006

LEGEND

PROPERTY LINE	—— - - ——	PROPOSED BUILDING FOOTPRINT	▨
EASEMENT LIMITS	— — — —	MATERIAL STORAGE AREAS	☒
DITCH CENTERLINE	—— – ——	WETLAND AREA	▒
ROADSIDE SWALE	—— · · ——	WHARF DREDGING AREA	⬡
BUILDING SETBACK		PAVEMENT AREA	▩
OVERHEAD DOORS	⊐	CONCRETE AREA	░
MAN DOORS	◀	RIP-RAP WATER EMBANKMENT STABILIZATION	▓
EXISTING BUILDING	▨	PROPOSED DEED RESTRICTED AREA	▨

0 150 300 450 FT
SCALE

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
FINAL DESIGN PLANS

NO.	DATE	DESCRIPTION
1	05/20/22	TOWN COMMENTS
2	06/06/22	TOWN COMMENTS
	06/08/22	GMP PLANS

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT OF ALBANY EXPANSION SITE

DRAWN JES
DESIGNED NSO
CHECKED AJF
SCALE AS SHOWN
DATE 05/10/2022
PROJECT 18641.00

ADAM J FROSINO
088870

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
SITE PLAN OVERALL

DRAWING NUMBER
SP-00

4 OF 68

Figure 2
Coal Ash Disturbance

GRADING NOTES:

1. THIS SHOWS THE COMPARISON OF THE FINISHED GRADE MINUS 48" TO THE EXISTING GRADE.

LEGEND

🟥 APPROXIMATE LIMITS OF IMPACTED COAL ASH. 4.86ØAC (211,707ØSF.) 15,660ØCU. YD.

LEGEND

PROPERTY LINE
EASEMENT LIMITS
DITCH CENTERLINE
ROADSIDE SWALE
BUILDING SETBACK
OVERHEAD DOORS
MAN DOORS
EXISTING BUILDING

PROPOSED BUILDING FOOTPRINT
MATERIAL STORAGE AREAS
WETLAND AREA
WHARF DREDGING AREA
PAVEMENT AREA
CONCRETE AREA
RIP-RAP WATER EMBANKMENT STABILIZATION
PROPOSED DEED RESTRICTED AREA

0 150 300 450 FT
SCALE

N:\18641.00 ALBANY PORT EXPANSION\DRAW\DRAWINGS\SHEET FILES - SITE PREPARATION\ASH CUT-FG MINUS 48 INCHES_09-30-2022.DWG

McFarland Johnson
60 RAILROAD PLACE
SUITE 402
SARATOGA SPRINGS, NEW YORK 12866
P:518-580-9380 F:518-580-9383
SaratogaROM@mjinc.com

PROJECT MILESTONE
BID PLANS

NO.	DATE	DESCRIPTION

CLIENT:
ALBANY PORT DISTRICT COMMISSION
ALBANY, NEW YORK

PROJECT:
PORT EXPANSION SITE - SITE PREPARATION

DRAWN	JES
DESIGNED	NSO
CHECKED	AJF
SCALE	1"=40'
DATE	APRIL 2022
PROJECT	18641.00

CONCEPTUAL
FIGURE
NOT FOR
CONSTRUCTION

IT IS A VIOLATION OF LAW FOR ANY PERSON, UNLESS THEY ARE ACTING UNDER THE DIRECT DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR, TO ALTER AN ITEM IN ANY WAY. IF AN ITEM BEARING THE STAMP OF A LICENSED PROFESSIONAL IS ALTERED, THE ALTERING ENGINEER, ARCHITECT, LANDSCAPE ARCHITECT, OR LAND SURVEYOR SHALL STAMP THE DOCUMENT AND INCLUDE THE NOTATION "ALTERED BY" FOLLOWED BY THEIR SIGNATURE, THE DATE OF SUCH ALTERATION, AND A SPECIFIC DESCRIPTION OF THE ALTERATION.

DRAWING TITLE
APPROXIMATE LIMITS
OF IMPACTED
COAL ASH PLAN

DRAWING NUMBER
GR-01
OF

Appendix B

DUSTTRAK II AEROSOL MONITOR MODEL 8530/8531/8532/8530EP, OPERATION AND SERVICE MANUAL

DUSTTRAK™ II
AEROSOL MONITOR
MODEL 8530/8531/8532/8530EP

OPERATION AND SERVICE MANUAL

P/N 6001893, REVISION M
DECEMBER 2014

DustTrak II 8530/31 Desktop and 8532 Handheld

DustTrak II 8530EP Monitor

Appendix C

Project Contact List

Project Directory

Albany County Department of Heal

No people are associated with Albany County Department of Heal

Albany Port District Commission

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